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ANNEX I DAM AND POWER PLAN

ANNEX - I

DAM AND POWER PLAN

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I. DAM AND POWER PLAN

I.1 DAM AND RESERVOIR

I.l.l San Fernando Reservoir

Through the study of alternative plans in Annex G, it was proposed to design a concrete gravity dam at San Fernando (catchment area of 1,665 km² with an average annual inflow of around 400 MCM). The effective storage capacity at the San Fernando damsite was determined to be 380 MCM for irrigation of 23,960 ha and power generation of 53.6 GWh. (Refer to Annex G.6)

The minimum operating level of the San Fernando reservoir was determined on the basis of estimated sediment yield. To allow sediment deposit of 67 MCM in 50 years, the minimum operation level was set at EL. 797.0 m. The full supply level was determined to be at EL. 826.5 m.

To determine the maximum reservoir water level, a flood routing study was made by applying the probable maximum flood and probable maximum discharge for a return period of 200 years. In the flood routing analysis, the width of spillway was pre-determined at 30 m at maximum, because of a narrow topographic shape of the damsite. The flood routing study indicated that the probable maximum flood with a peak discharge of 5,300 m³/s would be lowered to 3,380 m³/s, and the maximum reservoir water level would reach up to EL. 832.83 m, as shown on Table I-01 and Figure I-01. As a consequence, the maximum reservoir water level was designed to be at EL. 833.0 m.

I.1.2 Spillway and Freeboard

The spillway was designed to safely discharge flood for a return period of 200 years, regardless of natural flood control effect by the reservoir. The estimated flood peak discharge of 2,920 m³/s at San Fernando could be discharged at EL. 828.0 m without natural flood control

effect. With a flood control effect of the reservoir, the outflow discharge of the PMF from the spillway would be $3,380 \text{ m}^3/\text{s}$. The spillway crest elevation was set at EL. 819.0 m. The width of overflow section was topographically limited to 30 m. The spillway would be equipped with 3 sets of spillway gates with 10.0 m in width and 8.0 m in height.

A freeboard from the maximum reservoir water level up to the dam crest was determined on the basis of wave height of reservoir water and creep height on dam slope by S.M.B. method, as well as some possible risk of delay of the spillway gate operation. The wave height and creep height were estimated to be 1.0 m, and a freeboard of 1.0 m would be additionally provided for spillway gate operation. Consequently, the dam crest elevation was designed to be at EL. 835.0 m

I.1.3 Preliminary Design of Dam

The river bed at the damsite is approximately at EL. 740.0 m. Through the geological survey explained in Annex D.2.1, the weathered rock outcrops at the damsite are to be excavated up to the maximum excavation surface at EL. 735.0 m. Consequently, the San Fernando dam was designed to have a height of 100.0 m. The crest length at EL. 835.0 m would be approximately 320 m. The dam crest is designed to be 8.0 m in width.

Stability analysis for design of the San Fernando dam was made on the basis of design conditions summarized hereunder.

Max. water level	EL .	833.0 m
Full supply level	EL.	826.5 m
Min. operation level	EL.	797.0 m
Spillway crest elevation	EL.	819.0 m
Dam crest elevation	EL.	835.0 m
Foundation elevation	EL.	735.0 m
River bed elevation	EL.	740.0 m

Unit weight of concrete	2.35 t/m3
Unit weight of sediment load	1.0 t/m ³
Shearing strength	10 kg/cm ²
Earthquake acceleration	0.16 g
Internal friction angle	45°

The stability against sliding, bearing capacity and overturning were calculated in the following manner:

a) Stability against Sliding:

$$N = \frac{f \cdot \Sigma V + To \cdot B}{\Sigma H}$$

where,

- N: Safety factor against shearing sliding (≥ 4) f: Coefficient of internal friction angle
 - $(f = fan 45^{\circ})$
- To: Shearing strength of foundation rock (10kg/cm²)

Horizontal distance of resultant forces (m)

- B: Width of dam foundation (m)
- ΣV : Total vertical forces (ton)

b) Stability against Bearing Capacity:

$$q = \frac{\Sigma V}{B} (1 \pm \frac{6e}{B})$$

where,

q: Bearing capacity (t/m)

- ΣV : Total vertical forces (ton)
- B: Width of dam foundation (m)
- e: Eccentric distance (m)

c) Stability against Overturning:

$$X_{O} = \frac{\Sigma M}{\Sigma V}$$

where, Xo:

 ΣM : Total moment (t · m)

EV: Total vertical forces (ton)

The result of stability analysis is shown in detail on Table I-02.

For foundation treatment, joints and seams are to be adequately treated by consolidation grouting and, if required, by dental work. Double grout curtain is to be provided at the heel of the dam, and drainage holes are drilled in the bottom inspection gallery at the downstream side of the curtain grouting, to release uplift pressures.

An embedded pipe cooling system would be required in concreting the dam. A network of galleries would be incorporated in the dam for inspection and other purposes.

In accordance with such design criteria, a preliminary design of the San Fernando dam was prepared as shown on DWG-I-02 and DWG-I-03.

I.1.4 Diversion During Construction

Diversion works would consist of an upstream and downstream cofferdams and a circular shape diversion tunnel. In view of the height of mat concrete designed for dam safety at the bottom of dam, the upstream cofferdam was designed to be a concrete dam of 20 m in height above foundation rock or 15 m above the river bed (crest elevation of the cofferdam at EL. 755.0 m). The discharge capacity of river diversion tunnel was determined by applying the design flood for a return period of 1.5 years which was estimated at 260 m³/s. The river diversion flood routing was calculated as shown on Table I-03. As a consequence, the diversion tunnel was designed to be 6.0 m in diameter. The tunnel would be lined with concrete of 50 cm in average thickness. The diversion tunnel would be approximately 340 m in length, as shown on DWG-I-04.

1.2 POWER FACILITIES

1.2.1 Generating Equipment

The installed capacity of the generating plants to be installed at the San Fernando power station located at the toe of the dam was determined in accordance with the formula presented in Annex G.3.2 and the proposed reservoir water level. The rated reservoir water level was designed at EL. 815.0 m, and the maximum plant discharge was estimated at $32.2 \text{ m}^3/\text{s}$.

The plant discharge in the dry season would vary in the range of $8 \text{ m}^3/\text{s}$ to $32 \text{ m}^3/\text{s}$, in accordance with the requirement of water release for irrigation. A combined efficiency of turbine and generator of 18.2 MW with $8 \text{ m}^3/\text{s}$ discharge was estimated at 59%, and the efficiency of 9.1 MW was estimated at 74%. In view of a combined efficiency of turbine and generator, as well as mechanical phenomena under low load operation, it was designed to install two units of generators.

The maximum output of turbine was calculated on the basis of the following conditions:

Maximum plant discharge (Q)	16.1 m ³ /s
High water level	EL. 826.5 m
Rated water level	EL. 815.0 m
Low water level	EL. 797.0 m
Tailwater level at max. plant discharge	EL. 745.0 m
Rated effective head (h)	67.5 m
Expected efficiency of turbine at max. output and rated head (η)	88.3 %

Thus, the maximum output of the turbine was calculated at 9.4 MW as shown below:

 $Pt = 9.8 \cdot Q \cdot h \cdot \eta$ = 9.8 x 16.1 x 67.5 x 0.883 = 9,400 kW

The selected rated speed of turbine is 450 r.p.m, and its specific speed was calculated at 225.5 kW-m.

The rated capacity of turbine and generator was determined at 9,100 kW and 10,000 kVA, respectively, on the basis that the efficiency at maximum output would be 96.8% and power factor would be 91%.

In addition to two sets of turbines and generators as outlined above, a mini-hydropower would be installed to make use of water to be released for supplemental irrigation in the rainy season in the San Juan de Flores area, which is located immediately downstream of the San Fernando dam. The water release for irrigation in the rainy season would be in the range of 0.1 m^3 /s to 0.9 m^3 /s. The installed capacity of the minihydropower plant would be 500 kW. The rated speed of turbine would be 1,200 r.p.m, and the rated capacity of generator would be 625 kVA.

I.2.2 Power Station and Related Facilities

The power house was designed to be sited at the top of the San Fernando dam to equip turbines and generators as proposed above. It would be a reinforced concrete structure of 44.8 m in length, 30.0 m in width and 19.0 m in height. The power house would have adequate spaces for office, control room, cubicle room, assembly bay, and other electrical and mechanical auxiliaries. A layout of the power house and generating equipment, as well as outdoor switchyard is shown on DWG-I-05.

The power intake would be provided on the upstream face of the dam, with a bellmouth shaped opening. The conduit center line was designed to be at EL. 791.0 m, to allow intake at the low water level at EL. 797.0 m. The intake structure would include intake gates, trashracks and guides. The intake gates would be operated by stationary hoist from the operation deck. The penstock would be about 96.0 m in length, with a diameter from 3.5 m to 1.6 m.

Power generated at the San Fernando power station would be sent to the load center in Tegucigalpa. A 25 km long transmission line would be constructed to connect the power station with the receiving substation in Tegucigalpa. The voltage of this transmission line was

determined to be 69 kV, which is the standard voltage of the existing transmission line network. A single circuit line with 120 m^2 or 266.8 MCM ACSR conductors would be installed.

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TABLES

Table I-01 RESULT OF FLOOD ROUTING WITH PMF

B = 30 m

Hd=13.9 Gate=7.5 Spillway Crest=EL. 819.0 m

Ţ	Q In (m ³)	QOut	Н	Area	
	(m ²)		1 1	(A D)	С
<u>(hr)</u>	· · · · · · · · · · · · · · · · · · ·	(m3/s)	- (m)	(km2)	
0.5	60.0	60.0	826,5	20.6	1,98
1.5	60.0	60.0	826.5	20.0	1.98
2.5	65.0	65.0	826.5	20.6	1.98
3.5	75.0	75.0	826.5	20.6	1.98
4.5	85.0	85.0	826.5	20.6	1.98
5.5	.95.0	95.0	826.5	20.6	1.98
6.5	125.0	125.0	826.5	20.6	1,98
7.5	175.0	175.0	826.5	20.6	1.98
8.5	207.5	207.5	826.5	20.6	1.98
9.5	222.5	222.5	826.5	20.6	1.98
10.5	277.5	277.5	826.5	20.6	1.98
11.5	372.5	372.5	826.5	20.6	1.98
12.5	442.5	442.5	826.5	20.6	1.98
13.5	487.5	487.5	826.5	20.6	1.98
14.5	540.0	540.0	826.5	20.6	1.98
15.5	600.0	600.0	826.5	20.6	1.98
16.5	680.0	680.0	826.5	20.6	1.98
17.5	780.0	780.0	826.5	20.6	1.98
18.5	930.0	930.0	826.5	20.6	1.98
19.5	1,130.0	1,130.0	826.5	20.6	1.98
20.5	1,335.0	1,225.1	826.5	20.6	1.98
21.5	1,545.0	1,235.0	826.5	20.6	1.99
22.5	1,720.0	1,253.3	826.6	20.7	1.99
23.5	1,860.0	1,278.1	826.7	20.7	1.99
24.5	1,942.5	1,307.2	826.8	20.8	2.00
25.5	1,967.5	1,337.5	826.9	20.9	2.00
26.5	1,970.0	1,367.3	827.0	20.9	2.00
27.5	1,950.0	1,395.4	827.1	21.0	2.01
28.5	1,942.5	1,421.7	827.2	21.0	2.01
29.5	1,947.5	1,446.8	827.3	21.1	
30.5	1,980.0	1,471.7	827.3	21.1	2.02
31.5	2,040.0	1,497.7	827.4	21.2	2.02
32.5	2,140.0	1,526.5	827.5	21.2	2.02
33.5	2,280.0	1,559.9	827.6	21.3	2.03
34.5	2,465.0	1,599.9	827.8	21.4	2.03
35.5	2,695.0	1,648.5	827.9	21.5	2.04
36.5	3,000.0	1,708.5	828.1	21.6	2.05
37.5	3,380.0	1,783.4	828.4	21.7 21.9	2.05 2.06
38.5	3,827.5	1,876.6 1,991.5	828.7 829.0	22.1	2.08
39.5	4,347.5	T122T+3	0.29.0	64 · L	2.00

(to be continued)

Table I-01

B = 30 m	Hd = 13.9	Gate = 7.5	Spillway Crest	= EL. 819.0 m	
T (hr)	Q In (m3)	QOut (m ³ /s)	H (m)	Area (km ²)	C
40.5	4,737.5	2,126.2	829.4	22.3	2.09
41.5	4,992.5	2,273.2	829,9	22.6	2.10
42.5	5,160.0	2,426.1	830.3	22.8	2.1
43.5	5,250.0	2,579.9	830.7	23.1	2.1
44.5	5,237.5	2,729.3	831.1	23.3	2.1
45.5	5,122.5	2,868.3	831.5	23.5	2.1
46.5	4,980.0	2,993.3	831.8	23.7	2.1
47.5	4,800.0	3,102.8	832.1	23.9	2.1
48.5	4,597.5	3,195.5	832.3	24.0	2.1
49.5	4,372.5	3,270.8	832.5	24.1	2.1
50.5	4,097.5	3,327.2	832.7	24.2	2.1
51.5	3,772.5	3,362.9	832.7	24.3	2.1
52.5	3,457.5	3,377.7	832.8	24.3	2.1
53.5	3,152.5	3, 373, 5	832.8	24.3	2.1
54.5	2,865.0	3,352.1	832.7	24.3	2.1
55.5	2,595.0	3,315.6	832.6	24.2	2.1
56.5	2,315.0	3,265.4	832.5	24.1	2.1
57.5	2,025.0	3,201.8	832.3	24.0	2.1
58.5	1,767.5	3,126.3	832.2	23.9	2.1
59.5	1,542.5	3,041.7	831.9	23.8	2.1
60.5	1,360.0	2,950.8	831.7	23.7	2.1
61.5	1,220.0	2,856.6	831.5	23.5	2.1
62.5	1,095.0	2,760.9	831.2	23.4	2.1
63.5	985.0	2,664.7	831.0	23.2	2.1
64.5	887.5	2,568.9	830.7	23.1	2.1
65.5	802.5	2,474.2	830.4	22.9	2.1
66.5	730.0	2,381.1	830.2	22.8	2.1
67.5	670.0	2,290.3	829.9	22.6	2.1
68.5	615.0	2,202.2	829.7	22.5	2.1
69.5	565.0	2,116.8	829.4	22.3	2.0
70.5	515.0	2,034.0	829.2	22.2	2.0
71.5	465.0	1,953.8	828.9	22.0	2.0
72.5	425.0	1,876.2	828.7	21.9	2.0
73.5	395.0	1,801.5	828.4	21.8	2.0
74.5	365.0	1,729.9	828.2	21.6	2.0
75.5	335.0	1,661.0	828.0	21.5	2.0
76.5	310.0	1,594.9	827.8	21.4	2.0

