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CHOLUTECA RIVER BASIN AGRICULTURAL DEVELOPMENT PROJECT

UPDATING FEASIBILITY STUDY

VOLUME III ANNEXES

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ALTERNATIVE PLANS

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RESERVOIR OPERATION

G. ALTERNATIVE PLANS

G.1 ALTERNATIVE DAMSITES

G.1.1 Selection of Damsite

To store and secure water primarily for irrigation in the Choluteca plain, some alternative dam and reservoir sites were identified by the previous study in 1977-78, on the basis of available topographic maps on the scale of 1/50,000. These damsites are: (Refer to Figure G-01)

Damsite	Drainage Area	Approx. Annual Runoff
	(km2)	(MCM)
Zorillo	1,590	380
San Fernando	1,665	400
Oropoli	4,154	930
Morolica I	6,140	1,200
Morolica II	6,187	1,215

Out of these alternative damsites, the Oropoli site was found to be unfavorable for dam construction because of topographic conditions which were inferior to any other alternative damsites. The Morolica I and Morolica II sites are located some 5 km and 12 km downstream from Morolica town. At the Morolica I damsite, the Choluteca valley becomes narrowest, and is topographically superior to the Morolica II site. However, the drilling survey in 1977-78 revealed that the debris on the right abutment of the Morolica I damsite was extremely deep and outcrop of tuff near the damsite was judged as a moved rock mass. Since the bed rock was not identified at 30 m and 60 m in depth in two drill holes, the Morolica I damsite was concluded to be geotechnically unsuitable for dam construction.

Construction of a rock-fill type dam is technically possible at the Moralica II damsite, though the valley is relatively wide at the site. A solid bed rock composed of andesite is discernible at 10-12 m in depth

from the ground surface. Rock materials are available nearby, but impervious core materials are scarce and insufficient for a large rockfill dam construction. More serious problem for dam construction at the Morolica II site is the resettlement from the submergible area. The dam will inundate Morolica town and some other villages in the Morolica valley. It was estimated that about 700 households, including some 250 households in Morolica town, had to be resettled for dam construction. Due to such a social problem, the Morolica II damsite will not be preferred as a site for dam construction.

The San Fernando damsite and Zorillo damsite are located in the upstream reach of the Choluteca river. At the San Fernando damsite, the Choluteca valley forms a very narrow gorge, with both abutments rising 70-80° to the terraces at around EL. 835 m. The site offers favorable topographic condition for construction of an arch type or gravity type dam. As reviewed in Annex D.2.1, geological and geotechnocal conditions will permit construction of a concrete dam, though careful design is required due to low shearing strength in the foundation. Construction of a fill-type dam at San Fernando will result in higher cost due to availability of rock and impervious core materials and the need for treatment of steep slopes on both abutments. The study indicated that the construction cost of the rock-fill type dam would be higher than the gravity-type dam by about 12%.

The Zorillo damsite is conceived as an alternative to the San Fernando damsite if and when a concrete dam construction is not technically feasible at San Fernando. The Zorillo damsite is located at about 2 km upstream of San Fernando, or 0.5 km downstream from Hernando Lopez bridge. At Zorillo, rock outcrops are rhyolitic welded tuff which are hard to moderately hard, with bedding plane dipping $10-15^{\circ}$ S. Although vertical joints are strongly developed and open cracks are observed, they may be treated by excavation to a reasonable depth and by foundation grouting. The rock excavation will be required for 10-12 m in depth. In view of the strength of rocks and change in their hardness by stratum, it is considered questionable to construct a concrete dam at Zorillo.

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Through a review of possible alternative damsites in the Choluteca valley, the San Fernando damsite was selected as the most promising site for construction of a dam for storage of water primarily for irrigation purpose. The water stored by the San Fernando dam will also be beneficial to other purposes, which will be further studied subsequently.

G.1.2 Storage by San Fernando Dam

Available discharge at the San Fernando damsite is calculated in Annex C.3.2 (Table C-24) on the basis of hydrological record at the Hernando Lopez gauging station located at approximately 2 km upstream of the damsite. The mean monthly discharge at San Fernando ranges from 4.6 MCM (1.7 m³/s) in March-April to 88.0 MCM (34.0 m³/s) in September. The average annual runoff for the past 29 years is 400.5 MCM (12.7 m³/s).

In the Choluteca river basin upstream of the proposed San Fernando dam, SANAA envisages to construct the Guacerique dam and reservoir for water supply to the metropolitan area, as noted in Annex B.3. Detailed design of the Guacerique dam and water supply project has already been prepared, and it is scheduled to be constructed by 1991. In case that the Guacerique dam (catchment area of 189 km²) is constructed and 1.18 m³/s of water is taken on an average, the discharge available at the San Fernando damsite is estimated to decrease to approximately 385 MCM per annum on an average, on the assumption that a return flow from water supply is available at the rate of 50%. Such a decrease is to be counted in studying reservoir operation at the San Fernando damsite. Possibility of further utilization of the Choluteca water for municipal water supply is reviewed in Chapter G.5.

A storage capacity curve of the San Fernando damsite was drawn out on the basis of 1/5,000 scaled topographic maps prepared in 1967, as shown on Figure G-02. A gross storage capacity was estimated at around 280 MCM at the high water level at EL. 818 m (about 91 m in dam height), and 450 MCM at EL. 826.5 m (100 m in dam height). Since the sediment volume was estimated at 800 m³/km²/year or 1.33 million m³/year, as noted in Annex C, the effective storage was estimated at around 210 MCM at the high water level at EL. 818 m and 380 MCM at EL. 826.5 m.

G.2 PRIORITY USE IN IRRIGATION

G.2.1 Irrigable Area

Water stored in the San Fernando dam and reservoir would be utilized for irrigation as the primary objective. As reviewed in Annex D.4, Annex E and Annex F, the land irrigable by water stored by the San Fernando dam covers the Choluteca plain in a sizable scale, as well as in the middle reach valleys of the Choluteca river. The net irrigable area identified through the study is summarized as follows:

Western plain		16,000 ha
Eastern plain-A		4,600 ha
Eastern plain-B		5,200 ha
Middle reach valley	(existing)	680 ha
	(potential)	1,640 ha
San Juan de Flores	(existing & extension)	2,680 ha
Total		30,500 ha

Water requirement for irrigation in respective area was estimated in Annex H.1 in accordance with the cropping pattern proposed in Annex F. The monthly irrigation water requirement is reproduced and attached in Table G-01 for easier reference. The estimated irrigation water requirement would be first met by discharge available from the remaining river basin downstream from the San Fernando dam to the intake site, and supplementarily fed by water stored in the San Fernando reservoir, particularly during the dry season. The monthly irrigation water requirement in each area will form a basis for reservoir operation at San Fernando.

G.2.2 Priority Area

Alternative study on reservoir operation at the San Fernando dam will be made on various scales of irrigation area, in view of a possibility of limitation in the storage capacity at San Fernando. After reviewing

the socio-economic condition in respective areas, as well as the net incremental return estimated for each area in Annex F, it is proposed that priority of water use for irrigation will be accorded in the following order:

Priority	Area	
1 a	Western plain area	16,000 ha
1 b	San Juan de Flores area	2,680 ha
1 c	Middle reach valley (existing)	680 ha
2 a	Eastern plain - A	4,600 ha
2 b	Eastern plain-B	5,200 ha
3	Middle reach valley (potential)	1,640 ha

G.3 HYDROPOWER PLAN

G.3.1 Power Generation at San Fernando

Present situation of electric power, as well as power demand forecast by ENEE, has been reviewed in Annex B.2. With the completion of El Cajon hydroelectric project in 1985, the hydropower capacity will be increased to 423 MW. After completion of El Cajon project, some thermal plants are scheduled to be retired, and their capacity will be decreased to 87 MW in 1991 and 30 MW in 1994. Since the peak demand was estimated at 344 MW and required power capacity (inclusive of reserve capacity) at around 430 MW in 1991, the demand would be covered by the existing and committed plants. By the year 2000, however, the required power capacity would reach around 840 MW, and additional plants of about 380 MW should be installed.

The average energy produced by the existing and committed hydropower plants would reach around 2,090 GWh, but the firm hydropower energy would be around 1,620 GWh (average plant factor of 44%) or about 810 GWh in the dry season and wet season. Since the energy demand was estimated to reach 1,980 GWh in 1991, the demand in the dry season (990 GWh) will have to be partly covered by thermal plants. The energy demand in the year 2000 is estimated to reach 3,630 GWh (1,815 GWh in the dry season and wet season), and the plants to be additionally installed will have to cover about 850 GWh in the dry season and 460 GWh in the wet season. (Refer to Table G-02 and G-03)

It is presumed that the existing and committed hydropower will share, in the wet season, in upper portion of the base load and middle load of the demand estimated for 2000. In the dry season, the hydropower will share in the middle load and peak load portion. The remaining part of the load will be covered by plants to be additionally installed. On the other hand, water stored by the San Fernando dam will be primarily used for irrigation purpose, and the stored water should basically be released in accordance with the irrigation water requirement.

In view of the above situation, as well as the water requirement for irrigation (Refer to Table G-01), power generation in the dry season at the San Fernando dam was planned to be made by water released for irrigation (8 to 30 m³/s), and it will cover a part of base load of the power demand. In the wet season, inflow into the San Fernando reservoir is to be stored to recover water released for irrigation in the dry season. However, discharge in the range of $0.1 \text{ m}^3/\text{s} - 0.9 \text{ m}^3/\text{s}$ is to be released from the reservoir to cover the water requirement in the San Juan de Flores irrigation area which is located immediately downstream from the dam and little discharge is made available from the remaining basin. This water release can be utilized for mini-hydropower generation for auxiliary power supply for the power station and dam operation, as well as for rural electrification. In a wet year, some surplus water would be released in the rainy season through water turbine, and generated power will cover a part of peak load of the demand in central grid.

