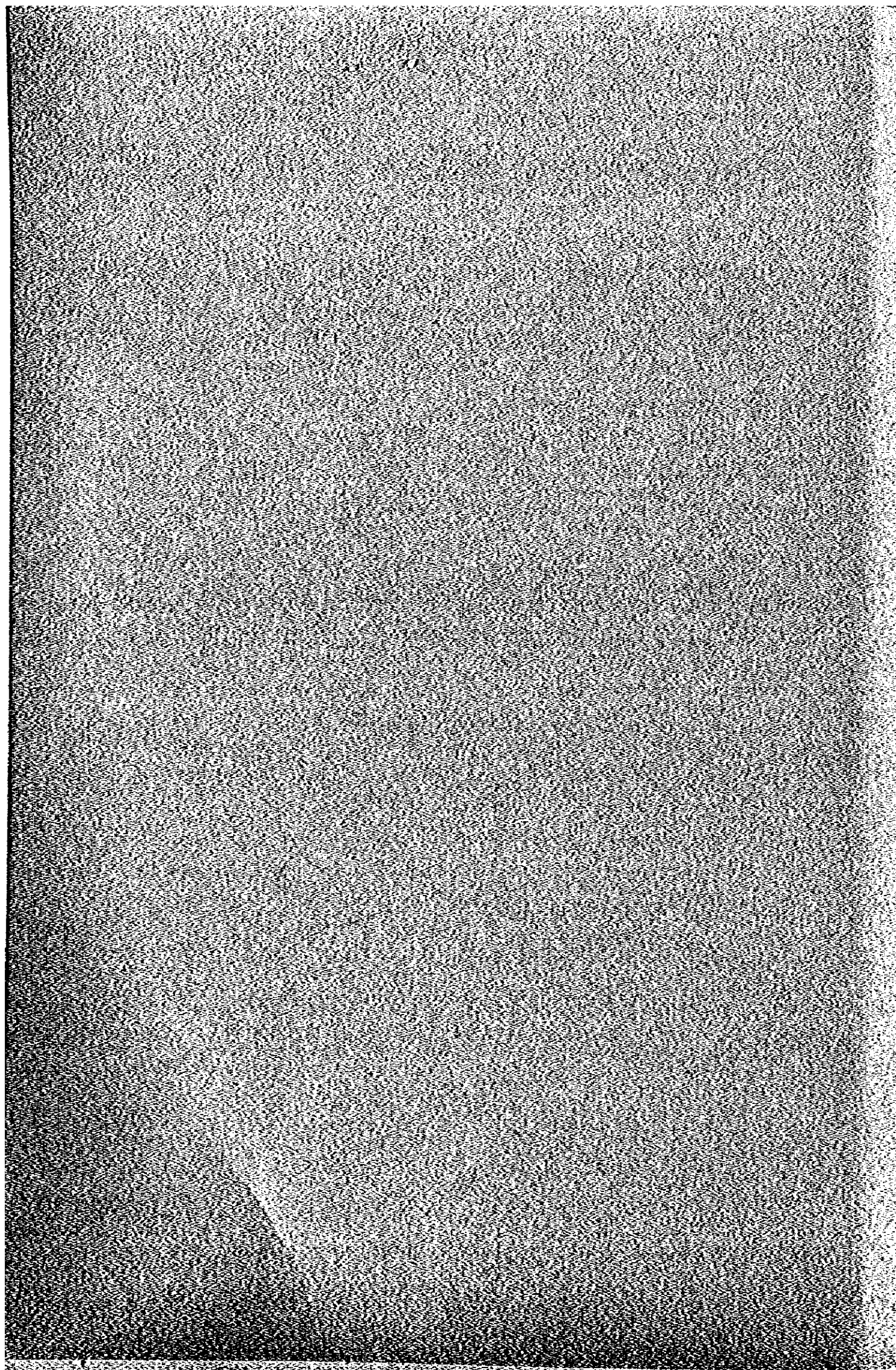


APPENDICES TO CHAPTER 8



APPENDIX TABLE 8-1 LIST OF CONTROL POINTS

<u>Code No.</u>	<u>Description</u>	<u>Code No.</u>	<u>Description</u>
101	Royal Palace	230	Chinese Christ Cemetery
102	Sa Patum Palace	231	Mosque
103	Suan Pakard Palace	232	Sri Uma Thavee Mosque
201	Makok Temple	233	Christ Church
202	Xavier Church	234	Mosque
203	Ta Phan Temple	235	Lum Satta Tum Temple
204	Fatima Church	236	Cemetery
205	Chang Saeng Temple	237	Chinese Cemetery
206	Patum Wanaram Temple	238	Sai Temple
207	Holy Redeemer Church	239	Reevaul Mosque
208	Dis Hong Saram Temple	240	Pai Nguen Temple
209	Mai Chong Rom Temple	241	Bang Klo Temple
210	Pha Sri Temple	242	Yannawa Temple
211	Mosque	243	Bangkok Church
212	Klong Ton Mosque	244	Wattana Wittaya Church
213	Church	245	Victory Monument
214	Samien Naree Temple	301	Metropolitan Water Works Authority (Filter Section)
215	Phaya Young Temple	302	Office of Children Welfare
216	Chummi Hattakarn Temple	303	Ministry of Foreign Affairs
217	Sa Bue Temple	304	Department of Highway
218	Daung Kae Temple	305	Police Fire Brigade Division
219	Maha Puttaram Temple	306	Ministry of Industry
220	Kaew Jam Fah Temple	307	Government Pharmaceutical Organization
221	Christ Church	308	Office of University Affairs
222	Muang Kae Temple	309	Asoke-Din Daeng Electricity Power, Sub Station
223	Suan Plu Temple	310	Asoke-Din Daeng Telephone, Sub Station
224	Sutti Wararam Temple	311	Department of Livestock Development
225	Don Temple		
226	Chinese Cemetery		
227	Pok Burma Temple		
228	St. Louis Church		
229	Christ Church		

<u>Code No.</u>	<u>Description</u>	<u>Code No.</u>	<u>Description</u>
312	Express Transportation Organization	341	National Energy Administration
313	Bangkok Mass Transit Authority	342	State Railway of Thailand
314	State Railway of Thailand	343	State Railway of Thailand (Printing House)
315	Industrial Estate Authority of Thailand	344	Electricity Power, Sub Station
316	Embassy, Great Britain	345	Bang Rak City Hall
317	Embassy, Switzerland	346	Bang Rak Police Station
318	Chid Lom Electricity, Sub Station	347	Revenue Department
319	Telephone Organization of Thailand	348	Communication Authority of Thailand
320	Embassy, Netherland	349	Yannawa Police Station
321	Embassy, Vietnam	350	Fish Marketing Organization
322	Embassy, USA	351	Embassy, Laos
323	Makasan Electricity Power, Sub Station	352	Embassy, Burma
324	Development of Fishery Industry	353	Embassy, USSR
325	Embassy, India	354	Alien Registration and Taxation Division
326	Embassy, Japan	355	Embassy, Singapore
327	Embassy, Nepal	356	USA Information Centre
328	Klong Ton Electricity, Sub Station	357	Electric Sub Station
329	Government Lottery Bureau of Thailand (Printing House)	358	Embassy, Spain
330	Klong Ton Railway Station	359	Saudi Arabia Royal Embassy, and Canadian Embassy
331	Hua Mak Railway Station	360	Embassy, Brazil
332	Embassy, Indonesia	361	Embassy, Austria
333	Thai Red Cross Council	362	Embassy, Israel
334	Bang Kuang Correction	363	Embassy, Egypt
335	State Railway of Thailand	364	Embassy, Cambodia
336	Metropolitan Water Works Authority (Pumping Station)	365	Embassy, Pakistan
337	Treasury Department (Mint Division)	366	Residence of Japanese Ambassador
338	Office of the National Environment Board	367	Residence of Italian Ambassador
339	Ministry of Finance	368	Residence of Egyptian Ambassador
340	National Women Council	401	Army Area
		402	Phayathai Police Station
		403	Police Department

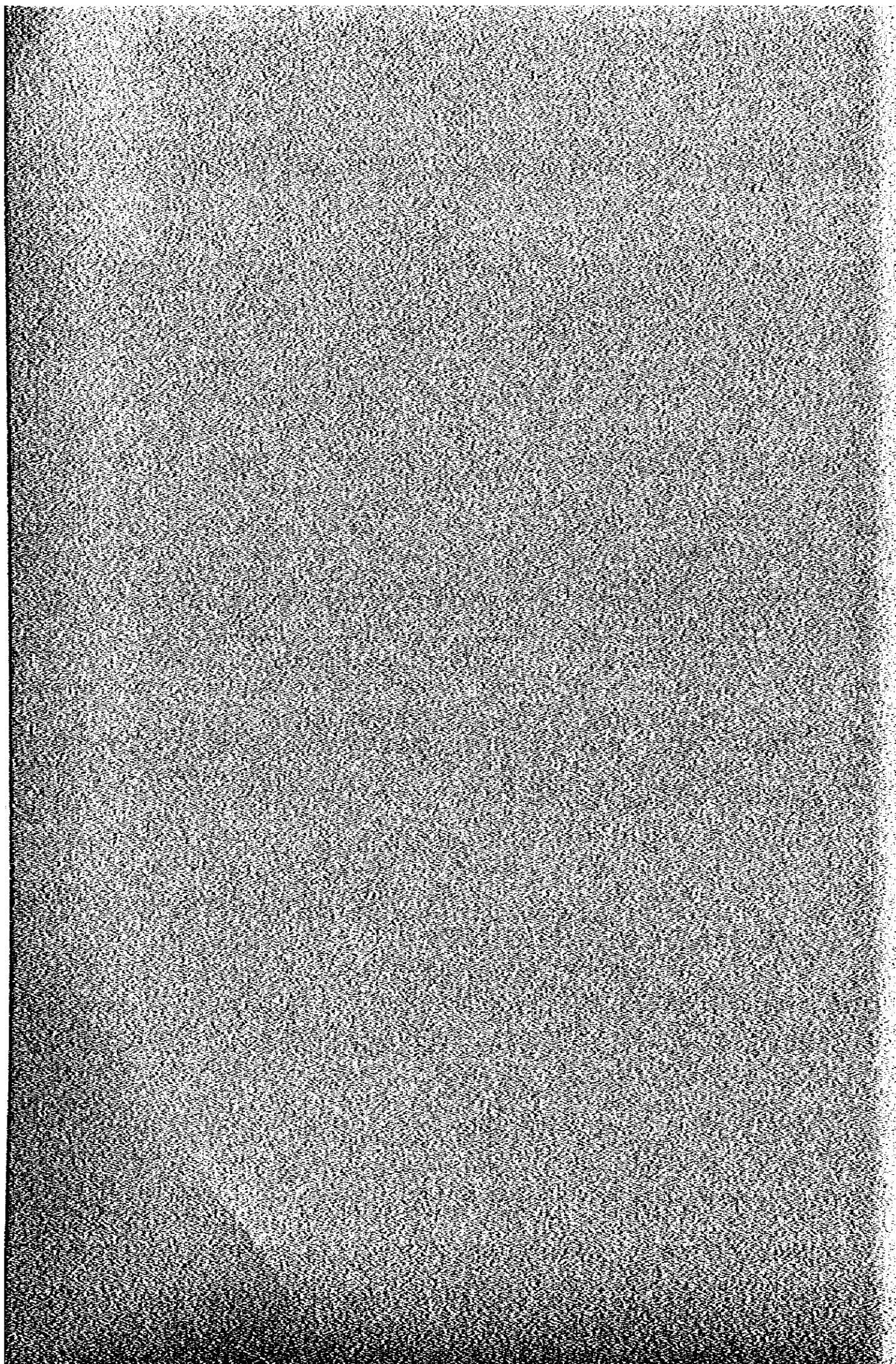
<u>Code No.</u>	<u>Description</u>	<u>Code No.</u>	<u>Description</u>
404	Commissioner's Office of the Provincial Police	609	Patumwan Technical College
405	Police Fire Brigade Division	610	Chulalongkorn University
406	Military Area	611	Uthen Thawai Technical College
501	King Mongkut Hospital	612	Traim Udom Suksa School
502	Nerve Hospital	613	Sri Ayuthaya School
503	Rama Hospital	614	Kittikun School
504	Monks Hospital	615	Indrachai Technical College
505	Rajvithee Hospital	616	Deficiency School
506	Decha Hospital	617	AUA
507	Phayathai Hospital	618	Don Bosco School
508	Makkasan Hospital	619	St. Dominic School
509	Phetchaburi Hospital	620	International School
510	Smittiwiet Hospital	621	Srinakharin Wiroj University (Prasarn Mitr Campus)
511	Prompt Mitr Hospital	622	Kanta Butr School
512	Camillian Hospital	623	Chalerm Sart School
513	Bangkok General Hospital	624	Charnwit Pittayarat School
514	Phetchawiet Hospital	625	Jan Jan Wittayarat School
515	Chulalongkorn Hospital	626	Dol Wittaya School
516	Jong Jitr Hospital	627	Klong Ton Mosk School
517	St. Joseph Hospital	628	Kasen Pittaya School
518	Lerd Sin Hospital	629	Sai Thip Pittaya School
519	St. Louis Hospital	630	Phadung Sit Pittaya School
520	Bang Rak Hospital	631	Raevadee School
521	Mahea Sak Hospital	632	Samsen Kindergarten School
522	Bangkok Christian Hospital	633	Samsen Vittayarat School
523	Bumrungrad Medical Center	634	Business Administration College
601	Mahidol University	635	Chaophaya Commercial School
602	Blind School	636	Satree Pratuang Wit School
603	Amnuay Silpa School	637	Phaya Young School
604	Phayathai School	638	Sritabut Bumrung School
605	Santiraj Bumrung School	639	Daung Kae School
606	Dusit Commercial School	640	School
607	Piboon Wattana School	641	Maha Puttaram School
608	Fatima School		

