Return Period	Excessive	e Value	Unit: mn Non-excessive Value				
	Calendar Year	Hydrological Year	Calendar Year	Hydrological Year			
2-year	1,798.9	1,773.2	1,798.9	1,773.2			
5-year	2,158.9	2,184.7	1,492.1	1,432.9			
10-year	2,371.2	2,433.0	1,350.2	1,279.1-			
20-year	2,560.4	2,657.3	1,241.7	1,163.2			
50-year	2,789.4	2,932.7	1,128.2	1,043.5			
100-year	2,952.6	3,131.3	1,057.2	969.6			

PROBABLE ANNUAL RAINFALL IN PROJECT AREA

	n ang sang ng mga sa					مەربىيە يەر بىرى مەربىيە بىرى بىرى بىرى بىرى بىرى بىرى بىرى بىر	Unit: m
Year	1Day	2-Day	3-Day	Yea	r 1-Day	2-Day	3-Day
1964	121.3	133.9	160.5	197	4 76.7	95.9	121.9
1965	98.2	176.7	177.0	197	5 133.3	118.2	142.8
1966	79.0	144.5	209.7	197	6 249.4	251.4	251.4
1967	87.1	136.8	136.8	197	7 55.4	94.7	142.8
1968	82.8	125.1	170.4	197	8 75.0	99.5	146.9
1969	67.0	106.3	118.4	197	9 182.1	332.1	374.1
1970	88.0	155.6	160.5	198	0 159.8	166.4	195.6
1971	128.2	203.9	211.0	198	1 130.0	130.0	131.0
1972	174.3	198.8	202.8	198	2 85.8	122.2	128.9
1973	151.3	233.8	249.1	198	3 85.0	102.0	132.4

MAXIMUM CONTINUOUS RAINFALL IN PROJECT AREA

PROBABLE CONTINUOUS RAINFALL IN PROJECT AREA

			unit: m
D <i>k</i> D <i>k</i> k		Excessive Value	
Return Period	1-Day	2-Day	3-Day
2-Year	103.8	140.8	165.4
3-Year	123.1	169.36	187.4
5-Year	145.9	202.4	213.6
10-Year	176.3	245.4	248.3
20-Year	207.2	288.0	283.4
50-Year	249.6	345.4	331.6
100-Year	283.2	390.1	369.8

		rn.	VDADUN) NATUC	UDU UT	TUAT	0 0111		and de antique a sub character a sub-	Unit:mm
Duration				Retur	n Peri	od (Ye	ars)			
وروب وروب وروب وروب وروب وروب وروب وروب	2	5	10	15	20	25	50	100	2001/	10001/
5 min	11.2 134.4	14.2 170.4	16.2 194.4	17.4 208.8	18.1 217.2	18.8 225.6	20.6 247.2	22.5 270.0		
10 min	18.0 108.0	23.5 141.0	27.2 163.2	29.3 175.8	30.7 184.2	31.8 190.8	35.3 211.8	38.7 232.2	tana ang sana ang sa Sana ang sana ang san Sana ang sana ang san	
15 min		30.3 121.2								
30 min	35.9 71.8								80.0 160.0	
60 min	45.8 45.8								105.0	
2 hrs	58.0 29.0	76.1 38.0				103.2 51.6				165.0 82.5
3 hrs									142.0 47.3	
6 hrs		107.9 18.0								251.0 41.8
12 hrs		140.6 11.7								400.0 33.3
24 hrs		173.4 7.2								540.0 22.5

PROBABLE RAINFALL AT ILOILO CITY

Note: Upper column value is in mm. Lower column value is in intensity of mm/hr.

 $\frac{1}{2}$ Estimate based on probability analysis.

			**************************************		Unit: u
Hour	10-Year	25-Year	Return Perio 50-Year	d 200-Year	1000-year
			an ha da anna _a (trudiganny sa pàr tha ba da an da sa firs da		
1	2.95	3.94	4,69	7.03	9.90
2	3.16	4.07	4.83	7.21	10.14
3	3.38	4.33	5.13	7.64	10.68
4	3.65	4,48	5.31	7.88	10.98
5	3.98	4.83	5.70	8.41	11.67
6	4.38	5.02	5.92	8.72	12.05
7	5.66	5.47	6.44	9.41	12.93
8	6.33	5.74	6.75	9.83	13.45
9	7.30	7.44	8.79	12.57	16.77
10	8.81	7.84	9.25	13.19	17.51
11	11.61	8.84	10.38	14.69	19.38
12	19.84	9.49	11.13	15.66	20.60
13	68.20	11.36	13.24	18.40	23.97
14	14,30	12.76	14.83	20.43	26.45
15	9.95	18.04	20.78	27.95	35.54
16	7.96	24.65	28.16	37.05	46.36
17	6.77	79.50	87.80	105.00	123.00
18	5.97	14.77	17.11	23.33	29.99
19	4.61	10.31	12.06	16.86	22.07
20	4.16	8.30	9.77	13.87	18.36
21	3,80	6.04	7.10	10.28	14.03
22	3.52	5.23	6.17	9.04	12.47
23	3.26	4.65	5.49	8.13	11.30
24	3.05	4.20	4.97	7.42	10.40
Total	216,60	271.30	311.80	420.00	540.00

DESIGN HYETOGRAPH FOR FLOOD ANALYSIS

Co-eff. **近**井 - 0 0.63 Runoff 12-0 0.40 0.84 0.53 0.56 0.58 0.85 18.0 0.67 0.37 0.64 0.47 0.51 11.76 95.95 63.64 17.73 35.35 102.73 18.98 119.59 22.10 44.06 62.65 11.58 23.08 23.45 Reinfall (MOM) 37.85 Annual 60.313 15.610 5.966 16.005 53 .033 5.860 10.932 53.191 4.678 86.172 76.396 12.299 (MOM) 6.964 20.130 25.386 Annual 1.69 0.15 1.91 0.19 1.68 0.19 0-50 0.51 0.22 19-0 2.42 0.39 0.35 2.73. 0.81 Mean (24-0) 1.66 0,04 0.39 0,17 2.53 0.51 Dec 0.21 0.31 2.74 0.36 1.14 0.34 0.88 1.81 0.29 0.61 0.08 0.43 3.27 0.80 0.23 Nov 2.33 0.31 1.40 4.58 0.86 1.87 0.7 0.40 3.34 01-0 0.12 0.60 2.08 5.5 2.29 0.13 0.57 1.36 00t 0.81 1.50 0.39 0.27 1.27 (0.80) (10:59) 2.43 0.26 7.72 1.76 0,30 0.76 0.72 0.97 5.60 0.26 0.57 2.52 0.37 Sep (01.0) (1.93) 6 # 0 3.06 0.55 0,40 0.88 1.36 2.59 0,12 7.81 Aug 1.49 0.57 7.02 0.27 Observed Monthly Mean Discharge (CMS) (0.20) (1.04) (0.85) Jul 0.49 0.85 0.27 1.61 0.21 0.23 2.87 0.27 0.35 4.43 0,30 0.51 (0.10) (3.24) (0:20) 0.41 Jun 144.0 0.20 0.30 0.22 0.42 2.73 0.64 0.10 0.04 76.0 10.12 (0.05) 0.50 0.14 0.22 10.0 0.06 0.34 0.03 May 0.28 0.26 0.06 0.0 0.05 0.13 0 (0.27) 0.04 0.60 0.05 0.18 0.03 0.05 0.18 1.45 0.03 0.05 Apr 0.06 0.31 0.17 0 0.20 3.02 0.04 0.29 0-09 0.05 Mar 0.06 0.03 0.37 0.39 0.06 70.0 1.09 0.08 Feb (0.44) (10.0) (0.39) 0.16 0, 13 10.0 0.27 0.61 0.07 1.05 0.45 0.06 0.02 0.08 0.21 Jan 2.40 0.18 0.12 0.07 0.35 0.34 0.44 0.15 1.41 0.18 0.33 0.05 0.33 1.37 0.27 Catipayan Catipayan Catipayan Catipayan Catipayan Serruco Serruco Serruco Serruco Serruco River Asue Asue Asue Asue âsue Year. 1979 1980. 1981 1982 1983

OBSERVED MONTHLY NEAN DISCHARCE

Note: Catchment Area: Catipayan (51.30 km²), Asue (9.48 km²), Serruco (18.90 km²)

Estimated values are shown with ()

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TABLE II-20 (1 of 2)

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	te Catly	NS 1969	52227 1/1 55156 1/3 5395 1/3	4789 4185 2193 202 202 202 202	33965 371 3572 373 3572 373	2335 4/1 2736 4/2 25550 4/2 25550 4/2	2369 5/1 2983 5/2 2489 5/2	7475 6/1 1781 6/2 2872 6/2	0026 7/2 8556 7/2 8556 7/2 8556 7/2	11122 8/1 1611 8/2 0781 8/3 0781 8/3	2799 9/1 9289 9/2 9392 9/3	635119/1 256518/2 419718/3 927618/3	125111/1 550411/2 447011/2 194211/3	18812/1 47812/2 39412/3	
	st C st	MCM	58124 0.6 58124 0.6 49849 0.5 57921 0.5		22744 8. 4 20952 9. 4 20957 9. 4 30654 9. 4	25570 0.2 23635 0.2 22698 0.2 71995 0.2		0982 0982 09864	77175 4.3 51175 4.3 76715 1.6 95025 7.0	82579 2.1 7791540.1 94253 3.0	10582 1. 2 61693 1. 2 433333 1. 6 15634 1. 6	10208	99799 1.1 57595 2.9 79520 5.4 27924 3.1	12941 1. 211 1385 0. 941 1385 0. 794 1985 0. 794	
	tleavan	W 8961		-000 7077	9000 1997	4444 - 46 99999 4444	6666 7997	-000 -000 -000	ษณุ⊶ต มหุ÷ตุ	-066 -066	-44-44 - 447	-066 -9995	96499 2622	13822 5005 5005	
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	leavan st C	MCM	2. 13654 2. 57875 1. 94282 1. 94282	0. 74719 0. 74719 0. 57494 2. 48060	0.71915 0.63281 0.65281 0.54084 0.192279	0. 42522 0. 37443 0. 33917 1. 13892	0. 31335 0. 31335 0. 32940 1. 27190 1. 90535	3. 49597 2. 27293 1. 48038 7. 15898	17149 17149 17245 99435	X. 52598 4. 72195 4. 47243 12. 38946	2. 74782 1. 69454 8. 90557 9. 34893	2. 50277 2. 91053 4. 03298 9. 47636	J. 61059 J. 23416 L. 07303 B. 96774	0. 85765 0. 92263 0. 74217 2. 53245 2. 53245	
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	rayan st C si	NCM	0. 75336 9. 5 0. 34795 9. 3 8. 78493 9. 3 2. 48994 0. 5	ස්ස්ට්ත්	0. 45575 0. 0. 37965 0. 1. 79495 1. 8 2. 62945 0. 9	8. 52258 9. 6. 45528 9. 6. 37646 9. 1. 34894 9.	8. 34216 9. 1 0. 30532 9. 1 1. 25054 1. 1 1. 89852 9. 7	8, 57592 9, 6 1, 62495 1, 9 3, 31359 7, 9 5, 57359 7, 9	1, 81764 2, 1 2, 73951 2, 7 6, 43814 6, 9 16, 79530 3, 9	1. 71550 1. 9 1. 47159 1. 6 4. 81447 5. 6 7. 95227 2. 9	3. 36791 3. 8 3. 97894 4. 6 1. 54041 1. 7 8. 88636 3. 4	73640 71840 45816 95936 95956 95056	1. 21056 1. 4 0. 82376 0. 5 6. 03136 7. 0 8. 11618 3. 1	2. 34133 2. 7 1. 89454 2. 1 1. 62754 1. 7 5. 85328 2. 1	
	10	\$ 1965	202	8888 2660 2660	2228	4444	2000 2000 2000 2000	8228 99999 99223	2012	00024 99990 402	8388 8388	1218/21 2418/21 2718/3	22172	1912/1 9912/3 2912/3 2012	
:	i st C site	MCM CMS	85583 8, 9918 73168 8, 5462 63125 8, 7273 27922 8, 8518	732/32 8. 848 76277 8. 848 44468 9. 575 94818 0. 774	44299 0. 5127 33295 0. 4444 44229 0. 4653 44229 0. 4653 26913 0. 4738	21239 0.2616 22554 0.3385 40519 0.4781 00411 0.3974	37754 9. 4367 91415 3. 4367 52252 3. 7654 71401 2. 5967	83848 1. 0339 29616 1. 5002 53024 4. 1438 77488 2. 2290	77613 5.5279 23531 1.4522 90395 9.2564 97039 2.6026	93737 5, 4592 84056 2, 4592 50955 3, 4518 52898 3, 1844	80196 2. 1955 82196 2. 1955 01328 3. 4275 63702 2. 5606	87282 3.3259 35522 2.7259 25497 2.3729 36411 2.89171	76086 3.19541 87022 5.79541 99611 6.93991 58720 5.24291	05715 2.381 21685 1.488 01683 1.682 22083 1.682	
	ti easan	19	6464 9494	ಥಿಷಿಷ್ಳಿ	4040 4040		2000 2000 2000	4444 9797	4490 4490	6000 00000 00000	പ്പിൾ	4994 4994 4966			

RIVER CATIPAYAN Ю ESTIMATED 10-DAY DISCHARCE

Catteasan

II - 45

TABLE II-20 (2 of 2)