G.3.2 Generating Capacity

The capacity of generating plants to be installed at the San Fernando dam will be determined in the light of water release for irrigation. Release for irrigation water requirement is normally largest in April. The average requirement for release in April would reach 21.9 m³/s for 19,360 ha (priority la - lc area) and 32.2 m³/s for 23,960 ha (priority 1 and 2a area). The capacity of water turbine was determined to release such a quantity of water at any water level. The averaged water level of the reservoir will be adopted as a rated water level to determine the rated output of generator.

The maximum plant discharge and rated reservoir water level will be calculated in accordance with the following formula:

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Qm = Qe /hr/hehm/hr = /hr/he

where, Qm: Maximum plant discharge at rated head (m³/s)
Qe: Average discharge in April (27.6 m³/s)
hr: Rated effective head (m)
he: Minimum effective head
 (Design LWL - Tailwater level - Loss head, m)
hm: Maximum effective head
 (Design HWL - Tailwater level - Loss head, m)

A mini-hydropower plant would be additionally installed to release water required for the San Juan de Flores area in the wet season and to supply auxiliary power source for power station and dam operation. The capacity of the plant is determined at 500 kW.

A water release valve would be installed to release irrigation water during the period of scheduled and/or forced outage of the power plant. The capacity of water release would be equal to the power plant discharge, or averaged required water at the low water level.

On the basis of the power generation plan as outlined above, the reservoir operation of the San Fernando dam is studied as described in Chapter G.5.

G.4 PRELIMINARY STUDY OF WATER SUPPLY

G.4.1 General

As reviewed in Annex B.3.1, demand of water supply has been rapidly increasing in the metropolitan area, and SANAA is obliged to seek for water supply sources to meet the growing demand. The Guacerique dam and water supply project, selected in SANAA's master plan as a priority scheme, has already been designed and it is scheduled for completion by 1991. Consequently, the reservoir operation of the San Fernando dam and reservoir will be made by taking into account the water to be taken out at the Guacerique dam for supply to the metropolitan area $(1.18 \text{ m}^3/\text{s})$.

The cost of water to be produced by the Guacerique dam project, as well as the Concepcion dam and other storage schemes envisaged in the master plan, is considerably high, and MRN requested to preliminarily study the possibility of water supply from the San Fernando reservoir to be planned for construction primarily for irrigation purpose. The study made herein on the water supply system is preliminary in nature, and it must be followed by further study in future. However, the preliminary study will serve to evaluate a possible case of water supply and operation of the San Fernando reservoir.

G.4.2 Preliminary Plan

Water of the Choluteca river is planned to be taken at a pumping station to be located at the low water level of the San Fernando reservoir. A site of the intake pump station is preliminarily selected nearby the Hombre river which is the largest tributary of the Choluteca river. Although data are unavailable on quality of water, it is considered that water quality is much better than the quality of mainstream flow that involves untreated sewage from Tegucigalpa metropolitan area. The intake elevation is provisionally set at EL. 797 m.

The average monthly minimum discharge of the Hombre river at the existing gauging station (catchment area of 334 km^2) located at several

kilometers upstream of the intake pump station is estimated at around $0.99 \text{ m}^3/\text{s}$. On the other hand, the Concepcion dam and water supply project, which was selected as the second priority storage scheme by the master plan, envisages to produce 1.37 m³/s of regulated water to cover the demand up to around the year 2002. In view of the above situation, as well as in the light of possible effects on the reservoir operation at the San Fernando dam, it is provisionally planned to take 1.0 m³/s of water at the intake pump station for supply to the metropolitan area.

Water is planned to be first pumped up by No. 1 pump station to a pondage to be constructed at EL. 1,030 m (L.W.L at EL. 1,026 m), and further boosted up by No. 2 pump station to a delivery pond to be constructed nearby Tegucigalpa at EL. 1,105 m. The static head is 233 m for No. 1 pump station and 79 m for No. 2 station, or 312 m in total. (Refer to DWG-G-01) No. 1 and No. 2 pumping stations are planned to be equipped with 5 units of pumps (including 1 spare unit) with a capacity of 15.0 m³/min. (Refer to Table G-04)

Water would be conducted mainly through a pipeline and partly by tunnel. The pipeline would run along the existing highway in view of the access for construction and maintenance. A ductile iron pipe is proposed for such an alignment. The diameter of the pipeline is determined at 1,100 mm, through the economic comparative study to minimize the investment and operational costs. The length of pipeline is estimated at around 19.5 km from the intake pump station to the delivery pondage. Water would be further led through a 3.8 km long tunnel to the receiving pondage at a filtration plant. A treatment plant will have a capacity of 87,000 m³/day.

A profile of the pipeline alignment, as well as preliminary layout of the pumping stations used for the preliminary cost estimate are illustrated on DWG-G-02 to DWG-G-05.

G.4.3 Preliminary Cost Estimate

An investment cost of the facilities required for water supply from the San Fernando reservoir has been preliminarily estimated. The construction cost of the intake pump station is estimated to be Lp. 12.9 million, including the cost of pumps and motors, valves, surge tank, water filling and drain pumps, excavation and concrete works of pump station, overhead crane, starter and control equipment and other auxiliary equipment and works. The cost of the booster pump station is also estimated to be Lp. 5.4 million in total.

The cost of pipeline and tunnel works is also estimated at around Lp. 35.2 million, including pipes, excavation backfill and lining. The cost of treatment plant is preliminary estimated at Lp. 13.0 million. In addition, cost of power transmission line, substation at pumping stations and expansion of substation in Tegucigalpa is preliminarily estimated at Lp. 13.6 million. The cost of replacement, engineering and contingencies is estimated to be around Lp. 35 million. Consequently, the total investment cost required for the water supply system was preliminarily estimated at around Lp. 120 million. (Refer to Table G-05).

The annual operation and maintenance cost is estimated at around 2% of the electric works and 14.3% of treatment plant, as well as at the rate of Lp. 0.174/kWh of electric power. It would amount to around Lp. 10.7 million per annum. On the other hand, the annual cost of investment is assumed at 10% of the total investment cost as applied in SANAA's evaluation. Consequently, the annual cost would amount to Lp. 20.2 million in total. Since the volume of water supply would be $31.5 \text{ million } \text{m}^3/\text{year}$, the cost of treated water at Tegucigalpa is calculated at around Lp. $0.72/\text{m}^3$.

G.4.4 Further Study

The cost of water preliminarily estimated hereinabove is much lower than the cost estimated for the Concepcion and other storage schemes contemplated in SANAA's master plan. It appears that the water supply from the San Fernando reservoir is worthwhile to be considered in the case study of reservoir operation at San Fernando, as an alternative to the expansion of irrigation area in the Choluteca plain and middle reach valleys.

It is noted, further, that the study made so-far was preliminary in nature and additional studies should be carried out on the technical and economic feasibility of the water supply system. It is suggested that such additional studies will cover, among others:

- a) Review of demand forecast of water supply which was made previously under unlimited condition of sprawl of urbanization in the metropolitan district,
- b) Analysis of water quality and hydrological condition of the Hombre river, as well as water quality of the mainstream of Choluteca river at the confluence with the Hombre river,
- c) Measures to restrict water contamination in the Zambrano valley in the upstream of the Hombre river,
- d) Comparative study on water intake at the low water level of the San Fernando reservoir and alternative intake by constructing a dam at a possible damsite at around EL. 870 m on the Hombre river,
- e) Study of alternative site for treatment plant and alternative alignment of pipeline, and
- f) Comparative study of other water supply systems contemplated by SANAA.

It is suggested that the additional studies be made in the shortest possible period or, at the latest, by the time of the initial stage of the detailed design for construction of the San Fernando dam.

G.5 RESERVOIR OPERATION

G.5.1 Alternatives

In the light of priority area for irrigation as discussed in Chapter G.2.2, the alternative study on reservoir operation at the San Fernando has been made in the following cases:

Case 1-1: Irrigation of 1st priority areas of 19,360 ha (16,000 ha in Western plain, 2,680 ha in San Juan de Flores and 680 ha in existing irrigation area in the middle reach valley),

Without intake of water for the metropolitan area.

Case 1-2: Irrigation of 19,360 ha, With intake for water supply at 1.0 m3/s.

Case 2-1: Irrigation of 23,960 ha (1st priority area and 4,600 ha in Eastern plain - B), Without intake for water supply.

Case 2-2: Irrigation of 23,960 ha, With intake of water supply at 1.0 m³/s.

Case 3 : Irrigation of 29,160 ha (lst and 2nd priority area, including Eastern plain - B), Without intake for water supply.

G.5.2 Reservoir Operation Program

The reservoir operation would be made in accordance with the following procedures: (Refer to Figure G-03)

1) Water Release from Reservoir:

Release of water from the San Fernando reservoir is basically made in accordance with irrigation water requirement. Quantity of release is calculated in accordance with the following formula:

 $Qout_{O} = \frac{\Sigma}{i} (Qdiv_{i} - Qres_{i}, O)$

where, Qouto: Release from reservoir

Q div i: Diversion water requirement

Qres i: Discharge from residual catchment area

The diversion water requirement is shown on Table G-01. The discharge from residual catchment area is calculated by deducting discharge at the San Fernando damsite from discharge at El Papalon (Ref. Table C-22), as shown on Table G-06. (Refer also to Figure G-04)

2) Water Supply:

Water is presumed to be taken at the Guacerique dam and San Fernando reservoir. At Guacerique, $1.18 \text{ m}^3/\text{s}$ would be taken out, and a return flow of 50% is presumed. Water spilled out from the Guacerique dam is also added by referring to the discharge record as shown on Table G-07. Pump-up from the San Fernando reservoir at $1.0 \text{ m}^3/\text{s}$ and its return flow at 50% are also presumed.