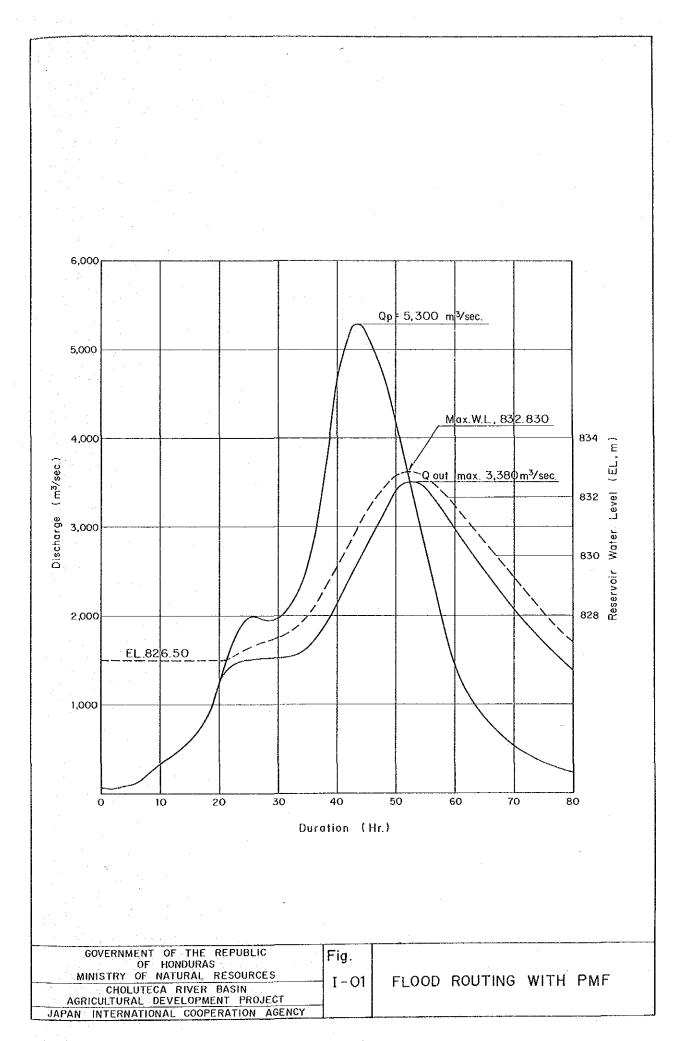
989,971,00 37,512.60 26,493.50 I,053,980.00 00.IT9, 989 (t.n) 989,971.00 X Moment 53,340.70 224,647.00 7,702.28 28,133.30 135,471.00 -26,670.30 -26,670.30 ц ц -1.360 (24.833) Ϋ́Η 3.303 (24.833) Overturning (B/6) 31.63 1 14.73 1 36.40 E l 25.31 ł 25.31 Arm Length RESULT OF STABILITY CALCULATIONS 1 ł 1 22.22 18.11 i l 1 75.17 75.17 X (m) 13,170.60 2,071.35 I 1,192.15 13,170.60 ł I 13,170.60 16,434.10 g g 124.967 83.554 Bearing Capacity Force -1,053.65 i J ŝ 7,686.62 522.78 (ton/m) 4,283.65 -1,053.65 2,107.29 772.89 (ton) Case 2: Reservoir Empty and Earthquake (K=0.08) Ш 95.625 93.233 Horizontal component of hydrostatic force Table I-02 Vertical component of hydrostatic force Inertia force caused by the earthquake on the dam itself Inertia force caused by the earthquake on the dam itself Case 1: H.W.L + Earthquake (K=0.16) Safety Factor 4.076 Sliding -26.641 Hydrodynamic pressure Weight of the dam Weight of the dam Soil pressure Total Total Case 2 Case 1

Table I-03 RIVER DIVERSION FLOOD ROUTING

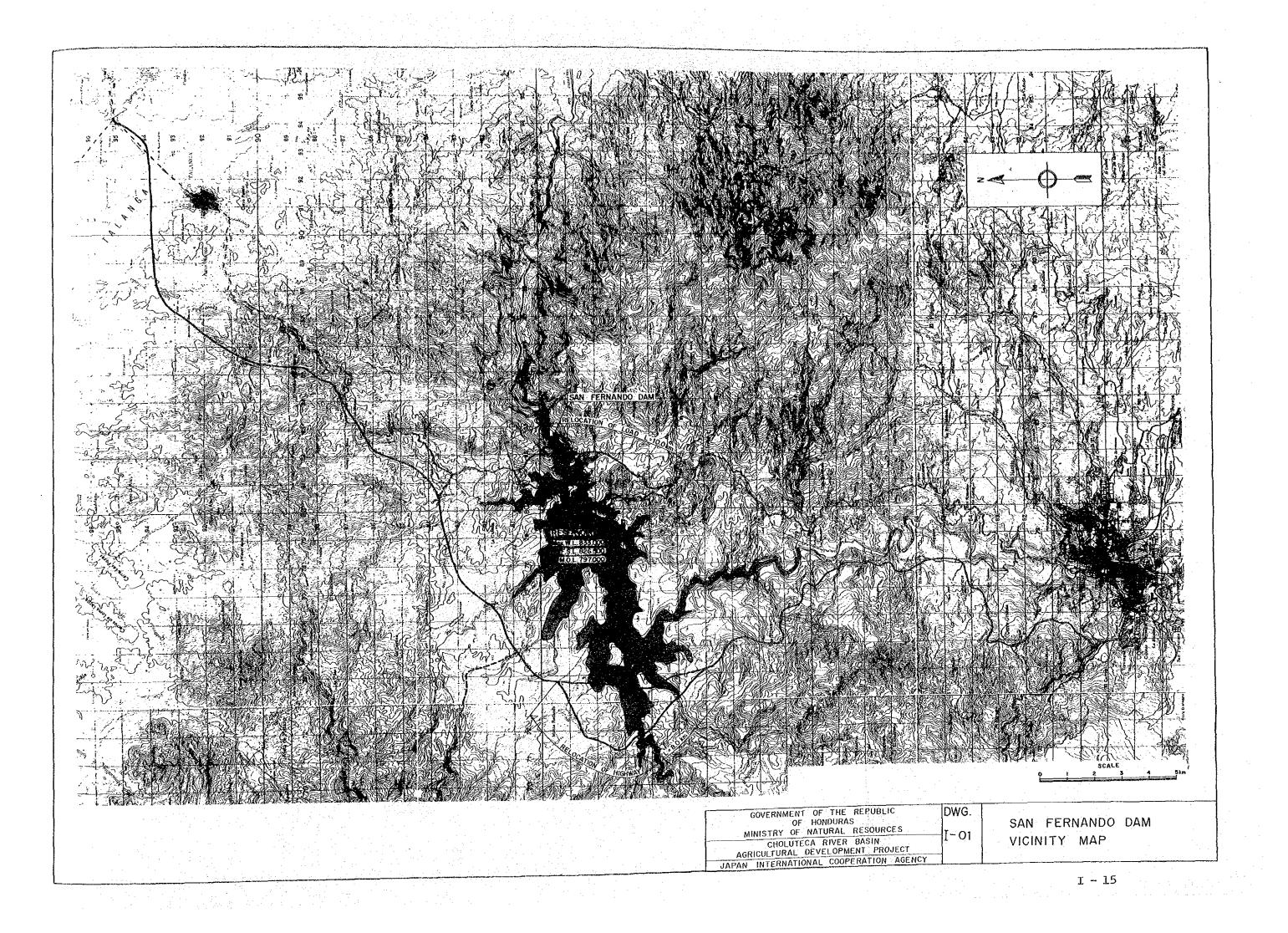
'ime	Inflow	Average Inflow	Pusai	Phai	Storage	R.W.L	Outflow
0	64	_			en de la companya de Companya de la companya de la company	747.2	63.8
1	68	238,168	0.01	0.25	0.13	747.3	67.2
2	73	254,902	0.00	0.27	0.13	747.5	72.5
3	84	283,348	-0.01	0.29	0.14	747.9	81.6
4	95	323,508	-0.02	0.32	0.15	748.3	93.8
5	119	385,420	-0.04	0.36	0.16	748.9	113.7
6	142	469,086	-0.07	0.42	0.18	750.0	135.9
7	174	569,485	-0.07	0.50	0.22	750.5	160.2
8	207	686,617	-0.05	0.61	0.28	751.3	185.4
9	224	775,303	-0.03	0.72	0.35	752.2	207.8
10	240	835,542	0.01	0.81	0.41	753.0	223.0
11	248	879,048	0.04	0.88	0.46	753.7	235.6
12	255	905,821	0.06	0.94	0.50	754.2	245.0
13	252	912,514	0.07	0.97	0.52	754.5	249.7
14	248	899,128	0.07	0.97	0.52	754.5	249.7
15	241	880,721	0.07	0.95	0.51	754.3	246.9
16	235	857,295	0.05	0.92	0.49	754.0	241.8
17	221	820,482	0.03	0.87	0.45	753.5	233.6
18	207	770,283	0.00	0.80	0.40	752.9	221.8
19	187	710,043	-0.03	0.71	0.34	752.1	206.1
20	168	639,764	-0.05	0.61	0,28	751.3	184.8
21	1.54	579,525	-0.07	0.53	0.23	750.6	165.0
22	140	529,325	-0.07	0.46	0.19	750.1	148.4
23	131	487,492	-0.06	0.41	0.18	749.8	132.0
24	121	454,026	-0.06	0.39	0.17	749.4	124.5
25	117	428,926	-0.05	0.37	0.16	749.0	116.8
26	112	412,193	-0.04	0.36	0.16	748.9	113.2
27	110	400,480	-0.04	0,36	0.16	748.8	110.2
28	112	400,480	-0.04	0.36	0.16	748.9	111.7
29	119	415,540	-0.05	0.37	0.16	749.1	117.2
30	125	438,966	-0.06	0.39	0.17	749.3	123.9
31	134	465,739	-0.06	0.41	0.17	749.7	130.7
32	142	495,859	-0.07	0.43	0.18	750.0	139.5
33	147	519,285	-0.07	0.45	0.19	750.1	145.4
34	151	536,019	-0.08	0.46	0.19	750.2	149.7
35	149	541,039	0.08	0.47	0.19	750.2	150.4
36	147	534,345	-0.07	0.46	0.19	750.1	147.9