<u>Code No.</u>	<u>Description</u>	<u>Code No.</u>	<u>Description</u>
642	Kaew Jam Fah School	701	Tiara Hotel
643	Krung Thep Pittaya School	702	Thai-Japanese Stadium
644	School	703	Century Theatre
645	Silom Commercial School	704	E M I Theatre
646	Yok Msn Suksa School	705	Athen Theatre
647	Assumption School	706	Hollywood Theatre
648	Satja Wittaya Wchool	707	President Theatre
649	Wat Suti Wararam School	708	Asia Hotel
650	Institute of Technology and Vocational Education (Satee Phanakorn Tai Campus)	709	McKenna Theatre
651	Satee Sri Suriyothai School	710	National Stadium
652	Kosol Wittaya School	711	Siam Centre
653	Nipat Wittaya School	712	Siam Square
654	Assumption Commercial School	713	Siam Intercontinental Hotel
655	Kindergarten School	714	Panthip Plaza
656	Krungthep Christain School	715	First Hotel
657	Padung Darunee School	716	Florida Hotel
658	Sathorn Wittaya School	717	Metro Theatre
659	Chinese Christain School	718	Paramount Theatre
660	Sawang Wittaya School	719	New Amarin Hotel
661	Chaun Chern Kindergarten School	720	O A Shopping Centre
662	Satja Tham School	721	Chawala Turkish Bath
663	Tri Ratana Suksa School	722	Chao Phraya 1 Turkish Bath
664	Kanok Technical School	723	Chao Phraya 2 Turkish Bath
665	Panit School	724	Indra Hotel
666	Chan Wetch Suksa School	725	Sports Club (Turf)
667	Kitti Commercial School	726	Royal Sports Turf Club
668	Benjawan Suksa School	727	Stella Theatre
669	Udom Wetch Kindergarten School	728	Star Theatre
670	Praae Maree School	729	Indra Clothing Market
671	Yana Wetch School	730	Petch Rama Theatre
672	Wat Pai Nguen School	731	DaDa Theatre
673	Wattana Wittaya Academy	732	Polly Theatre
		733	Hilton International Hotel
		734	Ploenchit Arcade
		735	Central Plaza
		736	President Hotel

<u>Code No.</u>	<u>Description</u>	<u>Code No.</u>	<u>Description</u>
737	Erawan Hotel	771	Trocade Hotel
738	Rajaprasong Shopping Centre	772	Peninsula Hotel
739	Imperial Hotel	773	Victory Hotel
740	Oscar Theatre	774	Princess Hotel
741	NaNa Hotel	775	New Fuji Hotel
742	Rajah Hotel	776	Ramada Hotel
743	Ambassador Hotel	777	Daily News
744	Manhattan Hotel'	778	Jim Thompson's
745	Chawala Hotel	779	Star Hotel
746	Golf Course	780	Reno Hotel
747	Bang Sue Supermarket	781	Opera Hotel
748	Siam Cement Factory	782	Heaven Hotel
749	Bang Sue Market	783	Picnic Hotel
750	Olympic Theatre	784	Amarin Hotel
751	Coliseum Theatre	785	Atlanta Hotel
752	Bangkok Centre Hotel	786	Golden Palace Hotel
753	British Club	787	Fortuna Hotel
754	Manorah Hotel	788	Federal Hotel
755	Royal Orchid Hotel	789	Prince Hotel
756	Oriental Hotel	790	Astar Hotel
757	Bang Rak Market	791	Nakorn Petch Hotel
758	New Hotel (Under construction)	792	Crown Bowl
759	Bangkok Dock	793	East Hotel
760	Warner Theatre	794	Asoke Bowl
761	Hyatt Rama Hotel	795	Siam Hotel
762	Siam Theatre	796	Metro Hotel
763	Thavee Phol Theatre	797	World Hotel
764	Chan Theatre	798	Star Bowl
765	Bangkok Palace Hotel	799	Morakot Hotel
766	Narai Hotel	800	Marketing Organization of Farmers
767	Sri Sathorn Theatre	901	Police Flat
768	Century Hotel	902	Din Daeng Flat
769	Bangkok Rama Theatre	903	Premruthai Housing Complex
770	Grace Hotel		

<u>Code No.</u>	<u>Description</u>
904	Chawala Housing Complex
905	Seree Housing Complex
906	Seree Housing Complex
907	Seree Housing Complex
908	Seree Housing Complex
909	Seree Housing Complex
910	Police Flat
911	Military Flat
912	Military Flat
913	Lottery Flat
914	National Housing Authority Flat
915	Police Flat

APPENDICES TO CHAPTER 9



APPENDIX 9.1 COMPARATIVE STUDY FOR GEOMETRIC DESIGN PHILOSOPHY

A comparative study was made to select one of the two philosophies mentioned in subsection 9.3.2 of the Text. Different types of design speeds and road widths which basically affect construction cost were chosen to compare the costs, considering design standards of the First Stage Expressway System, Road Structure Ordinance (Japan), Metropolitan Expressway Public Corporation (Japan) and Intra Urban Tollway (Jakarta) as shown in Appendix Table 9-1.

Six alternative cases as shown in Appendix Fig. 9-1 were studied based on the combination of design speeds (60 and 80 km/h) of expressways, design speeds (40 and 50 km/h) of interchange¹⁾ between expressways, and the cross-sectional composition of expressways by types²⁾. The result of the comparison is shown in Appendix Fig. 9-2.

The result of the above discussion indicates that these design speeds for the expressway and interchange made about 0.8 and 0.9 percent difference in the construction costs, respectively, and that the types of expressway widths make 6 percent difference. Based on the above mentioned factors, the following is found among others:

- Design speeds of throughways and interchanges do not introduce big differences into their construction costs;
- Wider lane width is preferable since massive traffic volume is expected on the proposed metropolitan expressways;
- Homogeneity with the First Stage Expressway System is necessary even if their characters are different; and
- Percentage of heavy vehicles of the Second Stage Expressway System is relatively lower than that of the First Stage Expressway System.

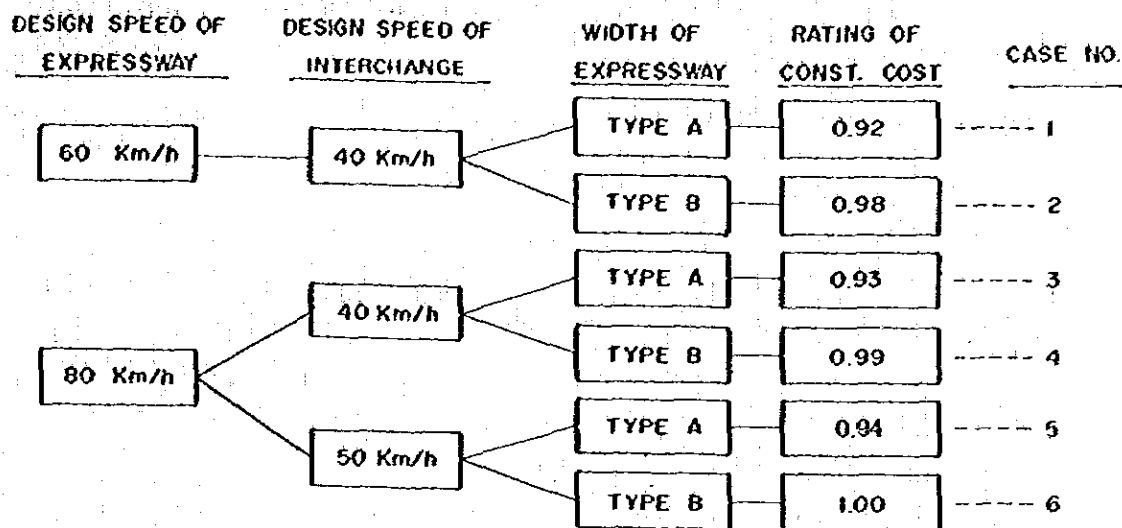
Notes: 1) Interchange does not include on-off ramps connecting with arterial roads.