3) Inflow into Reservoir:

On the basis of discharge estimated at San Fernando and presumed water supply, the inflow into the San Fernando reservoir is calculated as shown on Table G-08.

4) Evaporation:

Evaporation from the reservoir is estimated on the basis of evapo-ration record at La Venta meteorological station located close to the damsite. (Refer to Table C-16).

5) Reservoir Scale:

Reservoir scale is determined to guarantee release for irrigation and water supply. Failure in irrigation water supply may be allowed in dry years with a recurrence period of not less than 5 years.

6) Power Capacity and Output:

Power capacity and output are calculated in accordance with the procedures explained in Chapter G.3.2.

The water release from the San Fernando reservoir for irrigation of 19,360 ha and 23,960 ha is calculated as summarized on Table G-09 and G-10.

G.5.3 Result of Reservoir Operation Study

The result of reservoir operation is compiled in the attached sheets and summarized in Table G-11. For irrigation of 19,360 ha in the Western plain, San Juan de Flores area and existing irrigation area in the middle reach valley, an effective storage capacity of 200 MCM with the high water level at EL. 817.6 m would be required (Case 1-1). The annual energy output is calculated at approximately 45 GWh, with the average power capacity of 7.1 MW in the dry season. In case that intake of water supply is planned to be 1.0 m³/s (Case 1-2), the storage capacity would slightly increase to 210 MCM, and annual energy output would be reduced to 43 GWh.

For irrigation of 23,960 ha in the first priority areas and the Eastern plain-A (4,600 ha), the storage requirement is to be increased to 354 MCM (Case 2-1). Such a sharp increase in storage capacity is necessitated to secure adequate restoration of water storage for irrigation. In case that intake of water supply is planned to be $1.0 \text{ m}^3/\text{s}$ (Case 2-1), the storage requirement is further increased to 380 MCM, and annual energy output is calculated at 54 GWh with the maximum power capacity of 18.2 MW and average capacity of 11.1 MW in the dry season.

Further, in case that the irrigation area is expanded to 29,160 ha to additionally cover the Eastern plain-B, the storage requirement would be increased to 737 MCM (Case-3). As noted in Chapter G.1.2, the averaged annual inflow into the San Fernando reservoir for the last

29 years is 400.5 MCM, and the storage capacity will not be recoverable if water would be released to cover 29,160 ha. Therefore, this alternative to additionally irrigate the Eastern plain-B is found to be technically unjustifiable.

G.6 SELECTION OF RECOMMENDABLE PLAN

G.6.1 Economic Comparison

Four alternative plans (Case 1-1, 1-2, 2-1 and 2-2) have been preliminarily evaluated in terms of economic benefit/cost (B/C) ratio and benefit- ∞ st (B-C) value. The economic benefit is estimated on the basis of incremental net revenue of agricultural production between "with" and "without" project condition and power revenue from fuel saving. Benefit of water supply is provisionally assumed at Lp. 0.9/m³ which is equivalent to the water production ∞ st of the Guacerique project (production ∞ st of the Concepcion project is much higher).

The B/C ratio of Case 1-1 turns out to be 1.20 as shown on Table G-07. The ratio would be slightly decrease to 1.11, if water supply scheme is incorporated (Case 1-2). For irrigation of 23,960 ha and water supply at 1.0 m³/s with a storage capacity of 380 MCM (Case 2-2), the B/C ratio is calculated at 1.14. In case that intake for water supply is unnecessary (Case 2-1), the ratio would be 1.24. (Refer to Table G-12)

Additionally, a case study is made on the economic feasibility of irrigation on the Western plain by diverting natural flow of the Choluteca river at El Papalon without seasonal regulation by the San Fernando dam. The incremental benefit by such a supplemental irrigation in the rainy season would rather be limited, as reviewed in Annex F.6, and the B/C ratio is calculated at 0.46. Consequently, the execution of irrigation only in the rainy season is considered as economically unjustifiable.

G.6.2 Recommendable Plan

The plan recommendable for implementation is to be selected in the light of economic comparison as noted in the foregoing chapter. Some other factors are additionally taken into account in the selection of the recommendable plan. They are, among others:

- a) Even in case that intake for water supply is not decided to be made directly from the San Fernando reservoir through detailed study as recommended in Annex G.4.4, water of the Choluteca river in the upstream reaches may possibly be utilized for water supply in a long run, as contemplated in SANAA's master plan. Intake of water from the San Fernando reservoir and in the upstream reaches would have similar effect on storage requirement for irrigation at the San Fernando reservoir, and it is recommendable that the storage capacity be planned for the present to cover the water supply to the metropolitan area (Case 1-2 and Case 2-2).
- b) The site suitable for water storage in the Choluteca river basin is relatively limited and much more costly if compared with the San Fernando dam and reservoir. In view of such a cost of alternative storage scheme at the Morolica II site as previously reviewed in 1977-78 study, it is desirable to store as much water as possible at the San Fernando and to expand irrigation area on the Choluteca plain.
- c) A stage-wise development would be desirable. However, the dam heightening in stages would be practically difficult, because it involves dewatering of the reservoir for heightening and causes much inconvenience in irrigation water supply and power generation. Reduction of irrigation and power benefit for two years of dewatering and re-filling would amount to Lp. 62 million in financial terms, which is higher than the incremental cost of dam heightening. Consequently, it is recommendable that the San Fernando dam be constructed at the initial stage at the height designed for ultimate stage of development.

With the above in view, it is recommended that the San Fernando dam be designed to have a storage capacity of 380 MCM for irrigation of 23,960 ha and for power generation of 54 GWh. The result of reservoir operation is summarized as illustrated in Figure G-05. Characteristics of power output are also illustrated in Figure G-06 and G-07.

G.6.3 Possible Implementation Program

In view of the urgency of agricultural development on the Choluteca river basin and fund requirement for implementation, it would be possible to contemplate phased-out implementation of the recommended plan. A possible program for implementation will be as follows:

Stage-1:	Phase 1-1	a)	construction of the San Fernando dam
			with the storage capacity of 380 MCM.
		b)	irrigation of 12,400 ha of land on
			the right bank of the Choluteca river

Phase 1-2 a) irrigation of 3,600 ha of land on the left bank of the Choluteca river in the Western plain.

in the Western plain.

Stage-2: Phase 2-1 a)

irrigation of 4,600 ha of land on Eastern plain-A.

Regardless of the phased-out implementation as provisionally contemplated above, rehabilitation of the irrigation systems in the San Juan de Flores area and the middle reach valleys should be executed separately.

Power supply in the dry season in 1991 would be partly covered by thermal plants, as reviewed in Chapter G.3.1, and power generation at San Fernando may contribute to fuel saving. The economic evaluation will therefore be made on the condition that power plant will be installed at the time of dam construction. Since the additional cost required for the power scheme would be limited to the power house, generating equipment, substation and transmission line, the fund required for the power plant installation may possibly be arranged separately.



10 ^{3m3})	Total	287,280	22,388	28,467	338, 135	87,386	425,521	97,422	522,943			
(Unit:	Dec.	51,328	2,349 3,330	2,224	56,882	15,658	3,687 37,044 72,540	18,689	46,165 91,229			
	Nov.	336 26, 336		2,278	30,963	483 6,081 I5,658	37,044	9,121	46,165			
	. oct.		0 1,874	4 994	32 3,204	0 483	32 3,687	0 208	1,082 3,895		·	
	Aug. Sep.	032 208	65	021 874	8,118 1,082	Ö	118 1,082	1,425	9,543 1,08			
LNB	Jul. A	8,832 1,456 18,320 6,032	544	2,200 2,021	1,064 8,	4,002	25,066 8,118	2,169 1,	30, 235 9,		-	
REQUI REMENT	Jun.	1,456 1	158	86	11,738 1,700 21,064	805		338	2,843 3			
	May		992	1,914		5,028	82,084 16,766 2,505	2,345	11, 111			
DIVERSION WATER	Apr.	54,192	3,329	3,578	61,099	20,985		18,252	100,336			
	Mar.	47,936	4,226	5,333	57,495	, 12 , 691	1 73,186	15,241	88,427			
Table G-01	. Feb.	39,952 32,352	2,450 3,071	7 3,508	38,93I	5 7,457	57,055 46,388 73,	15,371 11,263	72,426 57,651			
	Jan.			3,457	45,859	11,196		15,37.	72,420			
	(Ha)	(16,000)	(680)	(2,680)	(19,360)	(4,600)	(23,960)	(5,200)	(29,160)			
	Area	l) Western Plain	2) Middle Reach	3) S.J. Flores	Total (1-3)	Eastern Plain - A	Total (1-4)	Eastern Plain - B	Total (1-5)			
		л (т	2)]	(f)	C *	4) 1	د .	2)	<u> </u>			
						G -	21					

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Table G-02

EXISTING AND COMMITTED HYDRO PLANTS

Plant	Installed Capacity (MW)	Annual Energy Output			
		Average (GWh)	(Plant Factor) (%)	Firm (Gwh)	(Plant Factor) (%)
Canaveral	28.5	197	(60)	149	(46)
Rio Lindo	80.0	576	(82)	449	(64)
El Nispero	22.5	71	(36)	40	(20)
El Cajon	292.0	1,243	(49)	986	(39)
Total	423.0	2,087	(56)	1,624	(44)

	199	1	200	00
	Wet Season	Dry Season	Wet Season	Dry Season
Power (MW)				
Power Demand $\frac{1}{2}$	344	344	669	669
Required Capacity /2	430	430	836	836
Existing Plant: <u>/3</u>	510	510	453	453
Hydro Thermal	(423) (87)	(423) (87)	(423) (30)	(423) (30)
Plants to be installed Energy (GWh)	-	-	383	383
Energy Demand $\frac{1}{2}$	989	989	1,817	1,817
Existing Plant:	1,204	989	1,362	970
Hydro Thermal	(1,204)/4	(812) <u>/5</u> (177)	(1,204) (158)	(812) (158)
Plants to be installed		. 	455	847

Table G-03 SYSTEM POWER AND ENERGY BALANCE

Note: /1: Refer to Table B-26

/2: Including reserve capacity of 25%

<u>/3</u>: Refer to Table B-18. Thermal plants at Santa Fe, San Lorenzo, S.P. Sula and Miraflores are to be retired by 1990, and P. Cortes I and La Ceiba are retired by 1994.