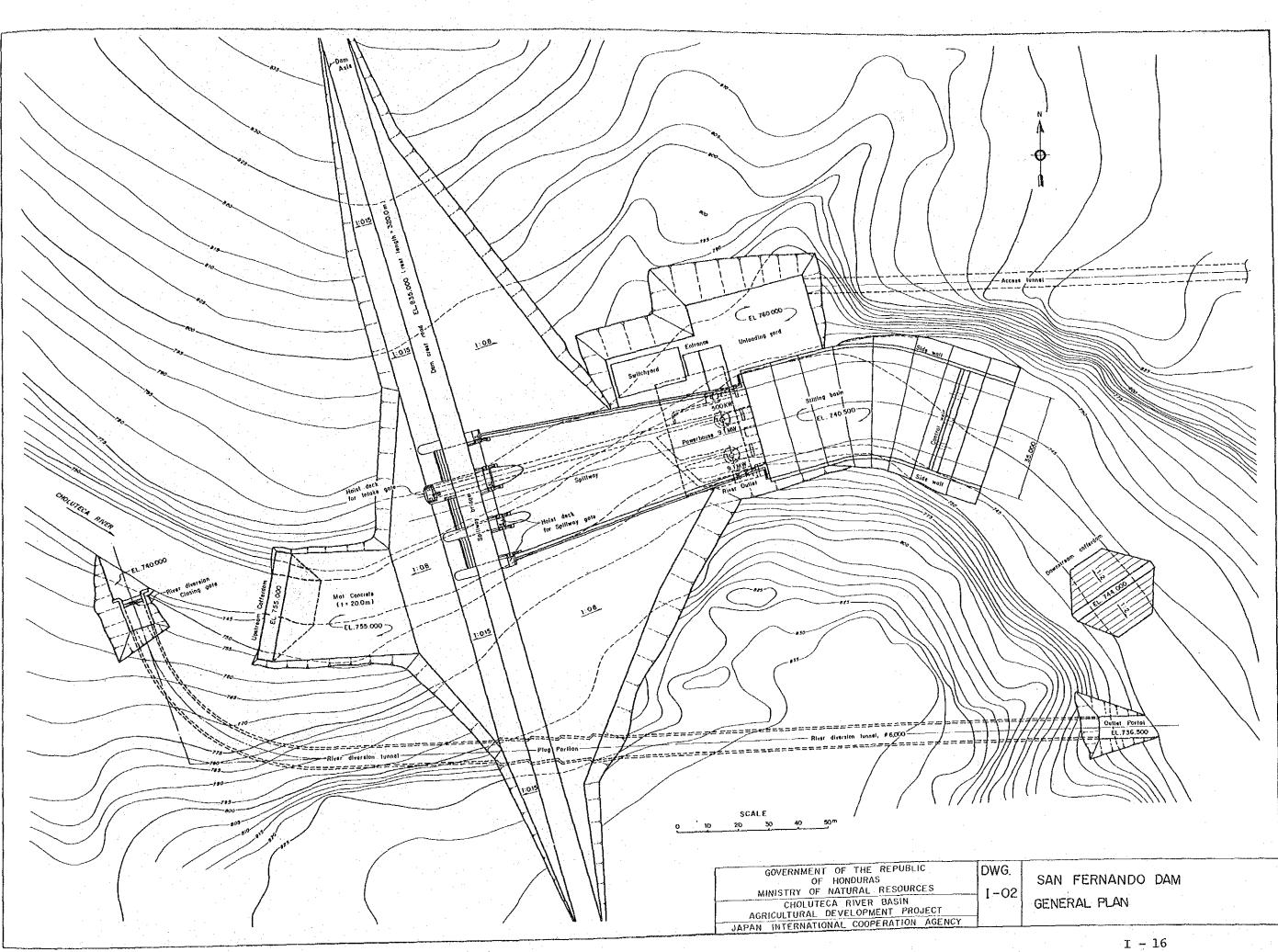
* R,W,L, 754.5 m > Crest of U/S cofferdam, EL. 755.0 m

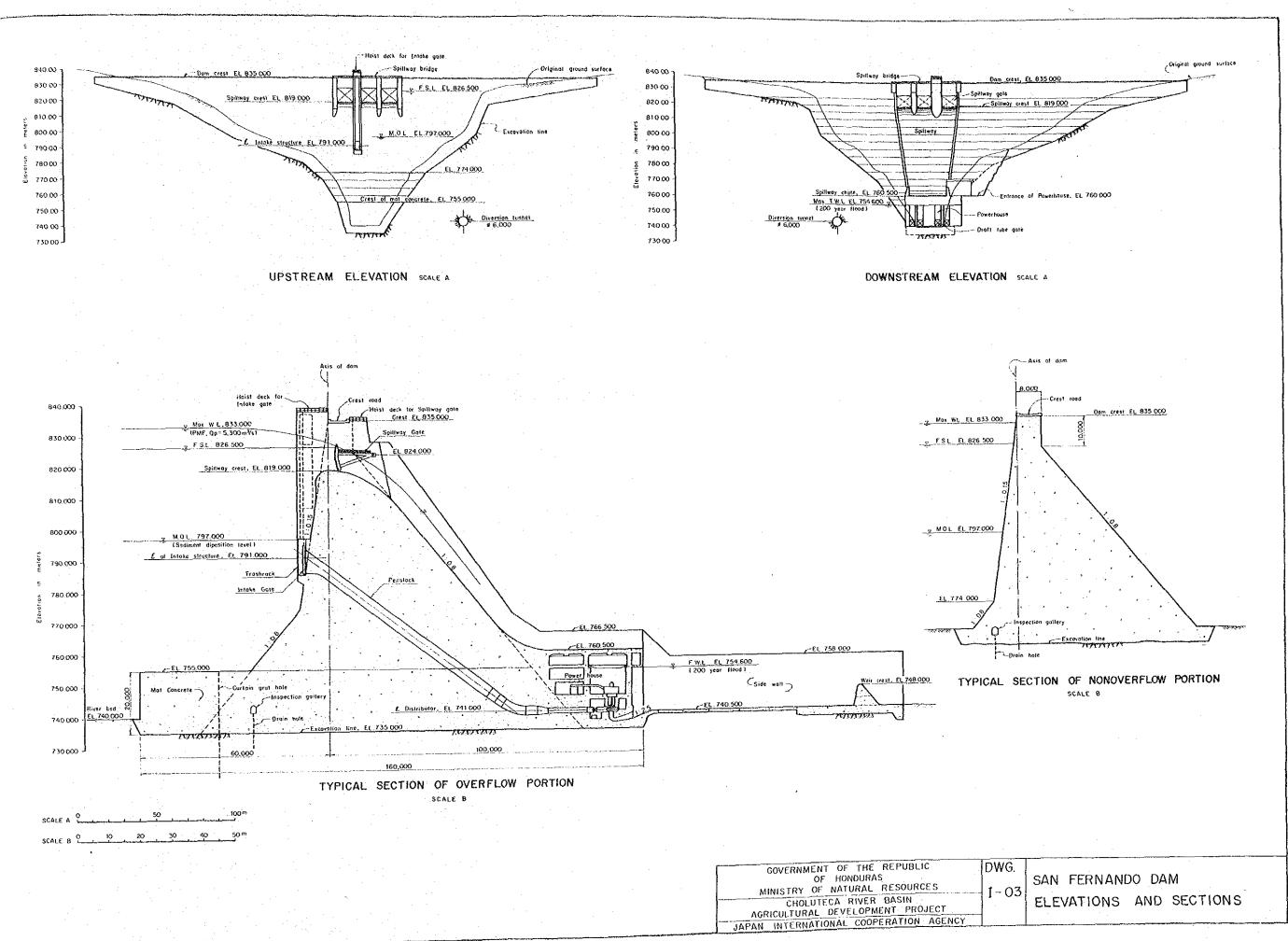


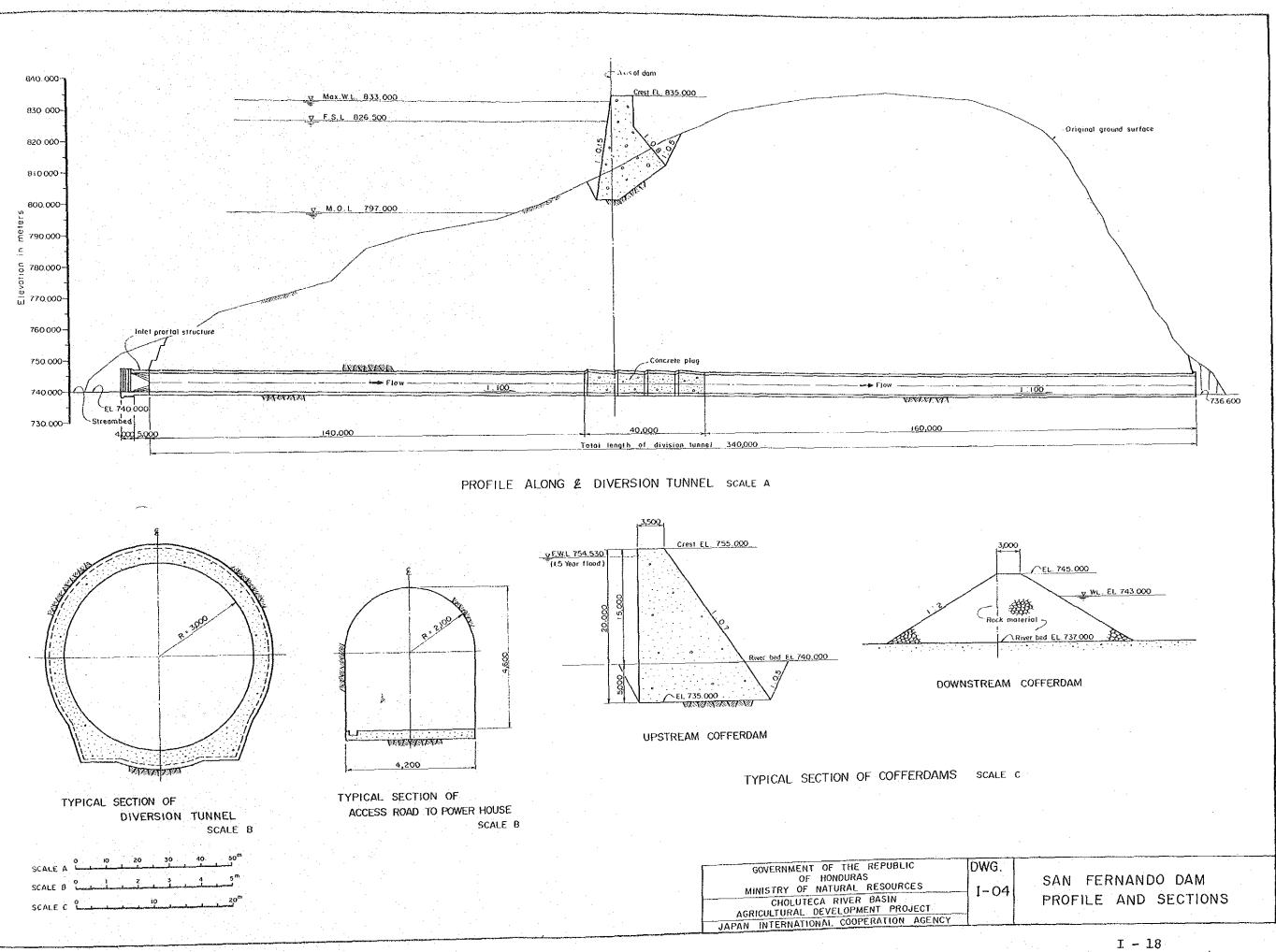


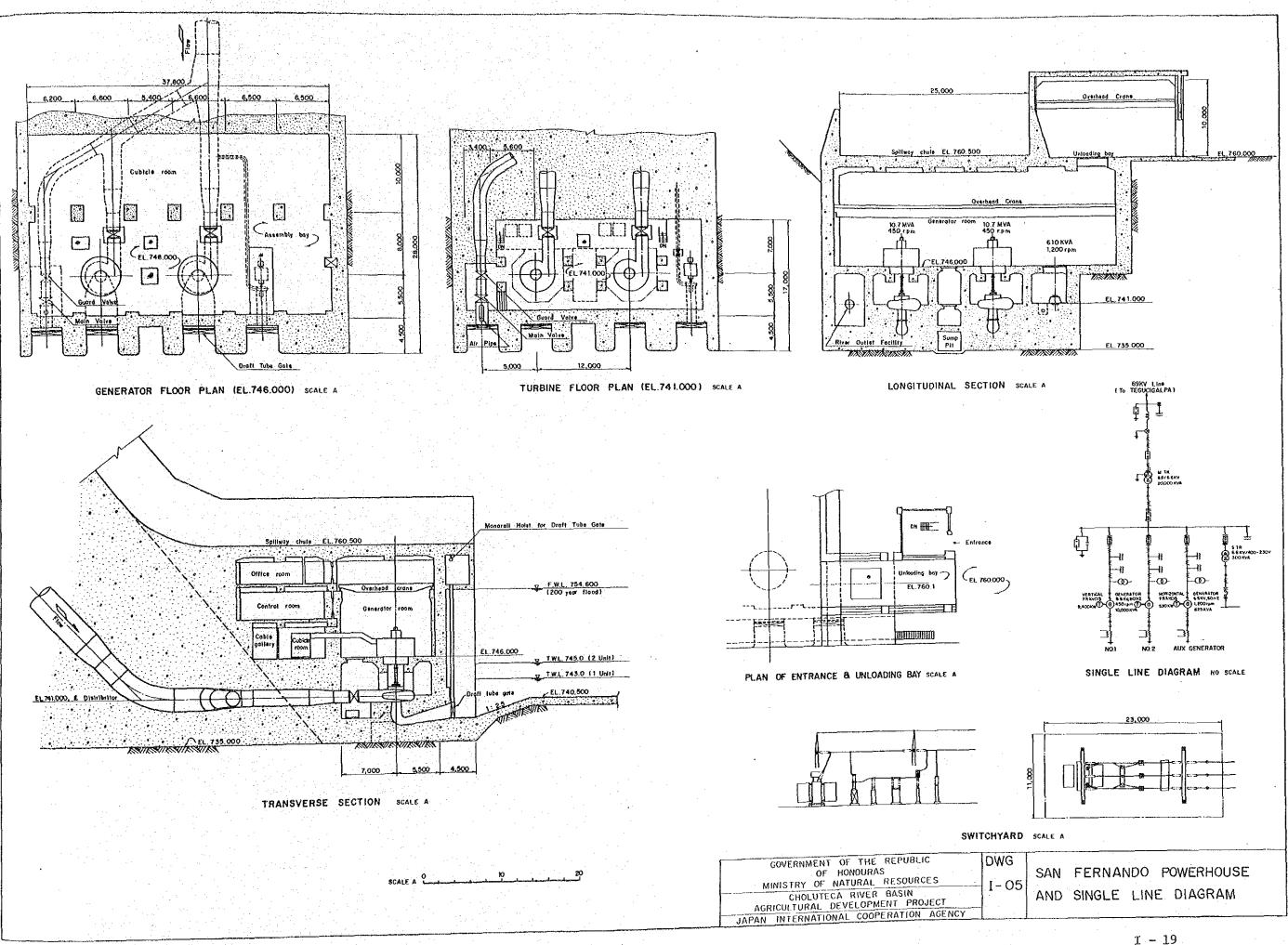
DRAWINGS











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ANNEX J

PROJECT EVALUATION

ANNEX - J

PROJECT EVALUATION

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J. PROJECT EVALUATION

J.1 CONSTRUCTION COST ESTIMATE

J.1.1 Basis of Estimate

The construction cost was estimated on the basis of work quantities measured through the preliminary design and the unit prices estimated for each item of work. All the costs were estimated at mid-1984 prices. The basis of estimate is briefly explained hereunder.