ESTIMATED 10-DAY DISCHARGE OF CATIPAYAN RIVER

	wan st C site Catieusan st C site Catieauan st C site Catieavan st C site Catieusan st C site MCM - CMS toop MDM - CMS test - MCM - CMS 1982 - MDM - CMS 1933 - MDM - CMS	229 1/1 8. 78759 8. 9117 1/1 1. 19179 1. 3556 1/1 8. 75272 9. 6591 1/1 8. 72272 0. 737 1/2 8. 7528 1/2 8. 7521 1/2 1/2 8. 7521	21. 8.5577 8.756 2.1 8.7145 8.8235 2.1 8.4552 8.5419 2.1 2.3549 8.4461 2.2 8.6527 8.7761 2.2 8.61199 8.6732 2.2 8.42521 8.4592 2.2 8.2552 8.2552 2.3 8.46738 8.599 2.7 8.4799 8.6737 2.3 8.22321 8.4532 2.2 8.2552 8.2552 2. 1.77155 8.1159 2. 1.76135 8.7581 2. 1.26815 8.4344 2. 8.95817 9.3563	8. 43413 8.5255 371 8.55729 8.7587 371 8.33218 8.3544 371 8.25474 0. 2.33779 8.4599 372 8.4547 8.3575 325 8.2522 8.25252 8.25252 2. 2.22275 8.5599 377 8.4547 8.1721 373 8.95439 1.8048 243 8.22253 4.07364 1.5209 371 8.47136 8.5867 3 1.57983 8.5293 8.52233 8.	a 8, 5564 4/1 8, 35405 8, 4445 4/1 8, 59315 9, 4559 4/1 8, 15724 8, 3, 4, 421 4/2 8, 2422 9, 2494 4/2 8, 25275 8, 3459 4/2 8, 13122 8, 6, 4, 2564 4/7 8, 57535 1, 1291 4/3 8, 25577 0, 3039 4/7 8, 54494 8, 5 0, 4397 4, 1, 17035 8, 5562 4, 2, 95544 0, 3758 4, 8, 54494 8,	7678 5/1 8. 25895 9. 5972 5/1 9. 22392 9. 2787 5/1 8. 17212 8. 1746 5/2 9. 57975 9. 6816 5/2 1. 17452 1. 2753 5/2 8. 54595 9. 2645 5. 2. 44479 9. 9128 5 8. 57518 5/2 2. 44479 9. 9128 5 8. 51095 9.	6.1 1.23328 1.2023 6.1 0.4015 0.466 6.1 0.3337 9. 6.2 0.53315 0.4171 6.2 2.75415 0.3133 6.7 0.87287 1. 6.3 0.83519 1.0244 6.3 2.75413 6.31535 6.5 1.2223 3.5 6.3 0.27511 1.0617 6. 9.14188 3.5278 6. 0.31223 3.5	2,09570 2,4152 7/1 1,022962 1,1919 7/1 2,77541 4, 2,20354 4,675 7/2 1,15999 1,1914 7/2 5,7292 5, 1,95527 1,1107 7/3 2,69572 7,7169 7/3 4,15931 4, 7,34781 2,7454 7/3 4,73959 1,7916 7/3 5,5570 5,	1. 41225 1. 6427 8-1 1. 91781 2. 2195 8-1 2. 75716 3. 44233 0.5721 8-2 1. 5191 2. 5529 8-2 3. 21575 2. 4. 62399 0.5721 8-3 4. 35577 5. 0592 8-2 1. 56899 1. 2. 99161 1. 0333 9 11. 39530 4. 4433 8 6. 35765 2.	53770 6.6224 9-1 1.62281 2.1997 9-1 2.39994 4. 75625 2.972 9-3 1.39162 1.2899 9-2 2.1773 7. 17725 5.777 9-3 0.83114 0.9628 9-3 14.36199 4.	25399 1.561101 1.35311 2.25761941 4.31778 4. 36781 1.591102 2.82122 2.357123 78161 1.6741103 2.121723 2. 78161 1.6445103 5.8515 2.187218 6.95579 5.	22255 1.41901/1 0.66179 3.75591/1 2.85295 7.7482 2.90411/2 0.57369 3.877518 2.5125 2.4125 2.4125 2.4125 2.4125 2.41552 2.415552 2.415552 2.415552 2.415552 2.4	43597 2. 820512/1 9. 45927 9.542112/1 1. 34937 2. 82557 9.655122 9. 44952 9.48412/2 1. 12255 1. 22771 1. 29151227 9.69178 9.632212-2 1. 12555 1. 22771 1. 29151227 9.69188 9.632212-3 1. 4555 1. 86275 1. 681812 1. 47574 0.352812 4. 58145 1.	118.1.4768 Year 47.72810 1.5134 Year 71.74858 2
	at C site Catiessan at C site Catiesuan at C site Catiesuan at C site C 2m - CMS (coo - Mnn - CnS (cat - MCM - ChS 1882 - MCM - CMS 1	229 //1 8, 28759 8, 3117 1/1 8, 13159 1, 3676 1/1 9, 59523 8, 6501 1/1 235 1/2 8, 55245 1/2 8, 5321 1/2 8, 33518 1, 0245 1/2 9, 5245 7/2 9, 52445 7/2 9, 52445 7/2 9, 52445 7/2 1, 1246 1/2 27 1/2 1, 1246 1/2 25 1, 0549 1 2, 395499 1, 1555 1, 22 25731 8, 3479 1, 2475 1, 2554 2, 27 27 27 27 2, 27 27 27 2, 27 27 27 27 2, 27 27 27 27 27 27 27 27 27 27 27 27 27	1. 0.65372 0.7781 2.1 0.61196 0.8237 2.1 0.4552 0.5419 2.1 2. 0.67227 0.7781 2.2 0.61196 1.592 2.2 0.42532 0.4593 2.2 3. 0.45673 0.7781 2.2 0.45119 0.6537 2.2 0.42532 0.4532 2.3 1.75158 0.1787 2.1 0.4513 0.4594 2.1 20815 0.4594 2	43415 8.2255 571 8.65729 8.7667 3.1 8.23219 8.2364 5.1 2773 8.4299 5.2 8.4447 5.2575 5.2 8.2225 6.2368 5.2 22219 8.4299 3.7 8.44521 8.4721 5.3 8.2493 1.8048 5.3 87364 1.3209 3.1 5.7136 8.5367 3 1.57382 8.3293 3	8, 5564 47/1 8, 35436, 9, 4445 47/1 9, 35315 9, 4558 47/ 9, 4251 472 8, 34122 9, 37594 442 8, 25592 442 4, 2266 447 9, 37558 1, 1221 477 8, 25577 0, 3079 473 0, 4897 4 1, 76056 9, 6562 4 9, 9, 95544 0, 3685 4	54 6. 59395 9. 5972 541 9. 22392 9. 2787 541 552 9. 51775 9. 0016 552 1. 1655 552 552 9. 52365 8. 3775 552 1. 1655 553 553 56 1. 648359 8. 5175 55 2. 444779 9. 9128 5	1 1.29328 1.5023 5.1 6.40315 9.4565 5.1 2 9.53315 9.6171 6.2 2.76413 5.1933 5.2 3 9.83558 1.0344 5.3 2.37432 5.3153 5.2 2 2.72511 1.0517 5.3.14138 3.3278 5	00670 2.4152 771 1. 22392 1. 1919 771 20034 4. 4152 773 721 1. 11395 7. 7442 72 25557 1. 11107 77 2. 26357 2. 7446 72 21781 2. 7434 77 2. 73966 1. 7316 7	41226 1.6427 8.1 1.91761 2.2195 8 84333 0.7761 8.2 5.1531 5.9523 8 63939 0.6723 8.3 4.83677 5.0922 8 63999 0.6723 8.3 1.1.96579 4.4455 8	53776 6.6224 9-1 1.82281 2.1937 9-1 57662 5.7572 9-2 1.391621 7.789 9-2 4772 6.5778 0.82114 0.9628 9-2 59639 4.0858 9 4.15557 1.6622 9 1	L 5661 10-1 L 3521 2 257618 2 0991 10-2 2 8212 2 334719 1.8745 10-3 1.81772 1.970318 1.8445 10 5 85515 2 197219	1. 415911/1 0. 65173 2. 2. 030411/2 0. 75339 0. 9. 235711/3 0. 52369 0 1. 460411	2. 2206/12/1 9. 46327 9. 543112. 6. 9695 12/2 9. 40/55 0. 469412. 1. 291812/3 9. 69178 9. 637212. 1. 681912 1. 47574 9. 537812	1. 4768 Year 47. 72810 1.5134 Year
	st C site Catingvan st C site Catinavan st C site Catinavan st C si 24 C Mis toon MDK - ryis test, MCM - CMS 1982 - MDM - C	229 1/1 8.72849 8.9117 1/1 1.9129 1.3576 1/1 8.95528 8. 235 1/2 8.52849 8.9117 1/1 1.9129 1.3576 1/1 9.95228 8. 237 1/2 1.41566 1.4906 1/3 1.20245 1.6019 1/2 1.13787 8.	1. 0.65372 0.7565 2v1 0.71145 0.8235 2v1 0.40552 0. 2. 0.61227 0.7581 2v2 0.61196 0.7032 2v2 0.42521 0. 4.45730 0.5957 2v3 0.41799 0.6137 2v3 0.25942 0. 1.77105 0.7159 2v1 1.74135 0.7291 2v1 1.20915 0.	43415 0.5255 371 0.55729 0.7607 371 0.32210 0. 27739 0.4599 372 0.45447 0.375 5.25 0.2577 0. 23219 5.2903 373 0.45447 0.375 5.25 9.25429 0. 27210 5.2903 373 0.45420 0.4751 373 0.55439 1. 07264 1.5209 3 1.57155 0.5667 3 1.57983 0.	8. 5564 4/1. 6. 72406. 0. 4445. 4/1. 9. 39315. 9. 8. 4221. 4/2. 6. 52422. 9: 2934. 5. 6. 25272. 0. 8. 4266. 4/2. 6. 51528. 1. 1291. 4/2. 9. 25227. 0. 0. 4897. 4. 1. 170565. 6. 5552. 4. 9. 35544. 0.	57. 0.52695 0.532 54. 9.2332 0. 572 0.51275 0.6815 572 1.63525 1. 573 0.52565 9.5755 572 1.1422 1. 164855 0.5155 57 2.44479 0.	1 1.23328 1.5023 5.1 0.48715 9. 2 0.5315 9.617 6.2 2.75413 5. 3 0.5375 1.6344 5.3 2.37432 8. 3 2.72511 1.6517 6. 9.14188 5.	09670 2.4152 7/1 1.92982 1. 20034 4.1675 7/2 1.15999 1. 95557 1.1187 7/3 2.60978 2. 34781 2.7434 7 4.77968 1.	41326 1.6427 8.1 1.21761 2. 84338 0.9761 8.2 5.15191 5. 63299 0.5761 8.3 4.33677 5. 90167 1.9835 9 11.29539 4.	53775 9.6224 9.1 1.82281 2. 57692 5.2972 9.2 1.59162 1. 47572 6.376 9.3 0.83114 0. 59050 4.0853 9. 4.1555 1.	L 5651 10-1 1. 95921 2. 2. 0901 10-2 2. 82122 7. 1. 8745 10-3 1. 91772 1. 1. 8445 10 5. 85815 2.	1. 415911/1 0. 65173 2. 2. 030411/2 0. 75339 0. 9. 235711/3 0. 52369 0 1. 460411	2 2206/12/1 9. 46927 9. 9.9695/12/2 9. 40/59 8. 1. 29/8/12/3 9. 69/78 9. 1. 68/13/2 1. 47574 9.	1.4768 Year 47.72810 1.
	st C site Catimavan st C site Catimavan st C site Di CMS igen with CMS igen MCM CMS	22 1 22 1 222 1 222 1 22 1 22 1 22 1 2	1. 0.65372 0.7765 2-1 0.71145 0.8237 2-1 0. 2. 0.45727 0.7781 2-2 0.61196 2.5092 2-2 0. 3. 0.45739 0.7781 2-2 0.45759 0.6537 2.7 0. 1.779158 0.7159 2. 1.75135 0.7291 2. 1.	43418 0.3256 3/1 0.65729 0.7607 3/1 0. 235719 0.4359 3/2 0.45447 0.375 3/2 0. 235719 1.4350 3/3 0.4445 0.4751 3/3 0. 07364 1.13209 3/1 1.57136 0.3967 3 1.	0, 5664 4/1 6, 13406 0, 4445 4/1 9, 9, 4201 4/2 6, 13412 8, 1349 4/2 8, 1, 4206 4/2 8, 137528 1, 1221 4/3 9, 0, 4397 4 1, 17056 9, 6562 4 9,	5/1 8.5395 9.6912 5/1 9. 5/2 9.51975 9.6916 5/2 1. 5/3 0.51975 9.6916 5/2 1. 5/3 0.5295 9.5155 5/3 1.	4 1.23928 1.5023 6/1 0 2 0.53515 9.6171 6/2 2 3 0.53755 1.0344 6/3 5 2 72511 1.0517 6 2 72511 1.0517 6	03570 2.4152 7.1 L 20554 4.5675 7.2 L 05557 L.1107 7.7 2 34781 2.7434 7 4	41926 1.6427 8.1 L 84338 0.9761 8.2 5. 63392 2.5723 8.3 4. 99167 1.9833 8 11.	53775 9.6224 9.1 1. 51632 5.2972 9.2 1. 47522 6. 3775 9.3 0. 59059 4. 0353 9 4.	1. 2451 10.1 1. 1. 2991 10.2 1. 1. 2451073 1. 1. 24510 3 1.	1. 415911/1 2. 039411/2 3. 935711/2 9. 935711/2	2, 2205,1271 6, 9535,1272 1, 29181273 1, 29181273 1, 58191273	1. 4768 Year
	st C site Catimavan st C site Catimavan st C site Di CMS igen with CMS igen MCM CMS	229 1/1 (2010) 1/2 (200) 1/2 (2000) 1/2 (2000) 1/2 (2000) 1/2 (200	1 0.65372 0.7565 2.1 0.71145 0. 2 0.65227 0.7781 2.2 0.61199 0. 3 0.46738 0.5987 2.3 0.45795 0. 1.79155 0.7159 2 1.76135 0.	43415 8.5256 3/1 8.55729 8. 59739 8.4599 3/2 8.4547 9. 22219 3.5983 3/3 8.44552 8. 87364 1.5209 3 1.57136 9.	0. 7564 4/1 0. 35405 0. 9. 4321 4/2 0. 35405 0. 0. 4397 4 1. 70055 2. 0. 4397 4 1. 70055 9.	84. 0.92835 0. 6.92835 0. 6.92835 0. 1.64855 0. 1.64855 0.	1. 23928 L 2. 23928 L 2. 23928 L 2. 23928 L 2. 72511 L	09670 2.41 29554 4.55 97557 1.11	41926 1. 24232 0. 63292 0. 90165 1.	53775 8. 57632 5. 47572 6. 59039 4.		2. 0384	10	1, 47
	st C site Catleavan st C site Catisavan st Di CMS foed MDN CNS foet MDM	229 1/1 6, 78769 6, 9117 1/1 1. 235 1/2 6, 78769 6, 9117 1/1 1. 271 1/3 11, 41666 1, 47061 1/3 1. 63 1 2, 96283 1, 0689 1 3	1. 0. 55372 0. 7565 2/1 0. 711 2. 0. 67237 0. 7781 2/2 0. 611 3. 0. 45733 0. 5987 2/3 0. 457 4. 79156 0. 7159 2 1. 751	43413 8.5255 3/1 8. 39719 9.4539 3/2 9. 22219 3.5983 3/3 8. 97364 1.5209 3 1.	0, 5564 4.71 9, 4321 4.72 9, 4205 4.73 9, 4397 4 1, 79 9, 4397 4 1, 79	2000 -000 -000 -000	12392 2922 29251 292555 29355		1. 4192 9. 2433 9. 63299 2. 9016		5553 9050 7815 7815	10.4 M 20.4 M 20	3275	-
	st C site Catiravan st C site Ca CM - CMS topo - MCM - CMS te	23 1/1 6, 78769 8, 9117 23 1/2 1, 41666 9, 9117 27 1/2 1, 41666 1, 47621 53 1/2 2, 95283 1, 0589	1. 0. 65372 0. 7765 2 0. 65272 0. 7761 27 0. 46738 0. 5987 27 1. 79156 0. 7159 2	43415 0.5256 3 39719 0.5256 3 22219 1.7903 3 87364 1.5209 3	0, 5564 0, 4321 0, 4325 0, 4355 0, 4355 0, 4355 0, 4355 0, 4355 0, 4355 0, 4355 0, 4355 0, 4355 0, 43555 0, 4355 0, 4355 0, 4355 0, 4355 0, 4355000 0, 435500000000000000000000000000000000000	ທີ່ທີ່ທີ່ກ			그 주요 문헌	ದ ಗ ಗದೆ.	ಷಷನ್		40-14	46. 571
	st C site Catiravan st C s Chi Chis tese hinn	23 1/1 8.73249 9. 33 1/2 9.53249 9. 53 1/3 1.41666 1.	1 0.65372 0. 2 0.65237 0. 3 0.67237 0. 1. 79156 0.	43415 0. 39739 9. 22219 3. 07364 1.	රේ ශ්රා එ		9351 4154 1528 5878	4432 7.1 1974 7/2 1662 7/3 5882 7/3	9945 8-1 4077 8-1 5431 8-2 5825 3-3	8487 9/1 7554 9/1 33344 9/2 2315 9	318118/1 474218/2 333218/3 637618/3	914311/1 578411/2 377911/3 630111/3	342212/1 256912/2 422412/3 407412/3	3213 Vea
	st C site Catira CMS food	2922	995		4400 1400 1400 1400 1400 1400 1400 1400		59122 3. 11372 3. 31333 6. 05239 8.	82890 4. 16239 4. 09916 5. 51065 5.	2. 39585 2. 94423 2. 94423 2. 22443 2. 22445 2. 22445 2. 22445 2. 22445 2. 22445 2. 22455 2. 224555 2. 2245555 2. 2245555555555555555555555555555555555	1, 49219, 4, 2, 39151, 2, 5, 95819, 4, 9, 95819, 4,	5, 45331 5, 5, 59321 5, 4, 13039 4, 1, 23359 4,	1. 74099 2. 4. 84215 4. 1. 13275 1. 6. 37273 2.	1. 15956 1. 1. 98518 1. 4. 28582 4. 6. 44785 2.	32.57758 2.
	5	87.52	0,0000	4559 4550 4550 5655 573 5655 573 5655 573	8888 2444 2465	44 455 455 455 455 455 455 455 455 455	10048 99990 406	12 222 1282 12 222 1282 12 222 1282 12 222 1282	88:58 85:38	9999 9999 9999	1000	854811/1 223611/2 397811/2 492011/3	435312/1 136612/2 667312/3 824712/3	4721 Vear 3
• :	ត្តដ	52499 5249 5249 5249 5249 5249 5249 5249	49668 9. 51 49856 9. 51 32187 9. 45 31612 9. 54	48244 8.48 20307 1.29 44090 8.46 95951 8.76	11148 0. 20 26539 2. 62 76291 0. 30 76291 0. 32 76291 1. 29	28436 9, 44 24436 9, 44 43844 1, 50 85635 1, 14	03646 7, 04, 09023 9, 25 74527 3, 17 25289 6, 49	19750 1. 78 78490 0. 90 1133618, 00 09890 7, 13	91295 2.32 33323 5.97 67313 3.97 97349 3.76	07415 2,49 98252 3,55 16279 1,54 11945 2,36	90833 2.200 87422 2.15 87422 1.15 81533 1.79	69275 1. 22 85715 1. 22 20766 1. 37 86736 1. 37	23731 - 4 55751 - 4 455751 - 4 42291 - 2, 6 42291 - 2, 6	95140 2.4
	Catleau 1979		99994 9999-	9000 1000 1000 1000	4444 	0440 0440 0440	2000 2000 2000 2000 2000 2000 2000 200	10222	00000 04100 01100	9971 9972 9972 9972 9972 9972	10000 1000	ี มีมีมี พฤท	-1044 -1044 -1044	Year 77.
	C site C CMS		6 0.4572 0 9.4572 0 9.4573 0 9.4573 4473 4473 4473	0.3511	9. 2585 9. 2585 4. 4535	5 9.4483 1.0354 0.9784 9 9.8391	9. 5604 1. 4463 1. 9156	11.9076 11.7550 11.6072	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	1225 1225 1225 1225 1225 1225 1225 1225	7 4. 9970 4 5. 4533 9 1. 6918 0 3. 9396	1. 1. 569311/ 1. 1. 562311/ 3. 1. 211211/ 3. 1. 947711	1.0395	3 2. 6277
	NCM NCM		0, 4015 0, 4015 0, 2651 1, 9394 1, 9394 1, 9394	0, 30536 0, 20536 0, 27335 0, 84856 0, 84856	0. 21194 0. 22482 1. 85506 4. 38982	0, 33736 0, 33736 0, 33782 0, 32282 2, 24749	6. 48419 1. 23197 1. 655497 1. 655497 1. 78858	9, 87035 1, 31628 1, 92758 1, 92758	2, 76602 5, 36520 6, 30152 14, 95538	1.1795 2.9305 6.1793 9.5884	4. 31737 4. 71204 4. 71204 1. 5223 10. 55150	2. 65137 1. 55137 1. 55691 5. 94643 6. 94643	6.575 6.275 6.275 6.275 7.275	53. 94690
	lite Catly CMS 1978	" mm-+0	2005 2/3 2259 2/3 2259 2/3	6314 411 8537 472 8537 472 8537 472	7334 4/1 5914 4/2 5086 4/3	8522 44-1 8522 44-1 8523 44-1 8524 44-1 8554 44-10000000000000000000000000000000000	4425 6/1 1078 6/2 7782 6/3 4426 6/3	28743 7/1 2874 7/2 9595 7/3 8128 7/3	8493 8/1 5546 8/2 6179 8/3 3979 8/3	6087 9/1 43772 9/1 5293 9/3 5293 9/3	297718/1 192418/2 781519/3 574119/3	22112226 11112226 11112226	952812/1 804512/2 976912/3	9935 Vear
	aan st C.a	1. 54637 1. 1. 12698 1. 2. 44737 2. 2. 12072 1.	1. 13049 1. 1. 53151 1. 1. 53855 2. 4. 23056 1.	39117 74051 55350 285380	් ත් ක් ක් ක්	0. 73952 0. 0. 77959 0. 1. 09978 1. 2. 18819 9.	24633 27645 40038	ี เว่า - ห้า	6. 78130 7. 1. 35135 1. 9. 36738 1. 9. 10193 3.	11799 J. 10574 J. 33241 1	1. 90745 2. 2. 75820 3. 1. 69319 1. 6. 35834 2.	0. 84263 0. 9. 81263 0. 1. 32520 1. 2. 97786 1.	9. 82329 9. 9. 63488 9. 1. 81789 1. 2. 53538 9.	69. 02728 1.
	site Catipavan CNS 1977 N	7777	3116 2/1 7683 2/2 7671 2/3 8244 2	ta si tea		3599 5/1 8899 5/1 7652 5/2 7652 5/2	1022 6/1 0576 6/2 8998 6/3 3532 6	7343 7/1 8392 7/2 4893 7/3 6783 7	9645 8/1 2559 8/2 2559 8/3 4955 8	1 Berge	536118/1 955218/2 699118/3 029518/3	916111/1 925911/2 896111/3 232711	177112/2 501312/3 170412	1397Vear 6
	ប ដូន្ល	21534 1. 215093 1. 21569 1. 21534 1.	7876 58755 581555 58555 595555 595555 595555 595555 595555 5955555 595555 595555 595555 595555 595555 5955555 5955555 5955555 59555555	57374 0. 43938 0. 57362 9.	48551 8. 36532 9. 33644 0. 19996 8.	31821 0 73552 2 68125 2 72799 1	95227 1. 91374 1. 64143 1. 50743 1.	62446 8. 44527 2. 41544 1. 49517 1.	63340 0. 49675 6. 03823 3. 53839 3.	71697 4. 33658 1. 13333 1. 23693 2.	14150 J. 14020 J. 14028 J.	80875 80875 82226 47184 7	7260810. 74594 4. 37712 2.	663393 2.
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:		ನ್ನುಗಳ	いい たいしんきょう	លល់លក លំសំសំគ	4444 4421 9.9999 9.9999 9.9999 9.9999 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	1 1. 10393 2 1. 20583 3 8. 78092 3. 09665	ONMN JAM	1 8 75759 3 1. 76515 5. 72838 5. 72838	~ NN ~ - GO	9/1 2 64686 9/2 6.47739 9/3 2.89437 9/12.83879	 K1 K2 K2 K2 K2 K2 K2 K2 K3 K3 K4 K	1 5.55855 1.39737 3.1.39737 9.43656	2/1 L 95189 2/2 L 95189 2/3 7, 94195 2/3 7, 94195 2/3 11420	IL 98. 50570
	Site Ca CHC 10	2. 6695 1. 5486 1. 5486	1.1352	1. 1555 0. 7607 0. 6608 0. 8528	0. 5174 9. 5174 9. 5171	95550 955500 955500 955500 955500 955000 955000 955000 955000 955000 955000 955000 955000 955000 955000 955000 95500000000	1, 1579 1, 2568 1, 2568	2. 5758 2. 7258 3. 5697 3. 3697	1762	7962 6143 5128	9,465110 4,246510 2,726510 5,399510	4. 073311/1 5. 605411/2 1.843511/3 3. 840811	5. 096312 5. 096312 2. 377642 3. 543512	2. 2265 Yea
	C) to use	47910 41751 28151 17842	98082 98082 98082	9, 39815 9, 65726 9, 65726 2, 28346	44335	0. 59407 9. 55991 1. 60771 1. 87178	1. 00042 1. 55871 1. 08584 3. 45497	22547	74597	1, 47414 1 1, 39491 1 1, 3921 1 4, 17825 1	10191 59105 59105	4.51900	2. 91117 4. 32549 2. 255971 9. 49636	70.21640
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TABLE II-21 (1 of 2)