/4: Estimated at a plant factor of 65%

/5: 1/2 of firm annual energy output

	Facilities	Outline
L	Intake Pump Station (No. 1)	
	L.W.L in Reservoir H.W.L of No. 1 Pondage Static Head	EL. 797.0 m EL. 1,030.0 m 233.0 m
	Pumps:	
	Discharge Head Unit Motor Type	15.0 m ³ /min. 240.0 m 4+1 (spare) 1,000 kW Horizontal shaft multi- stage volute pump
2.	Booster Pump Station (No. 2)	
	L.W.L of No. 1 Pondage W.L of Delivery Pondage Static Head	EL. 1,026.0 m EL. 1,105.0 m 79.0 m
	Pumps: Discharge Head Unit Motor	15.0 m ³ /min. 100.0 m 4+1 (spare) 400 kW
	Туре	Horizontal shaft double suction volute pump
3.	Pipeline and Tunnel	
	Pipeline: Length Diameter Pipe Tunnel length:	19.5 km 1,100 mm Ductile iron pipe 3.8 km
4.	Treatment Plant	
	Capacity:	87,000 m ³ /day
j.	Transmission Line & Substation	
	Transmission line No. 1 pump station No. 2 pump station Tegucigalpa s/s (expansion)	approx. 30 km 6,000 kVA 3,000 kVA 9,000 kVA

Table G-04 PRELIMINARY PLAN FOR WATER SUPPLY

Table G-05 PRELIMINARY ESTIMATE OF COST FOR WATER SUPPLY SYSTEM

	Amount (Lp. 10
Investment Cost:	
1. Intake pump station	12,9
2. Booster pump station	5.4
3. Pipeline and tunnel	35.2
4. Treatment plant	13.0
5. Transmission line and substation	13.6
6. Miscellaneous	4.9
Sub-total	85.0
eplacement Cost:	
7. Equipment related to No. 1 station	5,9
8. Equipment related to No. 2 station	4.1
Sub-total	10.0
Total (1-8)	95.0
9. Engineering and administration	10.0
10. Contingency	15.0
Total (1-10)	120.0

Table G-06 INFIOW AT EL PAPALON WEIR

2.90 175.70 .80 51.10 18.20 30.30 50.40 32.50 ACM 0 10 10 10 .. 0 22.80 21.90 27.60 17.80 0 50.00 173.80 475.20 147.20 0 108.80 455.80 331.30 70.00 0 51.60 427.00 443.70 232.50 0 36.10 27.10 534.00 48.30 0 144.20 89.70 31.40 7 273.40 294.60 84.30 0 463.60 686.90 158.50 3 560.80 321.30 94.60 0 247.10 293.00 56.80 21 60 38 50 58 80 58 80 58 80 29 60 23 60 25 90 25 90 25 90 75 20 25 90 75 20 114.20 24.90 25.00 61.70 35.00 25.20 138.70 138.70 44.50 227.60 65.90 25.00 275.30 107.50 70.30 406.40 466.20 129.40 378.80 109.50 123.90 273.10 910.50 107.60 903.80 132.90 115.90 309.70 120.30 48.40 LINU VON 8 27 8 6 7 8 241•20 241•20 5 3 83 80 78.20 102.20 183.70 128.00 251.80 148.20 231.00 160.00 360.40 158,00 163,60 182,60 182,60 215,90 130,50 001 42.40 73.10 14 41.40 109.20 24 184 40 584 40 266 40 101 80 S EP 11.80 96.00 45.30 39.50 104.90 451.90 80.50 51.40 43.40 445.50 139.10 422.50 21.90 16.00 88.10 185.90 55.00 41.20 19.60 81.00 61-20 67-10 67-10 06-30 41.90 92.10 24.80 173.80 73.40 AUG 0 147.80 26.40 239.50 186.70 44.80 9.00 154.70 119.00 139.00 142.80 109-70 82-10 71-10 73-40 284-00 164-40 103-60 43-40 398-50 240-70 17.50 94.90 20.30 68.50 75.40 34.50 JUL 218.40 75. J 63.00 34 • 110.10 51.50 184.80 340.90 NUL 54-60 1(5-60 2 71-20 2 744-80 3 112.40 116.90 104.90 24.50 94.20 94.20 41.00 174.30 16.00 64.50 44 90 71 50 80 60 733 70 73 60 MΛΥ 117 20 177 20 177 20 177 20 177 20 280 27 21.90 16.40 15.70 8.10 12.60 26.70 7.10 10.70 ਮ ਪੂ 13.50 13.50 13.50 13.50 28 °20 8 °20 8 °20 MAR 7 20 9 60 9 80 9 80 AT THE WEIR 18.60 14.20 23.50 23.50 26.50 13.20 н 1 1 1 1 1 1 1 1 1 1 1 1 1 1 21.60 7.30 8.80 4.40 17,90 13,20 16,20 18,10 23.40 17.50 25.40 28.00 22 10 16.40 30.70 16.30 26.10 18 20 16 80 14 40 18 70 30.80 13.40 14.00 23.60 23.60 16.00 15.90 JAN DATA J INFLOW 1 1962 1962 1965 1965 978 975 975 975 1957 1958 1959 1960 968 968 970 971 979 979 981 981 1982 1983

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mean	1.28	2.18	1.59	1.85	\$6.0	1.99	I.51	1.71	3.99	1.74	0.69	2.01	4.78	3.61	2.38	0.67	2.19	1.56	1.82	16-1	. 1•09.	1.46	1.94	1.75	I.73	1.06	1,20	I.85
dec.	0.28	0.39	0.49	0.62	16.0	0.45	0.57	0.34	0.29	0.48	0.13	0.49	0.95	0.84	0.28	0.20	0.58	0.40	0.60	0.46	0.17	0.25	0.64	0.55	0.43	0.54	0.67	0.48
• vou	0.51	0.94	0.98	1.42	0.63	I.37	0.93	0.44	1.33	0.71	0.45	1.69	1.69	1.67	0.55	0.03	1.77	0.86	I.39	0.93	0.06	0.37	1.13	0.77	0.66	0.76	1.68	0.95
oct.	L.68	3 37	3 62	6.91	2.07	6.41	3.35	2.70	3.25	3.25	1.65	2.39	10.56	5.80	5.87	0.32	9.12	2.91	4.06	5.13	0.35	1.39	5.45	4.05	1.58	2.70	3.05	3.81
sep.	2.27	0.79	0.33	2 62	1.18	2.74	1.21	3.44	35.85	2.83	1.56	3.05	13.69	15-33	8.19	0.03	5.82	4.84	11-44	0.20	0-91	2.47	5.68	3 - 53	4.21	1.33	2.84	5.13
aug	1.73	2.91	2.23	4.27	1.46	3.54	2.05	1.14	1.90	1.82	0.86	1.10	9.70	8.93	4.66	0.11	2.86	16.0	0.12	10-1	0.09	1,89	1.38	1,98	3.14	0.80	0.96	2.35
jul.	1.96	5.28	1.05	2.81	3.17	2.54	3.77	5.47.	1.43	4.50	1.18	2.58	5.82	5.77	2.08	1.09	3.20	3.09	0.87	2.78	1.11	3.91	2.61	2.36	2.59	0.87	I.79	2.80
june	4.88	8.97.	8.59	1.44	0.57	4.97	3.87	4.32	2.15	5.24	0.68	7.48	12.33	1.36	1.96	4.16	2.02	3.76	0.89	2.76	7.85	3.64	3.75	5.80	5.94	3.18	1.78	4.23
may	I.12	1.32	0.60	0.76	0.06	0.14	1.02	0.70	16.0	1.42	0.22	5.06	1.57	I.19	3,30	1.65	0,28	0.92	1.40	1.24	1.19	1.67	1.02	1.12	0.97	1.29	0.25	1,20
apr.	0.32	1:01	0.29	0.35	0.14	0.28	0.29	0.58	0.12	0.14	0*60	0.08	0.15	0.64	0.35	0-09	0.14	0.10	0.16	0.23	0.38	0.86	0.88	0.17	0.18	•		0.34
mar	0.15	0.74	0.23	0.23	0.15	0-26	0.37	0.59	0.15	0.14	0.23	.60*0	0.17	0.44	0.34	0.07	0.14	0.12	0.18	0.16	0.33	0.75	0**0	0.15	0.39	0.37	0.31	0.28
feb -	0.23	0 26	0.32	0.36	0.42	0.49	0.33	0.41	0.20	0.15	0.31	0.04	0.26	0.62	0.42	0.13	0.19	0.34	0.38	0.40	0.29	0.21	0.19	0.22	0.33	0.38	0.32	0.30
jan.	0.20	0.21	0.31	0.39	0.52	0.70	0.36	0.44	0.27	0.17	0**0	0.07	0.41	0.70	0.53	0.21	0.13	0.42	0.30	0.46	0.34	0.08	0.14	0.33	0.28	0.21	0.45	0.33
year	1957	1958	1959	1960	1961	1962	1963	1964	1965	1966	1967	1968	1969	0261	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980	1981	1982	1983	mean