1) Civil Works:

Construction contracts will be awarded to contractors to be selected through international competitive tenders. The unit prices of civil works include direct costs such as labor, materials and equipment, as well as indirect costs such as contractors' overhead and profit. They were estimated in the following manner:

a) Labor Cost:

The workable days for civil works were estimated on the basis of meteorological records, as shown in Table J-Ol. For instance, around 208 days are workable for common civil works and 252 days for concrete works. The labor cost was estimated on the basis of daily wage rates of labors as listed up on Table J-O2. The actual working hours were presumed to be 7.0 hours a day.

b) Material Cost:

The cost of construction materials includes cement, fuel and lubricants, steel and bars, wooden materials, explosives, electric power, etc. The unit prices of major materials are shown on Table J-03. The locally available materials will be procured in Honduras, and other materials will be imported from abroad. As an exceptional case, the cost of locally manufactured cement was presumed to be disbursable for 70% in foreign currency and 30% in local currency.

c) Equipment Cost:

The equipment cost includes the depreciation, maintenance and repair, as well as administration costs. The equipment to be required for each work item was selected according to the construction time schedules and work quantity. The unit prices of imported equipment were estimated on the basis of CIF Honduras and inland transportation cost.

d) Miscellaneous Cost:

About 5% of the labor, materials and equipment costs were added as miscellaneous cost to cover non-itemized construction costs.

e) Indirect Costs:

Contractors' overhead costs and profit were estimated as indirect costs. It was presumed that the indirect costs would be around 25-30% of the direct costs.

2) Electrical and Metalworks:

The cost of metalworks, such as gates, stoplogs, trashracks, penstocks, valves, etc., as well as such electrical works as generating equipment and substation, was estimated on the basis of current international unit prices per ton of similar works.

3) Land Compensation:

The cost of land compensation in the submergible area by the San Fernando dam (2,450 ha) was estimated in according with the land use interpreted by aerial photographs. (In economic evaluation, production forgone of the land is counted as negative benefits of the projects.) In addition, compensation of land for construction of irrigation and drainage canals on the Choluteca plain was estimated. Further, the cost of land expropriation, entitling and training for the possible resettlement area was incorporated in the estimate of land compensation.

4) Engineering Services and Administration:

The cost of engineering services was estimated to cover the fee of consultants to be retained for detailed design, assistance in preconstruction engineering and supervision of the construction works. The cost will also cover detailed topographic, geologic and soil-mechanic investigations required for the design and construction.

5) Physical and Price Contingencies:

The cost estimate was made on the basis of data and information available from the feasibility level investigation. Therefore, some allowances should be provided for such unknown factors as i) changes in items and quantities in the detailed design, ii) changes in the assumed geological conditions to be encountered during construction, iii) degree of accuracy of available topographic maps in some places, etc. The physical contingency was estimated at 10% of the direct construction cost. On the other hand, the price contingency was estimated to cover the price escalation. The escalation rate of the costs to be incurred in local currency was presumed at 6%, based on the price indices as noted in Annex A.2.6. The escalation rate of the costs to be incurred in foreign currency was assumed at 5% in the light of recent international price escalation in similar works.

J.1.2 Construction Cost

The direct construction cost of dam and power facilities was estimated at around Lp. 105.5 million, of which about Lp. 15.5 million would be for power facilities. Details of the estimated cost are shown on Table J-04 and J-05. On the other hand, the direct construction cost of irrigation and drainage facilities was estimated for each case of implementation program, i.e. irrigation of 23,960 ha (total irrigable area), 20,600 ha (Western plain + Eastern plain - A), 16,000 ha (Western plain) and 12,400 ha (lst phase in Western plain). The estimated cost is shown in detail on Table J-04 to J-06.

The total estimated construction cost, including land compensation, engineering and administration, physical and price contingencies, for each case of implementation program is shown on Table J-04, and summarized as follows:

		(Unit:	Lp.10 ³)
	Foreign Currency	Local Currency	Total
Dam, Power & Irrigation			
23,960 ha	269.5	107.3	376,8
20,600 ha	263.5	106.1	369.6
16,000 ha	218.2	84.6	302.8
12,400 ha	197.2	75.6	272.8
the second se			

J.1.3 Disbursement Schedule

The estimated construction cost will be disbursed in accordance with the progress of construction works. By referring to the construction time schedule presented in the Main Report, Chapter 7.1, a disbursement schedule was prepared for each case of implementation program as shown on Table J-07. Details of disbursement of dam and power facilities, as well as irrigation facilities, are shown on Tables J-09 and J-10. Annual disbursement in each case is summarized as follows:

		1. L. 1.	Deg.			공 : 1		(Unit:	Lp.106)
	<u>1985</u>	1986	1987	<u>1988</u>	1989	<u>1990</u>	<u>1991</u>	<u>1992-4</u>	Total
Dam, Power & Irrigation:									
20,600 ha	7.6	5.7	30.9	80.6	99.6	75.8	10.3	59.1	369.6
16,000 ha	7.6	5.7	30.9	80.6	97.5	71.8	8,7	-	302.8
12,400 ha	7.6	5.7	30,4	78.7	96.2	54.2		÷*	272.8
Middle reach	-		1 77	1.6	2.8	2.8		-	7.2

Further, in the cases of dam, power and irrigation of 20,600 ha and 16,000 ha, the cost of dam and irrigation was segregated from power facility cost, to facilitate financial evaluation of each component of the facilities, as shown on Table J-08.

J.2 ECONOMIC EVALUATION

J.2.1 Economic Cost

Economic evaluation was made on the basis of economic cost estimated for the implementation of the project. The economic cost is the cost to be required by the project from the view point of national economy. There are some costs which are not to be counted as economic costs in terms of national economy, even if those costs are financially required for implementation.

1) Economic Construction Cost:

The economic cost was estimated in terms of local currency. In the economic cost, such transfer payments as taxes and subsidies are excluded. Shadow rates were assumed in evaluating the economic value of labor, foreign exchange, etc. from the viewpoint of national economy.

a) Taxes:

Taxes to be imposed in Honduras are considered as a transfer payment which is only transferred within the national economy and are excluded from the economic cost. As for local materials, their market prices include some taxes which are not exempted or reimbursable. The amount of such taxes was deducted in estimating the purchase cost of local materials.

b) Shadow Wage Rate:

The wages of skilled labors appear to reflect the market mechanism and are taken into account as the economic cost. The unskilled labors are in excess of actual demand, and unemployment and under employment rate is substantially high as reviewed in Annex A.1.2. In such a case, the opportunity cost of unskilled labor, which is defined as the production value of unskilled labor to be sacrified in other fields by employment in the project, is considered to be economic cost of unskilled labor. As noted in Annex A.1.2 the opportunity cost of unskilled labor is evaluated at 0.5 of the wage to be actually paid to labor, and this shadow wage rate was applied in the project evaluation.

c) Shadow Exchange Rate:

The equipment, plants and materials, as well as agricultural products, to be imported from or exported to foreign countries will be estimated at a higher rate if they are valued in local currency. As noted in Annex A.2.4, the shadow exchange rate of foreign currency is evaluated at Lp. 2.5 per U.S. Dollar. Consequently, this shadow exchange rate was applied in estimating the economic cost of imported equipment, materials and services, as well as of exported or import-substituted agricultural products.

In the estimate of costs to be incurred in local currency, the taxes to be imposed on local purchase and the shadow wage rate of unskilled labor were calculated as approximately 5% of the principal costs estimated on the basis of market prices. On the other hand, the costs to be incurred in foreign currency were valued at 1.25times of the estimated financial costs.

The economic construction cost of the San Fernando dam, power and irrigation facilities was estimated for each case of implementation program, as summarized on Table J-11. The disbursement of such an economic cost was also estimated as indicated in Table J-11.

2) Operation, Maintenance and Replacement Cost:

In addition to the construction cost, the economic cost of the project comprises the operation and maintenance cost (O&M cost) and necessary replacement cost. They were estimated as follows:

a) O&M Cost:

The O&M cost covers the wages of operation and maintenance staff, regular maintenance and minor repair costs. The annual O&M cost was estimated at 0.1% of the San Fernando dam construction cost, 0.5% of the power facility costs, and 2.0% of irrigation facility costs. The annual O&M cost was estimated at Lp. 3.1 million for the San Fernando dam and irrigation of 23,960 ha, and of Lp. 2.35 million for the dam and irrigation of 19,360 ha.

b) Replacement Cost:

The economic life of civil works is 50 years, and the project life was assumed to be the same period for economic evaluation. On the other hand, the economic life of electrical works and metalworks was assumed to be 25 years, and a replacement cost after this useful life period was taken into account. A salvage value of about 10% was assumed in estimating the replacement cost.

J.2.2 Economic Benefit

Economic benefit accrues directly from increased agricultural production and electric power generation. The benefit was estimated in such a manner as explained hereunder.

1) Agricultural Benefit:

Agricultural benefit was estimated by incremental benefit, comparing "with" and "without" project conditions. For economic evaluation, economic prices of farm inputs and outputs were estimated in accordance with the following procedures:

a) The economic price of agricultural products was basically estimated by referring to the IBRD price forecasts. The projected 1995 world market prices were valued at 1984 constant prices. The farm-gate price was calculated by estimating transportation, processing and other charges, as shown on Table J-12. The farm-gate prices of some products not included in the IBRD forecasts were estimated by applying a ratio between economic prices and local market prices of similar products, as summarized on Table J-13.

b) The economic price on farm inputs was estimated by applying a ratio between economic prices and market prices calculated for some representative inputs, as shown on Table J-14. For the estimate of machinery cost, the shadow exchange rate was applied to the imported components (75%). Labor cost was estimated by applying the shadow wage rate of 0.5.

On the basis of economic prices of inputs and outputs, the economic net return of each crop was calculated respectively for "with" and "without" project conditions. A summary of net return of each crop is shown on Table J-15 and J-16, and their breakdown is presented in Table J-17 and J-18. Further, the net incremental benefit of agricultural development in the Western plain and Eastern plain - A was estimated in accordance with the cropping areas studied in Annex E and Annex F, as summarized on Table J-19 and J-20. Through the development of Western plain (16,000 ha), it is expected that the net incremental benefit would amount to Lp. 39.89 million. Likewise, the development of Eastern plain - A (4,600 ha) would bring about a net return of Lp. 14.65 million.

Benefit from the development in the middle reach valleys (2,680 ha in San Juan de Flores and 680 ha in other existing areas), for which water would be stored and secured by the San Fernando dam, was also estimated as an associated benefit of the project. The net return of each crop was estimated respectively for "with" and "without" project conditions in Table J-21 and J-22. The net incremental benefit would amount to Lp. 5.65 million in the middle reach valleys, as shown on Table J-25.

Consequently, the agricultural benefit annually accruable from the project was estimated to amount to approximately Lp. 45.5 million for the first stage development of 19,360 ha, and to Lp. 60.2 million in total by the second stage development of 23,960 ha.

The said benefit would be achieved in a build-up period of 5 years. In preparing a benefit flow, therefore, the benefit during the build-up period was calculated to gradually increase from 40% of the target in the first year, 55% in the second year, 70% in the third year and 85% in the fourth year, as shown in Table J-26.

2) Power Benefit:

As noted in Annex G.3, power plants would be installed to make use of water released for irrigation and to cover the base load of the demand in the dry season. Power would also be generated in the rainy season by mini-hydropower. In a rainy year, power would also be generated by surplus water to be released from the reservoir. For evaluation of power benefit, the capacity value and energy value of alternative power source was estimated as explained hereunder.

a) Capacity Value:

Although the installed capacity was determined at 18.2 MW (average capacity in April), power generation in the dry season would be rather fluctuated in accordance with water release for irrigation. An average capacity during the period from December to April would be 11.1 MW, and this was taken in evaluating the capacity value of the alternative power source which would be an oil-fired thermal plant. Although some peak power would be generated by surplus water in the rainy year, such a peak power generation would be unstable and additional capacity value was not counted in evaluation.

The capacity value was calculated on the basis of 50 MW class oil-fired thermal plant which would cost \$920/kW. As shown on Table J-27, capacity value was estimated at Lp. 2,630/kW or Lp. 29.2 million for 11.1 MW. The first operation and maintenance value was estimated at Lp. 52.6/kW or Lp. 584,000 per annum.

b) Energy Value:

The energy value of oil-fired thermal plant was estimated on the basis of prevailing fuel cost (\$27.18/bbl). As shown on Table J-28, the energy value was estimated at Lp. 0.1311/kWh. For an annual energy output of 53.6 GWh, the energy value was estimated at Lp. 7.02 million per annum. The annual variable operation and maintenance value was estimated at Lp. 536,000.

J.- 9

c) Replacement Cost:

The replacement cost to be required after the service life of 25 years was estimated to be about 90% of the initial investment cost.

On the basis of the above estimate, the annual economic power benefit was estimated as summarized on Table J-29. The annual benefit during the period of alternative power operation would amount to around Lp. 8.1 million.

3) Negative Benefit:

The San Fernando reservoir will flood an area of 2,450 ha. The present production in the area cannot continue after the completion of dam. Though it is small, the production forgone in the reservoir area was estimated at Lp. 0.38 million as shown in Table J-30. This was taken into account as a negative effect of the project.