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	e Discharse	HCH CHS	0. 18845 9. 2191 0. 13332 0. 1613 0. 133354 0. 1613 0. 43554 0. 1639	8. 09418 8. 0773 8. 02125 8. 0342 8. 05251 8. 0349 8. 22785 9. 0342	0. 67479 0. 8365 0. 07120 0. 0326 0. 07237 9. 0331 0. 22476 2. 0333	0.06559 9.0759 0.06161 8.0713 8.05849 0.0676 8.18551 0.0716	0, 05522- 0, 0649 0, 05243, 0, 2697 0, 05416, 0, 8573 0, 16218, 0, 8605	0. 04597 0. 0544 0. 19159 0. 0544 0. 06539 0. 0303 0. 23404 0. 0903	8, 35182 9, 4182 9, 20771 9, 33541 9, 25321 9, 2776 9, 93334 9, 3485	0. 23442 0. 2713 0. 26039 0. 2003 0. 61634 0. 6420 1. 11126 0. 4142	. 61899 9. 7157 - 91861 1. 1789 - 96735 9. 9992 - 49996 9. 9645	29585 1. 4995 59161 0. 6733 57771 0. 6973	. 22559 8. 9572 . 94793 0. 9914 . 30159 1. 7064 . 20159 1. 1479	59295 1. 7395 52417 9. 7224 75974 9. 7994 85687 1. 0778	33068 0.4544
	schar se Asu	MCM CHS 1973	4216 1. 8385 1/1 4153 8. 7426 1/2 6915 8. 4226 1/2 5231 9. 7665 1/3	227371 0. 4956 2/1 22737 0. 5326 2/2 20959 0. 2584 2/3 84645 0. 3378 2	17969 8, 2879 371 19048 8, 28879 372 15115 8, 15959 373 11142 8, 1969 3	12394 8.1594 4.1 12296 8.1515 4.2 11689 8.1553 4.2 36898 9.1423 4	11283 8. 1306 5.1 10732 8. 1249 5.2 11733 9. 1193 5.2 33463 8. 1249 5	21572 8.2497 6/1 25798 9.2591 5/2 32485 9.3751 6/3 89685 9.3113 6	23337 9. 2707 7/1 9 19241 9. 2227 7/2 9 15515 9. 1738 7/2 9 59144 9. 2268 7/3 9	11534 0. 1347 8/1 8 18472 9. 1347 8/1 9 18643 9. 1962 8/3 9 49749 0. 1521 8	77657 8. 4289 9/1 8. 29988 8. 3471 9/2 1. 22995 8. 3369 9/3 9/3 9. 95142 9. 3789 9/3 0.	31278 8. 261518/1 1. 22275 9. 257410/2 0. 21520 9. 222518/3 8. 75297 9. 281118	99589 1. 843511/1 9. 31962 8. 378911/2 9. 26397 8. 385511/3 1. 48953 8. 574711 2.	163 9.476412/1 1. 116 9.476412/1 1. 618 9.233112/3 9. 927 9.358212/3 2.	339 8.3189year 14.
	scharse Asue Di	M CMS 1972 M	362 0.5435 1/1 0.94 054 0.3549 1/2 0.64 255 0.3568 1/3 0.66 871 0.3915 1/2 2.035	8.522 8.22457 9.22457 9.22457 2.245	14154 8. 1638 3.1 8.1 54447 8. 6302 3.2 8.1 19464 8. 2042 3.3 8.1 19465 8. 32365 3 8.5	12245 8. 1418 4/1 8. 1 09331 8. 1039 4/2 8. 1 12648 8. 2158 4/3 9. 1 39879 8. 1539 4 8. 3	33975 0. 3312 5/1 0. 11 19591 0. 2267 5/2 0. 11 41352 0. 4352 5/3 0. 11 94928 0. 3544 5 0. 3	0.5531 6/1 0. 7.4.728 6/2 0. 1.4728 6/2 0. 0.9618 6 0.	11.0500 7/1 0. 11.0934 7/2 0. 11.0934 7/2 0. 1.0951 7/3 0.	5477 0.6421 571 0.11 1752 0.5398 572 0.18 0996 0.5335 873 0.16 3125 0.5984 8 0.49	8.4248 9/1 9. 8.4013 9/1 9. 6.6013 9/2 9. 9.5007 9 9.	1. 1984 18/1 6. 8. 9379 10/2 8. 9. 7927 18/3 8. 8672 18/3 8.	8.938111/1 8. 8.829911/2 8. 8.742911/2 9. 9.837911/3 9.	38 0.547412/1 8.4116 46 9.477712/2 8.2814 53 9.401512/3 9.2651 30 9.473912 9.3592	10 9. 5684Vear 10. 093
	scharse Asua D1	CMS 1971 MC	0. 1964 1/1 0. 46 0. 1522 1/2 0. 46 0. 1175 1/3 0. 27 0. 1541 1 1. 04	47 8. 1824 2/1 8. 21785 18 8. 1176 2/2 8. 21723 95 8. 1663 2/3 8. 14731 55 8. 1253 2 8. 57895	0. 1145 371 9. 1445 371 9. 1443 372 9. 8. 1194 3 8. 1194 3 9. 898 9. 9. 898 9. 9. 998 9. 9. 998 9. 9. 998 9. 9. 998 9. 9. 998 9. 9. 998 9. 9. 1194 9. 9. 1194 9. 9. 1194 9. 9. 1194 9. 9. 1194 9. 9. 1194 9. 9. 1195 9. 9. 1194 9. 9. 1195 9. 1195 10. 9. 1195 10. 9. 10	0.0635 4/1 0.0635 4/1 0.0637 4/2 0.0637 4/2 0.0561 4/5 0.0561 4/5 0.0561 4/5 0.0561 4/5 0.0561 4/5 0.0561 0.05601 0.05601 0.0561 0.0561 0.05601 0.0561 0.0561 0.05601 0.0561 0.05	2 8. 0732 5/1 8. 5 0. 1527 5/1 8. 5 0. 1527 5/2 9. 7 8. 1437 5 9 9.	27 0. 1945 6/1 0. 42649 91 0. 1527 6/2 0. 42649 72 0. 7254 6/3 1. 27249 91 0. 5275 6 2. 49291	0. 5556 7/1 0. 5556 7/1 0. 5346 7/2 0. 4333 7/3 0. 5417 7	0.1111 8/1 0.0 0.4111 8/1 0.0 0.4186 8/1 0.0 0.4186 8/1 0.0 0.4594 8/1 0.0	16 0. 3703 9/1 8. 42673 19 0. 4478 9/2 0. 34725 16 9. 5207 9/3 0. 52381 1. 9. 4464 9 1. 29785	8 8.372118/1 8.95764 12 9.667818/2 8.65724 8 9.444418/3 0.65731 1 8.493218 2.3259	NHMA	5 8.53512/1 0.47238 6.533212/2 0.41445 2 9.412912/5 6.32158 5 9.488112 1.25905	6 9. 33564ear 17. 9241
	charse Asue Dis	CMS 1978 NCM	22 0.1609 1/1 0.15269 11 0.1398 1/2 0.13148 13 0.1859 1/3 0.11167 13 0.1346 1 0.41284	11 8. 0355 2.11 8. 03647 55 8. 0343 2.22 8. 19118 5 8. 0792 2.25 8. 11435 19 8. 0378 2 2 8. 30455	1 0.0736 3/1 0.0337 13 0.0729 3/2 0.12337 18 0.061 3/3 0.07293 18 0.0691 3/3 0.071938 12 0.0733 3 0.319392	11 9, 9549 4/1 8, 95315 19 0, 9583 4/2 8, 05597 25 9, 0578 4/3 8, 95192 25 9, 9604 4 9, 16614	9, 0533 5/1 0, 953 9, 9713 5/2 0, 195 9, 2272 5/3 0, 147 9, 1212 5 0, 394	1 0.4518 6/1 0.0902 2 0.2235 6/2 0.1319 2 0.2655 6/3 0.1319 3 0.2970 6 0.9483	0 9,4329 7/1 0,49595 7 9,7977 7/2 0,59959 1 9,4497 7/3 9,45931 8 9,5275 7 1,46985	2 0. 3476 8/1 8. 30575 2 0. 4223 8/2 0. 30119 6 9. 35569 8/3 0. 30281 7 9. 4457 8 0. 9 9. 39249	3 8. 5758 9/1 8. 3203 5 9. 3379 9/2 8. 3863 6 9. 3485 9/3 8. 4495 5 8. 4466 9/1. 1571	6 9. 319318/1 9. 32145 5 8. 357318/2 9. 57792 7 9. 279010/3 8. 42249 8 9. 314418 1. 32891	3 8.239611/1 8.3935 6 9.293811/2 8.9522 2 8.138311/3 8.5556 9 8.213911 1.9213	7 0. 157612/1 0. 4537 8 0. 243712/2 0. 4529 3 0. 217612/3 0. 3915 7 0. 217612/3 1. 30733	8 8. 2456Veer 12. 58440
· .	arse Asue Disc	CM3 1969 MCM	6 8. 1552 1/1 0. 15982 9 9. 1161 1/2 0. 12091 7 0. 8958 1/3 0. 12094 3 0. 1215 1 0. 36043	2 9. 0317 2/1 9. 08251 2 9. 0876 2/2 9. 07355 1 9. 0863 2/3 9. 05473 2 9. 9886 2 3/3 9. 05473	1 0.0814 3/1 0.0688 3 0.0777 3/2 0.0623 7 9.0738 3/3 0.0523 8 0.8735 3/3 0.19645 8 0.8775 3 0.1964	7 0. 0697 4/1 0. 0557 1/1 2. 0. 0658 4/2 0. 05209 0. 0628 4/3 0. 04925 0. 0659 4 0 0. 04925	1 9: 9585 5/1 8: 94648 5 9: 9561 5/2 8: 95218 1 8: 0538 5/3 8: 21532 6: 0558 5/3 8: 22459	0.1135 6/1 0.1135 6/1 0.1255 6/2 0.1254 6/3	0. 3494 7/1 0. 3749 0. 3016 7/2 0. 6114 0. 2762 7/3 0. 4274 0. 3889 7 1. 4128	9. 2603 8/1 8. 3003 9.339 8/2 8. 3648 9.5141 8/3 8. 2548 8. 5593 8 1. 1936	0, 3518 9/1 0, 49745 0, 4182 9/2 0, 34375 9, 2035 9/3 0, 34375 0, 3385 9/3 1, 14215	0. 302110/1 0. 2650 1. 046110/2 0. 3005 0. 5330310/3 0. 2671 0. 623310 0. 6420	0.357011/1 0.29703 9.453311/2 0.17696 9.617311/3 0.17696 9.479211 0.55440	8. 354112/1 8. 14477 9. 220312/2 8. 21033 9. 212312/3 8. 22555 9. 280012 8. 58117	9. 2637 vear 7. 74499
	Brad Asue Disch	CMS 1968 - MCM -	8. 3275, 1/1 8, 1349 8. 3785, 1/2 8, 10749 8. 3175, 1/2 8, 69197 8, 3275 [8, 3254	B. 2659 2/1 0. 87919 B. 1948 2/2 8. 87572 B. 1559 2/3 0. 95711 B. 1929 2 8. 22292	8. 1359 5/1 8. 8793 9. 1959 5/1 8. 9571 9. 6924 3/3 8. 9. 6761 8. 1111 5 8. 2075	0.0876 4/1 0.86023 9.0931 4/2 0.05537 0.0794 4/3 0.05560 0.0834 4 0.17070	80000	0.4453 6/1 0.11540 0.2688 6/2 0.79805 0.2033 6/3 0.10938 0.3058 6 0.3 0.538	0. 5691 7/1 0. 30191 9. 3347 7/2 0. 26961 8. 3323 7/3 6. 26249 0. 4288 7 0. 92499	9. 5139 8/1 6. 27/37 0. 5426 8/2 0. 91172 0. 6853 8/3 0. 42965 0. 5553 8/3 1. 52324	8. 4946 9/1 0. 39395 9. 3929 9/2 0. 36133 8. 3142 9/3 0. 26497 0. 4806 9 0. 32935	9.334610/1 0.26196 0.415218/2 0.99354 0.580318/3 0.50449 0.433310 1.65937	6.558011/1 6.51765 6.548011/2 6.59161 8.571611/3 6.50354 9.485811 1.24234	8.299712/1 8.30592 8.254212/2 8.24721 8.260012/3 8.20181 8.249712 8.74994	0. 3065Vear 8. 33850
	se Asue Dischar	CMS 1967 MCM	2619 1/1 0.28392 2261 1/2 0.29346 1901 1/3 0.30176 2248 1 0.87724	1405 2/1 6. 17828 1858 2/2 0. 15829 8876 2/3 0. 11467	0349 3/1 0, 11741 0305 3/2 0, 93238 0764 3/3 0, 09778 0905 3 0, 29756	8721 4/1 8.87565 9587 4/2 8.97184 8548 4/3 8.05857 8585 4 8.21697	8614 5/1 0.86537 8628 5/2 0.96537 86115 5/3 0.16235 6115 5/3 0.12454 8149 5 0.25222	4108 6/1 0.38474 3478 6/2 0.23227 4378 6/3 0.17555 4149 6 0.79266	3918 7/1 8.49176 6486 7/1 8.49176 5181 7/2 8.34185 5135 7 1.14868	4399 B/1 0,44324 5737 B/2 0,4681 5737 B/3 0,57526 5737 9/3 0,57526	4699 971 8, 42735 4338 971 8, 42735 4926 973 0, 33942 4449 9 1, 83825	367810/1 0.33226 398318/2 0.35359 430318/3 0.47551 399916 1.16647	555111/1 8,46482 557511/1 8,46482 557511/2 8,47347 557511/3 8,75192 557111 1,25931	483812/1 8.25395 315812/2 8.25395 262612/3 8.1986 251112 8.1986	3248Vear 9,66617
	Asue Dischar	1S 1966 MCM	2334 1/1 0, 22619 0 1373 1/2 0, 19512 0, 1654 1/3 0, 18071 0, 1656 1 0, 60222 0,	1364 2/1 8,12169 8, 1008 2/2 8,02869 8, 0906 2/3 8,06055 8, 1113 2 8,27293 8,	0874 3/1 0.0735 0.0834 0.0831 0.0931 0.095961 0.0111 0.05961 0.0111090 0.73 0.017259 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.011254 0.00112564 0.00112564 0.00112564 0.00112566 0.00112566 0.00112566 0.00112566 0.00112556 0.00112556 0.00112556 0.00112556 0.00112556 0.00112556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.001125556 0.0011255556 0.0011255556 0.0011255556 0.001125555556 0.00112555555555555555555555555555555555	0993 4/1 0.05227 0. 0773 4/2 0.05938 0. 0712 4/3 0.05560 0. 0826 4 0.17764 0.	0688 57.1 8.05594 0. 6647 572 8.74480 0. 1279 373 0.59129 0. 0882 5 1.57903 8.	0888 5/1 8.35434 0. 1541 5/2 8.29377 9. 3111 5/3 9.42939 9. 1844 5 1.97539 8	2222	147 8/1 0. 37938 0. 43 8/2 0. 37938 0. 197 8/3 9. 35935 0. 378 8 1. 94357 0.	4955 9/1 8.35335 8. 5878 9/2 0.37413 0. 3882 9/3 8.42551 8. 4879 9 1.15369 8.	158818/1 0.31722 0 425318/2 8.34415 0 395418/3 0.40909 0 395218/1 1.07097 0	2222	87 12/1 0, 41833 0, 14412/2 0, 27232 0, 17312/3 0, 21955 0, 15512 0, 94049 0,	66Year 10.21998 0.
	Asue Discharse	1965 MCM CM	1/1 8. 28684 9. 1/2 8. 17651 9. 1/3 8. 15726 9.	2/1 8.11956 8. 2/2 8.897956 8. 2/3 8.96265 8. 8.26936 8.	3/1 0.07551 0. 3/2 0.07551 0. 3/3 0.19041 0. 3 0.32773 0.	4/1 0.05530 0. 4/2 0.055330 0. 4/3 0.05533 0.0 0.05148 0. 8.21489 0.	511 0. 85872 9. 572 0. 955972 9. 573 0. 12151 9. 6. 27616 9.	6/1 8.87698 8. 6/2 8.13315 8. 6/3 8.26377 9. 6 8.47792 8.	7/1 8.21423 9.2 7/2 8.21423 9.2 7/3 9.67594 9.71 7 1.13248 9.47	8/1 8.29782 9.34 8/2 8.29155 9.31 8/3 8.49394 9.51 8/3 1.96334 9.55	9/1 8.42936 8. 9/2 8.50714 8. 9/3 0.35646 8. 9/1 1.26466 8.	18/1 8.38999 0. 18/2 8.35745 0. 18/3 9.35745 0. 18/3 9.37579 0.	11/1 8.25781 9.25 11/2 8.21473 8.24 11/3 8.21473 9.82 11/3 8.75123 9.48	12/1 8.32719 8.57 12/2 8.30524 8.35 12/3 8.23262 8.35 12/3 8.325252 6.35 12 8.325555 8.35	Veter 8.72271 9.27
	SIG #1	964 NCM CMS	71 8.15335 8.2313 72 8.15525 8.1797 73 8.15375 8.1797 8.13875 8.1376	2/1 8.19735 8.1242 2/2 8.99643 8.1146 2/3 8.05394 8.1899 2 8.27372 8.1899	3/1 0.97344 0.0859 3/2 0.95369 0.0867 3/3 0.97391 0.0778 3/8 0.21705 0.0919	4/1 0. 96282 0. 0727 4/2 0. 05985 0. 0533 4/3 0. 05966 0. 08905 4/3 0. 19234 0. 0742	5/1 0. 05830 0. 0675 5/2 0. 25275 0. 2925 5/3 0. 31354 0. 3299 6. 62459 0. 2333	11 8. 15357 8. 1848 112 8. 15344 9. 1832 115 8. 165344 9. 1832 115 8. 53189 8. 3574 8. 63188 8. 2438	7/1 B. 50356 0. 5828 7/2 B. 50356 0. 5828 7/3 P. 25579 B. 2655 7/3 P. 23444 B. 2467 7. 1. 00179 D. 3740	11 0.32651 0.5779 0.27175 0.3779 13 0.37977 0.4286 0.99881 0.3726	11. 0. 26282 0. 3742 22. 0. 27552 0. 3785 73. 0. 34131 0. 3958 6. 87565 0. 3354	MNP	11/1 B. 35182 B. 4972 11/2 B. 66129 G. 7654 11/3 P. 65615 B. 7563 11 1. 64924 B. 6363	1 8.40141 8.4645 2 6.33012 8.3821 3 8.23373 9.3831 1.92531 8.3828	ar 9.00776 0.2649
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ESTIMATED 10-DAY DISCHARGE OF ASUE RIVER

STIMATED 10-DAY DISCHARGE OF ASUE RIVEH

TABLE II-21 (2 of 2)

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	CATIP	YAN	AS	UE	
	YEAR OCCURRED	MONTH (m ³ /s)	DISCHARGE OCCURRED	MONTH (m ³ /s)	DISCHARGE
	1964	APR	0.331	МАУ	0.068
	1965	MAY	0.335	MAY	0.065
• * •	1966	MAY	0,282	MAY	0.061
	1967	MAY	0.363	MAY	0.072
	1968	MAY	0.248	MAY	0.053
	1969	MAY	0.245	MAY	0.054
	1970	APR	0.245	APR	0.060
	1971	APR	0.633	APR	0.104
	1972	MAY	0.445	MAY	0.120
. '	1973	MAY	0.257	JUN	0.054
	1974	MAY	0.417	MAY	0.113
	1975	APR	0.454	APR	0.102
	1976	MAY	0.359	MAY	0.098
	1977	MAY	0.452	MAY	0.085
	1978	APR	0,260	APR	0.056
	1979	APR	0.386	APR	0.066
	1980	MAY	0.385	MAY	0.060
	1981	APR	0.395	JUN	0.084
н., н.	1982	MAY	0.271	APR	0.046
	1983	MAY	0.191	MAY	0.024
	Mean		0.348		0.072

ESTIMATED 10-DAY AVERAGE LOW WATER FLOW

PROBABLE 10-DAY AVERAGE LOW WATER FLOW

			· · ·					Ui	nit m ³ /s
Return Period	1/100	1/75	1/50	1/30	1/20	1/10	1/5	1/3	1/2
Catipayan	0.115	0.163	0.175	0.191	0.206	0.235	0.272	0.309	0.351
Asue	0.022	0.024	0.027	0.031	0.035	0.042	0.051	0.060	0.071

										n in in Saint Saint	Uni	t: H: Q:	m CMS
1984		Ju] H	Q.	Ju H	L.30 Q	Aug H	, 10 Q	Aug H	g.23 Q	Se H	p.2 Q	Se) H	2.3 Q
6:00 7:00 8:00	a.m							· .				0.84 0.81	6.74 6.29
9:00								1.			÷		÷
10:00					•						e e e e e e e e e e e e e e e e e e e		
12:00	p.m.				-			4 ¹			·		:
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2:00	• 、				i di second		· .				6.59		. •
3:00		. A. 191	.÷.,			2. S					13.13	÷	•
4:00		0.83	6.59			· · · · · ·		· · · · · · · · · · · · · · · · · · ·	7.53		15.00	1.	
5:00		0.94	8.34	1. A. 1. A. A.	· · · · ·	~ ~ ~			21.70		14.37		
6:00	· .	0.97	8.85		C ml	0.86	7.05		15.86	. – .	15.43		
7:00	-	0.94			6.74	1.02			12.34		14.79	1.	
8:00	1. A. A.	0.88	7.37	0.96	1 S.		12.73		9.72		13.54	5.	
9:00 10:00	• .	0.82	6.44	1.04 0.94	10.08 8.34		13.34 12.54		7.37		12.93 11.95		**. [*]

CATIPAYAN RIVER FLOOD RECORD

		: 		:		en Angelan			• • •			
1984	Se	p.16	Ser	5.17	Se	p.18	Ser	5.19	Se	p.20	Se	o.21
· .	Н	Q	Ĥ	Q	H	Q	Н	Q	H	Q	H	Q
6.00 -				•	1 56	21.20	9.94	0 2 1	1 01	10.08	1 10	17.42
6:00 a.m. 7:00						19.51	0.91		0.98	and the second process	A	16.08
8:00		1 - E				16.97	0.86		0.92	A DECEMBER OF	. –	14.79
9:00						15.21		• • • =		7.69		13.54
10:00						13.74			0.90			12.73
11:00			. *	ан сайта. Тарала		12.34			0.89			11.95
12:00 p.m	1.	·		1.		11.19			0.88			12.54
1:00					1.05	10.26	a ta la serie		0.87	7.21	1.19	12.93
2:00		n de la composición d	1.64	23.21	1.02	9.72		· · ·	0.85	6.90	1.09	11.00
3:00	2.42	46,30	3.98	115.09	0.99	9.19	0.94	8.34	0.84	6.74	1.02	9.72
4:00	1.96	32.03	3.30	81.04	0.97	8.85	2.04	34.37	0.83	6.59	0.96	8.68
5:00	1.55	20.96	2.10	36.14	1.01	9,54	2.76	58.52	0.82	6.44	0.92	8,01
6:00	1.48	19.27	1.80	27.47			2.96			6.14	0.87	7.21
7:00	1.23	13.74	1.58	21.70			3.34	82.87	1.1		0.82	6.44
8:00	0.98		1.32		1 N N 1	11.37	3.99	115.64			0.91	7.85
9:00	0.82	6.44	1.12		1.1	9.02		107.02			0.99	9.19
10:00		n an the state The state of the	0.94	8.34	0.97	8.85	3.76	103.36	*	1	1.19	12.93
			1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		1							· ·