Table G-07 MONTHLY DISCHARGE AT GUACERIQUE DAMSITE

Table G-08 INFLOW AT SAN FERNANDO DAMSITE

N01274 4011044 1044 12.4 11.2 0 11 10 11 10 16.6 35.0 2.6.55 11-4 15-7 30-7 30-6 NON 32.3 66.7 74.4 700.4 135 • 4 7 • 2 81 • 6 65 • 4 9.7 40.8 40.8 40.8 67.8 67.8 77.4 57°2 100 232.8 232.8 232.8 81.7 126°9 726°9 77°8 77°8 77°8 77°8 28.0 57.9 261.5 SΕΡ 18.0 29.5 20.00 25.00 20.000 2 4 0 4 0 4 6 4 0 AUG 35.0 4 6 4 8 6 8 6 6 8 6 8 7 8 8 6 13 4 30 4 109 4 77 2 77 2 10.01 241080 547080 547080 JUL 30.8 44.7 36.2 7.7 169.7 15240 884 2867 49867 49867 96-8 36-8 70.8 142.5 362.8 105.9 205.9 101 6-22 0 N 0 0 F N 0 F M N F F 5 F 0.0 4 R 0.0 0.0 4 28.0 (MCM) ΠAY. - N - N 0 10 2.5 4 10 K 4 10 M 7 4 4 6 M 6 5 APR 20 5 5 3. S 0.00000 60000 00444 40449 00-00 - いるいい - いるいい MONTHLY INFLOW TO RESERVUIR N A R 0 × 0 C N 6.1 04400 NN060NN 000130 101222 20 0 4 0 4 0 0 0 0 0 0 0 0 0 0 0 00000 011000 Е С. С. -000m0 04000 20004 200014 1000 5 5 2 6 2 6 8° • 20102 20102 20102 N V N 9000 1900 1900 1900 1900 1900 1900 1962 1978 1979 1979 1980 1982 1983 MEAN YEAR 1959 1959 1960

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Table G-09		ELEASE	AN L	- 20 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0 N N N N 8 N O N N 8 N O N N N M N M N	200000 20000 20000 20000	N8W4N	004 - 00 000 00 0 00 0 0 0 0 00 0 0 0 0	0 • 0 9 0 8 0 8 0 8 0 9
		REGUIRED R	E A R	1957 1958 1958 1965 1961	19662 19663 19665 19665	1967 1966 1966 1972 1972	1972 1973 1974 1975 1975	1977 1978 1979 1980	1982 1983

FOR IRRIGATION (23,960 HA) REQUIRED RELEASE

Table G-10

00100 001000 001000 40.00 40.00 40.00 40.00 21.4 0 9 E C UNIT : MCM 80..6 **NNNNN** 000000 00000 ง พพพพพ พ MMMMM SNNNN 44 NON 00000 -----1.00 000 00000 ------00 001 •• (MCM) 00000 000000 000000 6.0 SEP DISCHARGE 00000 NNNNN 00000 NNNNN AUG 00000 00000 00000 NNNNN 00000 00000 00000 0 2 2 2 MAXINUM IRRIGATION 00~00 00000 NNNNN NNNNN NNNNN 2 2 2 2 2 2 2 2 JUL - - -- - - -JUN ----------COCHO -----00000 ~ ~ ~ ~ ~ ~ ~ ~ ~ 0,000 ------¢.¢ MAY APR 144 0844 086 0440 0440 60.2 FOR IRRIGATION 59 - 7 59 - 7 59 - 5 59 - 5 59 - 5 59 - 5 59 - 5 59 - 5 59 - 5 59 - 5 59 - 5 59 - 7 57 • 1 60 • 6 55 • 3 55 • 0 MAR 44°9 50.07 49 33 9 н П П RELEASE 08440 3440 3450 4 4 4 4 4 JAN REQUIRED YEAR 1958 1958 1959 1960 1962 1965 1965 967 969 1970 1972 1979 1978 1979 1980 1982 1983

					je e se la la	
		Case 1-1	Case 1-2	Case 2-1	Case 2-2	Case 3
Irrigation Area	(ha)	19,360	19,360	23,960	23,960	29,160
Water Supply	(m ³ /s)	· · · ·	1.0	~	1.0	· _
Storage Capacity	(MCM)	200.5	207.2	354.0	387.6	737.5
Full Supply Level	(EL)	817.6	818.0	825.1	826.6	843.2
Min. Operating Level	(EL)	797.0	797.0	797.0	797.0	797.0
Max. Discharge	(m3/s)	21.8	21.9	32.1	32.2	43,4
Power Capacity	(MW)	7.1	7.0	10.9	11.1	15.8
Annual Energy	(Gwh)	45.2	43.4	55.4	53.6	63.4

Table G-11 RESULT OF RESERVOIR OPERATION STUDY

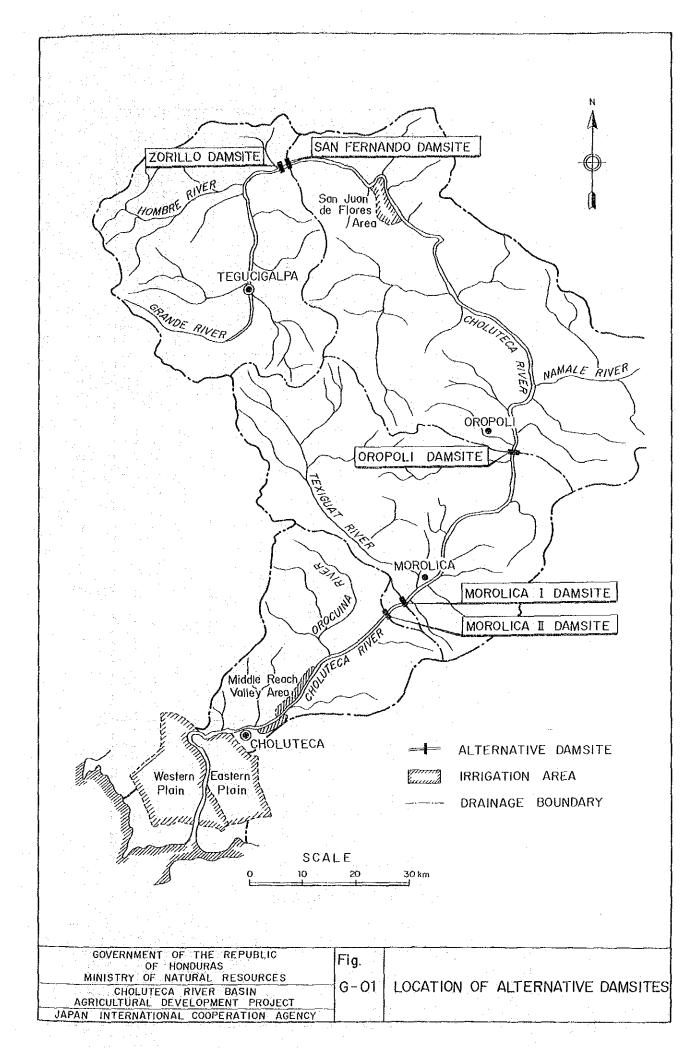
		Case 1-1	Case 1-2	Case 2-1	Case 2-2	Rainy Season Irrigation
Irrigation Area	(ha)	19,360	19,360	23,960	23,960	16,000
Water Supply	(m ³ /s)		1.0		1,0	가 다니 아이들은 바이지. 같은 아이들은 아이들은 바이지. 같은 아이들은 아이들은 아이들은 아이들이 아이들이 아이들이 아이들이 아이들
Storage	(MCM)	200	210	354	380	
Power Capacity	(MW)	7.1	7.0	10.9	11.1	<u></u>
Annual Energy	(GWh)	45	43	55	54	
enefit (Present Va	alue, Lp.	106)			ال المراجع م المراجع المراجع المراجع	
Irrigation	·	159	159	192	192	38
Power		36	37	51	50	
Water Supply $\frac{1}{2}$. *	_	90		90	
Total	· .	195	286	<u>243</u>	332	<u>38</u>
ost (Present Value	e, Lp.106	ō)				
Dam, Irrigation Power Plant	&	162	162	196	198	83
Water Supply	·	-	94	-	94	
Total		<u>162</u>	256	196	292	<u>83</u>
$B/C \text{ Ratio}^2$		1.20	1.11	1.24	1.14	0.46
B-C (Lp.106)		33	30	47	40	-45