J.2.3 Economic Internal Rate of Return

The economic cost estimated in Chapter J.2.1 and economic benefit estimated in J.2.2 were computed into an economic cost and benefit flow of each case of implementation, as shown on Table J-31. Further, the economic internal rate of return (EIRR) was calculated as summarized hereunder.

	EIRR (%)
Dam, Power & Irrigation	
20,600 ha (+ middle reach)	14.2
16,000 ha (+ middle reach)	13.7
12,400 ha (+ middle reach)	12.9

Judging from EIRR calculated above, the full scale development with irrigation of 20,600 ha in the Choluteca plain was found to be economically feasible, and it is recommended that the implementation program be set with a target to irrigate the Western plain (16,000 ha) and Eastern plain -A (4,600 ha). Even in case that the implementation is programed for the first stage development on the Western plain, EIRR would be well over the opportunity cost of capital in Honduras (at least 12%) and its implementation is thus economically justifiable.

J.2.4 Sensitivity Analysis

A sensitivity analysis of EIRR was made by assuming some changes in variables in benefit and cost. The result of sensitivity analysis is summarized as follows:

Dam,	Power	and	Irrigat	ion	(20,600 h	ia)
	Cc	ost		·	Benefit	Decrease
	Thor	000	````	<u> </u>	ÇQ.	10.9

COST		Benefit I	tecrease	
Increase	08	5%	10%	15%
08	14.2	13.7	13.1	12.4
5%	13.7	13,1	12.5	11.9
10%	13.2	12.6	12.0	11.4
15%	12.7	12.1	11.6	11.0

Dam, Power and Irrigation (16,000 ha)

Benefit Decrease					
0%	5%	10%	15%		
13.7	13.1	12.5	11.8		
13.1	12.6	12.0	11.4		
12.6	12.1	11.5	10.9		
13.1	11.6	11.1	10.5		
	13.7 13.1 12.6	0% 5% 13.7 13.1 13.1 12.6 12.6 12.1	0% 5% 10% 13.7 13.1 12.5 13.1 12.6 12.0 12.6 12.1 11.5		

Dam, Power and Irrigation (12,400 ha)

Cost		Benefit I	Decrease	
Increase	0%	58	10%	15%
0% 5% 10% 15%	12.9 12.4 11.9 11.4	12.3 11.9 11.4 10.9	11.7 11.3 10.8 10.4	11.1 10.7 10.2 9.8

It was observed that an increase in costs is less sensitive, though slightly as it is, than the decrease in benefit. (Refer to Figure J-Ol) Even in case that the cost is increased by 10%, or the benefit is decreased by 10%, EIRR would still fall within a range which economically justifies the implementation of the project.

J.3 FINANCIAL EVALUATION

J.3.1 Capacity-to-Pay

The financial evaluation was made, in the first place, from the viewpoint of farmers' economy by examining their capacity-to-pay. The capacity-to-pay was esamined in the case of i) a typical farm budget of farmers holding 10 ha, and ii) a typical budget of cooperatives with 20 members in 100 ha.

A typical farm budget of farmers holding 10 ha was estimated in Annex F.3.4 (Table F-14), and the net return was calculated for three types of cropping patterns as cited hereunder.

	N	<u>Net Return</u> (Lp			
Sugar cane, as major crop	· ·	15.2			
Cotton, as major crop	н М	20.5			
Paddy as major crop		18.8		•	

The net return is a repayment capacity at a farm level. Since water charge is actually ineffective, the operation and maintenance cost of irrigation facilities is provisionally considered as a charge which farmers have to pay for. From the financial cost of irrigation facilities estimated in Chapter J.1.2 (facilities for 16,000 ha), the operation and maintenance cost was estimated at around Lp. 105/ha. The charge of Lp. 1,050 per 10 ha would account for about 5-7% of the net return or capacity-to-pay of such a typical small farmer.

The capacity-to-pay of a typical cooperative with 20 members in 100 ha was evaluated on the basis of typical farm budget estimated in Annex F.3.4 (Table F-15). From the net return of cooperative, living expenses of member farmers were deducted at the rate of Lp. 4,550/member as in the case of typical small farmers. Consequently, the capacity-topay of cooperative would amount to Lp. 88,000 - Lp. 107,000. The charge for operation and maintenance (Lp. 10,500 per 100 ha) would represent about 10 - 12% of the capacity-to-pay.

From the above review, it was clear that the farmers' income will be greatly improved under "with" project condition, and even a small farmer will have enough capacity-to-pay water charge or operation and maintenance cost of irrigation facilities.

J.3.2 Financial Internal Rate of Return

The financial internal rate of return (FIRR) was additionally calculated to evaluate repayment capacity of capital investment. The project evaluation by FIRR was made respectively for irrigation plan and power generation plan.

1) FIRR of Irrigation Plan:

FIRR of irrigation plan was calculated on the following conditions:

- a) Benefit is estimated on the basis of revenue from marketable products in financial terms.
- b) Capital investment required for construction of dam (exclusive of power facilities) and irrigation facilities are considered as financial cost.
- c) Price escalation is applied to both capital investment (5% in foreign currency and 6% in local currency) and benefit (5%) during the construction period.
- d) Annual operation and maintenance cost of dam is presumed to be 0.1% of construction cost. Annual operation and maintenance cost of irrigation facilities is estimated at 2% of construction cost.

On the basis of above conditions, a flow of benefit and cost for irrigation plan was prepared as shown on Table J-32(1) for the full scale development of 20,600 ha on the Choluteca plain, and Table J-32(2) for the first stage development (16,000 ha on the Western plain).

FIRR was calculated at 13.1% for the full scale development and at 11.7% for the first stage development. This leads to the implication that repayment of investment in dam and irrigation facilities by farmers would be relatively hard, and it may be desirable that the irrigation project be subsidized by the government.

2) FIRR of Power Generation Plan:

FIRR of power generation plan was calculated on the following conditions:

- Benefit is estimated on the basis of revenue from power sales.
 Power revenue was Lp. 0.16 per kWh in 1983, and would increase at the annual rate of 5%.
- b) Power loss is estimated at 4.8% of generated energy.
- c) Since water release for irrigation is utilized for power generation and dam height is decided regardless of power generation, cost of dam is not allocated to power generation in this financial study.
- d) Capital cost is estimated for power station and related facilities. Price escalation is presumed to be 5% in foreign currency and 6% in local currency during the construction period.
- e) Operation and maintenance cost of power facilities is estimated at around 0.5% of construction cost. O&M cost of distribution systems (Lp. 0.0067/kWh) will be additionally counted.

On the basis of above conditions, a flow of cost and revenue for power generation plan was prepared as shown on Table J-33. FIRR of power generation plan was thus calculated at 34.0%. This fact indicates that the installation of a power plant at the San Fernando dam is financially viable and highly profitable.

A combined flow of benefit and cost for power generation and irrigation of 16,000 ha is presented on Table J-34. FIRR of the combined plan was calculated at 13.5%.

TABLES

Mondala	3mm	3-10nm	10-30	mn 30mm	Sunda	y & Enba	nkmont W	ordeah 1	
Month		- 11 - 1 		Over	Holid	lay Commo	n Filter	Rock	Concret
Jan.	8.4	1.2	0.2	0	6	24	24	25	24
Feb.	3.5	1.0	0.3	0	4	22	23	24	23
Mar.	2.2	0.4	0.3	0.1	7	23	23	23	23
Apr.	3.3	1.3	0.5	0.4	6	21	22	23	22
Мау	4.1	3.0	2.4	1.3	5	15	19	22	21
Jun.	7.3	5.4	4.7	1.4	4	8	14	20	17
Jul.	12.1	5.4	2.2	0.3	5	16	21	23	21
Aug.	12.4	4.0	2.4	0.3	4	17	21	24	22
Sep.	9.0	4.8	3.3	0,9	5	13	16	21	18
Oct.	6.7	5.5	3.9	0.7	7	9	14	18	16
Nov.	9.2	2.1	1.7	0.5	4	19	21	22	22
Dec.	10.8	1.5	1.2	0	6	21	22	24	23
•						208	240	269	252

Table J-01 ESTIMATE OF WORKABLE DAYS

The calculation formula of workable days:

1. Core: Monthly day - $(3-10mm) \times 1/2 - (10-30mm \text{ over}) \times 2 - \text{Holidays}$

2. Filter: Monthly day - (10-30mm over) x 2 - Holidays

3. Rock: Monthly day - (10-30mm over) x 1 - Holidays

4. Concrete: Monthly day - (3-10mm) x 1/2 - (10-30mm over) x 1 - Holidays

		-	
	14 M 10 M	(1) (1) (2)	
and the second		· · · · ·	
PTT 7 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	00	* - mair m	anam
Table J	~		CISP
TOOTO O	. VM .		

Description		Unit	Unit Price	(LPS)
Foreman		Man-day	60	
Operator (A)		H.	32	
Operator (B)		11	25	
Mechanic	•	H	32	
Electrician	· · · · · ·	11	40	
Driver		11	18	
Welder	· .	n - 1	37	
Blasting Worker		11	30	
Reinforcement Stee	el Worker	11	30	
Concrete Worker			18	
Carpenter		n	18	
Plasterer		B	18	
Mason		n	18	
Connon Labour	:	B	6	

ν.

Description	Unit	Unit Price (LP.)
Dynamite	kq	12.8
Detonator	NOS	1.5
A.E. Agent	kg	3.0
Release Agent	1	2.8
Reinforcement Bar	ton	800
I-Shaped Steel	ton	900
Square Pipe 60x60x2.3	kg	1.0
Plywood	m ²	9.8
Water Stop W330x9	m	20
Joint Filler	m ²	30
Metal Form	m ²	28,2
Light Steel Channel	kg	0.96
Steel Angle	kg	0.9
Anchor Bolt (Set)	(set)	132
Timber, Plant	m ³	220
Cross Bit Dia 65mm	NOS	220
Rod L-Im	11	120
Rod Sleeve	u .	96
Rod Shank	11	200
Cross Bit Dia 44mm	If	66
Diamond Bit Dia 44mm	Carat	1,160
Diamond Reamer 66mm	Carat	1,180
Boring Rod (2m)	NOS	106
W-FO	kg	3.7
Gasoline	1	0.95
Diesel Oil	1	0.61
Lubricant	1	3.06
Grease	kg	3.9

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(Dam, Power and Irrigati	on 23,960 h	ia)	
		(Unit:	Lp.103)
Description	Foreign Currency Component	Local Currency Component	Total
1. SAN FERNANDO DAM AND POWER STATION			
1.1 Access road and preparatory works	5,368	3,191	8,559
1.2 River diversion works	3,720	1,170	4,890
1.3 Dam and spillway	50,369	18,241	68,610
1.4 Intake, penstock and outlet	700	58	758
1.5 Powerhouse and tail race	2,372	1,133	3,505
1.6 Generating equipment	9,272	736	10,008
1.7 Transmission line and sub-station	1,130	850	1,980
1.8 Highway relocation	5,120	2,080	7,200
Sub-total	78,051	27,459	105,510
2. CHOLUTECA PLAIN IRRIGATION SYSTEM			la an Alfred
2.1 Preparatory works	1,937	1,467	3,404
2.2 Intake weir	13,974	3,716	17,690
2.3 Main canal	15,486	3,925	19,411
2.4 Branch canal	27,565	7,560	35,125
2.5 Secondary canal	2,714	1,052	3,766
2.6 Drainage canal	5,154	1,371	6,525
2.7 Farm road	7,570	2,206	9,776
2.8 On-farm construction	4,297	1,605	5,902
2.9 Clearing and reclamation	10,175	3,416	13,591
Sub-total	88,872	26,318	115,190
3. IRRIGATION IN MIDDLE REACH	4,900	1,062	5,962
4. LAND COMPENSATION		8,750	8,750
5. ENGINEERING AND ADMINISTRATION	17,640	8,857	26,497
6. CONTINGENCIES			
6.1 Physical contingency	18,948	7,246	26,194
6.2 Price contingency	61,077	27,657	88,734
Sub-total	80,025	34,903	114,928
Total	269,488	107,349	376,837

Table J-04(1) SUMMARY OF COST ESTIMATE

Table J-04(2) SUMMARY OF COST ESTIMATE

(Dam, Power and Irrigation 20,600 ha)

		·	(Unit:	Lp.103)
	Description	Foreign Currency Component		Total
1.	SAN FERNANDO DAM AND POWER STATION	· .		
	1.1 Access road and preparatory works	5,368	3,191	8,559
	1.2 River diversion works	3,720	1,170	4,890
	1.3 Dam and spillway	50,369	18,241	68,610
	1.4 Intake, penstock and outlet	700	58	758
•	1.5 Powerhouse and tail race	2,372	1,133	3,505
	1.6 Generating equipment	9,272	736	10,008
	1.7 Transmission line and sub-station	1,130	850	1,980
	1.8 Highway relocation	5,120	2,080	7,200
	Sub-total	78,051	27,459	105,510
2.	CHOLUTECA PLAIN IRRIGATION SYSTEM			
• :	2.1 Preparatory works	1,937	1,467	3,404
·	2.2 Intake weir	13,974	3,716	17,690
	2.3 Main canal	15,486	3,925	19,411
	2.4 Branch canal	27,565	7,560	35,125
	2.5 Secondary canal	2,714	1,052	3,766
	2.6 Drainage canal	5,154	1,371	6,525
	2.7 Farm road	7,570	2,206	9,776
	2.8 On-farm construction	4,297	1,605	5,902
	2.9 Clearing and reclamation	10,175	3,416	13,591
	Sub-total.	88,872	26,318	115,190
3.	LAND COMPENSATION	Web	8,750	8,750
4.	ENGINEERING AND ADMINISTRATION	17,150	8,750	25,900
5.	CONTINGENCIES			
	5.1 Physical contingency	18,409	7,128	25,537
	5.2 Price contingency	61,077	27,657	88,734
	Sub-total	79,486	34,785	114,271
	Total	263,559	106,062	369,621