CATIPAYAN RIVER FLOOD RECORD

Unit: H : m

						Q : CMS
1984	Sep.22	Sep.23	Sep.30	Oct.7	0ct.8	Oct.9
	H Q	H Q	H Q	H Q	H Q	H Q
6:00 a.m.	1.22 13.54	0.97 8.85	1.60 22.20	0.94 8.34		
7:00	1.20 13.13	0.94 8.34	1.59 21.95	0.93 8.18	. 1.	
8:00	1.17 12.54		1.48 19.27	0.90 7.69		
9:00	1.11 11.37	0.93 8.18	1.38 16.97	0.89 7.53		
10:00	1.05 10.26	0.92 8.01	1.27 14.58	0.89 7.53		
11:00	1.01 9.54	0.90 7.69	1.15 12.14	0.87 7.21		
12:00 p.m.	0.96 8.68	0.89 7.53	1.05 10.26	0.84 6.74		
1:00	1.21 13.34	0.88 7.37	0.94 8.34	0.82 6.44	0.84 6.74	
2:00	2.20 39.18	0.87 7.21	0.91 7.85		0.88 7.37	
3:00	2.55 50.80	0.85 6.90	0.87 7.21		0.92 8.01	1.10 11.19
4:00	1.83 28.30	0.84 6.74			1.11 11.37	1.19 12.93
5:00	1.70 24.77	0.83 6.59	0.84 6.74		1.22 13.54	1.30 15.21
6:00	1.58 21.70	0.81 6.29	0.82 6.44		1.21 13.34	1.42 17.87
7:00	1.45 18.57	0.80 6.14			1.17 12.54	1.41 17.64
8:00	1.30 15.21				1.12 11.56	
9:00	1.13 11.76				1.06 10.44	
10:00	1.03 9.90				0.89 9.19	1.31 15.43

1984	Oct.14	0c	t.19	0et	t.20	0et	t.21	No	v.5		
	H Q	Н	Q	Н	Q	Н	Q	Н	Q .	Н	Q
· · · · · · · · · · · · · · · · · · ·			··.								
6:00 a.m.	2.42 46.30	1.20	13.13	0.84	6.74	1.04	10.08				
7:00	3.38 84.71	1.20	13.13	0.82	6.44	1.02	9.72				
8:00	1.28 14.79	1,22	13.54	0.82	6.44	0.99	9.19	· .			
9:00	1.25 14.16	1.28	14.79	0.81	6,29	0.97	8.85	0.84	6.74		
10:00	1.23 13.74	1.34	16.08	0.81	6.29	0.96	8.68	2.90	63.99		
11:00	1.19 12.93	1.41	17.64	0.80	6.14	0.96	8.68	3.99	115.64		
12:00 p.m.	1.16 12.34	1.44	18.33			0.95	8.51				
1:00	1.12 11.56	1.40	17.42		÷ 4.	0.93	8.18				
2:00	1.10 11.19	1.70	24.77	0.80	6.14	0.91	7.85				
3:00	1.07 10.63	1.49	19.51	0.83	6.59	0.88	7.37				
4:00	1.03 9.90		16.97	1.48	19.27	0.85	6.90				
5:00	1.00 9.37		15.21	2.04	34.37	0.82	6.44				
6:00	1.00 9.37		13.54	2.40	45.63	1.23	13.74				
7:00	1.09 11.00		12.73	2,38	44.96	1.56	21,20				
8:00	1.07 10.63		11.56		42.99		20.47				
9:00	1.06 10.44	and the second second	10.63		41.38		19.99				
10:00	1.06 10.44				40.11	1.49					

FLOOD HYDROGRAPH FOR DAM DESIGN

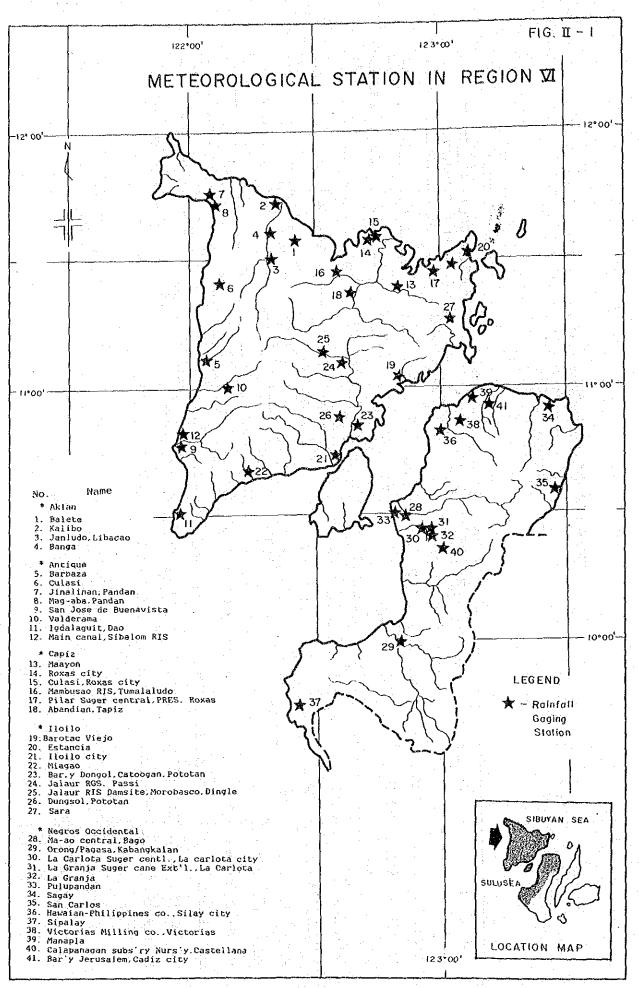
Hour	10-Year	200-	Year		Year
· . · · ·	Rainfall Drainage	Rainfall	Drainage	Rainfall	Drainage
	20 60	7.0	6.0	9.9	6.1
1	3.0 6.0		6.4	10.1	7.0
2	3.2 6.1	7.2		10.7	9.9
3	3.4 6.2	7.6	7.7	11.0	15.3
4	3.7 6.5	7.9	10.0		23.3
5 6	4.0 7.0	8.4	13.4	11.7	34.7
6	4.4 7.8	8.7	18.4	12.1	34.7 49.4
7 8	5.7 9.0	9.4	24.9	12.9	
8	6.3 11.2	9.8	33.6	13.5	68.9
9	7.3 14.3	12.6	45.6	16.8	95.7
10	8.8 19.1	13.2	66.2	17.5	138.0
11	11.6 27.5	14.7	91.5	19.4	171.9
12	19.8 46.0	15.7	125.0	20.6	200.5
13	68.2 130.8	18.4	156.2	24,0	226.0
14	14.3 358.7	20.4	187.4	26.5	258.2
15	10.0 269.6	28.0	221.1	35.5	295.8
16	8.0 202.8	37.1	281.2	46.4	365.6
17	6.8 166.1	105.0	414.6	123.0	519.7
18	6.0 138.9	23.3	703.7	30.0	847.1
19	4.6 117.7	16.9	512.6	22.1	628.0
20	4.2 100.0	13.9	307.8	18.4	476.0
21	3.8 87.1	10.3	246.8	14.0	387.6
22	3.5 76.9	9.0	174.2	12.5	314.9
23	3.3 68.5	8.1	145.5	11.3	263.1
24	3.1 62.3	7.4	174.2	10.4	227.7
25	54.7	• • •	145.5	44 - C.	191.4
26	36.5	1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -	95.6		124.3
27	25.6		65.7		84.5
28	18.2		48.0		61.0
29	14.3		35.8	-	44.9
30	11.7	· .	26.6		33.0
31	10.0		19.7		24.0
32	8.7		13.6	4 . L	16.4
<u>∍</u> ∠ 33	7.9		11.0		12.8
	(+) 7 0				
34	7.2 6.8 6.5 6.3		9.2		10.4
35	0.0 2 E		8.0		8.8
36	0.5		7.2		7.7
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NAME	: ADDRESS :	: COURCE : COORDINATES PERMIT: DATE : METHOD :IRRIGABLE: AMOUNT : AMOUNT : AMOUNT : SOURCE : ANTITUDE:LONGITUDE: NO. :GRANTED : OF : AREA :AVAILABLE:GRANTED : (IAS.) : (IPS) :(IPS)
Andres Biclar	: Ajuy	: : : : : : : : : : : : : : : : : : :
Estrella Borromeo	: : Ajuy	: : : : : : : : : : : : : : : : : : :
Arturo Canicula	: Ajuy	; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;
Primo Delfin	: Ajuy	:Finangtan ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;
Virginia Capalla	; Ajuy	: : : : : : : : : : : : : : : : : : :
Primo Delfin	: Ajuy	:Finangtan : : : : : : : : : : : : : : : : : : :
Perlita Illustrisimo	: Ajuy	:Balogo :11°09' :122°59' : : : : : : : : : : : : : : : : : : :
Estrellina Padernal	: Sara	: : : : : : : : : : : : : : : : : : :
Imelda Isa Inc.	: : San Dionisio	: : : : : : : : : : : : : : : : : : :
Serruco Isa Inc.	: : Sara (Ajuy)	: : : : : : : : : : : : : : : : : : :
Epifania Penaflor	: Sara	: : : : : : : : : : : : : : : : : : :

REGISTERED WATER RIGHTS

TABLE II-26

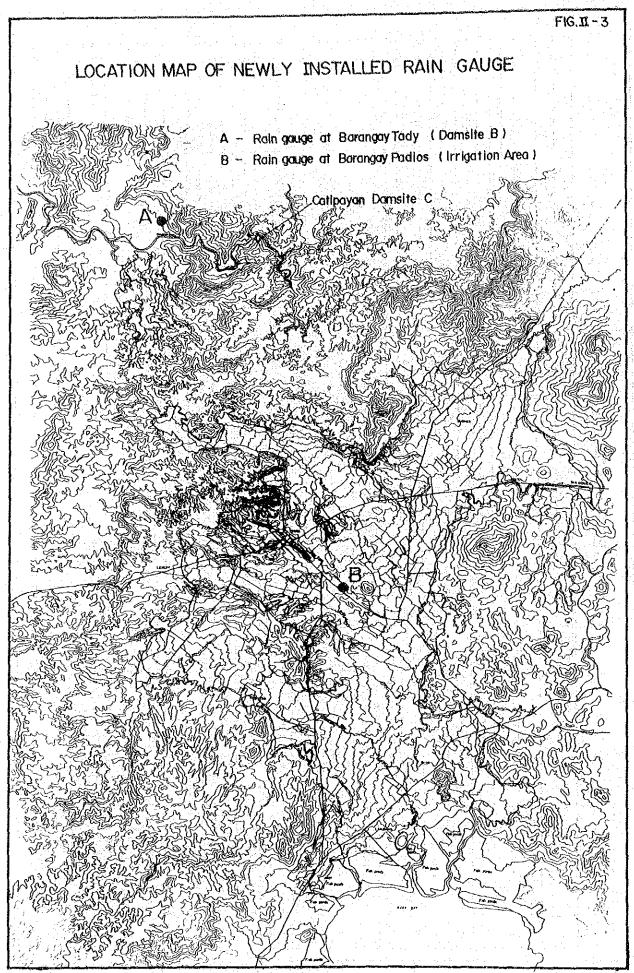


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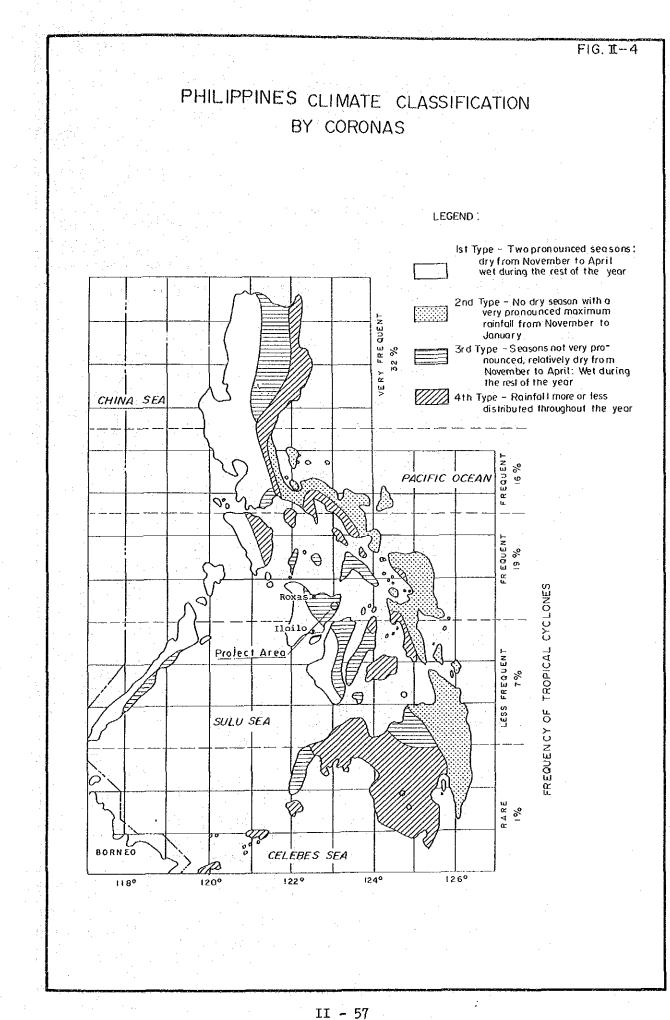
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DO	Year										-	I				Year	-				
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DATA PERIOD			1949	1949		, 19.52		1957				:			1950		- -				
Ш 			ar., 19	U D		L L L	-	Mar.							Jan.,		_				
AVAIL AB	, 65		from M	from		from		from							from		, e5				
AVA	Elev. (m)	34.9	1,5	1.0	25.0	15.0	°.0°.6	20.0	30.0	40.0	35.0		34.9	1.5	0.1	 Catchment	Area (km ²)		9	51.3	18,9
	tí on Long	122° 59'	122° 45'	122° 34'	122 51	122°57'	123° 07'	122° 38'	122° 27'	122° 27'	122° 30'		122° 59'	122° 45'	122°34	u o	Long		122° 59'	122°59'	122°59'
	Locat		11,35'	10°42'	1103	11° 27'	11°.23'	10° 52'	,60 ,11	Dingle 1 10'	11° 14'	·	11° 16'	11 35	10° 42 '	Location	Lat		11°17'	11° 10'	11°13'
	Station Baioton	Brgy Aguirre, Sara Iloilo	Ipiz	Iloite City	Barotac Viejo, Iloilo	Pilar Sugar Central, Pres. Roxas	Estancia , Lloilo	Dongsol, Pototan, I loilo	Jalaur RGS, Pass:	Jalaur RIS Damsite, Dingle	Abandian, Tapaz, Capiz	- Met eorologicy -	Brgy, Aguirre, Sara, Jiolio	Roxas City, Capiz	L loilo City		Kiver , Signon	River Water Gage-	Asue River, Brgy Agulrre	Catipayan River, Armidel Sara	Serruco River, Emprogo, Sara

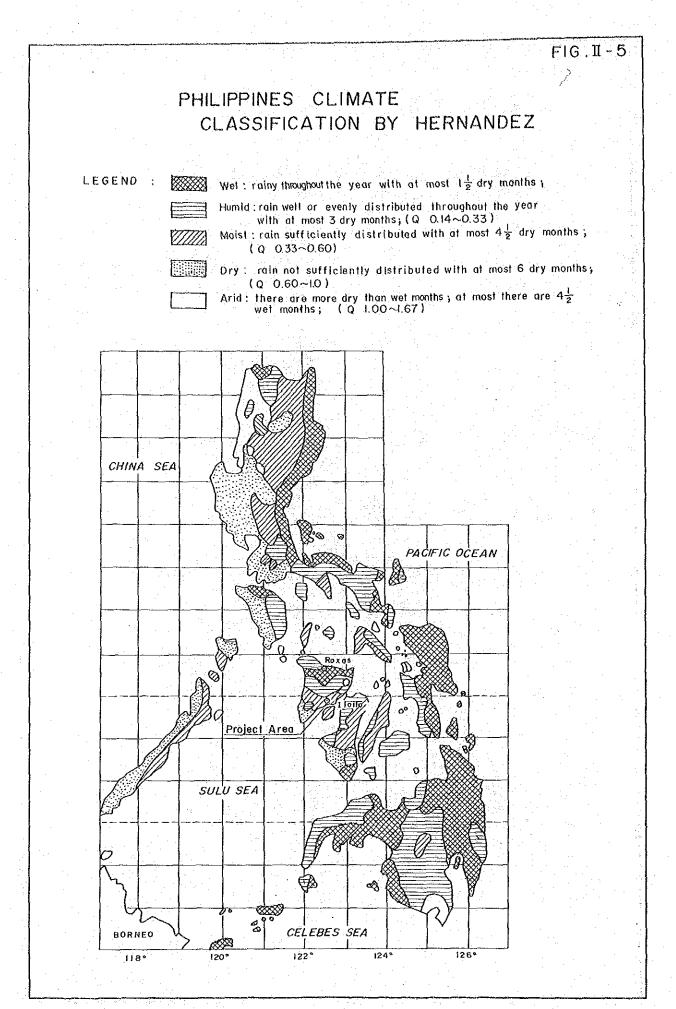


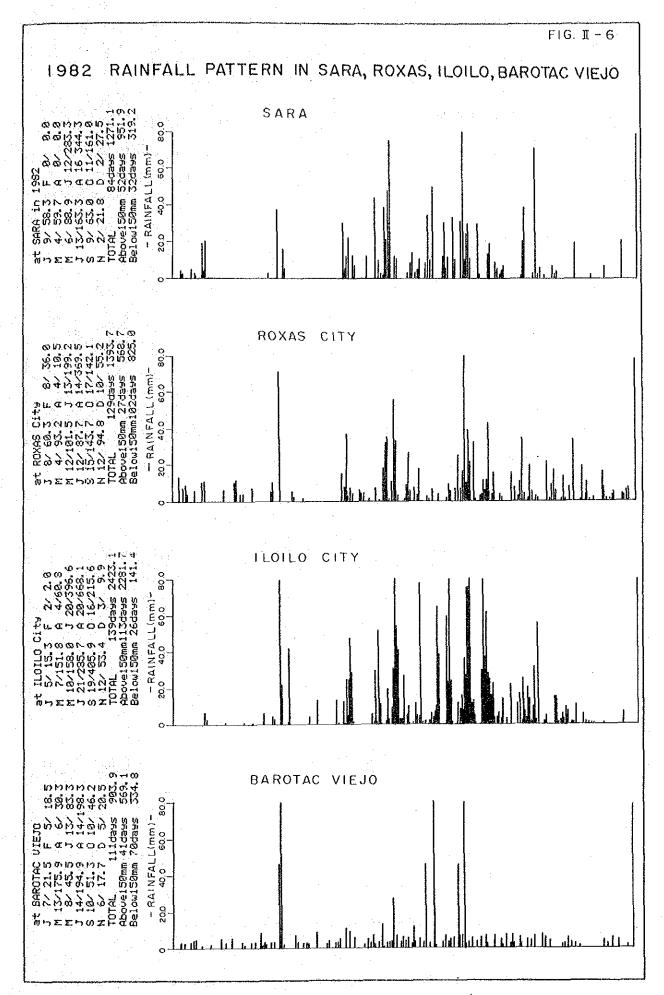
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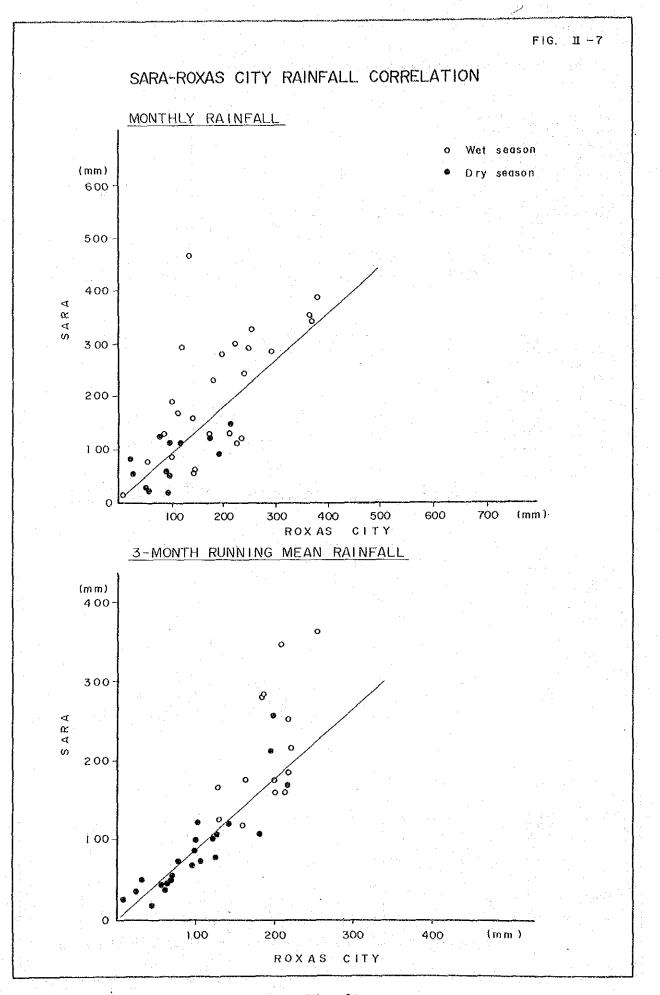