- 2) Type A : lane width of 3.25 meters and outer shoulder width of 1.50 meters
Type B : lane width of 3.50 meters and outer shoulder width of 2.00 meters

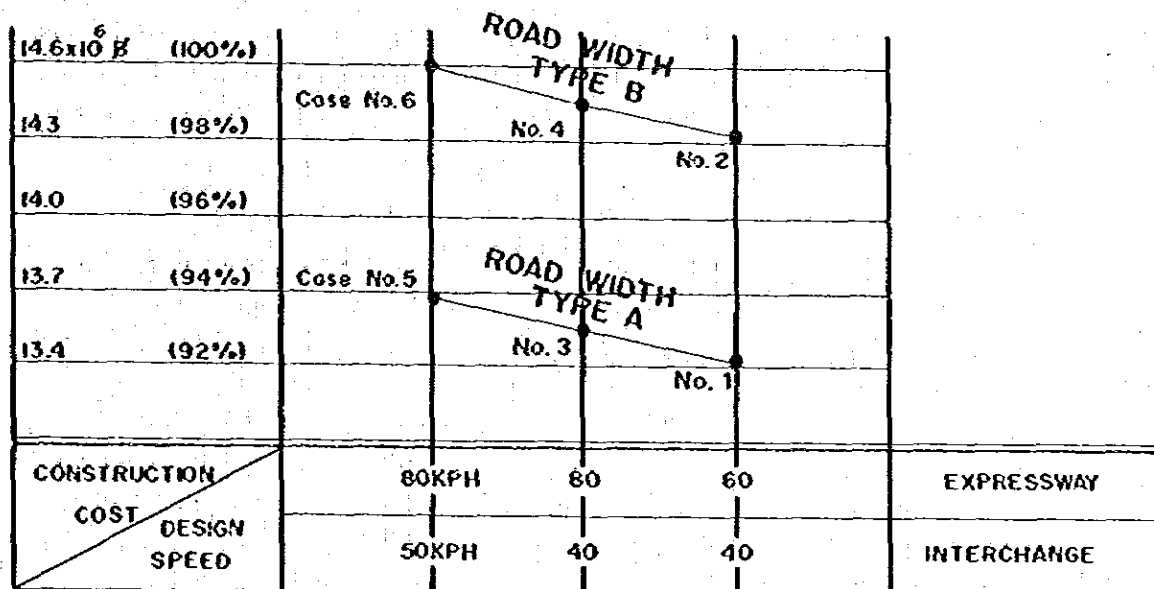
APPENDIX TABLE 9-1 DESIGN STANDARDS FOR URBAN EXPRESSWAYS IN BANGKOK, TOKYO AND JAKARTA

	First Stage Expressway (Bangkok)	Metropolitan Expressway (Tokyo)	Intra Urban Tollway (Jakarta)	Road Structure Ordinance (Japan)
Design Speed : Expressway Interchange	80 km/hr 50	(Inner Ring) 60	80	60-80
Lane Width	3.50 m	3.25	3.60	3.25-3.50
Outer Shoulder Width	2.00 m	1.25	1.75-2.00	1.25-1.25
Inner Strip	0.33 m	0.50	0.50	0.50-0.50
Median	2.00 m	2.00	2.00-5.00	1.75-2.25
Crossfall (On Tangent)	2.50 %	1.50	2.00	1.50-1.50
Minimum Radii (Horizontal)		120	230	120-230
Maximum Gradient	5.00 %	5.00 (Max. length 500m)	6.00	5.00-4.00
Stopping Sight Distance		75.00	115.00	75.0-110.0

APPENDIX FIG. 9.1 RELATIONSHIP BETWEEN DESIGN STANDARDS AND CONSTRUCTION COSTS



APPENDIX FIG. 9.2 COMPARATIVE STUDY OF DESIGN STANDARDS AND CONSTRUCTION COSTS



NOTES: TYPE A: LANE WIDTH OF 3.25 METERS AND OUTER SHOULDER WIDTH OF 1.50 METERS.

TYPE B: LANE WIDTH OF 3.50 METERS AND OUTER SHOULDER WIDTH OF 2.00 METERS.

APP. FIG. 9-1

RELATIONSHIP BETWEEN DESIGN STANDARD AND CONSTRUCTION COSTS

APP. FIG. 9-2

COMPARATIVE STUDY OF DESIGN STANDARD AND CONSTRUCTION COSTS

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

APPENDIX 9.2 BASIC, POSSIBLE AND DESIGN CAPACITIES

Basic capacity, possible capacity and service level were discussed in this appendix to get hourly design capacity of the Expressway.

(1) Basic Capacity

This is defined as the maximum volume per hour able to pass the section of one lane or one carriageway under ideal conditions of road and traffic flow.

The ideal condition is only accomplished providing the following conditions are met:

- Not less than 3.5 m lane width;
- Not less than 1.75 m lateral clearance;
- Good geometrical standard;
- Only passenger cars running; and
- No speed limit.

2,500 veh/hour is recommended in the Japanese standard and this is based on data surveyed at many sites on existing roads in Japan.

(2) Possible Capacity

Actual road and traffic conditions are different from the ideal conditions. The basic capacity is adjusted using several factors which affect the capacity in order to meet the expected actual road conditions. The factors affecting basic capacity are summarized as follows:

a) Lane Width

In the Japanese standard the minimum lane width for mixed traffic so as not to affect the traffic capacity, is designated to be 3.5 m.

The coefficients of the effect of lane width are shown as follows:

<u>Lane Width (m)</u>	<u>Coefficients</u>
3.50	1.00
3.25	0.94
3.00	0.85
2.75	0.77

b) Lateral Clearance

The lateral clearance is the distance between the edge of lane and obstacles such as retaining wall, electric pole, guardrail, etc.

These obstacles affect the traffic capacity if they are put within 1.75 m from the edge of the lane. The coefficients of the effect of lateral clearance are as shown below in the case of a multilane road.

<u>Lateral Clearance</u>	<u>1.75 m</u>	<u>1.50 m</u>	<u>1.25 m</u>	<u>1.00 m</u>	<u>0.75 m</u>	<u>0.50 m</u>	<u>0</u>
Insufficient clearance only on one side	1.00	1.00	0.99	0.98	0.97	0.95	0.90
Insufficient clearance on both sides	1.00	0.99	0.98	0.97	0.94	0.90	0.81

c) Truck and Buses

Heavy vehicles such as trucks and buses reduce basic traffic capacity not only by the body size but also by the reduced power to weight ratio. This results in lower vehicle speeds especially on steep and long slopes.

The effect of heavy vehicles on the traffic capacity is evaluated by converting one heavy vehicle to the equivalent number of passenger cars (sedan) which give the same influence to the capacity.

Coefficients to affect the traffic capacity due to the influence of heavy vehicles are calculated by applying the following formula:

$$C = \frac{100}{100 - P + E \times P}$$

C : Coefficient to affect traffic capacity by mixing heavy vehicle

P : Heavy vehicle mixing ratio (%)

E : Sedan equivalent of heavy vehicle

d) Degree of Development along the Roadside

On a road on which no interference is expected from outside the road, for example an access-controlled expressway, the running speed of vehicles is affected only by internal factors such as traffic volume and horizontal and vertical alignment.

On the other hand, the running speed of a non-access-controlled road such as an Arterial Street is affected by merging vehicles from access streets and pedestrians crossing the road. This results in a reduction of the traffic capacity of a non-access-controlled road.

The coefficient is evaluated depending on the density of houses, buildings or factories along the corridor. No adjustment is necessary for those sections of street on which there is no interference for reasons such as difference in level, pedestrian barriers, etc. Also no adjustment is necessary on the inner lanes of a multi-lane highway, as the outer lanes act as a barrier.

<u>Degree of Development along Roadside</u>	<u>Coefficient</u>
Not developed	1.0 – 0.9
Partially developed	0.9 – 0.8
Fully developed	0.8 – 0.7

e) Other Considerations

Effects from lack of overtaking sight distance and mixture of motorcycles should also be considered. However, the former only affects the capacity of 2-lane, 2-way

roads and the latter is considered in the assessment of traffic assignment by converting motorcycles to sedans by applying the sedan equivalent of a motorcycle. These factors are therefore ignored in the capacity calculations.

The factor discussed in (3)-(a) to (3)-(d), are used to convert the Basic Capacity into a Possible Capacity.

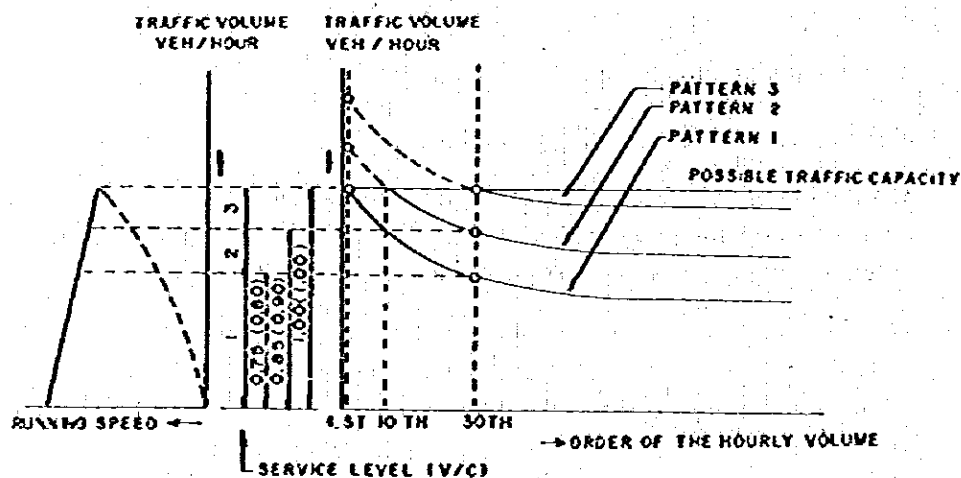
(3) Design Capacity (per hour)

Service level is expressed by the ratio of the traffic volume (V) and the Possible Capacity (C) – V/C . In an urban area the service level is designated as below:

Service level-1	$V/C = 0.80$
Service level-2	$V/C = 0.90$
Service level-3	$V/C = 1.00$

The design traffic capacity is calculated by multiplying V/C of required service level to the possible traffic capacity.

The Fig. 9-3 shows the relationship between service level and peak hour traffic volume. Summary of the basic criteria on the traffic condition in each service level are as follows:



APPENDIX FIG. 9-3 SERVICE LEVEL AND PEAK HOUR TRAFFIC VOLUME

Service Level-1: At the target year of design, the annual maximum peak hour traffic volume is less than the possible capacity per hour. Vehicles in the 30th highest annual hourly volume can keep stable flow at certain speeds, but selection of speed is restricted.

Service Level-2: At the target year of design, 10th highest annual hourly traffic volume reaches possible capacity and this sometimes causes serious traffic jams during these peak ten hours. Vehicles in the 30th highest annual hourly traffic volume are unable to keep uniform speeds and the speed changes at random.

Service Level-3: At the target year of design, the 30th highest annual hourly traffic volume exceeds possible capacity and this causes serious traffic jams during these peak 30 hours. Vehicle in the flow of the 30th highest annual hourly traffic volume is always forced to change speed and sometimes is forced to stop.

Even if service level-3 is adopted, it is theoretically possible to say that during only 30 hours in 1 year (8,760 hours) does the traffic volume exceed possible capacity.

However, from the nature of traffic flow, it should be considered that a serious traffic jam might remain for longer than theoretically indicated.

By multiplying the Possible Capacity by the appropriate V/C ratio, which depends on service level, it is possible to derive the Design Capacity on hourly basis.

APPENDIX TABLE 9-2 CONSTRUCTION COSTS OF OVER-CROSSING AND UNDERCROSSING SCHEMES

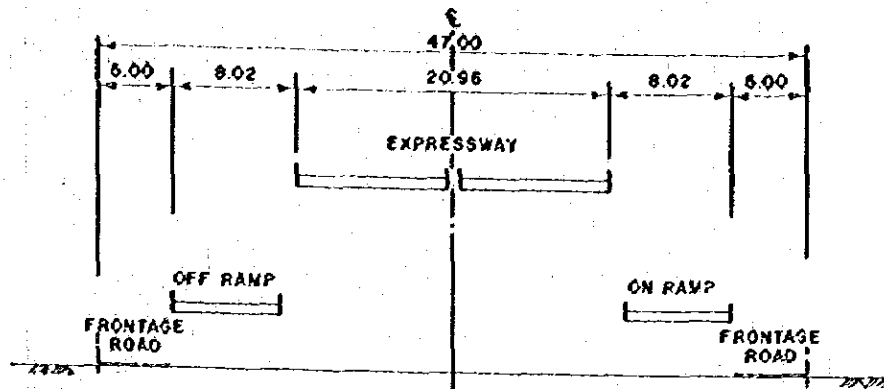
Overpassing Scheme Sta. 1+720 – 2+580	
<u>Construction Item</u>	<u>Cost (Million Baht)</u>
Phahol Yothin Crossing Section (L = 120 m)	39.08
Viaducts Section (L = 370 x 2 m)	176.78
Ramp Section	94.45
Miscellaneous	15.51
Sub-Total	325.82
Contingencies	32.58
Engineering Cost	17.92
TOTAL COST	376.32 ¹⁾
Underpassing Scheme Sta. 1+720 – Sta. 2+580	
<u>Construction Item</u>	<u>Cost (Million Baht)</u>
Tunnel Section (L = 320 m)	336.00 ¹⁾
Approach Section (L = 200 x 2 m)	83.52
Embankment Section (L = 70 x 2 m)	14.98
Over-Bridge	13.36
Drainage Facilities	5.90
Ramp Section	46.55
Miscellaneous	25.02
Sub-Total	525.33
Contingencies	52.53
Engineering Cost	28.89
TOTAL COST	606.75 ¹⁾