		(Unit:	Lp.103)
Description	Foreign Currency Component	Local Currency Component	Total
, SAN FERNANDO DAM AND POWER STATION			an taona an di Galaistan Antonio Galaistan Antonio
1.1 Access road and preparatory works	5,368	3,191	8,559
1.2 River diversion works	3,720	1,170	4,890
1.3 Dam and spillway	50,369	18,241	68,610
1.4 Intake, penstock and outlet	700	58	758
1.5 Powerhouse and tail race	2,372	1,133	3,505
1.6 Generating equipment	9,272	736	10,008
1.7 Transmission line and sub-station	1,130	850	1,980
1.8 Highway relocation	5,120	2,080	7,200
Sub-total	78,051	27,459	105,510
CHOLUTECA PLAIN IRRIGATION SYSTEM	anda Angalan sa Sangaran sa		in a second
2.1 Preparatory works	1,495	1,248	2,74
2.2 Intake weir	6,987	1,858	8,84
2.3 Main canal	12,941	3,261	16,20
2.4 Branch canal	18,348	5,009	23,35
2.5 Secondary canal	2,714	1,052	3,76
2.6 Drainage canal	4,559	1,185	5,74
2.7 Farm road	5,328	1,563	6,89
2.8 On-farm construction	3,350	1,248	4,59
2.9 Clearing and reclamation	8,259	2,873	11,13
Sub-total	63,981	19,297	83,27
LAND COMPENSATION		4,690	4,690
ENGINEERING AND ADMINISTRATION	15,600	7,600	23,20
CONTINGENCIES			n sa sa sa s Tangan sa
5.1 Physical contingency	15,764	5,907	21,67
5.2 Price contingency	44,774	19,680	64,45
Sub-total	60,538	25,587	86,12
Total	218,170	84,633	302,80

Table J-04(3) <u>SUMMARY OF COST ESTIMATE</u> (Dam, Power and Irrigation 16,000 ha)

Table J-04(4) SUMMARY OF COST ESTIMATE

(Dam, Power and Irrigation 12,400 ha)

	· 	(Unit:	Lp.103)
Description	Foreign Currency Component	Local Currency Component	Total
1. SAN FERNANDO DAM AND POWER STATIO	J.		
1.1 Access road and preparatory w	orks 5,368	3,191	8,559
1.2 River diversion works	3,720	1,170	4,890
1.3 Dam and spillway	50,369	18,241	68,610
1.4 Intake, penstock and outlet	700	58	758
1.5 Powerhouse and tail race	2,372	1,133	3,505
1.6 Generating equipment	9,272	736	10,008
1.7 Transmission line and sub-sta	tion 1,130	850	1,980
1.8 Highway relocation	_5,120	2,080	7,200
Sub-total	78,051	27,459	105,510
2. CHOLUTECA PLAIN IRRIGATION SYSTEM			
2.1 Preparatory works	1,495	1,248	2,743
2.2 Intake weir	6,988	1,857	8,845
2.3 Main canal	12,943	3,262	16,205
2.4 Branch canal	15,968	4,373	20,341
2.5 Secondary canal	1,841	674	2,515
2.6 Drainage canal	3,432	893	4,325
2.7 Farm road	3,495	1,017	4,512
2.8 On-farm construction	2,345	874	3,219
2.9 Clearing and reclamation	2,888	1,004	3,892
Sub-total	51,395	15,202	66,597
3. LAND COMPENSATION		4,000	4,000
4. ENGINEERING AND ADMINISTRATION	14,140	6,550	20,690
5. CONTINGENCIES			
5.1 Physical contingency	14,360	5,322	19,682
5.2 Price contingency	39,254	17,076	56,330
Sub-total	53,614	22,398	76,012
Total	197,200	75,609	272,809
and the second			

Table J-05 CONSTRUCTION COST ESTIMATE SAN FERNANDO DAM AND POWER STATION

u <u>u u u u u u u u u u u u u u u u u u </u>			Foreign	Currency		Jurrency	a second s	otal
Itens	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amount
			(Lp.)	(Lp.103)	(Lp.)	(Lp.103)	(Lp.)	(Ip.103)
1. Access Road & Preparatory Works	•							
		10.0	1 40 000	0 00	100.000	2 000	240 000	4 202
New access road	km	18.0	140,000	1	100,000		240,000	4,320
Preparatory works	L.S.	•		2,848	n AN Ang Standar	1,391		4,239
Sub-total				5,368		3,191		8,559
2. River Diversion Works								
Excavation in open	m3	1,000	28	28	10	10	38	38
Excavation in tunnel	m3	15,000	104	1,560	86	540	140	2,100
Cofferdam, impervious fill	m ³	1,450	12,2	18	3.8	5	16	23
Cofferdam, rockfill	m ³	3,380	16.4	55	4.6	16	21	71
Concrete, potal str.	m3	300	96	29	38	11	134	40
Concrete, tunnel linning	m3	4,600	252	1,159	58	267	310	1,426
Concrete, tunnel plug	m3	1,300	116	151	58	75	174	226
Reinforcement steel	ton	430	1,140	490	260	112	1,400	602
Tunnel steel support	ton	75	1,580	119	-		1,580	119
Drilling and grouting	L.S.	•		69	· · ·	56	a 	125
Care of river	L.S.	•		42	a de la composición de la comp	78		120
Sub-total				3,720		1,170	a a ser e estas Re	4,890
3. Dam and Spillway					. •			
Excavation, weatherd rock	m3	36,000	14.2	511	2.8	101	17	61.2
Excavation, rock	m ³	157,960	20.4	3,215	6.6	1,040	27	4,255
Concrete, dam	m3	472,000	84	39,648	30	14,160	114	53,808
Concrete, pier	m3	2,180	100	218	38	83	138	301
Concrete, retaining wall	ξm	1,980	96	190	36	71	132	261
Concrete, stilling basin	m ³	15,540	96	1,492	36	559	132	2,051
Reinforcement steel	ton	636	1,140	725	260	165	1,400	890
Spillway bridges	L.S.			140		55	14. 1	195
Spillway gate	ton	144	12,500	1,800	700	101	13,200	1,901
Anchor bar	ton	30	2,000	60	400	12	2,400	72
Foundation treatment	L.S.		÷ .	2,370		1,894		4,264
Sub-total				50,369	· · · ·	18,241		68,610

(to be continued)

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Table J-05

				Currency	To Tamata a description of the party of	urrency		otal
	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Anount	Unit Cost	Amount
			(Lp.)	(Lp.103)	(Lp.)	(Lp.103)	(Lp.)	(Lp.103)
1. Intake, Penstock & Outlet								
Intake structure	m ³	50	100	5	38	2	138	7
Reinforcement steel	ton	5	1,140	6	260	1	1,400	7
Intake gate, trash rack	ton	40	5,600	224	400	16	6,000	240
Penstock	ton	90	3,300	297	300	27	3,600	324
Outlet valve	L.S.			168		12		180
Sub-total				700		58		758
5. Powerhouse & Tailrace								
Concrete, substructure	m3	3,200	100	320	38	121	138	441
Concrete, superstructure	£n1	5,300	114	604	60	318	174	922
Anchor block	Em	150	100	15	38	6	138	21
Reinforcement steel	ton	415	1,140	473	260	108	1,400	581
Architectural works	L.S.			960		580		1,540
Sub-total				2,372		1,133		3,505
6. Generating Equipment			,					
Turbine and governor	L.S.			3,794		258		2,026
Generator	L.S.			2,802		122		1,462
Overhead crane	L.S.			1,304		76		690
Transformer	L.S.			1,372		280		826
Sub-total				9,272		736		10,008
7. Transmission Line and Sub-station							-	
69 kV line and line post	km	25.0	45,200	1,130	34,000	850	79 , 200	1,980
Sub-total				1,130		850		1,980
8. Highway Relocation								
New highway	km	8.0	640,000	5,120	260,000	2,080	900,000	7,200
Sub-total				5,120		2,080		7,200
Total				78,051		27,459		105,510

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Table J-06(1)

CONSTRUCTION COST ESTIMATE IRRIGATION SYSTEM FOR WESTERN PLAIN: 16,000 HA

			Foreign Currency			al	Total	
Item	Unit	Q'ty	Unit	Amount	Unit Cost	amount	Unit Cost	Amoun
. Communication System and (hartore				nga sa katalan Anga sa katalan			
Communication System and (Marcers			e e e e e e e e e e e e e e e e e e e				
Office & quarters	L.S.			152	- -	593		74
Workshop & others	L.S.		.	121	- ² - - - 5.	145	-	261
Laboratory	L.S.		·	82	· • •••	235	-	31
Temporary transmission	L.S.		-	261	i a	94	-	35
Water supply	L.S.		' 	344		74	-	418
Telecommunications	L.S.	1 .	-	212	-	35	-	24
Health Services	L.S.	· .	·	323		72	.	39:
Sub-total			· .	1,495		1,248		2,74
	1 . 				• •	•		
Headwork (El Papalon Intak	æ Weir)							
Earthworks					н 1911 - 1911 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 - 1914 -			
Excavation, river bed	m ³	29,500	. 7	207	1.4	41	8.4	248
Backfill	m ³	6,600	8.4	55	2.6	17	11	72
Concrete works	1				÷			
Mas concrete (Type-B)	m ³	10,200	100	1,020	34	347	134	1,36
Structural (Type-A)	_m3	2,400	102	245	40	96	142	341
Foundation (Type-D)	m ³	2,700	102	275	38	103	140	378
Reinforcement bar	ton	240	1,130	271	270	65	1,400	33
Concrete form	m2	17,300	9	156	11.2	194	20.2	350
Rock riprap	m3	300	68	20	20	6	88	20
Vertical concrete block	m ³	1,100	170	187	70	77	240	264
Operation deck	m	10	406	4	174	2	580	(
Sluice gate	ton	32	7,800	250	400	13	8,200	26
Trash rack	ton	41	5,200	213	400	16	5,600	22
Steel sheet piles	ton	300	1,714	51.4	6	2	1,720	51
Stop log	m3	30	0	0	120	4	120	4
Dike enbankment	m3	344,000	8	2,752	2.4	826	10.4	3,57
Temporary. works		ana Ang ang ang ang ang ang ang ang ang ang a				e de la composition d La composition de la c		
Coffer dam	m3	9,700	1.0	97	2.8	27	12.8	124
Sheet pile	ton	380	1,714	651	6	2	1,720	65.
Access road	m	1,000	70	70	20	20	90	90
Sub-total	· · ·			6,987		1,858		8,84

(to be continued)

Item Main Canal Canal Earthworks Excavation	Unit	Q'ty	Curr Unit Cost	Amount	Curr Unit Cost	Amount	Unit	
Canal. Earthworks							Cost	Amount
Earthworks								
 International Activity of the second s						•		
Excavation								
	m3	279,400	3	838	0.6	168	3.6	1,000
Embankment	m ³	569,200	5.4	3,074	1.6	911	7	3,98
Stripping	m ³	113,200	0.4	45	0.2	23	0,.6	68
Sod facing	m2	169,200	0,2	34	0.4	68	0.6	10
Concrete lining	m ³	32,400	222	7,193	48	1,555	270	8,74
Related structures								
Earthworks								
Excavation	m3	8,700	7	61	1.6	14	8.6	7
Backfill	т3	2,300	9.2	21	2.8	6	12	2
Concrete works								
, Structural (Type-D)	۶ _m	2,700	100	270	40	108	140	37
Foundation (Type-A)	m3	600	100	60	36	22	136	8
Reinforcement bar	ton	195	1,140	222	600	117	1,740	33
Concrete form	m ²	9,700	9	87	11.2	109	20.2	19
Stop log	m ³	20	0	0	120	2	120	
Precast concrete pipe	m	300	60	18	20	6	80	2
Gates	ton	97	7,700	747	380	37	8,080	78
Rock riprap	m ³	40	66	3	20	1	86	
Bifurcation structure								
Earthworks		4						
Excavation	m ³	3,000	7	21	1.4	4	8.4	2
Stripping	m3	350	0.6	0	0.2	0	0.8	
Embankment	m3	1,700	8.4	14	2.6	4	11	·]
Backfill	m3	760	9.4	7	2.8	- 2	12.2	
Concrete works								
Structural (Type-A)	r ³	720	102	73	38	27	140	10
Foundation (Type-D)	rm ³	150	1.02	15	38	6	140	-
Reinforcement bar	ton	60	1,140	68	600			
Concrete form	m²	2,900	9	26	11.2		20.2	
Trashrack	ton	8.5	5,200	44	400		•	
Stop log	m3	2	0	0	130	0	1.30	16,20

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Table	J-06	(1)	
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(Unit: Lp.103)

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(to be continued)

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Table J-06(1)