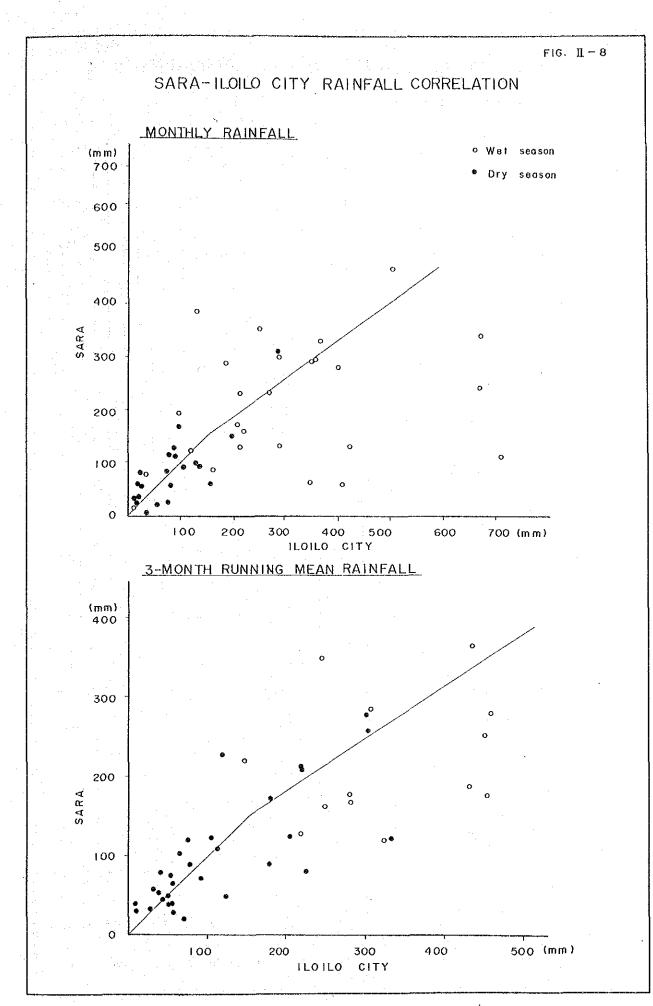
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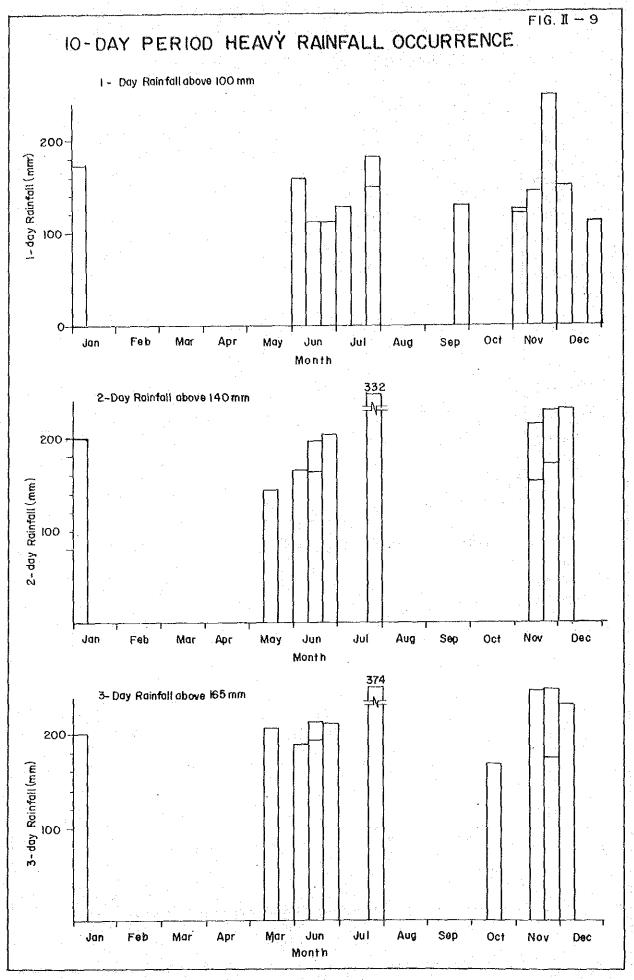




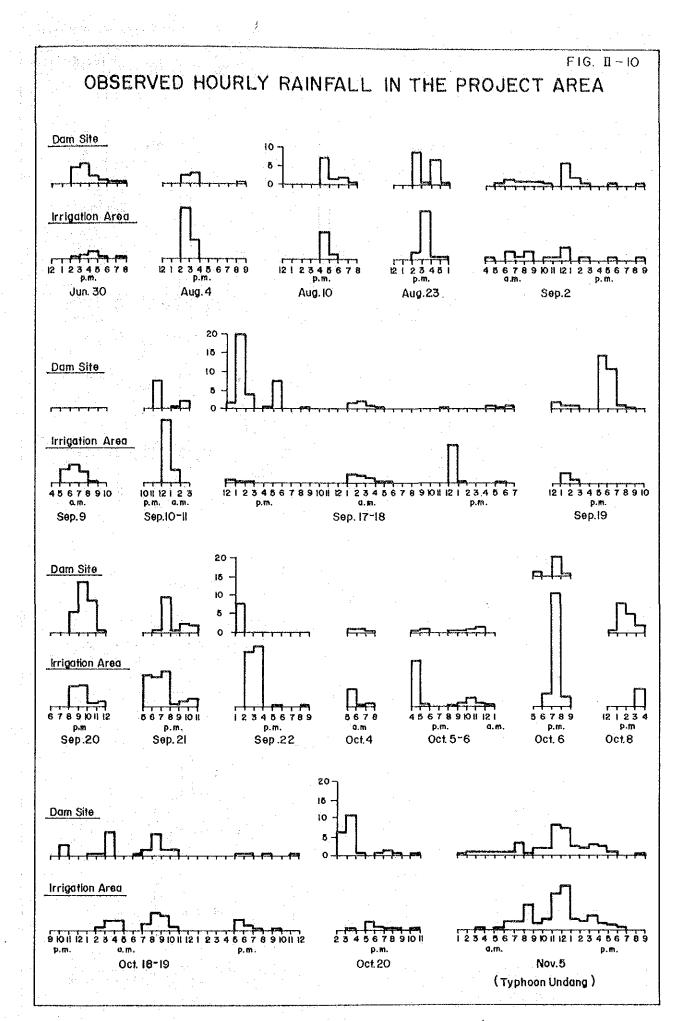


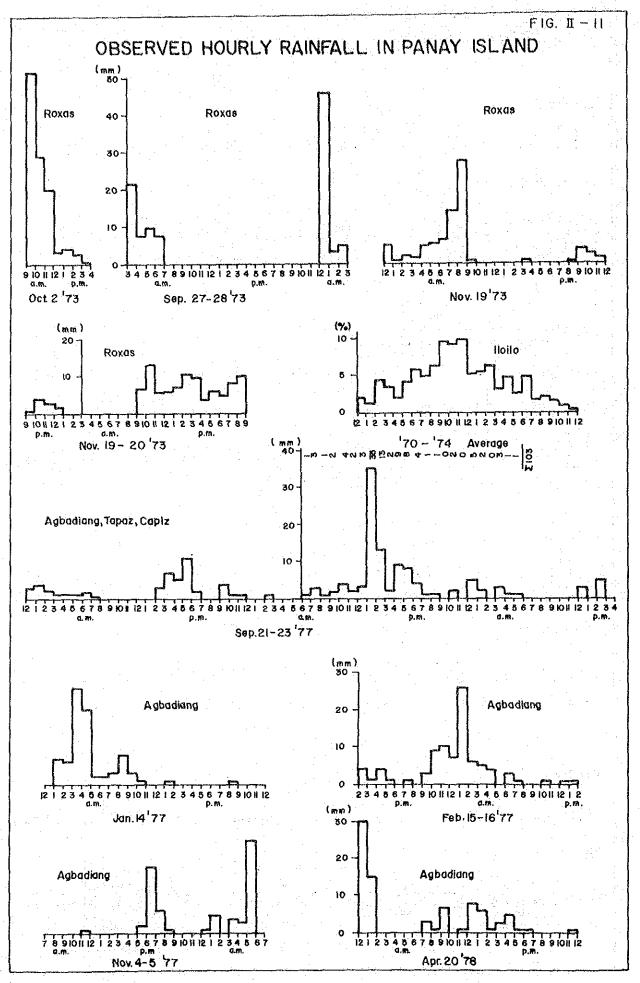


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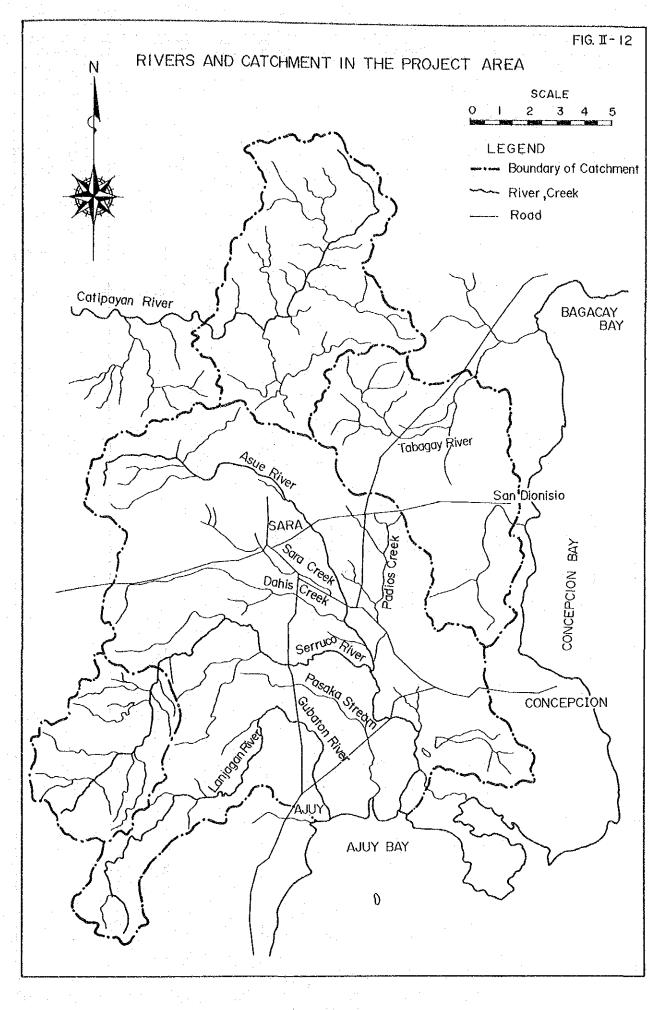


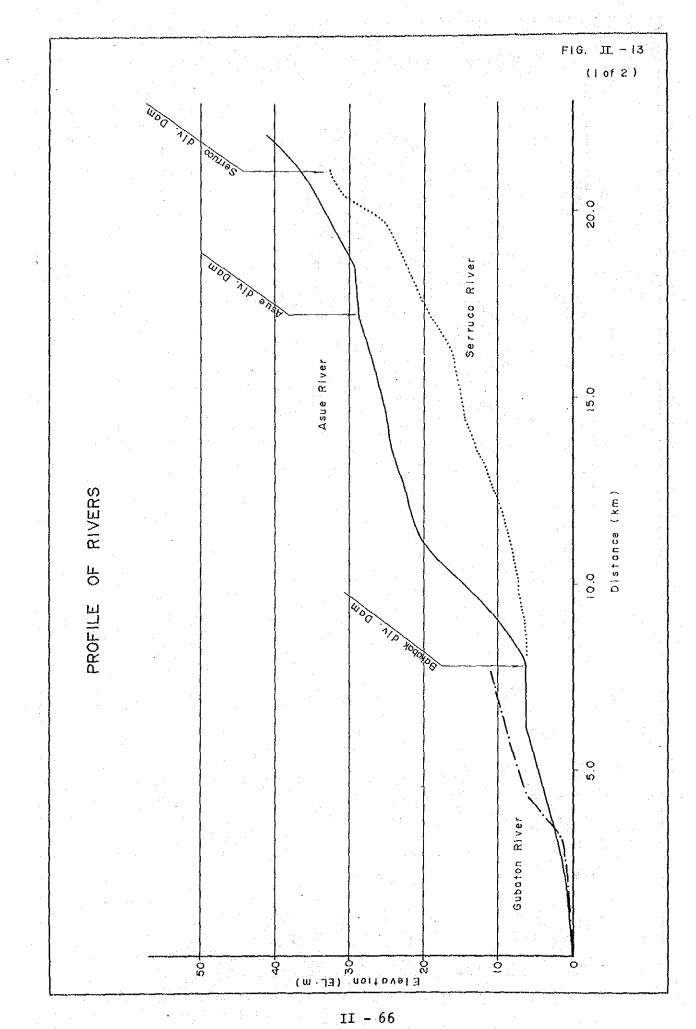
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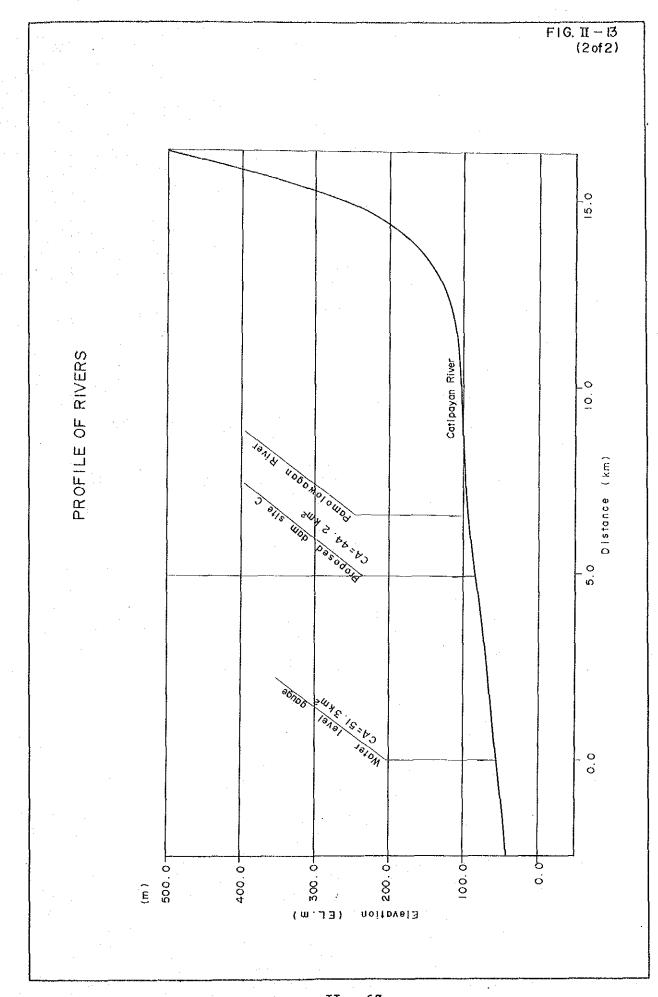




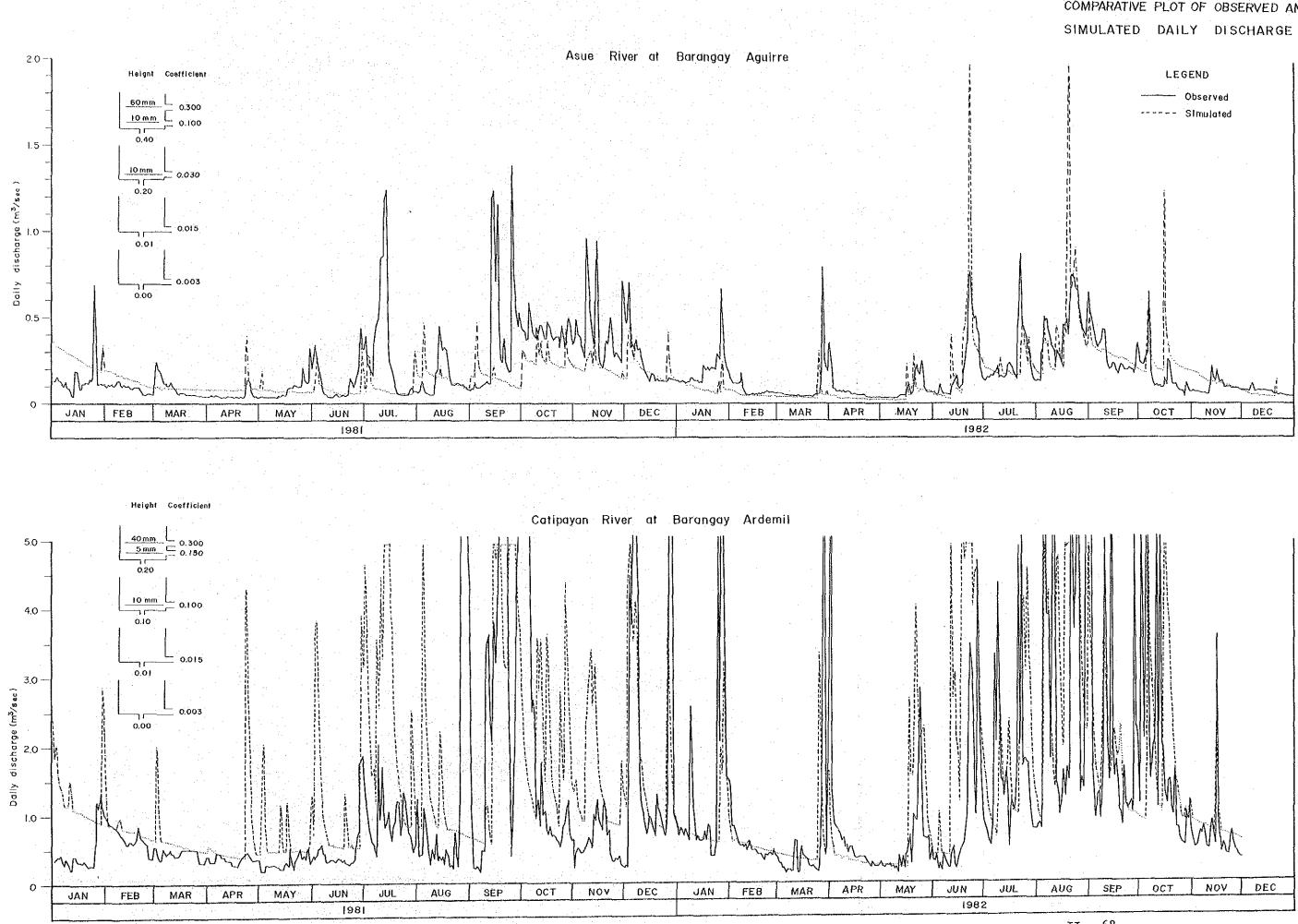
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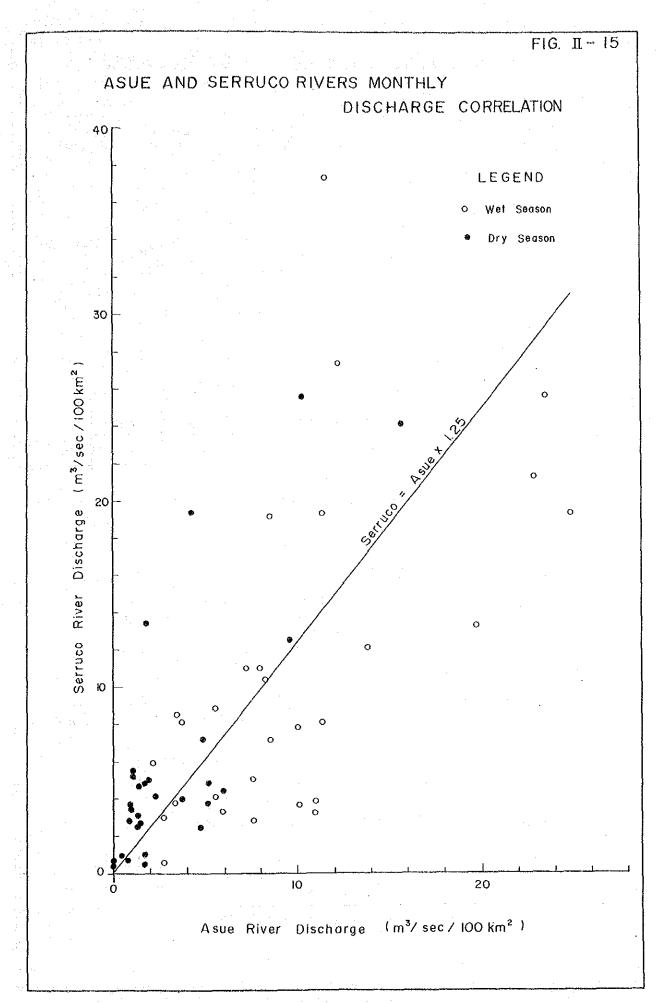