Notes: 1) Excluding land acquisition and compensation

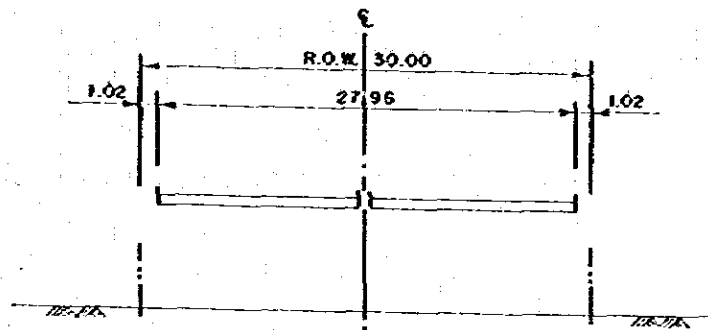
2) Estimate based on the cast-in-situ wall construction method

**APPENDIX TABLE 9-3 CONSTRUCTION COSTS OF VIADUCT
AND EMBANKMENT SCHEME**

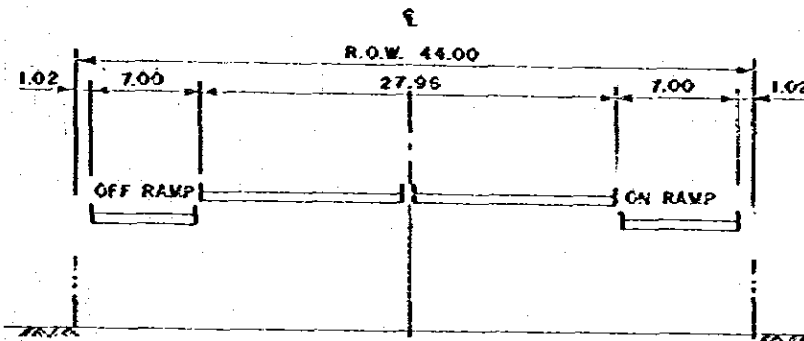
Embankment Scheme		Sta. 7+500 – 8+450 of East Route
Embankment and Drainage Cost		22 x 10 ⁶
Pavement Cost		16 x 10 ⁶
Sand Drain and Pre-loading Cost		96 x 10 ⁶
		134 x 10 ⁶ Baht
Viaduct Scheme		Sta. 7+500 – 8+450 of East Route
Viaduct (24,377 m ³)		238 x 10 ⁶ Baht



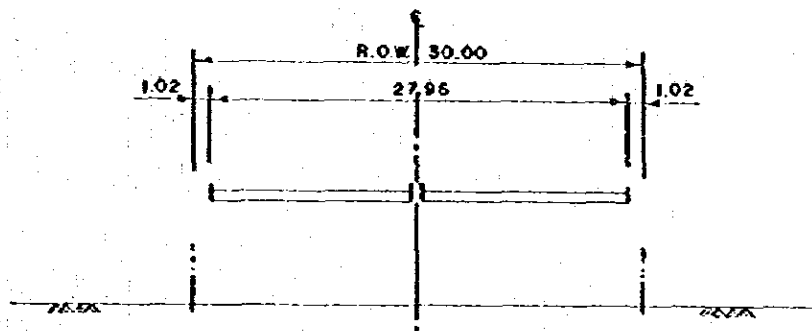
NS ④, STA. 4+400



NS ③, STA. 4+400



NS ②, STA. 3+100

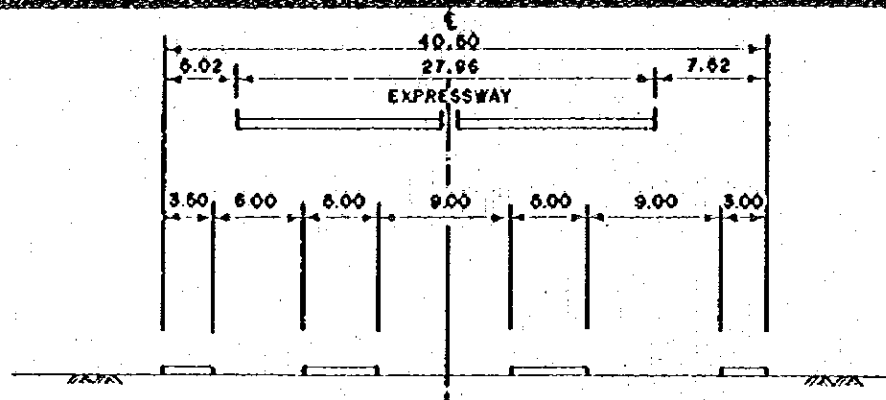


NS ①, STA. 1+500

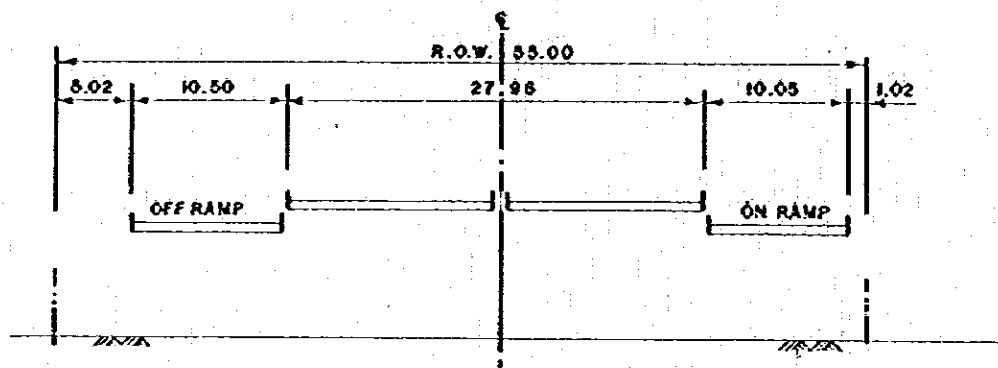
APPENDIX
FIG. 9-4 (1)

CROSS SECTION WITH R.O.W.
(N.S. ROUTE)

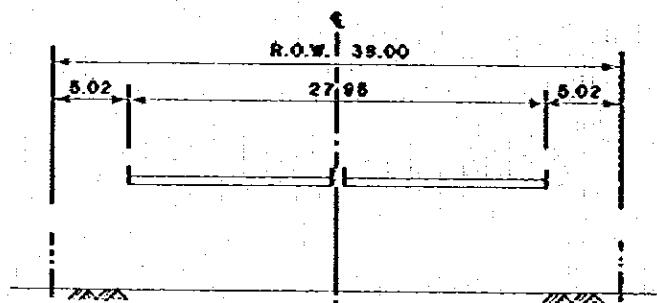
THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK



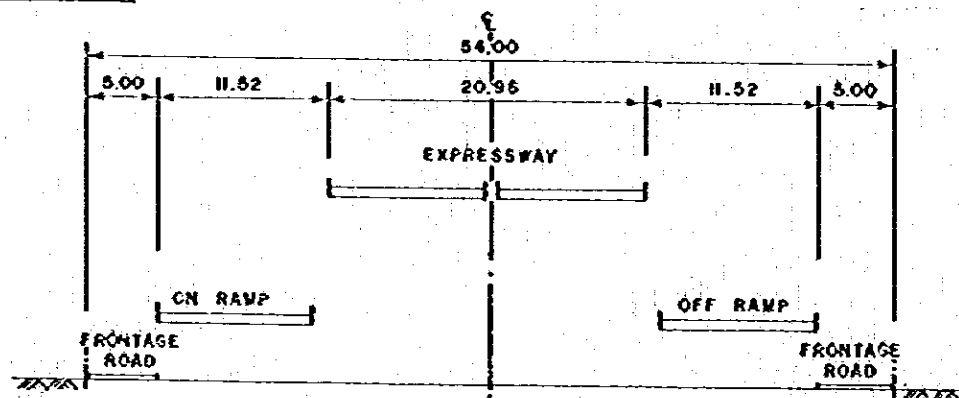
NS ⑧, STA. 6 + 650



NS ⑦, STA. 6 + 300



NS ⑥, STA. 5 + 500

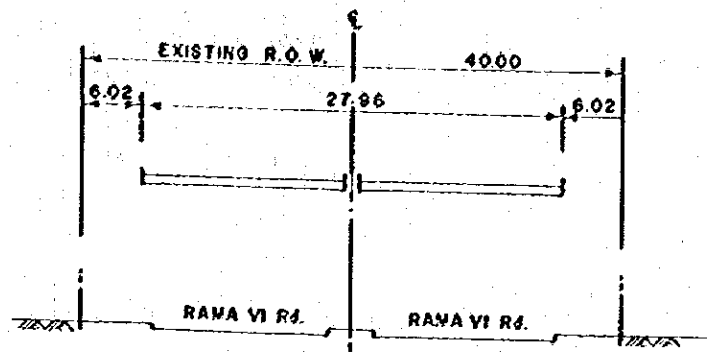


NS ⑤, STA. 4 + 800

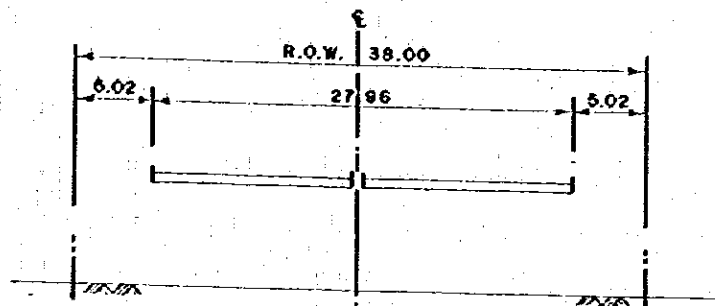
APPENDIX
FIG. 9-4(2)

CROSS SECTION WITH R.O.W.
(N.S. ROUTE)

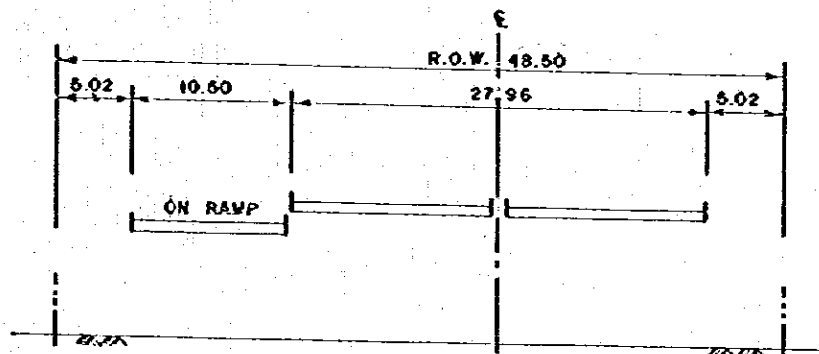
THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK



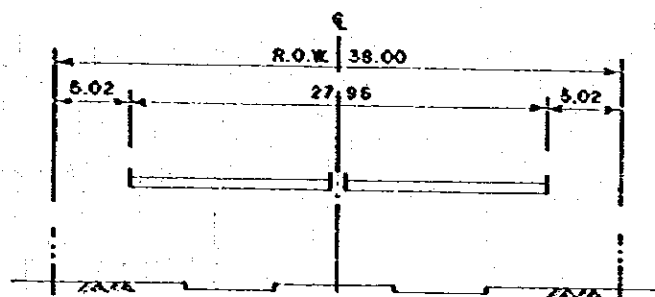
NS ⑫, STA. 10+000



NS ⑪, STA. 8+050



NS ⑩, STA. 7+650



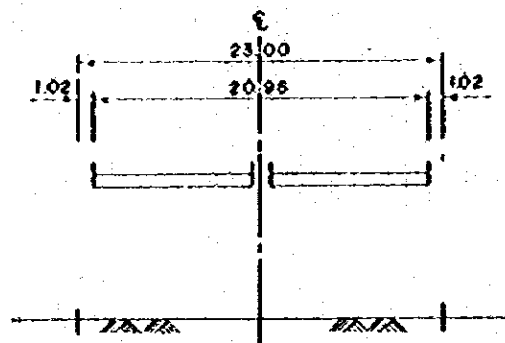
NS ⑨, STA. 7+000

APPENDIX
FIG. 9-4(3)

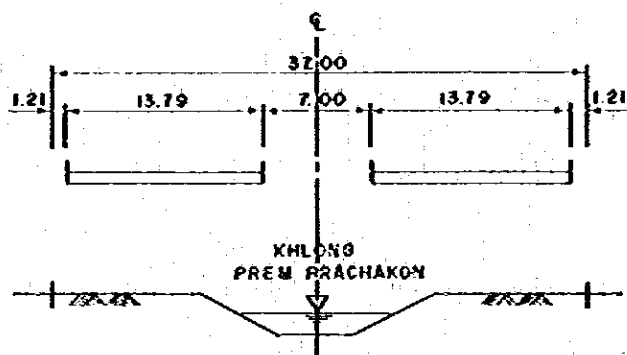
CROSS SECTION WITH R.O.W.
(N.S. ROUTE)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

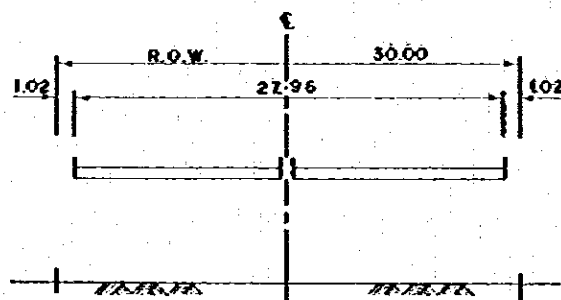
NS (16) STA. 19+000



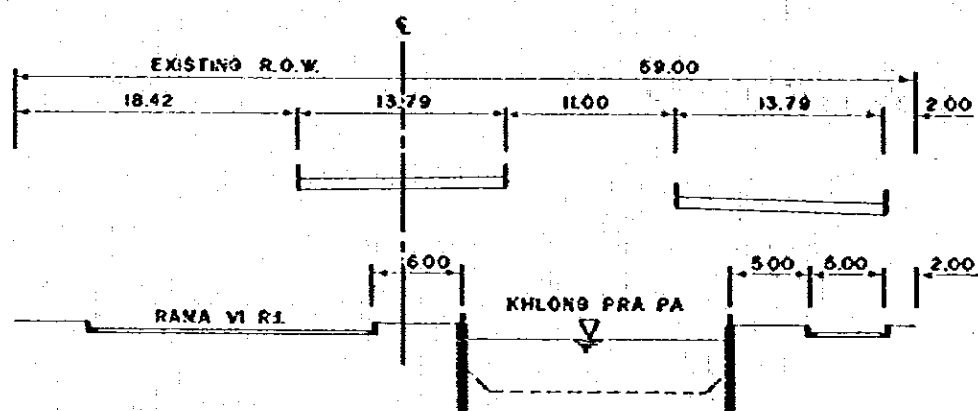
NS (15) STA. 17+600



NS (14) STA. 15+000



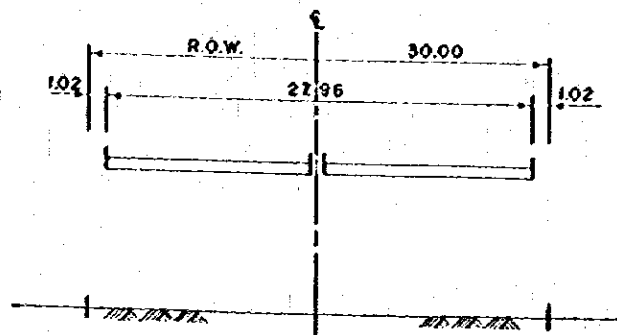
NS (13) STA. 11+600



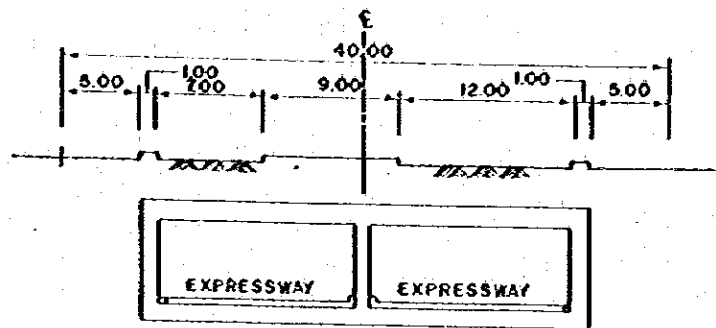
APPENDIX
FIG. 9-4 (4)

CROSS SECTION WITH R.O.W.
(N.S. ROUTE)

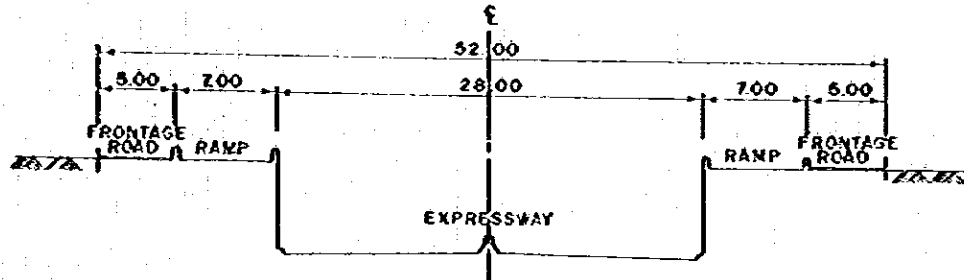
THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK



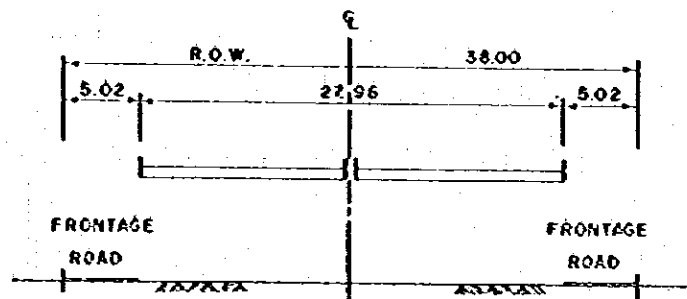
E ④, STA. 3+000 AND STA. 4+500



E ③, STA. 2+200



E ②, STA. 2+000

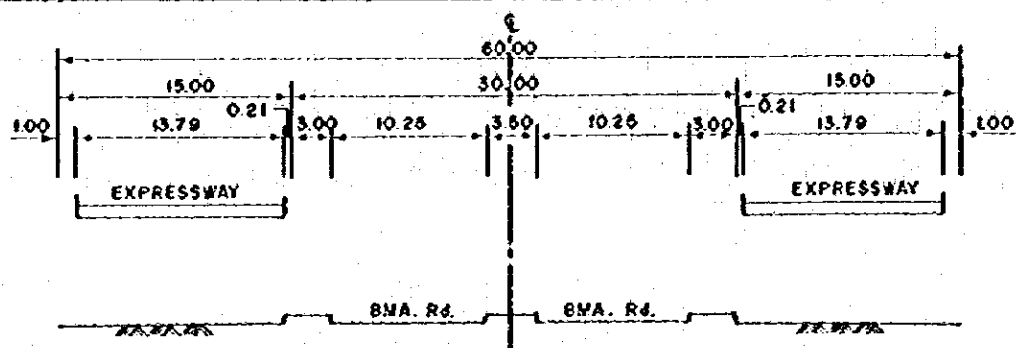


E ①, STA. 1+550

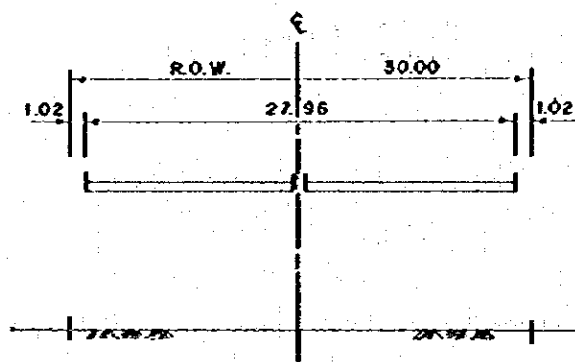
APPENDIX
FIG. 9-4(5)

CROSS SECTION WITH R.O.W.
(E ROUTE)

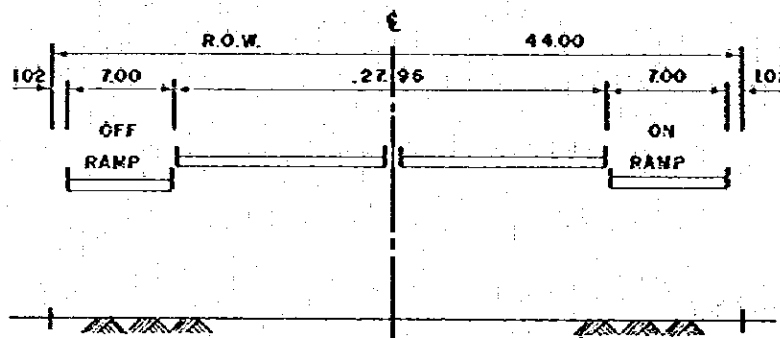
THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK



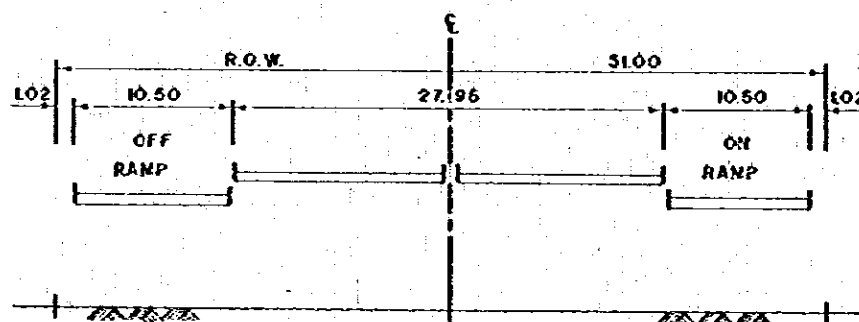
E - (8) STA. 8 + 000



E - (7) STA. 6 + 400



E - (6) STA. 5 + 850

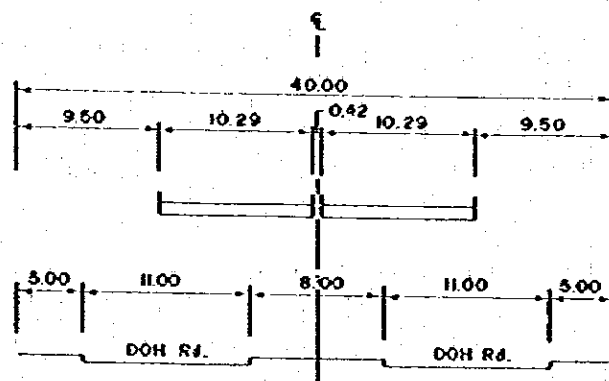


E - (5) STA. 5 + 200

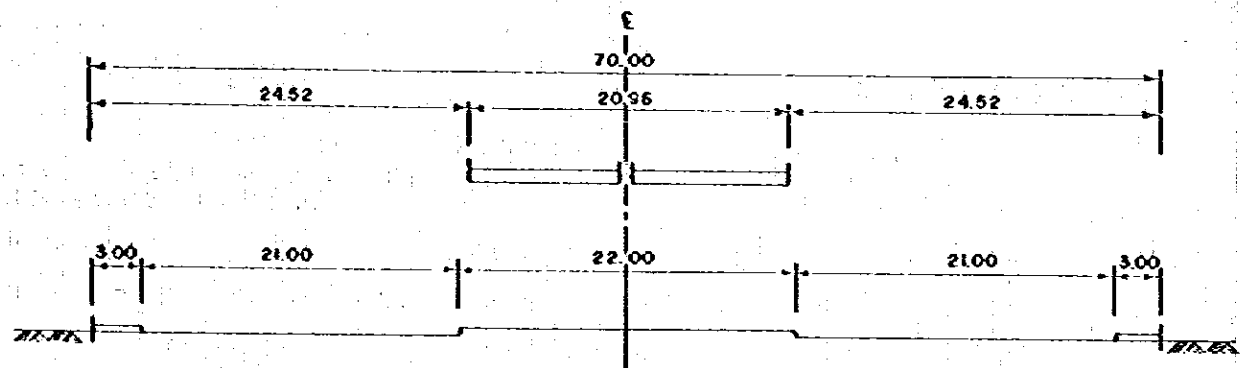
APPENDIX
FIG. 9-4 (6)