Table J-06(1)	•		· · ·				Unit:	Lp.10 ³)
Item	t too i d	h 01+	Fore	u.gn ency	Loc			tal
Treati	Unit	t Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amount
5. Secondary Canal					h- (*****	<u> </u>	
Canals								
Earthworks	-							
Excavation	m3		4.4	0	1.8	0	6.2	0
Embankment	m3	212,600	7.8	1,658	2.4	510	10.2	2,168
Stripping	m3	61,900	0.6	37	0.2	12	0.8	49
Sod facing	m2	167,500	0.2	34	0.4	67	0.6	101
Related structures								
Earthworks								
Excavation	m3	6,060	7	52	1.6	10	8.6	52
Backfill	m3	3,900	8,2	32	2.6	10	10.8	42
Concrete works								
Structural (Type-A)	m3	2,115	108	228	42	89	1.50	317
Foundation (Type-D)	m3	870	98	85	42	37	140	122
Reinforcement bar	ton	130	1,140	148	600	78	1,740	226
Concrete form	m2	5,810	8.6	50	24	139	32.6	189
P.C. pipe	L.S.	•	-	5	-	52	_	57
Concrete block lining	m3	66	170	11	72	5	242	16
Stop log	m3	8	0	0	120	1	120	1
Gate & metalworks	L.S.			384	-	42		426
Sub-total				2,714		1,052		3,766
6. Drainage Canal								
Earthworks								
Excavation	m3 -	1,295,000	3	3,885	0.6	777	3.6	4,662
Related structures								
Earthworks								
Excavation	m3	5,400	7.6	41	2	11	9.6	52
Backfill	т3	2,000	8.4	17	2.6	5	11	22
Concrete works								
Structural (Type-A)	m3	2,700	11.0	297	40	108	150	405
Foundation (Type-D)	m3	120	98	12	42	5	140	17
Reinforcement bar	ton	200	1,140	228	600	120	1,740	0
Concrete form	m ²	6,100	8.6	52	24	146	32.6	198
P.C. pipe (Total length 40m)	L.S.	•	-	1	-	5	-	б
Rock riprap	m ³	400	66	26	20	8	86	34
Sub-total				4,559		1,185		5,744

(to be continued)

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Table J-	-06(1)
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(Unit: Lp.10³)

			Fore	n ency	Loc Curi	al ency	Te	tal
Item	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amount
2 D1								a second
7. Farm Road		·						
Main farm roads		·	*	•			la se se	
Stripping	m3	36,800	0.6	22	0.2	7	0.8	29
Enbankment	m3	123,050	8	984	2.4	295	10.4	1,279
Gravel metalling	m3	20,700	38	787	9	186	.47	973
Secondary farm roads		· . · ·		· · ·	1.4.8	•		
Stripping	m3	74,430	0.6	45	0.2	15	0.8	60
Enbankment	6 m	186,000	8	1,488	2.4	446	10.4	1,934
Tertiary road		· · · ·	1.5			an the second		e station A
Stripping	m ³	103,360	0.6	62	0.2	21	0.8	83
Embankment	m3	200,640	8	1,605	2.4	482	10.4	2,087
El Palenque causeway					ana di Sana Li sana di Sana		ta an an	
Earthworks	÷.,							
Excavation	Em .	1,100	7.6	8	1.8	2	9.4	10
Backfill	_m 3	600	8.4	- 5	2.6	2	11	
Concrete works	-	÷.,		÷ .				
Structural (Type-A)	rm ³	650	108	70	42	27	150	97
Foundation (Type-D)	щЗ .	200	. 98	20	42	8	1.40	28
Reinforcement bar	ton	30	1,180	35	280	. 8	1,460	43
Concrete form	m2	500	9	5	11.2	6	20.2	11
Temporary works		•					•. •	a e je t
Coffering	mЗ	24,000	8	192	2.4	58	10.4	250
Sub-total				5,328		1,563	. <u>.</u>	6,891

(to be continued)

Table	J-06(1)

and the second	•••••	0.1	Fore		LOC	al ency	To	tal
Item	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amount
8. On-Farm Development		e e service Service de service	х	e e e e e e e e e e e e e e e e e e e				
Canals				•				
Tertiary canal	· · ·	· · ·						
Excavation	m3		4.4	0	1.8	0	6.2	0
Embankment	m3	277,900	7.8	2,168	2.4	- 667	10.2	2,835
Collector drain				-7200				.,
Excavation	m ³	182,400	4.4	803	1.8	328	6.2	1,131
Related structures			•					
Earthworks	111							
Excavation	m ³	1,226	7.6	9	2	2	9.6	11
Backfill	m3	477	8.4	4	2.6	1	11	5
Concrete works		•					5.00 L	:
Structural (Type-A)	m3	1,518	108	164	42	64	150	228
Foundation (Type-D)	m ³	243	98	24	42	10	140	34
Reinforcement bar	ton	45	1,140	51.	260	12	1,400	63
Concrete form	m2	14,134	9	127	11.2	1.58	20.2	285
Stop log	m3	46	0	0	120	6	120	6
Sub-total	:			3,350		1,248		4,598
9. Clearing and Reclamation		1. AL						
Land reclamation	ha	11,970	690	8,259	240	2,873	930	<u>'1</u> _11,132
Sub-total	· · .			8,259		2,873		11,132
Total		•		63,981		19,297		87,278

(Pasture land (6,960 ha) x Lp. 900/ha + Existing farm land (4,030 ha) x Lp. 650/ha + Paddy field (4,050 ha) x Lp. 400/ha) \div 11,970 ha Note: /1:

Table J-06(2)

the second second

CONSTRUCTION COST ESTIMATE IRRIGATION SYSTEM FOR EASTERN PLAIN - A

			Fore	eign Tency	Loc	al ency	IC	otal
Item	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Anoun
. Communication System and Q	uarters							
Office & others	L.S.	:	· ·	121		34		159
Workshop & others	L.S.			35	· 	42		7
Laboratory	L.S.		***	24		68	_	9:
Temporary transmission	L.S.			76	. ·	27	-	10
Water supply	L.S.		· -	100	·	22		12
Telecommnications	L.S.		. 🛥	35	-	10		4
Health services	L.S.	· .	-	51		16	· . .	6
Sub-total			18	442		219	·	66
		1. 1. 1. 1. 1. 1.		· · · ·	н 1			
, Headwork (Las Bases Intake	weir)						an tan Arang arang	an a
Earthworks								
Excavation, river bed	mЗ	29,500	7	207	1.4	41	8.4	24
Backfill	_m3	6,600	8.4	55	2.6	17	11	7
Concrete works	•			•	;	÷ .		
Mas concrete (Type-B)	m 3	10,200	100	1,020	34	347	134	1,36
Structural (Type-A)	m3	2,400	102	245	40	96	142	34
Foundation (Type-D)	m3	2,700	102	275	38	103	140	37
Reinforcement bar	ton	240	1,130	271	270	65	1,400	33
Concrete form	m2	17,300	9	156	11.2	194	20.2	35
Rock riprap	m3	300	68	20	20	6	88	2
Vertical concrete block	m3	1,100	170	187	70	77	240	26
Operation deck	m	10	406	4	174	2	580	÷.
Sluice gate	ton	32	7,800	250	400	13	8,200	26
Trash rack	ton	41	5,200	213	400	16	5,600	22
Steel sheet piles	ton	300	1,714	514	6	2	1,720	51
Stop log	m3	30	0	0	120	4	120	
Dike embankment	m ³	344,000	- 8	2,752	2.4	826	10.4	3,57
Temporary works			· .				н 1 с. н.	· · ·
Coffer dam	m3	9,700	10	97	2.8	27	12.8	12
Sheet pile	ton	380	1,714	651	6	2	1,720	65
Access road	m	1,000	70	70	20	20	90	9
Sub-total	• •			6,987		1,858	et a ser e	8,84

(to be continued)

Table J-06(2)							Unit:	Lp.103)
		e disterie Vielanderen	Fore	eign rency	Loc	al: Tency	Te	otal
· Iten	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amount
3. Main Canal								
Canal Earthworks				•				· · ·
Excavation	m ³	20,700	3	62	0.6	12	3.6	74
Enbankment	£m	125,450	5.4	677	1.6	201	. 7	878
Stripping	m3	26,050	0.4	10	0.2	5	0.6	15
Sod facing	m2	61,200	0.2	12	0.4	24	0.6	36
Concrete lining	m ³	6,450	222	1,432	48	310	270	1,742
Related structures		· · ·	-					
Earthworks								
Excavation	m ³	2,306	7	16	1.6	4	8.6	20
Backfill	m3	610	9.2	6	2.8	2	12	8
Concrete works		· .	•					
Structural (Type-D)	m3	716	100	72	40	29	140	101
Foundation (Type-A)	m3	1.59	1.00	1.6	36	6	136	22
Reinforcement bar	ton	52	1,140	59	600	31	1,740	90
Concrete form	m2	2,570	. 9	23	11.2	29	20.2	52
Stop log	۶m	6	. 0	0	120	l	120	1
Precast concrete pipe	m	80	60	5	20	2	80	. 7
Gates	ton	20	7,700	154	380	8	8,080	162
Rock riprap	m3	10	66	1	20	0	86	1
Sub-total				2,545		664		3,209

able J-06(2)

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(to be continued)

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Item	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amouni
Branch Canal								
Canal	· .	· · ·	, ¹ · · ·					
Earthworks							1 . 	
Excavation	m3	0	3	0	0.6	0	3.6	C
Enbankment	m3	506,700	7.8	3,952	2.4	1,216	10.2	5,16
Stripping	m ³	103,400	0.6	62	0.2	21	0.8	8
Sod facing	m2	242,200	0.2	48	0.4	97	0.6	14
Concrete lining	m3	19,500	232	4,524	48	936	280	5,46
Related structures		· · ·						
Earthworks			•				n de la composition de la comp	
Excavation	m3	3,154	7	22	1.6	5	8,6	2
Backfill	m3	1,750	8.2	14	2.6	5	10.8	1
Concrete works							- Internet	·
Structural (Type-A)	m3	1,425	110	157	40	57	150	21
Foundation (Type-D)	m3	- 399	100	40	40	16	140	5
Reinforcement bar	ton	74	1,140	84	600	44	1,740	12
Concrete form	m2	3,500	8.6	30	24	84	32.6	11
P.C. pipe	L.S.		-	- 3	-	39	<del>.</del>	4
Concrete block	m	8	170	1	70	1	240	i i se
Stop log	m3	6	0	0	130	1	130	
Rock riprap	εm	27	60	2	20	1	80	
Cate & metalworks	L.S.		-	278	· -	28	н 2015 — <b>н</b>	30
Sub-total				9,217	: · · ·	2,551		11,76

(to be continued)

able J-06(2)	ar the age and the second s		Fore	ign	Loc		Unit:	
Iten	Unit	Q'ty	Curr	ency	Photo: The Street Statistics	rency		tal
		,	Unit Cost	Amount	Unit Cost	Amount	Unit Cost	Amount
. Drainage Canal					· .			
Earthworks								
Excavation	m3	150,750	3	452	0.6	90	3.6	542
Related structures		i de la compañía de l Esta de la compañía de						
Earthworks						·		
Excavation	_m3	1,570	7.6	12	2	3	9.6	15
Backfill	m3 _	580	8.4	5	2.6	2	11	-
Concrete works								
Structural (Type-A)	mЗ	522	110	57	40	21	150	7
Foundation (Type-D)	m3	23	98	2	42	1	140	
Reinforcement bar	ton	39	1,140	44	600	23	1,740	6
Concrete form	m2	1,770	8.6	15	24	42	32.6	5
P.C. pipe (Total length 40m)	L.S.			0	-	2	. =	•
Rock riprap	m3	120	66	- 8	20	2	86	10
Sub-total				595		186		78
	· ·							
. Farm Road								
Main farm roads				1 - A				
Stripping	m3	22,720	0.6	14	0.2	5	0.8	19
Embankment	m3	75,970	8	608	2.4	182	10.4	79
Gravel metalling	m ³	12,780	38	486	9	11.5	47	60
Secondary farm roads	10	16,700	30		-		•	
Stripping	m ³	27,876	0.6	17	0.2	6	0.8	2
Enbankment	ш <del>о</del> . 1113 -	69,690	8	558	2.4	- 167	10,4	72
	112	0,000						
Tertiary roads	т ³	34,612	0.6	21	0.2	7	0.8	. 2
Stripping Embankment	m3	67,188	8	538	2.4	161	10.4	69
Embankment	TIF-	011200	a	2,242	2.47	101	TO ' 4	2,88

(to be continued)

			Fore	ugn Tency	Loc Curr	A STATE AND A STATE	To	tal
Iten	Unit	Q'ty	Unit Cost	Amount	Unit Cost	Amount	Uhit Cost	Amount
. On-Farm Development	[.] .							
Canals				· · · .			. • ² *	
Tertiary canal	tetre i					· · · ·		
Excavation	m3	0	4.4	0	1.8	0	6.2	0
Enbankment	m3	86,600	7.8	675	2.4	208	10.2	883
Collector drain			· · · · ·		· ·			
Excavation	m3	30,540	4.4	134	1.8	55	6.2	189
Related structures			ha dha an ta	an a				
Earthworks	at the	1. A.		• •	ант. 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 - 1919 -		· · · ·	s. a sig
Excavation	°m3	627	7.6	5	. 2	1	9.6	6
Backfill	m3	268	8.4	2	2.6	1	11	3
Concrete works		i se M	5.					
Structural (Type-A)	m3	532	108	57	42	22	150	79
Foundation (Type-D)	m3	84	98	8	42	. 4	140	12
Reinforcement bar	ton	. 16	1,140	18	260	4	1,400	22
Concrete form	m2	5,359	; 9	48	11.2	60	20.2	108
Stop log	m3	15	0	0	120	2	120	2
Sub-total				947		357		1,304
						e esta		
Clearing and Reclamation				· · ·		1 A.	•	
Land reclamation	ha	3,795	505	1,916	1.43	543	·	2,459
Sub-total		•		1,916	· ·	543	n in the National the	2,459
		· · · · ·					· · · · · ·	
Total				24,891		7,021	· .	31,912