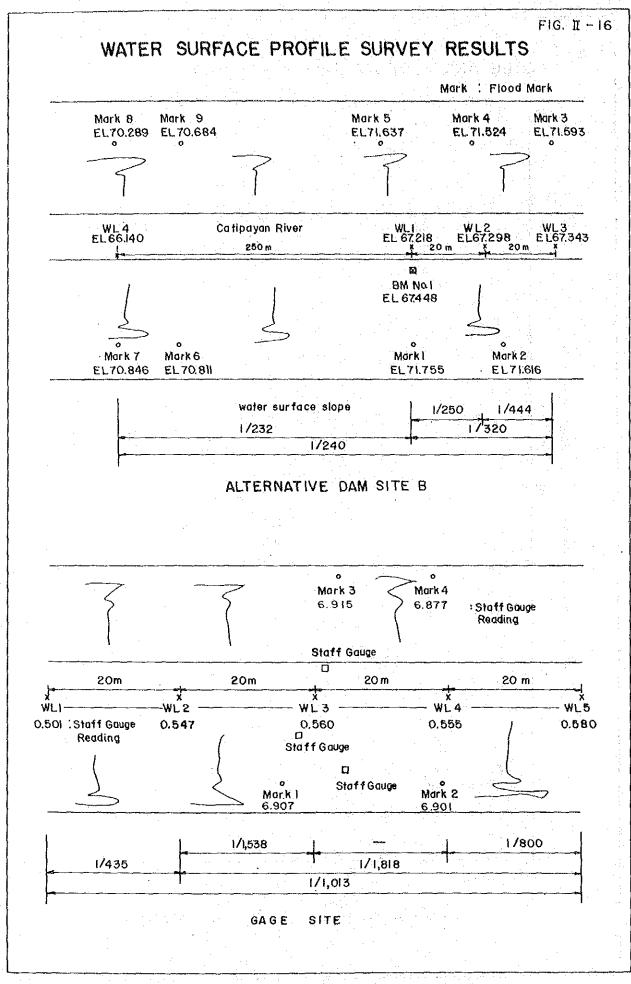


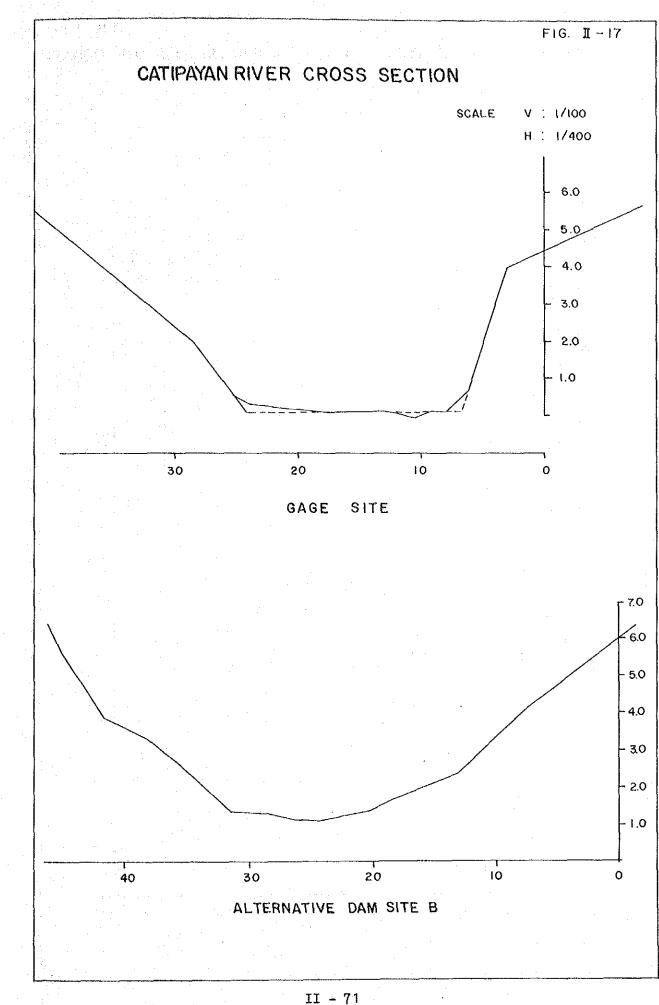
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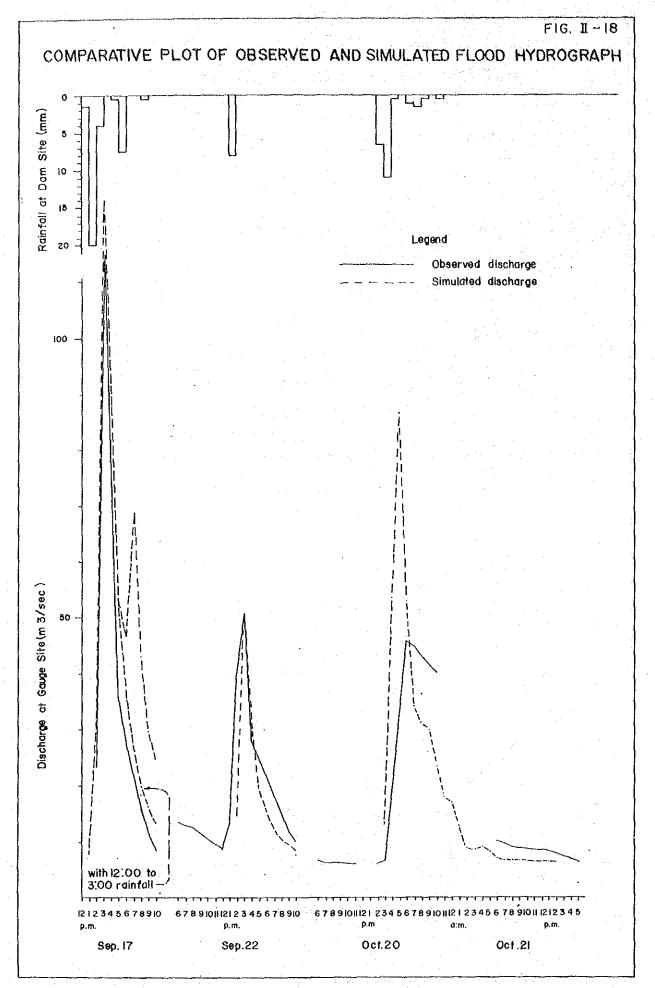


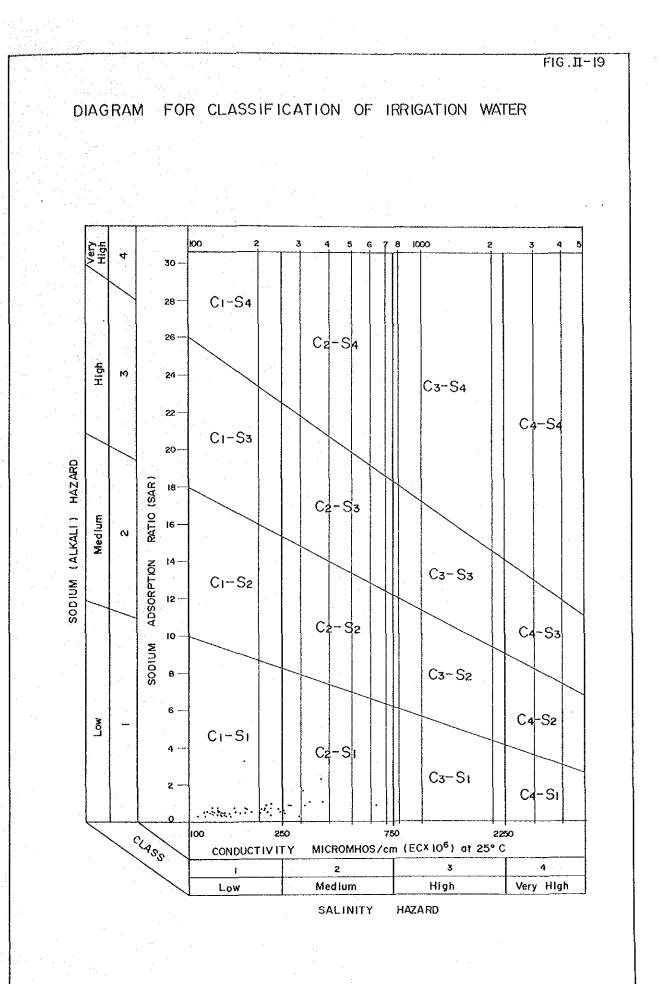
COMPARATIVE PLOT OF OBSERVED AND











APPENDIX III

GEOLOGY AND EMBANKMENT MATERIALS

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APPENDIX III

GEOLOGY AND EMBANKMENT MATERIALS

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APPENDIX III

GEOLOGY AND EMBANKMENT MATERIALS

1. TOPOGRAPHY AND GENERAL GEOLOGY

1.1 Topography

The Project area is slightly long and slender-shaped and is about 20km in length, trending to the north-south and about 10km in width trending to the east-west. The town of Sara is located in the center of the study area in Iloilo Province. The area is also divided into two different basins. The first is the Asue River basin in the southern area where the irrigation service area is located; the second is the Catipayan River basin in the northern area where there is a proposed dam site.

In the Asue River basin, the alluvial plain, which is surrounded to the north, east and west by hills and mountains, tends to Ajuy Bay located on the southern edge of the Project area. The Asue River which is the main river in the basin springs from the northern hills, flows southward where it is joined by tributaries east of the alluvial plain and finally flows into Ajuy Bay. Other rivers which have their source in the western mountains at levels of 100 - 200m are from north to south, the Alobayog, Serruco, Pasaka and Gubaton rivers. Two of the former join the Asue River slightly south of the center of the alluvial plain.

The Alobayog and the Serruco rivers flow into the Asue River in the downstream portion, while the Pasaka and Gubaton rivers directly flow into Ajuy Bay. The Tabagay River whose basin is located in the northeast portion of the Project area flows northeast of the Project's irrigation area to Bagacay Bay. The irrigation area under the Project is composed of an alluvial plain extending along the Asue River basin and high land surrounding the same.

The Catipayan River basin is mainly composed of gentle mountains, though narrow flat lands are spread over some areas along the river. The Catipayan River originates in the Agusbey mountains (peak elevation: 834m), and flows westward through the mountain area which is located in

the northern end of the Project area. The river, after passing the Project's study area, flows 10km before reaching the Maayon River which flows 20km to the confluence of the Panay River.

The proposed dam site is situated 8km north-northwest of Sara Town in the mountain area. The site is a gentle mountainous area with a slope of $20^{\circ}-45^{\circ}$, maximum elevation difference of 80-100m and a river width of about 20m. The proposed trans-diversion canal route passes along the mountain slope beside the Catipayan River, skirting the mountains, foothills and narrow flat land before reaching the Asue River basin.

In the Project area, alluvial plain is used for paddy while corn is grown in the hills. Mountains are mainly used for rice fields except for a small area of corn fields.

1.2 General Geology

The geology of northeastern Panay is composed of 2 kinds of bedrock and alluvial sediments. A stratigraphical table is presented as TABLE III-1.

One type of bedrock consists of volcanic sediments such as andesitic to basaltic pyroclastic rocks and volcanic flows which are named the Sibara Formation, and are presumably Creataceous to Tertiary in age. The Sibara Formation is basically composed of pyroclastic rocks and consists of latilli tuff, tuff, agglomerate and lava flow. It is massive, gray to dark green and its source is volcanic sediments with inconsistent thickness. Moreover, as the environment of sedimentation was different, the same is poorly graded. It was transformed by gravity and intrusions of Sara Diorite before becoming consolidated, resulting in disturbance of the sedimental structure.

The other type of bedrock consists of diorite known as Sara Diorite which is probably Tertiary (Paleocene) in age and intrudes into the Sibara Formation. The area covers about 600km² in the north-northeast area and is milkish gray to reddish gray. It mainly consists of quartz, hornblend and pyroxene, has fine to medium granular texture, and is assumed to have been formed during the Paleocene Period on the basis of measurements of the geological age by the K-Ar methods. The mass distributed around the Project area is intensively weathered as deep as the bedrock. Alluvial sediments lie thinly on the bedrock and consist of loose sand and gravel interbedded with silt and clay. In the Asue River basin, alluvial sediments extend in the central plain area, diorite in the northern to western mountainous areas and pyroclastic rocks in the eastern mountainous areas. In the Catipayan River basin, pyroclastic rocks are extensively distributed around the proposed dam site and reservoir areas. However, diorite is narrowly distributed in the southern part of the basin and in the northwestern ridge of the Catipayan area as a small intrusive mass from the Asue River basin. Alluvial sediments are scattered along the narrow flood plain in the Catipayan River and relatively broad flood plains in the Asue River basin as shown in FIG. III-1.

2. GEOLOGICAL INVESTIGATIONS

2.1 Existing Survey

NIA has planned and executed geological investigations since March, 1983 in the proposed dam site located in the upper reaches of the Catipayan River, and the proposed trans-diversion tunnel, which is about 3.5km south of the proposed dam site. The contents of geological investigations and drilling and water pressure tests, are outlined below while the quantities of the same are shown in TABLE III-2.

2.2 First Survey

The Team undertook the above mentioned geological investigations in cooperation with NIA counterparts, and carried out the First Survey including First Stage Field Work from June to August and First Stage Home Office Work from August to October, 1984.

Contents of geological investigations which were carried out for the period of the First Survey consist of drilling, seismic exploration and soil-rock tests, details of which are shown in TABLE III-3.

2.3 Secondary Survey

After the First Survey, the Team continued geological investigation, carrying out the Secondary Survey including Second Stage Field Work from October to December, 1984, and Second Stage Home Office Work from December, 1984 to March, 1985.

The contents of geological investigations which were undertaken for the Secondary Survey include the remaining items from the First Survey and supplementary works such as drilling and water pressure tests at proposed dam site C. Details are shown in TABLE III-4.

The geological investigations mentioned above are classified as shown in the table with regards to the site. The locations of the same are shown in FIG. III-3 and III-4.

Site	Investigation	Drilling	Permea- bility Test	Seismic Explo- ration	Soil Rock Test Test
Dam	Alternative Dam Site B	12 holes 540.20(m)	131 nos.	3 lines 1155(m)	
Dan	Proposed Dam Site C	1 hole 20(m)	5 nos.	1 line 550(m)	
Borrow	Impervious Material	3 holes	· · · · · · · · · · · · · · · · · · ·	4 lines	38 nos (13 pits)
Quarry	Semi-Pervious & Pervious Material	160.92(m)	199	1980(m)	10 nos. - (3 holes)
Tunnel	Trans-diver- sion Tunnel	3 holes 151.14(m)		1 line 880(m)	n an an Arrange an Arrange Arrange an Arrange an Arrange Arrange an Arrange an A

3. RESULTS OF GEOLOGICAL INVESTIGATION

3.1 Drilling

Drilling was conducted according to the core method at all dam sites, proposed embankment material sites and tunnel sites. The results are shown in FIG. III-5 to III-7 (Bore Hole Log). In each bore hole log, the grade of bedrock is described by the observation of drilling cores such as rock type, state of weathering, hardness, state of fractures and rate of recovery.

Bedrock confirmed by drilling were all andestic to basaltic pyroclastics belonging to the Sibara Formation. The depth of the highly weathered portion of bedrock, which is soil-like in appearance is relatively shallow with a depth of about 2m at the bottom of gullies, and about 10m on the mountain ridge and plateau area.

Results of investigations were arranged according to geological cross sections accompanied by the results of the other investigation, and used to evaluate the potential of bedrock as a foundation for the dam and tunnel construction. Results were also used to estimate the volume of embankment material.

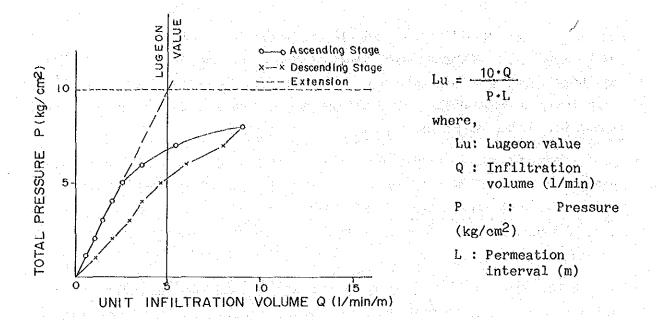
The standard to classify the grade of bedrock is shown in TABLE III-5. In general, bedrock of higher grade than C_L-C_M class are adaptable as a foundation for rock fill type dams, while a wide distribution of rocks graded higher than C_M class bedrock is preferable for tunnel construction.

3.2 Permeability Test

Water pressure tests were carried out in drill holes at dam sites. The interval of each test was 3.05m, and the results of each test are shown in TABLE III-6 by lugeon value.

Lugeon values obtained for each test were small. This indicates that minimal foundation treatment is required and in general bedrock is considered adequate for dam foundation. Lugeon values of more than 5 were obtained at the riverbed of site B, DDH-1,2 at depths from 30m to 37m indicating that deep foundation treatment is necessary. However, the lugeon values in this portion were still less than 50, corresponding to slightly fractured zone rather than fault shear zone.

Lugeon values are given on the following page, with 1Lu being equal to 1.0 x 10^{-5} m³/sec in permeability coefficient and the lugeon value for a unit length of test was determined as in the following figure. In general, 3-5Lu is acceptable as a foundation for a rock fill type dam.



3.3 Seismic Exploration

*

Seismic exploration was conducted at the dam site, proposed borrow/quarry sites for embankment material and the tunnel site. Measurements were carried out by NIA experts.

On the basis of recorded data, a 1/500 scale velocity layer cross sectional diagram was prepared. Velocity layers were broadly divided into the 4 strata shown in the table below.

Velocity Stratum	Velocity (km/s)	Rock characteristics
1st Stratum	0.4 - 0.6 (0.3)	Topsoil, talus, weathered rock soil
2nd Stratum	0.9 - 1.5 (0.8)	Heavily weathered rock (including portions of talus)
3rd Stratum	2.0 - 2.6	Moderately weathered rock
4th Stratum	4.0 - 5.3 (3.8)	Slightly weathered rock to fresh rock
Low Zone	Velocity unknown (3.2)	Shear zone, fractured zone, deep strata weathered zone

VELOCITY STRATUM OF THE PROJECT AREA

Through comparison of velocity layer classification for seismic waves and drilling results, the geological characteristics of each layer were estimated. The first velocity stratum (0.4-0.6km/s) consists of cover material including topsoil, talus, weathered rock soil, etc. The second velocity stratum (0.9-1.5km/s) is mainly composed of heavily weathered rock in the form of loose earth and sand, including a portion of unconsolidated sedimentary deposits. On the basis of core examination, this heavily weathered portion of bedrock is determined as D_L-C_L class.

The third velocity stratum (2.0-2.6 km/s) is composed of moderate to mildly weathered bedrock. Although sections composed of rock fragments and blocks exhibit hardness, cracking is pronounced throughout the stratum along which weathering has progressed. This stratum is determined as C_L - C_M classes including some other classes.

The forth velocity stratum (4.0-5.3 km/s) constitutes the foundation velocity stratum for the site area and is composed of slightly weathered to fresh bedrock. This stratum is determined as more than C_H class. Within the forth velocity stratum, low velocity zones are present wherein velocity values are relatively less than those of adjacent areas. The velocity of such zones was observed at 3.2 km/s. Although the precise nature of the relationship and continuity with surrounding formations remains unclear, these low velocity zones may be the result of such factors as fault cracked zones, deep strata weathering and deterioration from folding.

3.4 Soil Test

Soil tests were conducted for the samples which were taken from test pits excavated for 2-3m at proposed sites for impervious materials in the vicinity of dam sites. Soil profiles are shown in FIG. III-9. As shown in TABLE III-7 typical samples were classified into SC-CH. Natural water content (Wn) of the same ranged from 16-35%, dry density (rd) from $1.5-1.7t/m^3$ and permeability coefficient (K) from $1x10^{-6}-10^{-8}$ cm/sec. Although there is a high content of fines, the soil test values indicate the same is suitable for impervious material.

3.5 Rock Test

Drill cores which were classified into D-CH class sampled from DDH-R1, R2, R3 quarry sites were used for rock test specimens. The results of rock tests are shown in TABLE III-8.

Judging from the typical physical values of each type, rocks of more than C_M class are adequate rock materials in terms of gravity and chemical stability.

Rock Classificatio	Specific Gravity n (Apparent)	Water Content (%)	Soundness Test (%)
D-CL	2.72	3.5	57
c_L	2.73	2.2	56
CM	2.77-2.81	0.2-0.3	2-3
C _M -C _H	2.75-2.94	0.5-2.0	6-29
CM	2.77-2.87	0.2-0.5	3-7

For some C_M - C_H class and C_H class rocks, a high value was obtained for soundness tests. This is due to use of rock specimens extracted by hammer. When samples used in soundness tests are obtained by normal blasting excavation techniques, the value is usually 2-3% of those for the C_M class shown in the table.

4. GEOLOGY AT THE DAM AND RESERVOIR SITES

4.1 Dam Site

4.1.1 General

Alternative dam site B and proposed dam site C which is about 2.5km upstream from site B are situated in the upper reaches of the Catipayan River. Although the bedrocks of these dam sites are andestic to basaltic pyroclastic rocks and are composed of several rock types, these volcanic rocks should be considered as one mass in regard to bedrock for the construction of a dam.