CROSS SECTION WITH R.O.W.
(E ROUTE)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK



E - ⑩ STA. 11+000



E - ⑨ STA. 9+900

APPENDIX
FIG. 9-4(7)

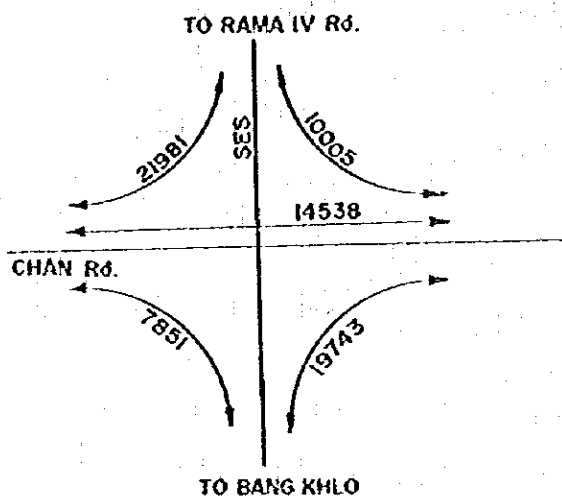
CROSS SECTION WITH R.O.W.
(E ROUTE)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

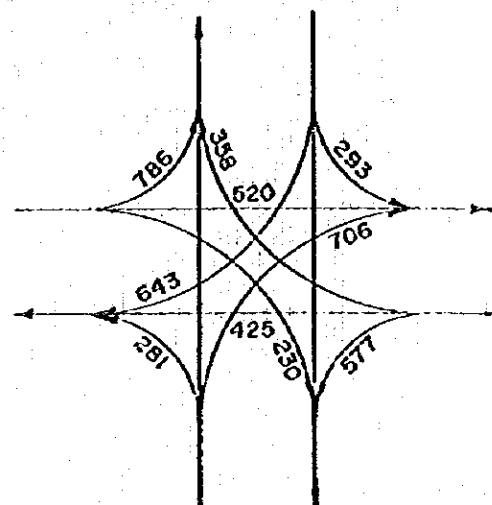
NODE NO. 747

YEAR : 2000

NAME OF INTERSECTION : SES (N - S) x CHAN ROAD



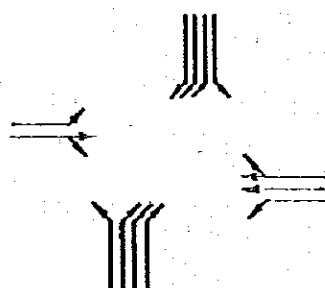
VEH / DAY



VEH / HOUR

TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $\gamma = V/Cp$	MODIFIED γ	PHASE TIME (SEC.)
1 786 / → 520 230 \	VL = 868 VT = 574 VR = 254 } 1696	1 x 1800 1 x 2000 } 3800	0.446 *	0.53 *	77 + 2
2 358 425 ← 577 \	VR = 395 VT = 469 VL = 637 } 1501	1 x 1800 2 x 2000 } 5800	0.250 *	0.30	43 + 2
3 643 / 1293 \	VR = 710 (VL = 323) } 710	3 x 1800 } 5400	0.131		
281 / 706 \	(VL = 310) VR = 779 } 779	3 x 1800 } 5400	0.144 *	0.17	24 + 2
TOTAL			0.840	1.00	150

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE : 2000 VEH (PCU) / GREEN HOUR
TURNING LANE : 1800 VEH (PCU) / GREEN HOUR

2. PEAK FACTOR : 6.5 %
3. PERCENTAGE OF HEAVY VEHICLES : 13.0 %
4. DIRECTIONAL DISTRIBUTION OF FUTURE DESIGN HOURLY VOLUME : 55.0 %

APPENDIX
FIG. 9-5(1)

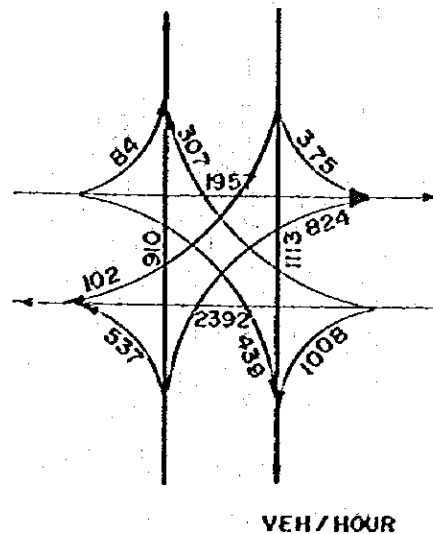
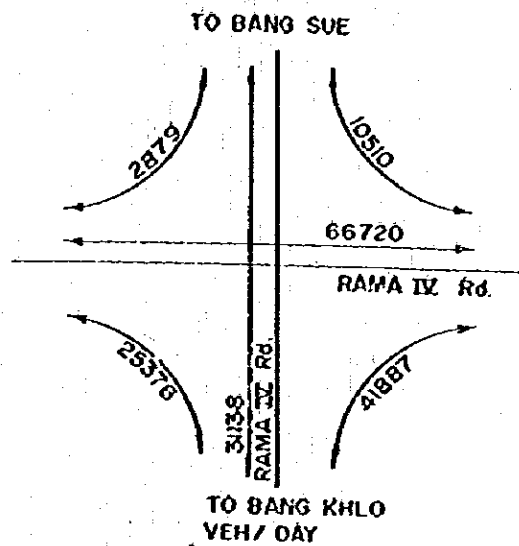
TRAFFIC ANALYSIS OF INTERSECTION (1)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 736

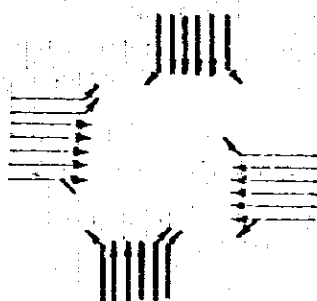
YEAR : 2000

NAME OF INTERSECTION : SES (N--S) x RAMA IV ROAD x RAMA VI ROAD



TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $Y = V/C_p$	MODIFIED Y	PHASE TIME (SEC.)
1 84 1957 439	VL = 92 VT = 2160 VR = 484 2736	2x18000 5x2000 13600	0.20	0.20	28 + 2
2 307 2392 1008	VR = 338 VT = 2160 VL = 1112 4090	5x2000 1x1800 11800	0.34	0.34	49 + 2
3 102 113 373	VR = 113 VT = 1228 (VL = 414) 1341	1x1800 3x2000 7800	0.17	0.17	24 + 2
4 537 910 824	(VL = 592) VT = 1228 VR = 909 2137	2x2000 2x1800 7600	0.28	0.29	41 + 2
TOTAL			0.99	1.00	150

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR
TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE
DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9 - 5 (2)

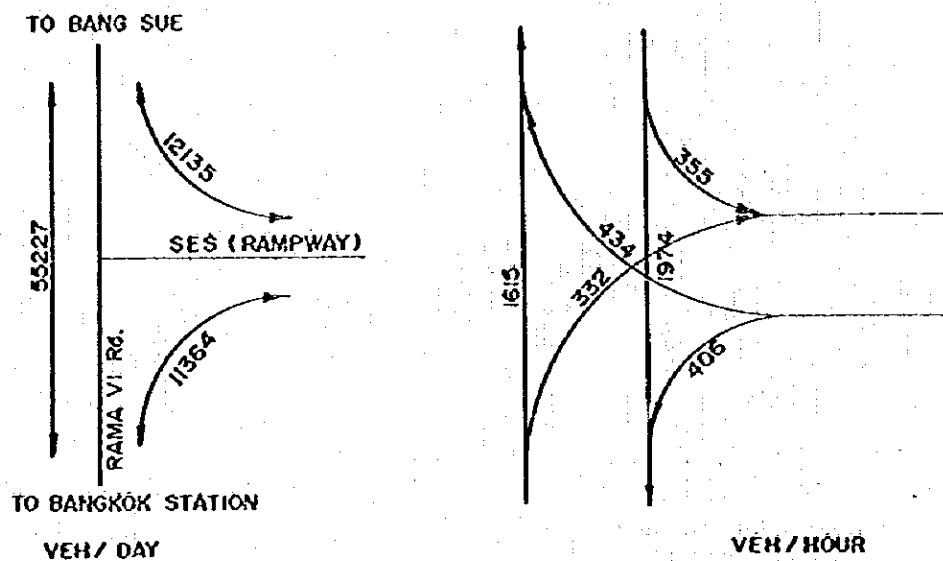
TRAFFIC ANALYSIS OF INTERSECTION(2)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 770

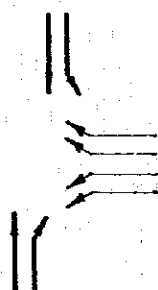
YEAR : 2000

NAME OF INTERSECTION : SES (N-S) x RAMA IV ROAD



TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $\gamma = V/Cp$	MODIFIED γ	PHASE TIME (SEC.)
1 	VT = 1783 VT = 2179 VL = 392 } 2571	$2 \times 2000 = 4000$ $2 \times 2000 = 4000$ $1 \times 1800 = 1800$ } 5800	0.446^* 0.443	0.57	66 + 2
2 	VR = 367	$1 \times 1800 = 1800$	0.204^*	0.26	29 + 2
3 	VR = 479 VL = 448 } 927	$4 \times 1800 = 7200$	0.129^*	0.17	19 + 2
4 					
TOTAL			0.77	1.00	120

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH (PCU) / GREEN HOUR
TURNING LANE = 1800 VEH (PCU) / GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9-5(3)