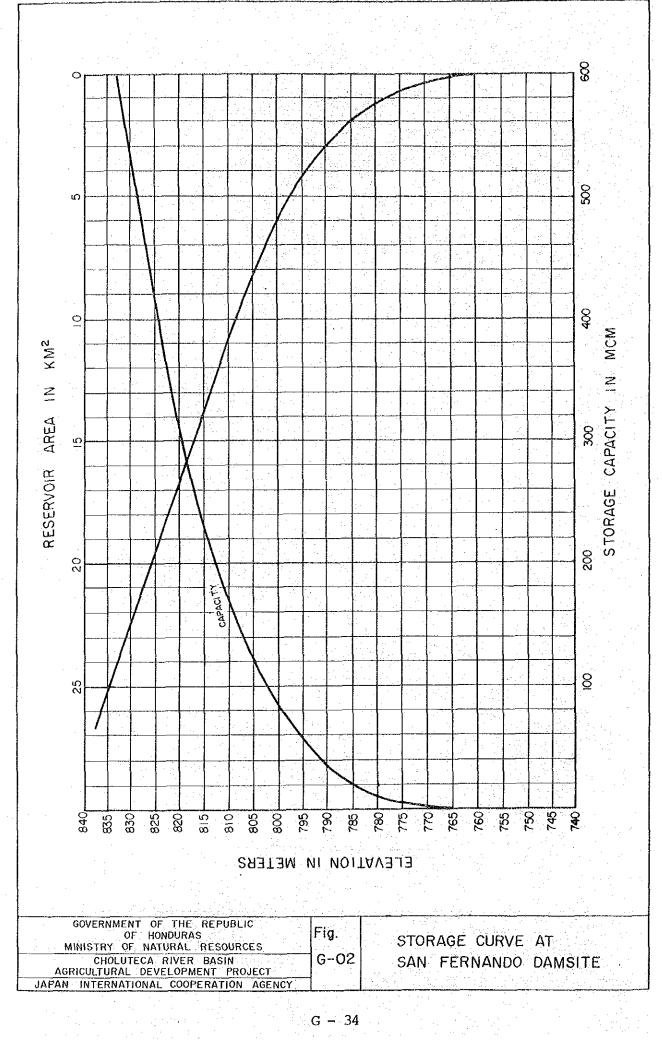
Table G-12 COMPARISON OF ALTERNATIVES

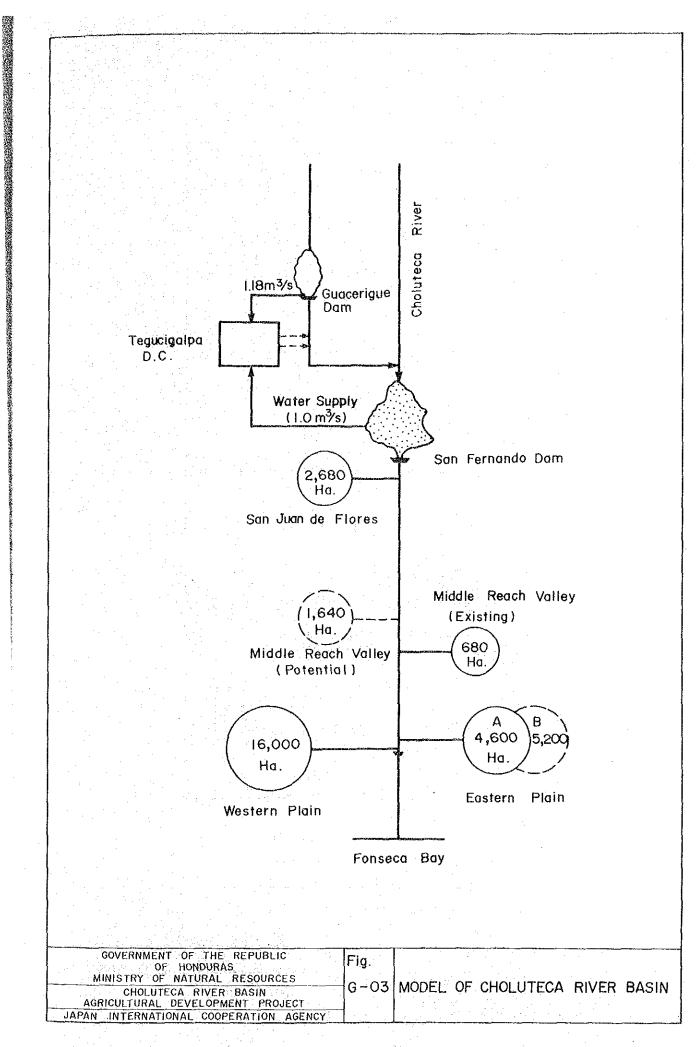
Note: <u>/1</u>: Presumed at Lp. $0.9/m^3$ which is estimated for Guacerique water supply project

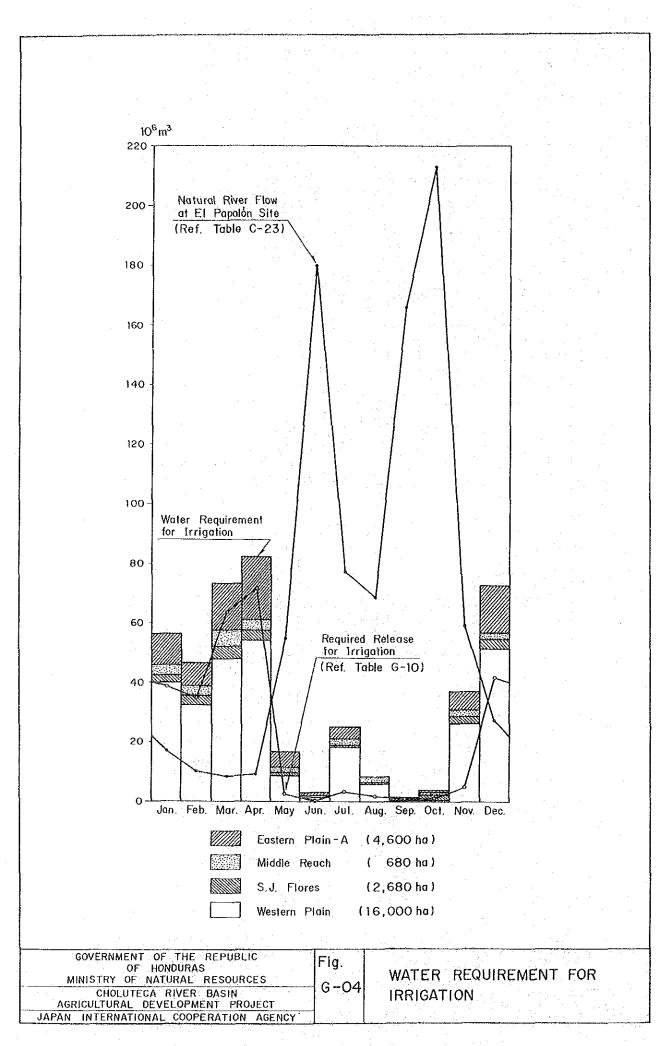
/2: Calculated at the discount rate of 12%