Outcrops of fresh bedrock can be seen along the riverbed of the Catipayan River and its tributaries. Bedrocks observed are classified petrographically into several rocks including latilli tuff, fine tuff, agglomerate, tuff breccia, andesite and fine to coarse grained basalt. The structure of these rocks appears to approximate a bed strike to the northeast with a dip to the northwest. However, the continuity of each layer is not traceable as the thickness of each rock type varies from a few centimeters to several meters. This is due to sedimentation which occurred when the meta-volcanic rock mass was formed as described in section 1.2. The intensity of weathering and the state of fracturing, which are important considerations in dam construction, do not seem to have arisen from changes in the rock type of each layer.

Accordingly, volcanic rocks containing many rock layers should be considered as one mass in regards to bedrock for the construction of a dam.

4.1.2 Alternative Dam Site B

At alternative dam site B, elevation of the riverbed is approximately 68m while the width is about 40m. Inclination of slope at both sides is about 20°; however, a flat portion runs along the left side at elevation 70m and 125m.

The depth of bedrock excavation and curtain grouting is shown in FIG. III-10. The excavation line determined that C_L-C_M class rock with more than 2km/s in seismic velocity is adequate for the foundation of a fill type dam. The average depth of excavation would be approximately 8m, while the maximum depth of excavation would be 20m on the left bank due to thick alluvial deposits. The bottom curtain grouting line has been determined at a lugeon value of more than 5 observed by water pressure test and in consideration of the highest possible water level of the reservoir. As a result of the above the maximum depth of grouting is estimated at about 30m from the excavation line at the riverbed while deep grouting is not required at this site.

Although several faults were recognized by examination of drilling cores, the fault cracking zone is impermeable as the width of the cracking zone is narrow and filled by calcite veins.

4.1.3 Proposed Dam Site C

(1) Dam

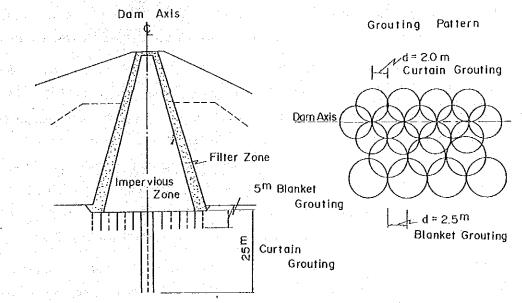
At proposed dam site C, elevation of the riverbed is approximately 85m, while the width is about 20m. However, the slope of both sides is gently inclined ranging from 25° to 30° which is steeper than that of B site. The slope of the left side is moderately inclined ranging about 15° from the hill side at approximate elevation of 140m to the ridge of the hill. The depth of excavation and curtain grouting around the dam axis is shown in FIG. III-11.

Excavation was estimated by correlation of drilling data and the results of seismic exploration. The average depth of excavation is approximately 8m. Deep excavation as required on the left bank of site B is not required at this site. The depth of curtain grouting was estimated by lugeon value, in consideration of the highest possible water level of the reservoir and also correlation of seismic velocity layer cross sectional diagram. The maximum depth of curtain grouting is only about 25m.

On the basis of water pressure tests at dam site B, required volume for grouting is presumed as presented on the following page. Supposing that the interval of the grouting hole is 2.5m zigzag for curtain grouting and 2.0m zigzag for blanket grouting, required quantity of cement is estimated at approximately 700t as shown in the table.

	Interval of Grouting Hole	Average Length	Number of Grouting Holes	Lugeon Value of Improve- ment	<u>11/</u> Total Quantity of Cement
Curtain Grouting	2.0m Zigzag	15m	270 Holes (90 x 3)	10Lu (15Lu → 5Lu)	243.0t
Blanket Grouting	2.5m Zigzag	5m	1008 Hoels (72 x 14)	15Lu (20Lu -> 5Lu)	453.6t

1' 60 kg/m of cement is considered necessary to improve 10Lu.



(2) By-pass Tunnel

Outcrops of C_M class rock are observed along creeks at an inlet portion of the proposed by-pass tunnel site; hence the inlet presents no geological problems for planning. At an outlet of the tunnel, D-CL class rock is found along the vicinity's creek indicating that a relatively long open cut will be required for the same portion. C_M -CH class rock is distributed at the tunnel portion

Since ground water is abundant in both the inlet and outlet portions, care should be taken to eliminate water which wells up during construction (FIG. III-12).

(3) Spillway

 $D-C_L$ class rock outcrops are observed along the mountain slope around the proposed service spillway crest portion, while the proposed stilling basin site is composed of C_L class rock. Since slightly weathered C_L class rock is enough for spillway bedrock, excavation depth will not be so deep (FIG. III-13).

4.2 Reservoir Site

The basin is slightly long, extending about 10km with a north-south trend and a width of about 4km with an east-west trend. The area ranges from the south facing mountain slopes (EL = 700-800m) of Mount Agudo and Mount Alpasco in the north to the southern foot of the same (EL = 200m).

The area of the basin is about 44.2km^2 with a dam reservoir submerged area of about 2.10km^2 for proposed dam site C (FIG III-14). When the watershed area is divided into 4km^2 blocks, the highest and lowest elevation in each block is as shown in TABLE III-9. According to the same, the average elevation is about 500m in the north and about 150m in the south, indicating that the area is moderately inclined to the south. The height of undulation is from 300 to 600m in the north and from 100m to 200m in the south. The south area is topographically flat.

Bedrocks distributed in the area are composed of volcanic rocks corresponding to the Sibara Formation as at the dam site and diorite. While volcanic rocks are predominately distributed in 80% of the area, diorite is restricted to the south ridge of the area, distributed at elevations of 200-300m.

Outcrops of these bedrocks revealed significant differences in weathering condition. Highly weathered volcanic rocks to gravel/sand conditions contain appropriate clayey materials, and seem to be limited to a depth of 5 to 10m on the mountain ridges. On the other hand, weathered to gravel/sand diorite contains less clayey material, and is generally weathered more deeply, especially on mountain ridges to a depth of 20m or more.

As shown in FIG. III-15, several aerial photos revealed northeast and southwest trends. As these trends are not associated with large landslides or slope collapse, and do not appear to be caused by large

fault shearing zones, the same should not present any significant hindrance.

As vegetation cover in the reservoir area is obviously very poor, and traces of small landslides and collapses in the surface area were observed at steeply sloped sections on the hillside and around mountain ridges in the reservoir area, implementation of sodding or other measures should be considered after reservoir construction to prevent sedimentation.

5. EMBANKMENT MATERIALS

5.1 Quarry/Borrow Area

If the dam at site C is assumed to be a rock fill type, the following kind and amount of embankment material are necessary.

Impervious (core material)	137,200m ³
Semi-pervious (filter)	40,000m ³
Pervious (transition, rock, riprap)	619,000m ³
Total	796,200m ³

As described in the next section, volcanic rocks distributed around the dam site are adequate for embankment material; however, the amount of riverbed gravels in the dam site vicinity is insufficient and materials should be collected from the same site. The overburden and totally weathered portion of the bedrock (D-C_L class) would be used for core material, the totally to moderately weathered portion (D-C_M class) for filter material, moderately weathered to fresh portion (better than C_L class) for transition and rock material and the fresh portion (better than C_M class) for riprap material. In addition, it may be necessary to crush fresh portions of the bedrock for concrete aggregate.

Quarry 4 is recommended as the most suitable site for excavation, with a large area for construction and a short transportation distance.

Excavation should be coordinated with embankment planning, and intransit settling should be minimized. FIG. III-16 is an example of excavation planning. The volume of each material is estimated as below and total volume is about 2 million m³ correlated with the 800,000m³ required for embankment.

Estimated Volume of Emb	ankment Materials at Quarry 4	Velocity Stratum
Overburden, D-CL class	950,000m ³ (Core, Filter)	1st, 2nd
$C_L - C_M$ class	746,000m3 (Filter, Transition))
Better than C_{M} class	31,900m ³ (Rock, Riprap)	4th
Total	2,015,000m ³	<u></u>

5.2 Material Characteristics

The design value of each material is determined by the results of soil/rock tests and according to the values of existing dams. The physical value of compacted materials, such as density and mechanical properties varies according to quality and gradation of the material, and the compaction method. It is thus possible to determine the physical value of embankment material if selection and mixing of the material is Design values of the materials are estimated as made before compaction. shown in TABLE III-10, on the basis of the results of laboratory tests. For reference, TABLE III-11 presents general characteristics and adoptability of soils for dam embankment.

5.2.1 Impervious Materials

Soil tests are based on extraction at test pits from candidate embankment material sites. The results are shown in TABLE III-7. As a result of gradation tests, details of which are shown in FIG. III-17, the majority of data is inconsistent with the USBR Standard guideline. The specimen seems to have been greatly compressed as it contains less than 10% gravel. The depth of excavation is shallow (about 4m), while test specimens mainly consist of fine material. The material might be adequate for core material, classified as SC or GC, since it will be mixed with coarser material and in actuality deeper excavation will be necessary (see

TABLE III-5). In this case, a gradation curve for the same might be represented by the under ridge of the gradation curve shown in FIG. III-17.

1) Specific gravity (Gs)

The Gs value is estimated at 2.61 based on the average of test data.

2) Water content (W%)

As optimum water content (W opt) is considered to be about 20%, W% is determined to be 24% in order to obtain a 95% wet side dry density from maximum density.

3) Wet density (Tt)

As the dry density of the 95% wet side (7d 95 wet) is about $1.60t/m^2$, ℓ t is estimated at $1.98t/m^3$ from the formula ℓ t = ℓ d (1 + $\frac{W}{100}$).

4) Shearing Strength (C t/m^2 , \emptyset^0)

The angle of internal friction (\emptyset°) is estimated at 32° and cohesion (C t/m²) is disregarded in consideration of gradation, dry density and void ratio, etc.

5) Permeability Co-efficient (Kcm/s)

Although K $(1x10^{-6} - 10^{-8} \text{ cm/sec})$ was observed in the tests, the K value could be decreased to $1x10^{-5}\text{cm/sec}$ by mixing sand and gravel.

5.2.2 Semi-Pervious - Pervious Material

As a rule, coarse materials used for semi-pervious - pervious materials must be massive and both mechanically and chemically stable. Furthermore, the same must have appropriate shearing strength and drainage ability. Shearing strength and drainage ability can be broadly determined on the basis of gradation and degree of compaction of the material.

It is prescribed in the USBR Standard that the coefficient of permiability required to satisfy the function for filter material should be from 1.0×10^{-5} cm/s to 1.0×10^{-3} cm/s, and filter material is also provided by the formula as given below in regard to correlation with the gradation of material.

Diameter of a particle of 15% passing for filter material greater than 5 Diameter of a particle of 15% passing for core material

Diameter of a particle of 15% passing for filter material less than 5 Diameter of a particle of 85% passing for core material

If the gradation of core material is assumed to be represented by the under ridge of the gradation curve shown in FIG. III-18, the diameter of a particle of 15% passing should be 0.05mm and the diameter of a particle of 85% passing should be 2.0mm. Therefore the diameter of a particle of 15% passing for filter material should be greater than 0.25mm and less than 10mm.

Furthermore, shearing strength correlated with the gradation of coarse material is proposed by adaptable values as tabulated below with regards to relative density. Approximately 70% of relative density should also be available for coarse material by mechanical compaction of embankment material on the basis of actual results of dam construction.

	Relative Density		
Diameter and Gradation	more than 70% dense	less than 50-70% moderate	50% loose
Equally mixed with fine to coarse sand	34-380	32-36°	28-30°
Well graded coarse sand to poor graded sand, sand & gravel	r 37-380	32 - 340	28-30°
Mixed with well graded sand to sand & gravel	40-45°	36-410	33 - 36°

$$D_{\delta} = \frac{\max \left(\delta - \delta \min\right)}{\delta \left(\delta \max - \delta \min\right)} \times 100$$

\$\sigma: natural density (t/m³)

max:	maximum	density	(t/m3)
min:	minimum	density.	(t/m3)
1.1.1			

Rock materials at the site obviously satisfy the above basic standards and characteristics for embankment material. Accordingly the design values shown above should be used where coarse material falls within the gradations shown. If the material is not included within these gradations, the gradation should be adjusted. On the basis of examination of drilling core, rock classification for embankment material as described in section 5.2.2 is the most suitable classification for each dam zone and adjustment of gradation may be unnecessary.

The design values shown in TABLE III-10 were determined on the basis of the considerations mentioned above and by reference to test values and actual construction results as shown in FIG. III-19 to III-22.

SITE GEOLOGY AT MAJOR IRRIGATION FACILITIES

Major irrigation facilities from the dam site to the end of the canal are the trans-diversion canal from the dam site to the transdiversion tunnel, the trans-diversion tunnel and the proposed diversion dams at 3 sites in the Asue River basin.

6.1 Trans-Diversion Tunnel and Canal

6.

(1) Trans-Diversion Tunnel

The length of the trans-diversion tunnel is approximately 500m and the elevation of the formation line is approximately 80m. The object of excavation is the volcanic rocks which are similar to the rocks at the dam sites as shown in FIG. III-23. The depth of overburden and highly weathered portion of bedrock at both the inlet and outlet sites is about 10m.

While the slope near the inlet is about 25° , the outlet is about 10° and the shallow portion of covered rock (approximately 10m) extends toward the mountain side for about 80m, which is a disadvantage in construction. Moreover, as ground water was observed at a shallow depth of 6m from the surface at both sites, extraction of seepage should be considered for construction and the scale of supports should be determined. It appears that the less weathered portion of bedrock, better than C_M class, is distributed in the tunnel section. It might be possible to use smaller supports for tunnel excavation, but seepage at the same should also be considered.

(2) Trans-Diversion Canal

The trans-diversion canal will run westward through the hilly areas along the Catipayan River for about half of its total length and then change direction to the south towards the flat land to about 2km downstream from alternative dam site B. The canal will then pass through the foot of the mountains and the alluvial plain until it reaches the inlet of the proposed trans-diversion tunnel.

While the construction of a canal along the Catipayan River may require large scale excavation on the steep hill sides, the overburden and highly weathered portion of the volcanic rocks should be excavated in consideration of slope stability. The southern portion of the canal from the Catipayan River presents no difficulty for construction as the geology of the area consists of alluvial sediments, overburden and highly weathered diorite with a gentle slope. It is therefore estimated that minimal cutting and banking works will be required.

6.2 Diversion Dams

Diversion dams are planned at Asue, Bakabak and Gubaton in the Asue River basin as shown in FIG. III-24. In the Asue River basin, alluvial sediments composed of unconsolidated sand, clay and gravel are distributed on the plain, while bedrocks composed of diorite and volcanic rocks are distributed in the mountain slope and isolated hills as described in the section on general geology.

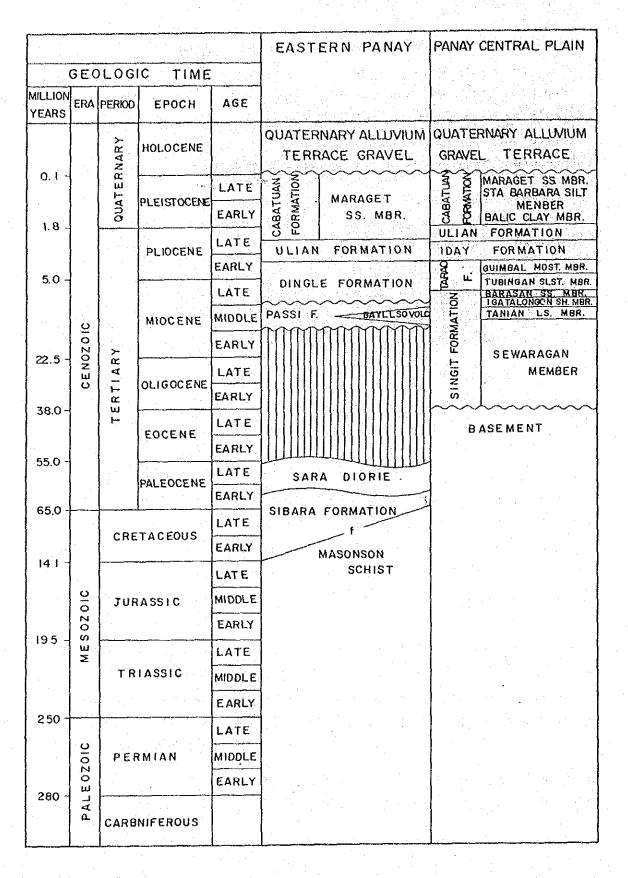
Bedrock is broadly divided into two areas by a geological border which runs north-south passing about 2km east of Sara Town. The area west of the border is predominantly composed of diorite, while the eastern area is composed of pyroclastic rocks which correspond to the rocks found at the proposed Catipayan dam site.

Outcrops of these bedrocks in mountain slopes and isolated hills revealed a significant difference in weathering conditions. As described in section 4.2, diorite is generally deeply weathered especially on mountain ridges where the rocks are highly weathered to a depth of 20m or more.

Weathered diorite contains less clayey material. On the other hand, the highly weathered portion of pyroclastic rock is limited only to a depth of 5 to 10m, and the weathered portion contains appropriate clayey materials. Accordingly, as a foundation for lighter structures both diorite and pyroclast have sufficient strength. In the case of heavy structures, however, pyroclastic rock is advantageous.

Diorite outcrops were observed in the riverbeds and vicinity of the proposed diversion dam sites in Asue and Gubaton, and the same are totally weathered. Outcrops of bedrock were not clearly observed in the vicinity of the proposed diversion dam site at Bakabak. The depth of alluvial sediments is assumed to be shallow at the diversion dam site according to the existing isolated hill on the right side, which is composed of volcanic rocks totally covered by overburden.

STARTIGRAPHICAL TABLE



EXISTING SURVEYS

1) Drilling

Location	Bore Hole No.	Site	Ground Elevation (m)	Drill- ing Length (m)	Permeability Test (nos.)	Note
	DDH-1	dam axis	83.10	50.00	11	Left bank
	2	- do -	72,45	60.00	17	riverbed
	-3	- do -	89.61	48.89	15	right ban
	- 4	downstream	86.56	60.00	-	- do -
Alterna-	~5	- do -	70.02	50.18	15	riverbed
tive	6	upstream	73.21	60.00	17	- do
Dam Site B	-7	dam axis	124.04	30.00	6	left bank
	-8	- do -	114.60	30.54	8	right bar
	-9	downstream	70.28	60.00	18	riverbed
	-10	- do -	108.56	30.00	7	left bank
	-11	upstream	84.50	30,21	8	- do -
	-12	- do -	97.62	30.38	9	right bar
Total	<u>12</u> Holes			<u>540.20</u> (m)	<u>131</u> nos.	
Trans	DDH-T1	inlet	116	40.00	- CE	foot of mt.
Diversion Tunnel	-T2	center	164	90.27	-	saddle of mt.
. willing 1	-T3	outlet	96	20.87	-	foot of mt.
Total	<u>3</u> Holes			<u>151.14</u> (m)		

2) Test Pit and Soil Tests

				Number	of Soil T	ests	
Location	Pit No.	Excavated Depth (m)	Specific Gravity	Grain Size Analysis	Natural Moisture Content	Atter- berg limit	Compao- tion Test
Borrow-1	TP-1	4.0	5	5	5	5	1
-	2	1.8			· · · · ·	-	
Borrow-1	3	3.5	Ц	4	4	4	1
Borrow-2	-4	4.0	5	5	5	5	. 1
-	-5					-	-
Borrow-2	-6	3.0	Ц	4	4	4	1
- do -	-7	2.0	3	3		3	1
- do -	-8	4.0	5	- 5	5	5	1
- do -	-9	4.0	5	5	5	5	1
– do –	-10	3.9	5	5	5	5	1
Borrow-1	-11	4.0	-	•••• •••			-
Total	11	34.2m	36 nos.	36 nos.	36 nos.	36 nos.	8 nos.

3) Microscopic Analysis

			•
Location	Hole No.	Number of Specimens	1
Alternative	DDH - 3	2	
Dam Site B	DDH - 6	5	- <u>11</u>
	Total	7	
		,	

FIRST SURVEY

1) Drilling

١

Location Bore Hole No,		und ation m)	Dril Len (m	gth		Remarks
Quarry DDH-R1 Site "-R2 -R3	" 2 14	0 0.01 5.90	60.1 50.0 50.8	0	A11	core drilling - do - - do -
Total 3 2) Seismic Explorati	on		160.9	2 (m)		
Location	Site	Line	No.	Length	n (m)	Remarks
Alternative Dam	downstream	В –		385		· · · · · · · · · · · · · · · · · · ·
Site B	dam axis upstream	В – В –		44(330		
Proposed Dam Site C	dam axis	C -	1	550)	Interval of detector is
	quarry - 1	Q -	1	44()	5m each
Quarry Site	quarry - 2	Q		550)	
	quarry - 3	Q -		495	5	. ·
	quarry = 4	Q -	4	44()	
Tunnel Site	inlet to outle	t T-	1	770)	
 Total		9 Li	200	4,400)տ	

3) Test Pit and Soil Tests

n an	- <u> </u>	Excavated		N	lumber of	[Soi]	Test	S	**************************************
Location	Pit No.	Depth (m)	S.G	G.S.A	N.M.C	A.L	C.T	P.T	P.N.T
Quarry 2	TP - 12	3.0	1	1	1	1	1	1	- 1
Dam Site B	TP - 13	3.0	1	1	1	1	1	1	. 1

S.G: Specific Gravity

G.S.A: Grain Size Analysis

N.M.C: Natural Moisture Content

A.L: Atterberg Limit

C.T: Compaction Test

P.T: Permeability Test

P.N.T: Penetration Test

4) Rock Test

Specific Gravity Water Absorption Soundness Test Location Hole No. ໌<u>3</u>. Quarry 1 DDH - R1 3. 3 ŧt. 2 - R2 3 3 3 u 3 - R3 4 4 4 Total 10 nos, 10 nos. 10 nos.