•	
Dam, Fower and irrigation 20. Items	$\frac{20,600 \text{ ha}}{1\text{ st} (1985)} \frac{1}{2\text{ hd}} \frac{(1986)}{(1986)} \frac{3\text{xd}}{3\text{ xd}} \frac{(1987)}{(1987)} \frac{4\text{th}}{4\text{th}} \frac{(1983)}{(1983)} \frac{5\text{th}}{(1983)} \frac{(1983)}{6\text{ th}} \frac{7\text{th}}{(1990)} \frac{(1991)}{7\text{th}} \frac{8\text{th}}{(1992)} \frac{(1992)}{8\text{th}} \frac{8\text{th}}{(1992)} \frac{(1992)}{7\text{th}} \frac{8\text{th}}{1000} \frac{(1992)}{7\text{th}} \frac{8\text{th}}{1000} \frac{(1992)}{7000} \frac{8\text{th}}{10000} \frac{(1992)}{7000000000000000000000000000000000000$
1. Dam Works	199 2,873 11,557 3,786 25,888 10,363 21,029 7,
2. Power Station 3. Irrigation Systems	2,080 320 800 590 7,508 1,430 2,360 430 6,602 2,324 22,302 8,462 14,813 4,213 10,925 3,286 3.
	14,881 5,517 40,659 12,838 48,209 16,006 34,314 10,933 3,555 1,125 3,71
4. Land Acquisition 5. Fnoineering and Administ.	1,970
6. Physical Contingency 7. Price Contingency	174 307 161 1,574 829 4,336 1,630 5,071 1,933 3,602 1,423 442 115 348 220 2,736 1,741 10,303 4,697 15,396 7,186 13,985 6,559 1,896 1,
Grand Total	2,029 3,725 1,991 20,051 10,857 57,998 22,625 71,176 28,445 53,601 22,
Items	.
	65,303 24,689 89,
2. Power Station 3. Irrigation Systems	12,748 2,770 15 3.399 8.733 2.471 88.872 26.318 115
	71 166,923 55,777 22
5. Engineering and Administ.	260 310 190 17,150
6. Physical Contingency 7. Price Contingency	7,128 27,657
	6,798 16,204 5,242 263,559 106,062 369,62
Middle Reach 3,360 ha	(Unit: Ip.103)
Itens	Ist (1985) 2nd (1986) 3rd (1987) 4th (1988) 5th (1989) 6th (1990) 7th (1991) Total Total F.C. L.C. F.C. L.C.
2. Power Station 3 Invigation Systems	1 920 377 1 920 377 4,900 1.062 5.
4. Land Acquisition 5. Envineering and Administ	1 I 1 I 1 I
	106 31 192 38 192 3 

	2 2	80 88 0	200	95		
IP.103)	15.518	83,278 188,788 4 690	23,200	04,44/ 302,795		
(Unit: Total	F.C L.C 65,303 24,689 12.748 2.770	1.1		44, //8 19,069 218, 174 84,621		
<u>91)</u>		015 6 015 14	600 L 162 L	895 4 2,672 21		
7 <del>th</del> (1991		, <u>335 1,</u> ,335 1,	390	,029 2,		
	L.C 1 7,217 430	, 930 3, -	680 1,161	3,331 1,744 18,122 6,029		
6 <del>th</del> (1990)		<u>10,926 3,283 3,335 1,015</u> 34,315 10,930 3,335 1,015 -		985 602		
	L.C F.C 10,363 21,029 1.430 2.360	4,213 10 16,006 34 800		o, opu 13, 26,325 53,		
(FINANCIAL) 5th (1989)	F.C L.C F.C 25,888 10,363 21,029 7.508 1.430 2.360	815 211 16	and the second second			
E	0 8 8	8,462 14,815 12,838 48,211 1.920		4,03/15,		
1 44		303 8, 660 12, - 1		10,303 4,097 12,399 57,999 22,625 71,178		
SEMENT	L.C F.C 2,873 11,557 320 800	2,324 28,303 5,517 40,660 1_970 _				
DISBURSEMENT rd (1987) 4t	.C L 199 2,8 080		î -	/30 1,/41		
, m	й <b>й</b> 101	450 14, 8	નેત			
Table J-07(2) 985) 2nd (1986)		630 4	2,440 1,1 307 1	725 1,9		
Table			.740 2. 174	029 3,		
5		)    ₹     1 ₹   	4,840 1,740 2,440 1,160 484 174 307 161	5,590 2,029 3,725 1,991 20		
1 16,00		•	is t			
Dam, Power and Irrigation 16,000 ha	1. Dam Works 2. Power Station	3. Irrigation Systems Sub-total 4 Tand Acomisition	<ol> <li>Engineering and Adminit</li> <li>Physical Contingency</li> </ol>	. Frice Contingency Grand Total		
et e service de la companya de la co				J - 38		

	Table J-07(3) DISBURSEMENT SCHEDULE (FINANCLAL)		1
Law, rower and initigation is	, tuo la 1st (1985) 2nd (1986) 3rd (1987) 4th (1988) 5th (1989) 6th (1990) 7th F.C. L.C. F.C.	(Unit: (1991) Total L.C F.C L.C	Ip. 1(3) Total
1. Dem Works 2 Power Station	630 450 6,199 2,873 11,557 3,786 25,888 10,363 21,029 7,217 2 080 320 800 540 7 508 1 430 2 350 430		89,992 15 518
3. Irrigation Systems	2,272 27,912 8,192 14,619 4,078 2,338		66,597
Sub-total 4. Land Acquisition	630 450 14,805 5,465 40,269 12,568 48,015 15,871 25,727 8,307 1,650 - 1,650 - 650	129,446 42,661 1 [.] 4,000	172,107
5. Engineering and Administ.	2,400	· ·	20, 690
6. Physical Contingency 7. Price Contingency	484 174 307 161 1,567 797 4,267 1,556 5,022 1,745 2,713 889 266 115 348 220 2,723 1,674 10,138 4,484 15,245 6,488 10,534 4,095	14,360 5,322 39,254 17,076 1	19,682 56,330
Grand Total	5,590 2,029 3,725 1,991 19,955 10,436 57,074 21,598 70,482 25,684 40,374 13,871	197,200 75,609 2	272,809

J - 39

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	2,346		Total 15 510	
8th ( 715	6, 849			710 330 30 31 11 10 12 10 10 10
	3,513		(Unit Total .C L.	<b>ì</b> (À) 🕂 🕂
	6,755		μ. F	12, 12, 12, 13, 13, 13, 13, 13, 13, 13, 13, 13, 13
)) 217 500 550 650 650 650	494			
10 10 10 10 10 10 10 10 10 10	12 216		E.C	
	,193 49		(1990) L.C	
6, 1 2, 10, 10, 10, 10, 10, 10, 10, 10, 10, 10	26		E.C.	2, 360 120 3, 691
12, 4, 14, 5t	60,078		1.1989)	
	21,709 03) a1 992	194 750 950 890 734 510	25th ()	N TENED TO THE MELTER TO THE
8664 97 97	,850 LP.10 Tota 89,9	115,194 205,196 8,750 8,750 24,950 24,950 23,890 83,734 346,510	1988) 1.C	
) -C -C - -C - -C - -C - - - - - - - - -	0,372 56 (Unit: I.C 4,689	314 003 750 826 687 687	900 - ((	800 86 1,150 1,150
(1987) 2, 1, 1,	o tal e	83 51, 50 8, 50 8, 64 6, 23 101,	(1987) L.C	1 (1) <b>1</b> (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)
NH NO OF	17,248 17,248 5,303 65,303	88,880 154,183 16,450 17,064 57,126 57,126	3rd ( F.C	2,080
	1,991 1994) L.C	2,471 2,471 190 2,315 5,242	( <u>1986</u> )	
Znd (1) F.C 630 630 2,440 2,440 348	3,725	8,733 8,733 8,733 310 904 6,257 16,204	2nd (1	
1 1111040	6	399 399 366 366 773 798 798		
lst (1985) F.C L.C 4,840 l,74 484 l7 266 l1	5,590 2,02 9th (1993) F.C L.C	<u>12,227 3,</u> 12,227 3, 440 1,267 2, 7,678 2, 21,612 6,	F.C	1 <b>3 1 1 1 1 1</b>
Shr 2	194 S.	21, 11, 12, 12, 12, 12, 12, 12, 12, 12,		
00 ha		List.		t.
and Irrigation 20,600 r Items Dam Works Power Station Irrigation Systems Sub-total Land Acquisition Engineering and Administ Physical Contingency Price Contingency	tal	Fower station Irrigation Systems Sub-total Land Acquisition Engineering and Administ. Physical Contingency Price Contingency Grand Total	orks	Irrigation Systems Sub-total Sub-total Land Acquisition Engineering and Administ Physical Contingency Price Contingency Grand Total
and Irrigation 20,4 Items Dam Works Power Station Irrigation Systems Sub-total Ind Acquisition Engineering and Admu Physical Contingency Price Contingency	Grand Total Items ks	Lower station Irrigation Systems Sub-total Land Acquisition Engineering and Admi Physical Contingency Price Contingency Grand Total	Generating Works Items Works	Irrigation Systems Irrigation Systems Land Acquisition Engineering and Admi Physical Contingency Price Contingency Grand Total
and Irrigatic Items Dam Works Power Station Irrigation Syu-to Engineering a Physical Conting	Gra Ite Dam Works	Fower station Sy Irrigation Sy Sub-to Land Acquisit Engineering a Physical Cont Price Conting Grand	er Generating Items Dam Works	Rection 1 Acquired to a superior of the second
<b>A</b>	J. Dem	2. FOW 3. Irri 5. Eng 5. Eng 7. Priv	Power ( 1. Dem	
קןן (אמעיביער בייסער) שויז איז איז איז איז איז איז איז איז איז א	1 1 1 1 1 1 1 1 1 1 1 1 1		Berth Ling A	
		J - 40		
			•	

DISBURSEMENT SCHEDULE (FINANCIAL)

Table J-08(1)

lst		: • .• •						 	5	(Unit: Ip.	Tp.103)
	(1985) 2nd (1986) L.C F.C L.C	3rd (1987) F.C L.C	4th F.C	(1988) 5th L.C F.C	(1989) L.C	6th (1990) F.C L.(	7th (	<u> </u>	Total F.C ]		Totàl
1. Dan Works	- 630 450	6,199 2,87	3 11,557	3,786 25,888 10,363 21,029	10,363 2		7,217 -	ש.   	65,303 24,689		89,992
2. Power Station		1	1	1	I	1	1	<b>)</b>	1	1	1
3. Irrigation Systems		6,602	4 28,304 8	2,324 28,304 8,462 14,815 4,213 10,927 3,283 3,336 1,034 63,984 19,316	4,213 1	0,927 3,	283 3,336 ]	L,034 €	3,984 19		83,300
Sub-total	630 450	450 12,801 5,19	7 39,861 12	5,197 39,861 12,248 40,703 14,576 31,956 10,500 3,336 1,034 129,287 44,005	14,576 3.	1,956 10,	500 3, 336 2	L, 034 12	9,287 44		173, 292
4. Land Acquisition -	1	- 1,970	1	1,920	800	1	) 1	J.	4		4,690
5. Engineering and Administ. 4,840	4,840 1,740 2,440 1,160	740 750		2,640 1,470 2,100	980	1,580	650 560		600 14,900 7	7,350 2	22,250
6. Physical Contingency 484	4 174 307 161	1,354	792 4,250 1,564	1,564 4,280	1,636	3,354 1,115	115 390	163	14,419 5	5,605 2	20,024
7. Price Contingency 266	115 348	2,354	3 IO,099 4	1,663 10,099 4,507 12,995 6,081 13,022 5,139 1,744	1 6,081 I	3,022 5,.	139 1,744	906	40,828 18,631		59,459
Grand Total 5,590	2,029 3,725	1,991 17,249 10,372		56,850 21,709 60,078 24,073 49,912	24,073 4	9,912 17,	17,404 6,030 2	2,703 15	6,030 2,703 199,434 80,281	1 ··· · ·	279,715
Power Generating Works							• •		Ð	Unit: Iv	[Lo.103)
Items Items	(1985) 2nd (1986) L.C F.C L.C	3rd (1987) F.C L.C	4th F.C	(1988) 5th L.C F.C	(1989) L.C	6th (1990) F.C L.(	F.C.	(1991) L.C	F.C	1 . 1	Total
1. Dam Works	1	l	3	1	I		1 1	Ĩ	, <b>1</b> ,	: <b>1</b>	1
2. Power Station	1	2,080 320	008 00	590 7,508	L,430	2,360	430 -	. 1	12,748 2	2,770 1	15,518
3. Irrigation Systems			1	1	1	1	1	1	. 1	1	'
Sub-total	1	2,080 320	20 800	590 7,508	l,430	2,360	430 -		12,748 2	2,770 l	15,518
4. Land Acquisition	1 1 5	1	5	ŀ	ł	١	1 1	ı	ł	ţ	I
5. Engineering and Administ.	1	· 120 5	50 60	70 400	100	120	30 -	I	700	250	950
6. Physical Contingency -	1	- 220 3	37 86	C60 791	153	248	46	ł	1,345.	302	l,647
7. Price Contingency	I I J	382	78 204	190 2,401	1 569	963		I	3,950 I	1,049	4,999
Grand Total		2,802	485 1,150	916 11,100	2,252	3,691	- 817	1	18,743 4	4,371 2	23,114

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