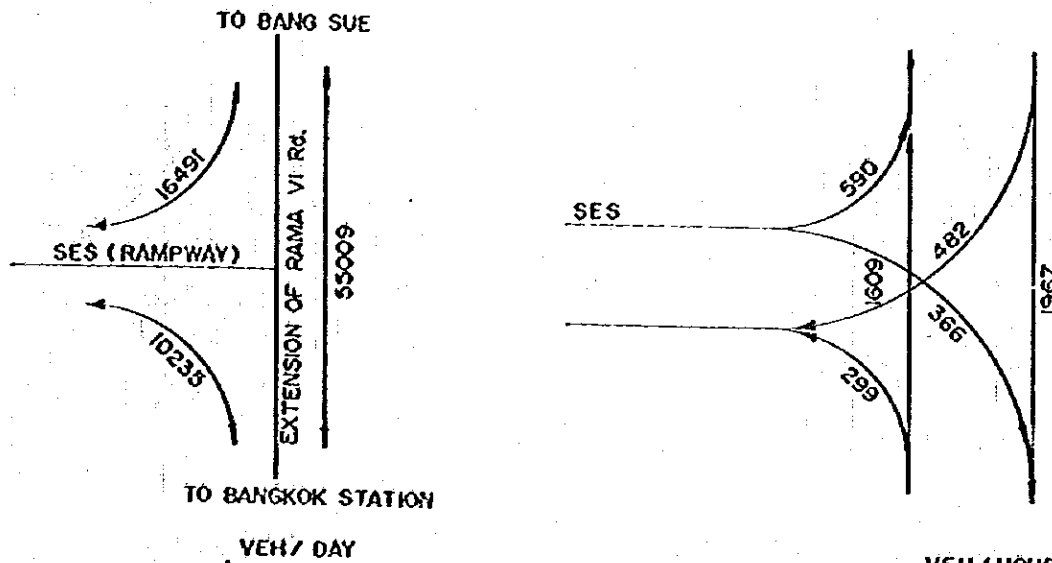
TRAFFIC ANALYSIS OF INTERSECTION(3)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 770

YEAR : 2000

NAME OF INTERSECTION : SES (N-S) x EXTENSION OF RAMA VI ROAD



TRAFFIC PHASE (VEH/H)		TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $Y = V/C_p$	MODIFIED Y	PHASE TIME (SEC.)
1	1609 299	VL = 330 VT = 1776 VT = 2172	1 x 1800 2 x 2000 2 x 2000	0.363 0.543	0.61	71 + 2
2	482	VR = 532	2 x 1800 = 3600	0.148	0.17	19 + 2
3	590 366	VL = 651 VR = 404	2 x 1800 1 x 1800	0.195	0.22	24 + 2
4						
TOTAL				0.886	1.00	120

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR
TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE
DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9-5(4)

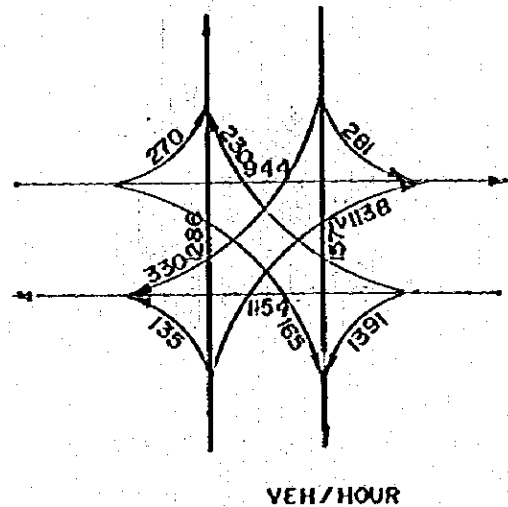
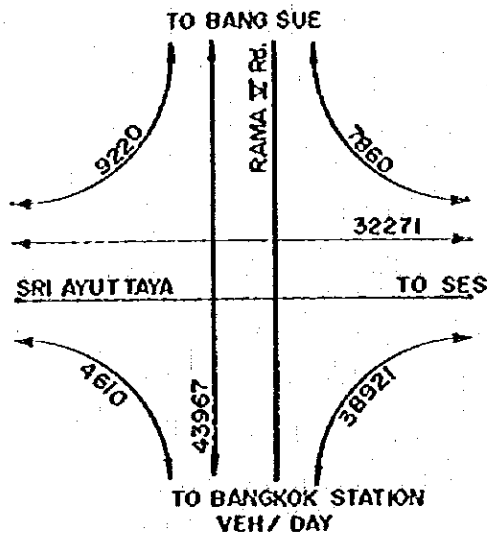
TRAFFIC ANALYSIS OF INTERSECTION(4)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 734

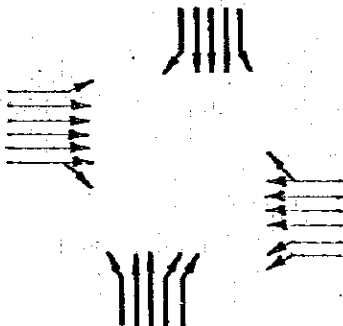
YEAR : 2000

NAME OF INTERSECTION : SES(N-S) x RAMA V ROAD x SRI AYUTTAYA ROAD



TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $Y = V/Cp$	MODIFIED Y	PHASE TIME (SEC.)
1 270 944 165	VL=298 VT=1042 VR=182 1522	1x1800 5x2000 11800	0.12	0.12	16 + 2
2 230 1154 1391	VR=254 VT=1274 VL=1536 3064	4x2000 2x1800 11600	0.26	0.27	39 + 2
3 330 1572 281	VR=364 VT=1735 (VL=310) 2099	1x1800 3x2000 7800	0.26	0.26	37 + 2
4 135 1286 1138	(VL=149) VT=1735 VR=1256 2676	2x2000 2x1800 7600	0.35	0.35	50 + 2
TOTAL			0.99	1.00	150

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR
TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR : 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES : 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE
DESIGN HOURLY VOLUME : 55.0 %

APPENDIX
FIG. 9 - 5(5)

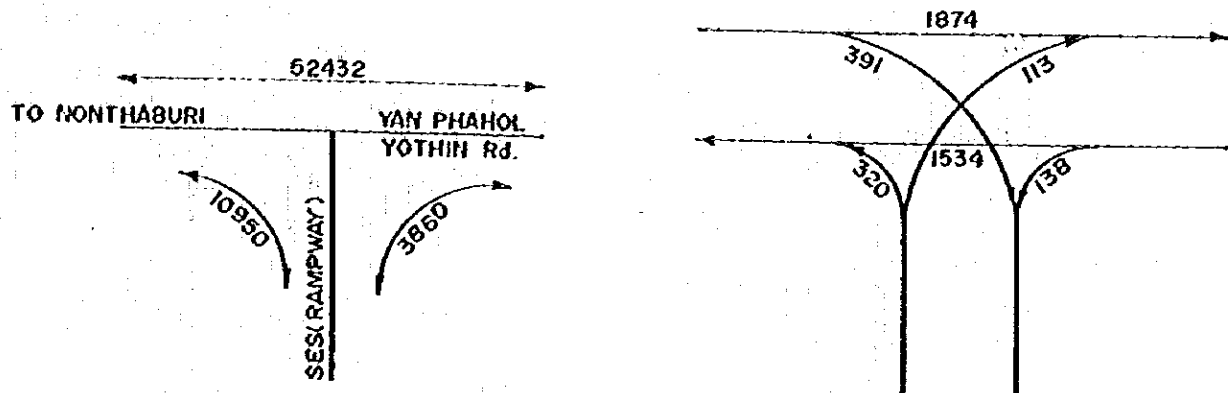
TRAFFIC ANALYSIS OF INTERSECTION(5)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 711

YEAR : 2000

NAME OF INTERSECTION : SES (N-S) x YAN PHAHOL YOTHIN ROAD

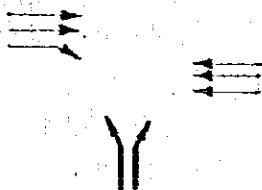


VEH / DAY

VEH / HOUR

TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $Y = V/Cp$	MODIFIED Y	PHASE TIME (SEC.)
1 1874 1534 138	VT = 2069 VT = 1694 VL = 152 } 1846	2x2000 = 4000 2x2000 } 5800 1x1800 }	0.517* 0.318	0.58	68 + 2
2 391	VR = 432	1x1800 = 1800	0.240*	0.27	30 + 2
3 320 113	VL = 353 VR = 125 } 478	2x1800 = 3600	0.133*	0.15	16 + 2
4					
TOTAL			0.890	1.00	120

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH (PCU) / GREEN HOUR

TURNING LANE = 1800 VEH (PCU) / GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE
DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9-5 (6)

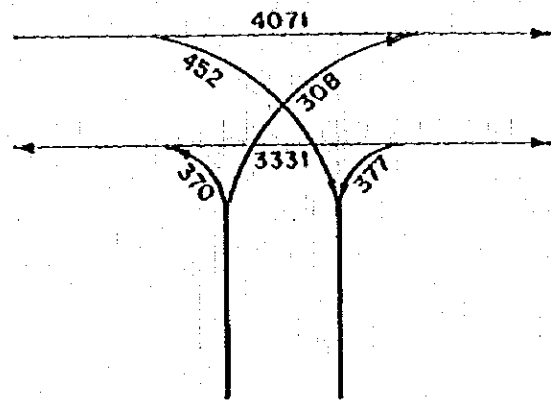
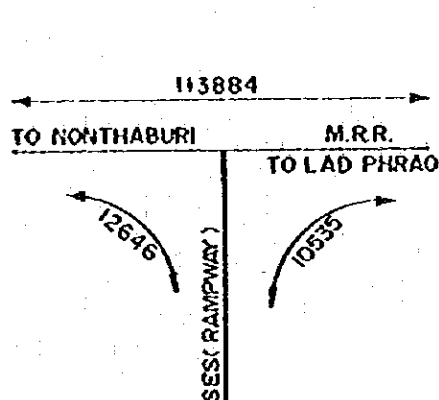
TRAFFIC ANALYSIS OF INTERSECTION (6)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 709

YEAR : 2000

NAME OF INTERSECTION : SES(N-S)X MIDDLE RING ROAD

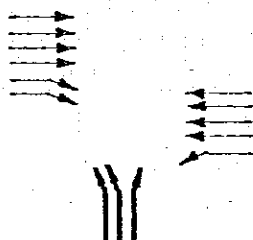


VEH / DAY

VEH / HOUR

TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $\gamma = V/Cp$	MODIFIED γ	PHASE TIME (SEC.)
1 407L 3331 377	VT=4494 VT=3677 VL=416 4093	4x2000 = 8000 4x2000 } 9800 1x1800 }	0.562 * 0.418	0.66	78 + 2
2 452	VR=	2x1800 = 3600	0.139 *	0.17	18 + 2
3 370 308	VL= 408 VR= 340 748	3x1800 } 5400	0.139 *	0.17	18 + 2
4					
TOTAL			0.840	1.00	120

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR
TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE
DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9-5(7)

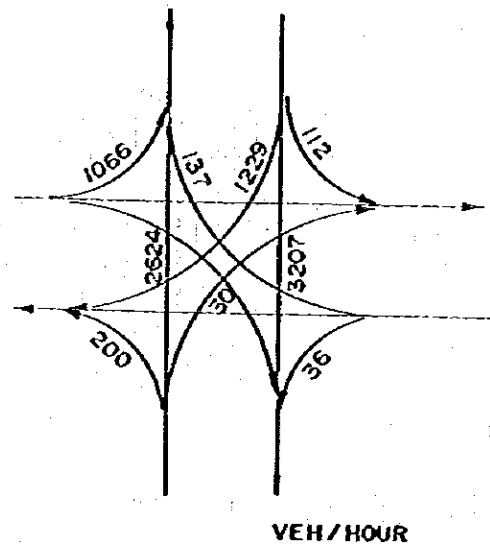
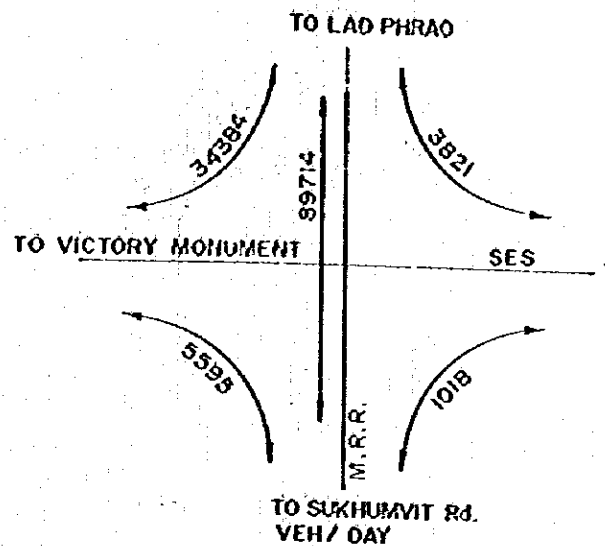
TRAFFIC ANALYSIS OF INTERSECTION(7)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 717