SECONDARY SURVEY

(Drilling)

1<u>.</u>

Location	Bore Ho No.	le Site	Ground Elevation (m)	Drilling Length (m)	Permeability Test (Nos.)	Note
Proposed Dam Site C	DDH – C	1 dam axis	315	20.0	5	left bank
Total	1 hol	e		20.0 (m)	5 nos.	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
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			III -	05		

CLASSIFICATION OF ROCK QUALITY IN DAM FOUNDATIONS

Classification	Characteristics
W-1 <u>1</u> / H-1 A J-1	Rock-forming minerals ^{2/} are fresh and not weathered or altered. Joints and cracks are very closely adhered with no weathering along their planes. A clear sound is emitted when hammered.
W-1, 2 H-1 B J-1	Rock-forming minerals are weathered slightly or partially altered, the rock being hard. Joints and cracks are closely adhered. A clear sound is emitted when hammered.
W-1, 2 H-1 C _H J-2	Rock-forming minerals are weathered but the rock is fairly hard. The bond between rock blocks is slightly reduced and each block is apt to be exfoliated along joints and cracks by strong hammering. Joints and cracks sometimes contain clay and other material which may be colored by limonite. A slightly dull sound is emitted when hammered.
₩-3 H-2, 3 C _M J-2, 3, (4)	Rock-forming minerals are weathered and the rock is slightly soft. Exfoliation of the rock occurs along joints and cracks by normal hammering. Joints and cracks sometimes contain clay and other material. A somewhat dull sound is emitted when hammered.
W-3, 4 H-3, 4 J-4, 5 C _L	Rock-forming minerals are weathered and the rock is soft. Exfoliation of the rock occurs along joints and cracks by light hammering. Joints and cracks contain clay. A dull sound is emitted when hammered.
W-5 H-4,5 D J-5	Rock-forming minerals are weathered, and rock is very soft. There is virtually no bond between rock blocks, and collapse occurs at the slightest hammering. Joints and cracks contain clay. A very dull sound is emitted when hammered.

Note: 1/ Refer to the following table

	WEATHERING		FLRINESS		Jointing
¥-1	SOUND		VERY HARD (METALLIC SOUND HARDEN BROKEN BY RAISHER)	J-1	(Joint/s -Slightly Jointed
¥-2	SLIGHTLY WEATHERED (VISIBLE OXIDATION AT JOINT)	H-2	BARD (DEATH SOUND EASILY BROKEN BY HANNER)	3-2	1-5 Joints/a -Jointed
¥-3	HODERATELY WEATHERED (HARIX SLIGHTLY WEATHERED)	H-3	HODERATELY HARD (EDGES HARDLY HARDLY BROKEN BY HANGER)	1-3	5-10 Joints/m -Very Jointed
¥-4	the second se	1	SLIGHTLY HARD (HARDAT SQUEEZED BY FINDERS)	J-4	11-20 Joints/m -Intensely Jointed
	TOTALLY WEATHERED (OHLY TRACES OF THE ORIGINAL ATRIOTURE)			1-5	>20 Joints/s -Ground

SUMMARY OF LUGEON VALUES

SITE B $SITE C$ S	i sa				н н. 1		2 · · ·						07 7 0 0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							SITE E	3 .					SILEC
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			Hole	No	• •								
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	5 . 11				1 . A. A.							. 1	I
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	Ueptn	DDH	1	1 A A A A A A A A A A A A A A A A A A A		·						1 A A A A A A A A A A A A A A A A A A A	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			- 2	- 3	- 5	<u> </u>	$\frac{-7}{1}$	- 8	-9 K /	- 10		- 12 N/1	- <u>C~I</u>
60 J	10 - 20- 30 - 40- 50 -	20 30 11 28 4 3 2 4 5	33 8 0.5 26 23 23 42 11 355 2 1 0.5 0.5	 0.5 2 0.5 0 0.1 0.5 25 2 0.5 6 0 0	26 8 3 15 8 1 1.5 1 0.5 0.5 1 1.5 1	42 22 35 5 0.5 0.5 3 1 0.5 1 0.5 2 0.5 0	42 37 35 2	48 12 17 5 4 7	13 4 9 2 3 2 3 2 1.5 1.5 2.5 1 2 1 2 1 2 18 0	24 30 6 15 3	56 31 1 0.5 6 0.5	1.5 1.5 1 1.5 5 0.5 0.5	0

Lu: Lugeon value Q:"Unit Infiltration Volume (l/min) P: Pressure (kg/cm²) L:Tested Length (m)

10 Q

PIL

1 u ...

	r. Note									penetration	resistance	78.73kg/cm ²	-		.*			· . ·	:			•	:			* .*	•	•						³ penetration	
Coefficient of	Permeability (k) cm/s		• •		:					1.7×10-6-	7.1x10-8																	• • • •				•		4.5-7.8×10-8	· ·
Щ, н	Max. Dry Dens. tf/m3	1.382					1.722		- - - -	1.662	1.366					COC • 1	•		1.531		10 10 10	CCC+)	•	· .		1.453						•	•	1.716	
Compaction	Opt Moist. Content %	30.7	•		•	1	17.6			21.4	32.0				10	C2.42	•	•	23.6		2 1 6	0.4		•	•	28.5								19.50	
Natural	Moisture of Soil	37.41	27.46	41.34	42.83	34-26	19 28	10 15	24.70	28,92	36.66	22.09	36.61	46.45	46.81	20 55	34:32	24.68		•		18.15	16.72	25.89	24.66	36.12 26.35		47.18	19 hh	30-68	30.06	68 68 00 00 00 00 00 00 00 00 00 00 00 00 00	10-10	28.32	• •
Specific	Gravity of Soil	2.56	2,55	5 1 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1 5	2.54	2-57	2 57	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0 V 0 V 0 V	2.78	2.56	2-57	2.55	2.57	i N N	20 20 20 20 20 20 20 20 20 20 20 20 20 2	5 69	2.65	2-58	ന് പ്ര പ്ര		0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2.54	2.55	2.54	5 5 7 7 7 7	יי היי 10-	2.51	2.59	2 51	2 48	ର ଜୁନ୍ମ ଜୁନ୍ମ	ก นี้ ก ก	5-32	ня на на ла
Limit Minus #40	Plastic Limit %	21.19	20.33	26.43	22.52	15.68	11.50	0 13	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	44,40 44,40	29.95	30.22	32.79	35.97	25, 88 25, 88	20.05 70	24.57	20.26	21.02	11.18	00.1	16.48	14.93	14,88	13.55	22.22	41,25	28.76	19.9	20.88	25.3	24.38 17 23	10 10	18.23	
Atterberg	Liquid Limit X	54.0	54.7	63 . 3		42.7	31.90	27-02 00 02	20.00	04.00	53.10	58-30	59-95	67 - 00	57.20	#7-00 #2-00	48.05	42.10	43-00	51 20 51 15	00. CO	37.40	38.50	38-30	36.9	#5.9 #2.7E	72.05	54.35	42.3	42.4	55.0	80 0 5 1 0	0.00 1.00	- 86 - 86	
Classi-	fication Symbol Minus #4	HO	H	HO I	5	년 i	U to	ວ ເ ທີ່ທີ່		200	с Н	HO	CH O	풘	E C	2 2	50	3 S S	2 2	ರೆಂ	56	້ວບ	ਰੇ	ر م	ਠੀ	ಕ್ಕೆ	20	; F	ೆರೆ	ਾ ਨੀ	HO	ភ្នំ	ວະ ວັ <i>ช</i>	S S	
Analysis	Gravel	0.00	00.00	0.0	00.00	000		р 1 1 1 1	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2.93 0.93	3.78	4.29	0.00	32.0		14.10	8 75	2.53	12 12	() () ()		ក្ខុណ្ណ ខេង	4.24	1.25	7.19	- 10 - 10		0.0	0.0	0.91	0	0.61	• .	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
Åna	Sand %	31.49	22.09	10,93	23 79	30.21		00 20 10 20	2 CC	27.30	29.95	42.74	31.30	43.28	20.53	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	37 22	61.03	21.39	24 51	10 17	28.28	43.05	47.58	45.37	24.86	16.48	13.23	23 . 84	43.36	23.21	27 23		12.83	
Gradation	Clay Silt % %	6		ດ່	ċ.	പ്	N 2	50 50 50 50 50 50 50 50 50 50 50 50 50 5	ว์ ณี	, .	റ	N	യ	m	ጉበ	ու) =	·ω	vo	<u>⊷</u> =	τu	D [~~	ົ		<u>~ (</u>	78.95	ח ה	Ó	SO I	ഹ	o	ณน	си	48.59	
Sample	Identifi- cation		1A 	na (21		TP-3T	4 ជ ហ ក	n Co n co	TP-12	7	A 1	8	4 5 1		54 64	68,	29	TP -7	4L	a - at	γœ	20	ပ္ပ	i ca i	6-41	н С С С С	5	Ð 0	TP-10	TOA	108		ш-13 СШ	
	Årea				· 		3		÷.,		-	2 ¹ -							· · ·	· ·· ·	ana ana			.1-					•			• • •		SITE-B	.*

SUMMARY OF SOIL TESTS

· · · · · · · · · · · · · · · · · · ·			SUMMAR	SUMMARY OF ROCK TESTS	TESTS					
Bore Hole No.	IC	DDH - R1	· · · · · · · · · · · · · · · · · · ·	· · ·	DDH - R2			DDH	I – R3	
Sample Number	No.1	No.2	No.3	No. 1	No.2	No.3	No. 1	No. 2	No.3	No.4
Sample Depth m	15.2 - 16.3	19.6 - 20.4	46.9 - 48.2	9.1 - 9.8	17.0 - 18.5	39.5 - 40.3	11.3 - 19.6	23.1 -	35.0 - 38.0	40.0 - 42.8
Rock Type	Agglomerate	E	F	Ē		= = = =	Lapilli tulf	#	Agglomerate	 F
Rock Classification	CM	G C	с _Н	CM - CH	СН	C _M – C _H	D - CL	CL	CM - CH	C _M - C _H
Specific Gravity Buik (Dwv)	2 7 2 K	040	0 756	000 000	0 785	0 870	2 405		5	2 701
Bulk (SSD)	2.471	2.798	2 761	2.846	2.814	2.901	2.520	2.557	2.636	2.734
H Apparent	2.768	2.812	2.770	2.892	2.868	2.944	2.719		.75	2.793
H Coefficient. N of Absorption %	0.553	0.271	0.505	2.420	1.044	0.716	4.803	3.964	2.569	1.213
Density Dry	2.717	2.750	2.722	2.753	2.784	2.804	2.364	2.514	2.590	2.758
Wet g/cm ³	2.725	2.754	2.727	2.773	2.797		2.446	2.570	2.641	2.781
Effective Void Ratio	0.01508	0.00753	0.00490	0.02445	0.02905	0.02189	0.11380	0.09653	0.06612	0.03272
Porosity %	1.517	147.0	. 0.577	2.417	2.894	2.208	11.548	9.725	6-577	3.294
Water Content %	0.28	0.15	0.19.	0.74	0.46	0.66	3.48	2.24	1.97	0.85
Degree of Saturation 🖇	97.952	100	96.476	98.614	97.554	97.027	88.474	90.523	93.778	96.201
Soundness Test % Loss After 5 Test Cycles	3.42	1.98	2.93	6.022	6.973	12.626	57.175	55.940	29.143	8.988

STRAMARY OF POCK TESTS

TABLE III-8

			e de la composition d La composition de la c	Unit: m
Block No. of the Basi	Highest Elevation n in the Block	Lowest Elevation in the Block	Undulation	Average Elevation
1	790	190	600	490
2	720	185	535	452.5
3	305	178	127	241.5
4	442	162	280	302
5	385	98	287	241.5
6	385	110	275	247.5
7	303	95	208	199
8	280	95	185	187.5
9	331	157	174	244
10	260	85	175	172.5
11	245	94	151	169.5
12	254	170	84	212
13	432	220	112	326
14	340	168	172	254
15	240	98	142	169
16 :	222	130	92	176
	Total Average		3,599 224.9	4,084.5 255.3
	<u> </u>			

UNDULATION IN CATIPAYAN RIVER BASIN

	Specific Gravity	Water Content (\$)	Void Ratio (t/m ³)	Wet Density (t/m3)	Dry Density (t/m3)	Saturated Density (t/m3)	Cohesion Angle of Internal (t/m ³) Friction	Angle of Internal Friction(0)	Coefficient of Permeability (m/sec)
Overburden and D - C _L Class Bedrock	2.61	24.0	0.63	86	1.60	. 99	ī	350	1 × 10 ح
	2.61	26.0	0.37	1-95	1.90	2.67	Т.	370	1 x 10 ⁻³
Transistor Better than CL Class Bedrock	2.61	26.0	0.37	1.95	1.90	2.67		390	1 x 10 ⁻¹ -10-3
	2.65	2.0	0.39	1.94	1.90	2.19	I	420	1×10^{-1}

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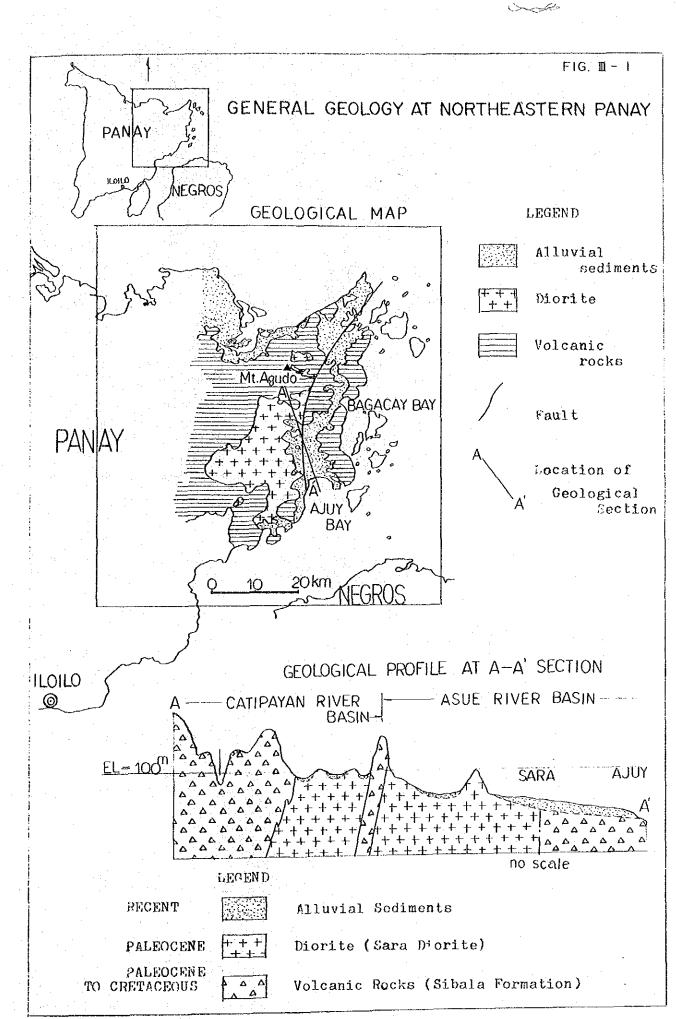
1 100 C

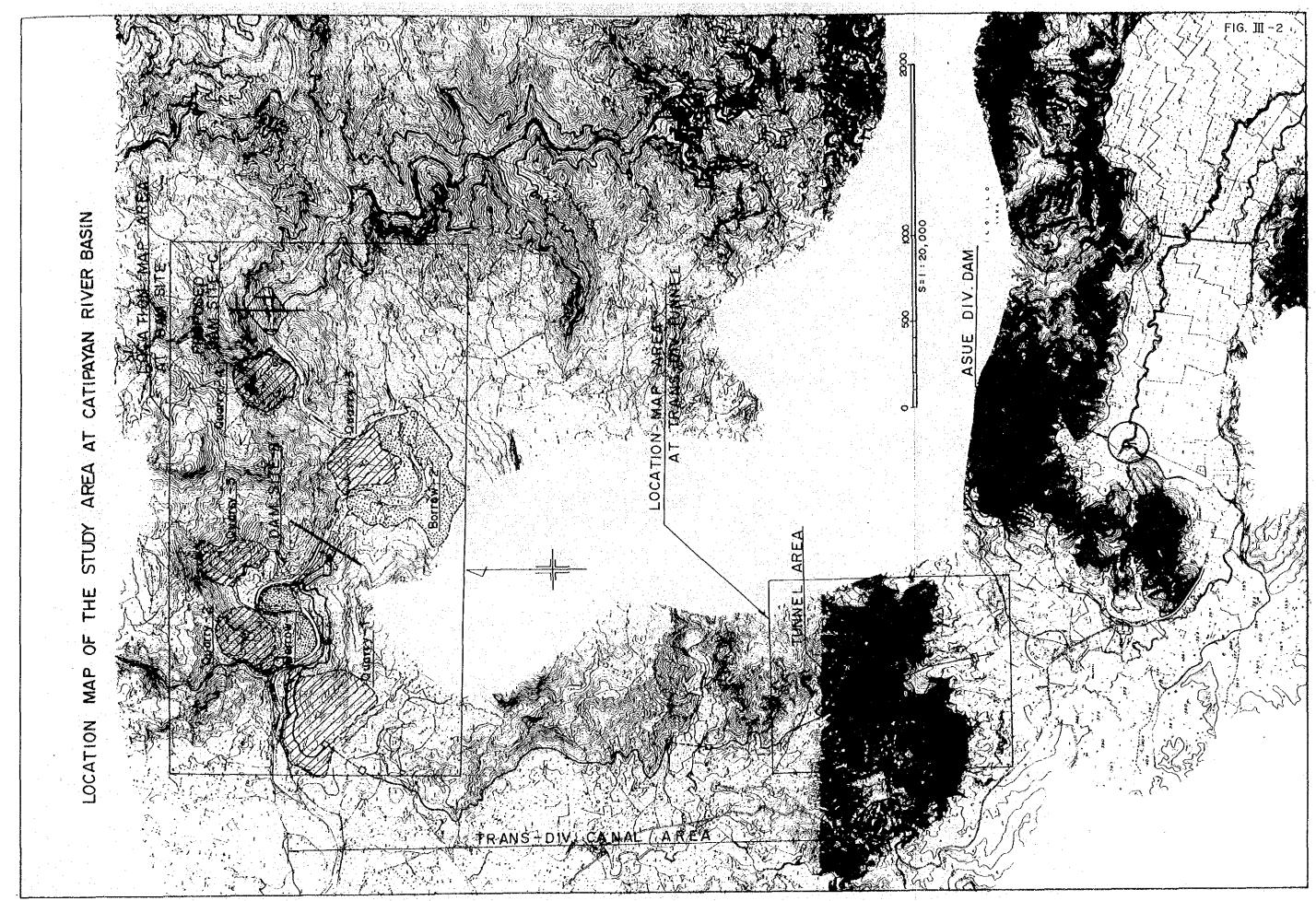
TABLE III-10

<u>\$</u>____

	e I a E		• .						TABLE III
	Pervious Zone	N 1				1			
Adoptability <	Impervious Zone	ب ا ع	1	ઌ૾ૺઌૻૺ ૫		م م	5 0 ,		
A	Earthfill Dam	110	• •	।। स	r N	აიკი	ວ ດ .	7	2000 1000 1000 1000 1000 1000 1000 1000
	Workability for Embankment	88 000 00 0 0 0		0 57 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		adoptable 5 2400000000000000000000000000000000000	auopvarte not	auopvanie 7	P - 1
ristics	Compress-1/ ability	excellent almost none almost none	good	excellent adoptable small	80 00 00 00 00 00	medium good to	large	not adontable	not adoptable
Characteristics	Shearing1/ Strength	almost none good good	very small	almost none very small	small	adoptable medium	adoptable adoptable	Large Large	large - condition
	Permeability after Compaction	excellent Is Is	s good to adontable	excellent good	is good to scotto	aucy can adoptabl	δ. <u>δ</u> . φ.	adoptable not adoptable	at e c
	Soil Classification Group	GW pervious GP very pervious GM semi-pervious	to impervious GC impervious	SW pervious SP pervious SM somi-porvious		ML semi-pervious CL impervious		CL impervious a	OH impervious mot PT - adoptabl after compaction and satur

CHARACTERISTICS AND ADAPTABLLITY OF SOILS





III - 34

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