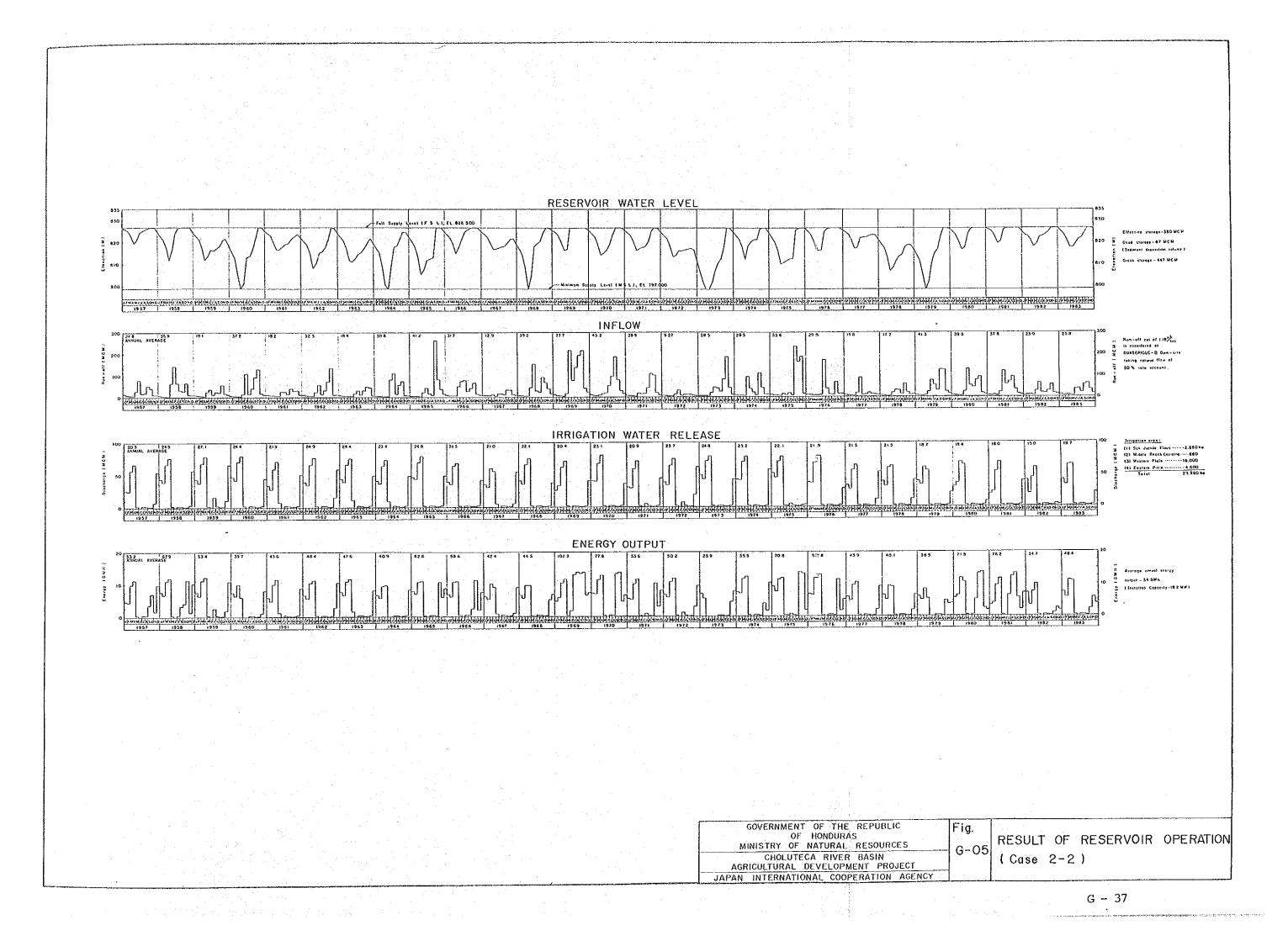


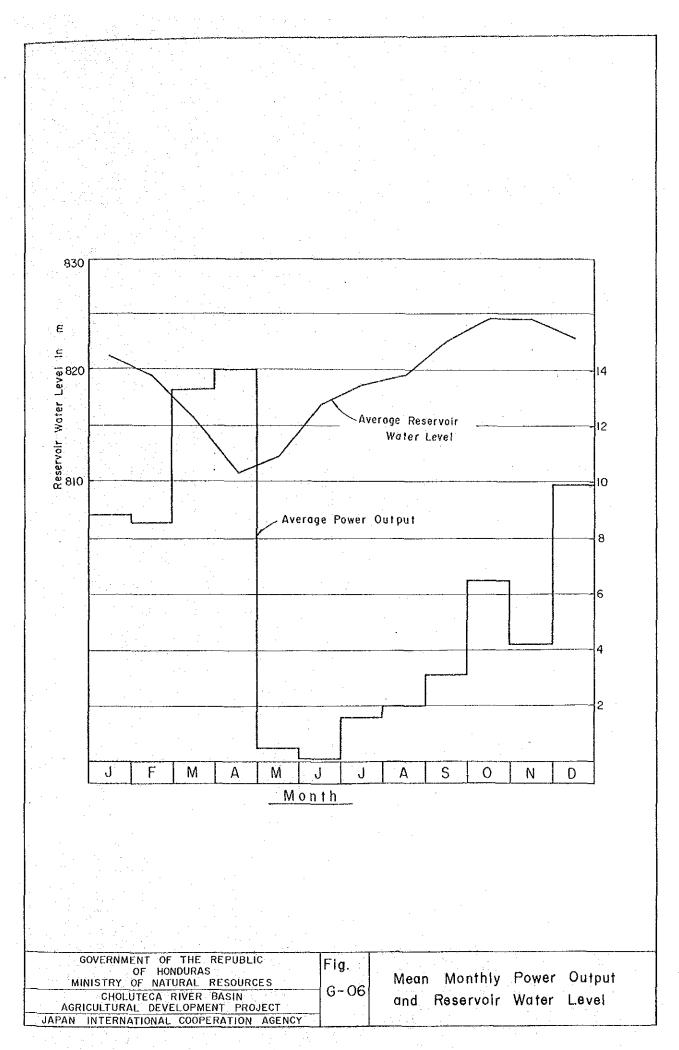






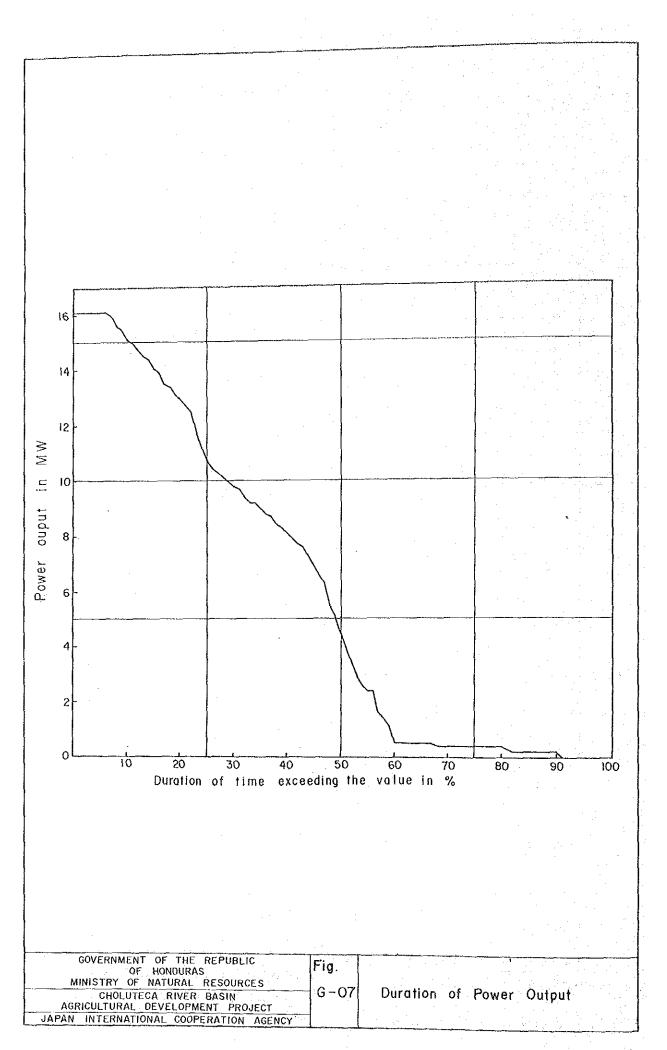






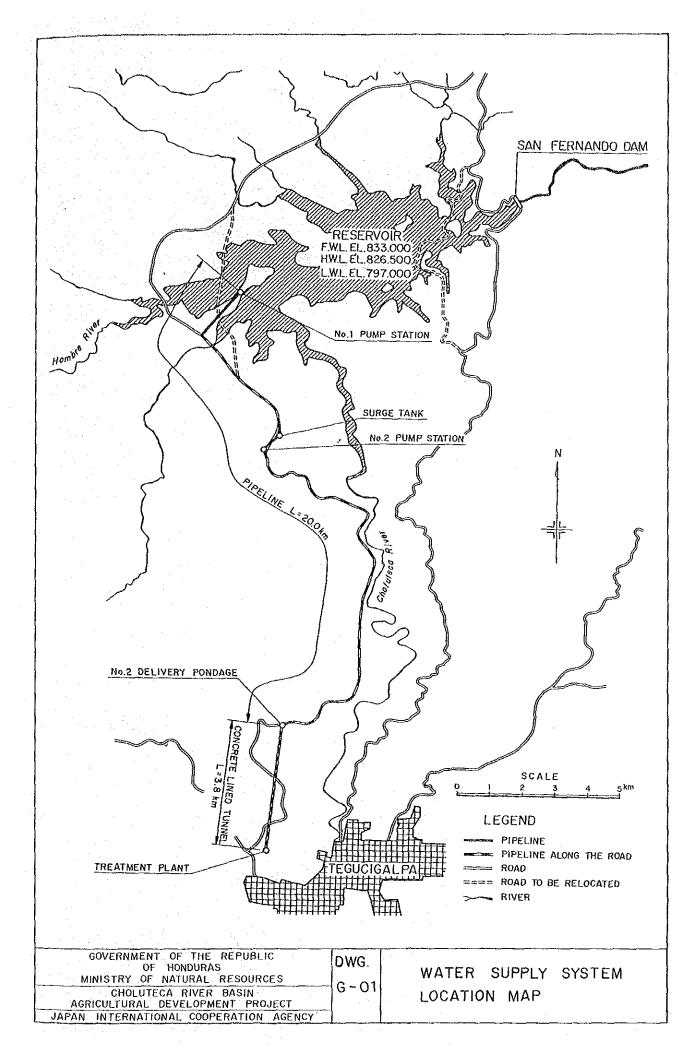
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