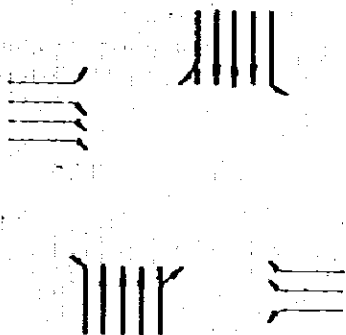
YEAR: 2000

NAME OF INTERSECTION: SES(E)xMIDDLE RING ROAD



TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $\gamma = V/C_p$	MODIFIED γ	PHASE TIME (SEC.)
1 1229 112 3207	VR=1357 VT=3541 VL=124 } 5022	1x1800 4x2000 } 9800	0.512 [*]	0.58	82 2
2 2624 200 30	VL=221 VT=2897 VR=33 } 3151	1x1800 4x2000 } 9800	0.322 [*]	0.37	50 2
3 1006 164	(VL=111) VR=181 } 181	3x1800 } 5400	0.034		
137 36	VR=151 (VL=40) } 151	2x1800 } 3600	0.042 [*]	0.05	12 2
TOTAL			0.876	1.00	150

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR
TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE
DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9-5(8)

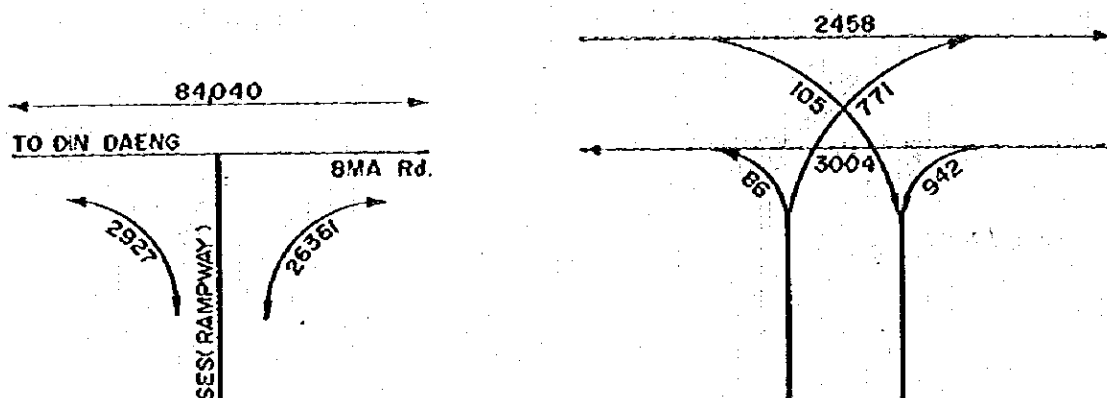
TRAFFIC ANALYSIS OF INTERSECTION(8)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 718

YEAR: 2000

NAME OF INTERSECTION: SES(E) x BMA ROAD ALONG THE NORTHERN BANK OF SAM SEN CANAL



VEH/ DAY

VEH/HOUR

TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $Y = V/Cp$	MODIFIED Y	PHASE TIME (SEC)
1	$\begin{matrix} 2458 \\ 3004 \\ 942 \end{matrix}$ VT=2714 VT=3316 VT=1040 } 4356	$\begin{matrix} 2 \times 2000 = 4000 \\ 2 \times 2000 \\ 2 \times 1800 \end{matrix}$ } 7600	0.679^* 0.573	0.74	81 + 2
2	$\begin{matrix} 105 \\ 86 \end{matrix}$ VR=116	1x1800 = 1800	0.06^*	0.07	12 + 2
3	$\begin{matrix} 771 \\ 942 \end{matrix}$ VL=95 VR=851 } 946	3x1800 = 5400	0.175^*	0.19	21 + 2
4					
TOTAL			0.914	1.00	120

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR

TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR = 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES = 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE DESIGN HOURLY VOLUME = 55.0 %

APPENDIX
FIG. 9-5(9)

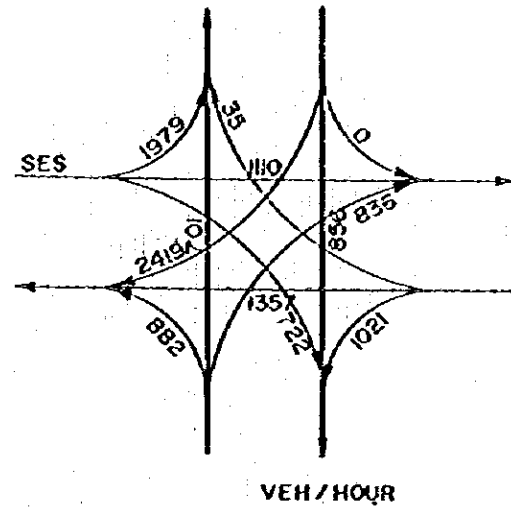
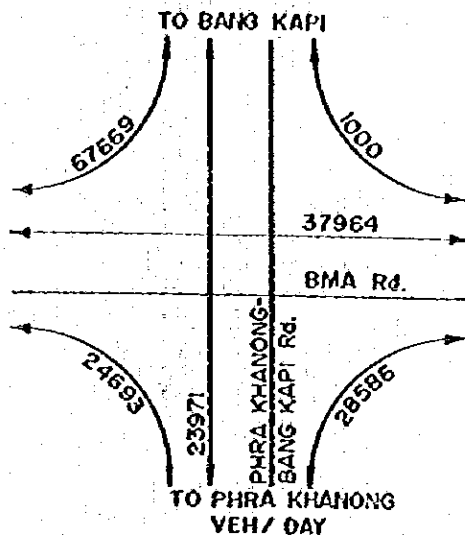
TRAFFIC ANALYSIS OF INTERSECTION(9)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 721

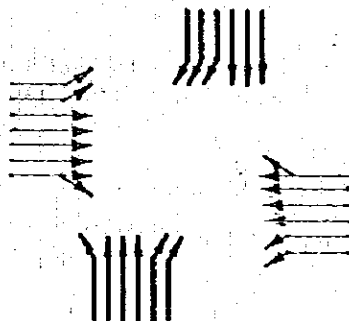
YEAR : 2000

NAME OF INTERSECTION : SES (E) x PHRA KHANONG - BANG KAPI ROAD
BMA ROAD



TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO Y = V/Cp	MODIFIED Y	PHASE TIME (SEC.)
1 1979 1110 722	VL = 2184 VT = 1225 VR = 797 4206	2x1800 5x2000 13600	0.30	0.30	43+2
2 35 1357 1021	VR = 38 VT = 1498 VL = 1127 2663	4x2000 2x1800 11600	0.22	0.22	31+2
3 2419 882 1021	VR = 2670 VT = 945 3615	3x1800 3x2000 11400	0.31	0.31	44+2
4 882 1021 356	(VL = 973) VT = 773 VR = 922 1695	3x2000 2x1800 9600	0.17	0.17	24+2
TOTAL			1.00	1.00	150

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH (PCU)/GREEN HOUR
TURNING LANE = 1800 VEH (PCU)/GREEN HOUR

2. PEAK FACTOR

= 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES

= 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE DESIGN HOURLY VOLUME

= 55.0 %

APPENDIX
FIG. 9 - 5 (IO)

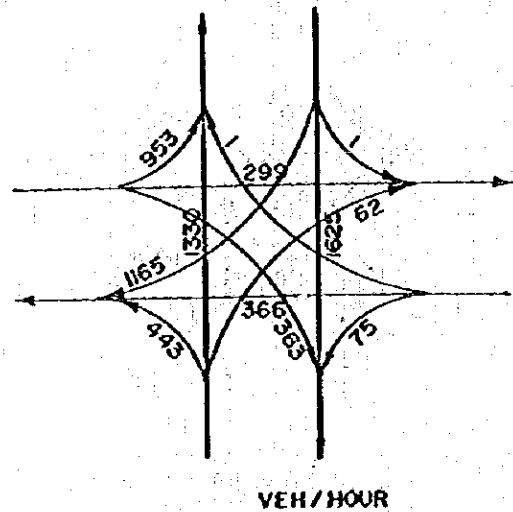
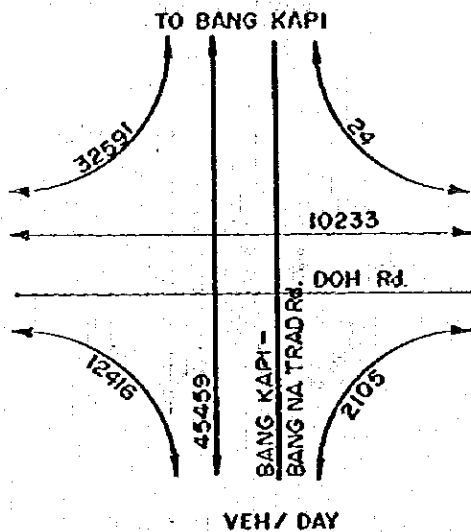
TRAFFIC ANALYSIS OF INTERSECTION (IO)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK

NODE NO. 722

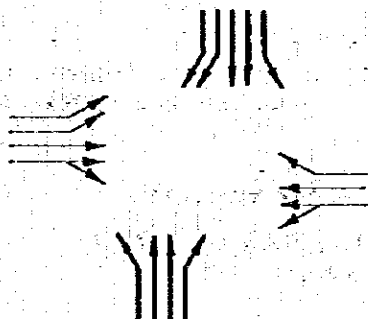
YEAR: 2000

NAME OF INTERSECTION: SES (E) x BANG KAPI - BANG NA TRAD ROAD
DOH ROAD



TRAFFIC PHASE (VEH/H)	TRAFFIC VOLUME V (PCU/H)	POSSIBLE CAPACITY Cp (PCU/H)	INTEGRATED CONGESTION RATIO $Y = V/Cp$	MODIFIED Y	PHASE TIME (SEC.)
1 953 299 363	VL = 1052 VT = 330 VR = 401 1783	2x1800 2x2000 7600	0.235	0.24	34 + 2
2 366 75	VR = 1 VT = 404 VL = 83 488	2x2000 1x1800 5800	0.084	0.08	12 + 2
3 1165 1330	VR = 1286 VT = 1794 VL = 1 3080	2x1800 2x2000 7600	0.405	0.41	57 + 2
4 443 62	(VL = 478) VT = 1468 VR = 68 1536	2x2000 1x1800 5800	0.265	0.27	39 + 2
TOTAL			0.989	1.00	150

LANE ARRANGEMENT



1. TRAFFIC CAPACITY PER LANE

THROUGH LANE = 2000 VEH(PCU)/GREEN HOUR
TURNING LANE = 1800 VEH(PCU)/GREEN HOUR

2. PEAK FACTOR

= 6.5 %

3. PERCENTAGE OF HEAVY VEHICLES

= 13.0 %

4. DIRECTIONAL DISTRIBUTION OF FUTURE DESIGN HOURLY VOLUME

= 55.0 %

APPENDIX
FIG. 9 - 5(II)

TRAFFIC ANALYSIS OF INTERSECTION(II)

THE SECOND STAGE EXPRESSWAY SYSTEM IN THE GREATER BANGKOK