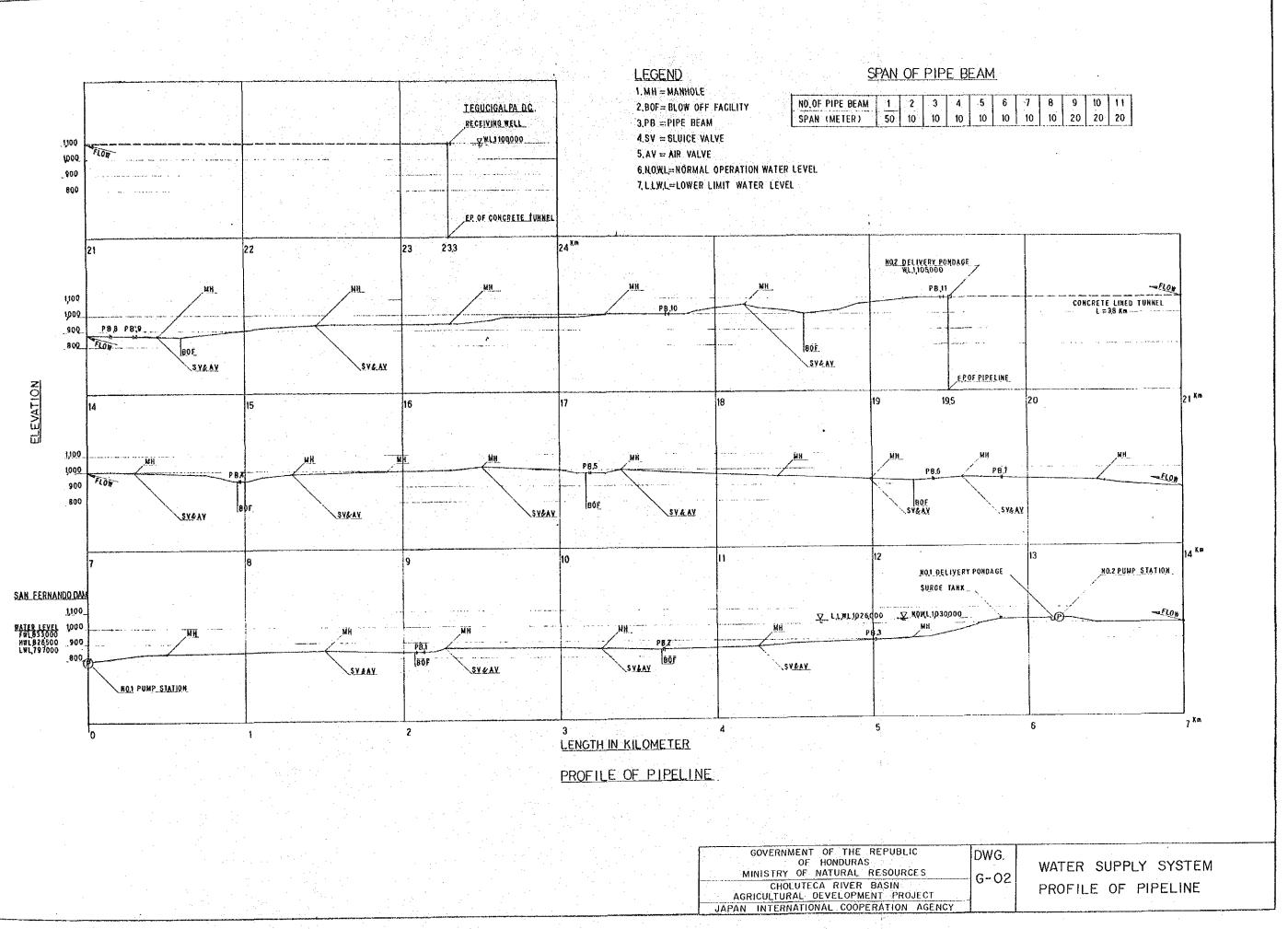
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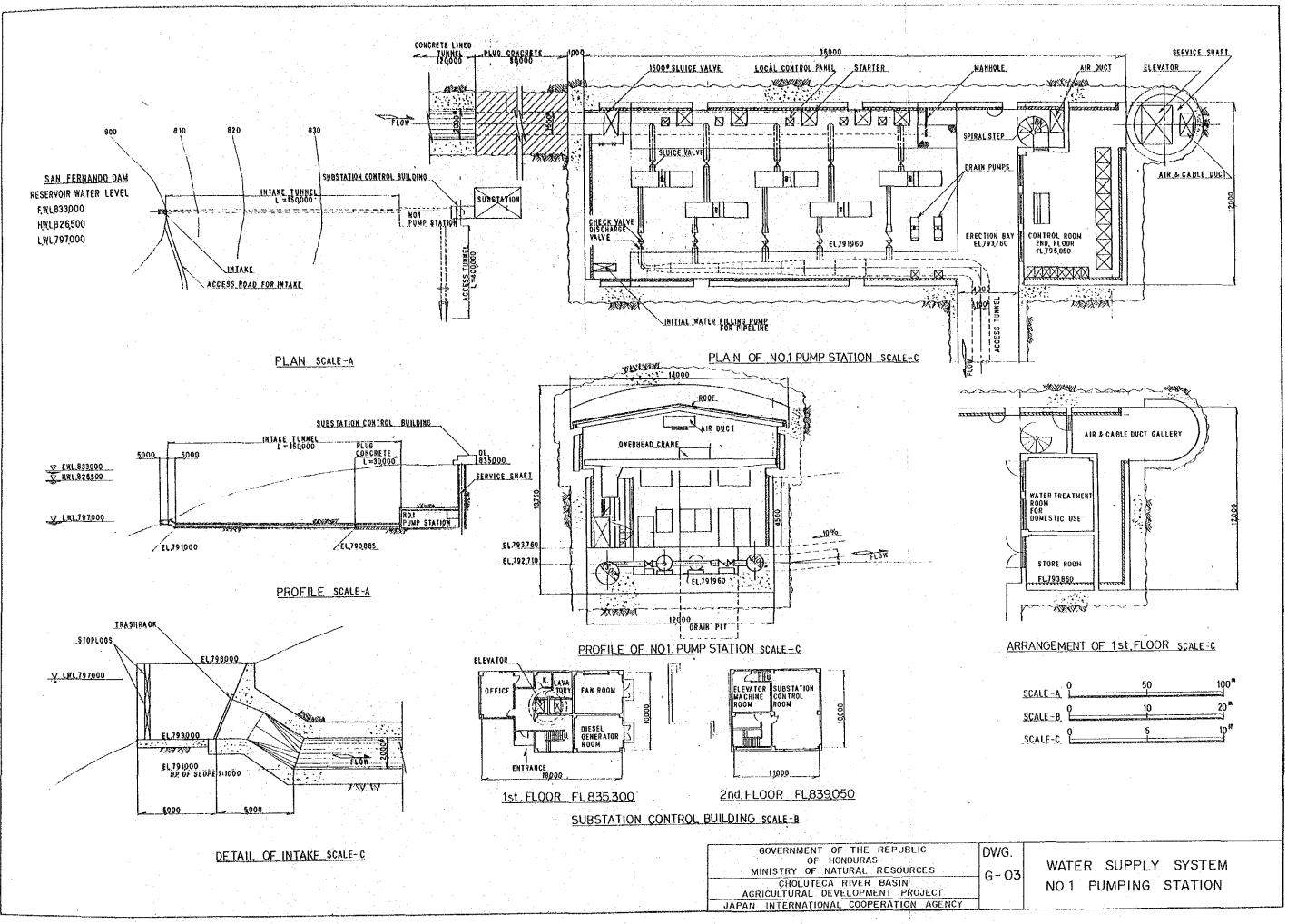


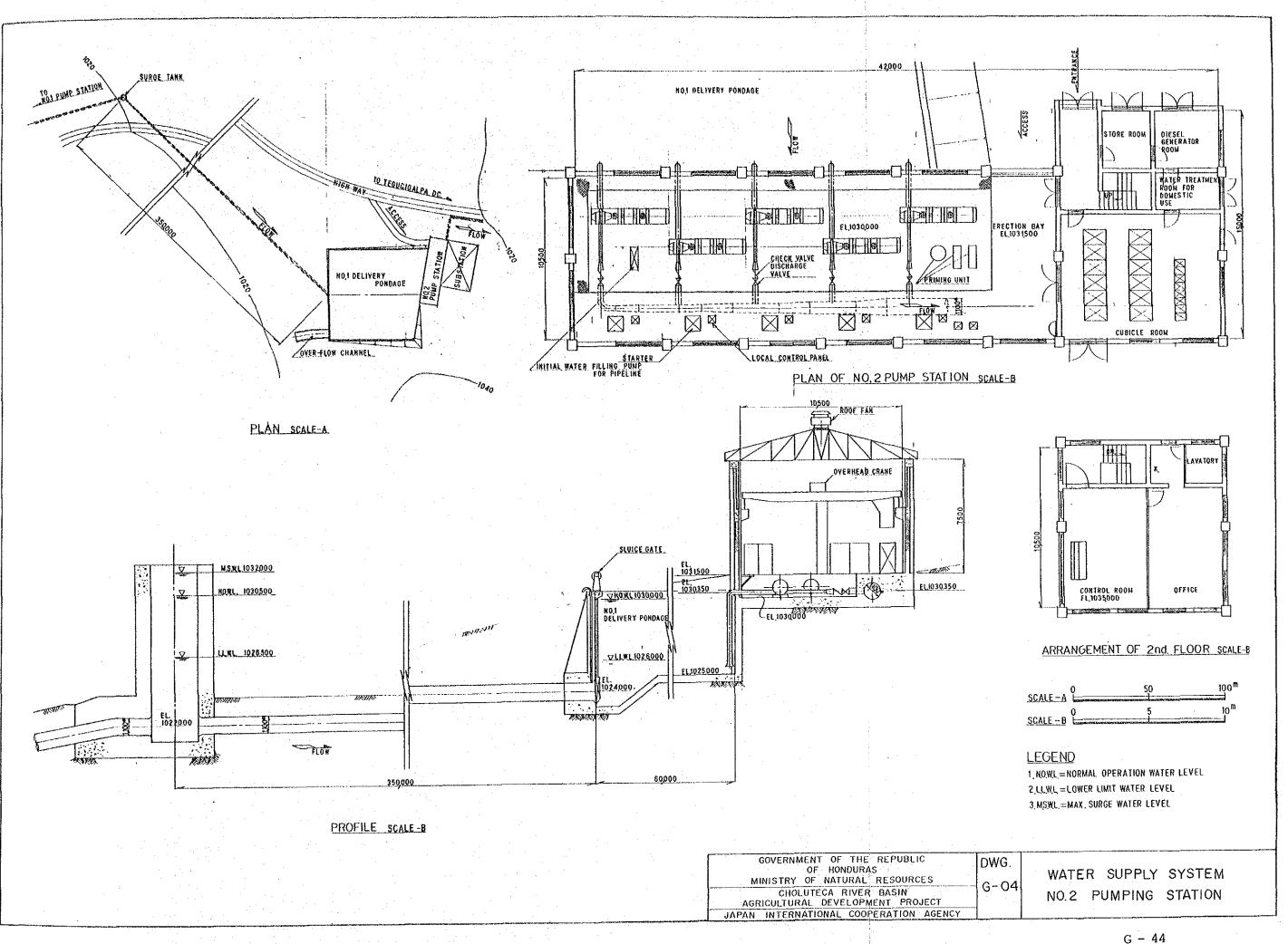
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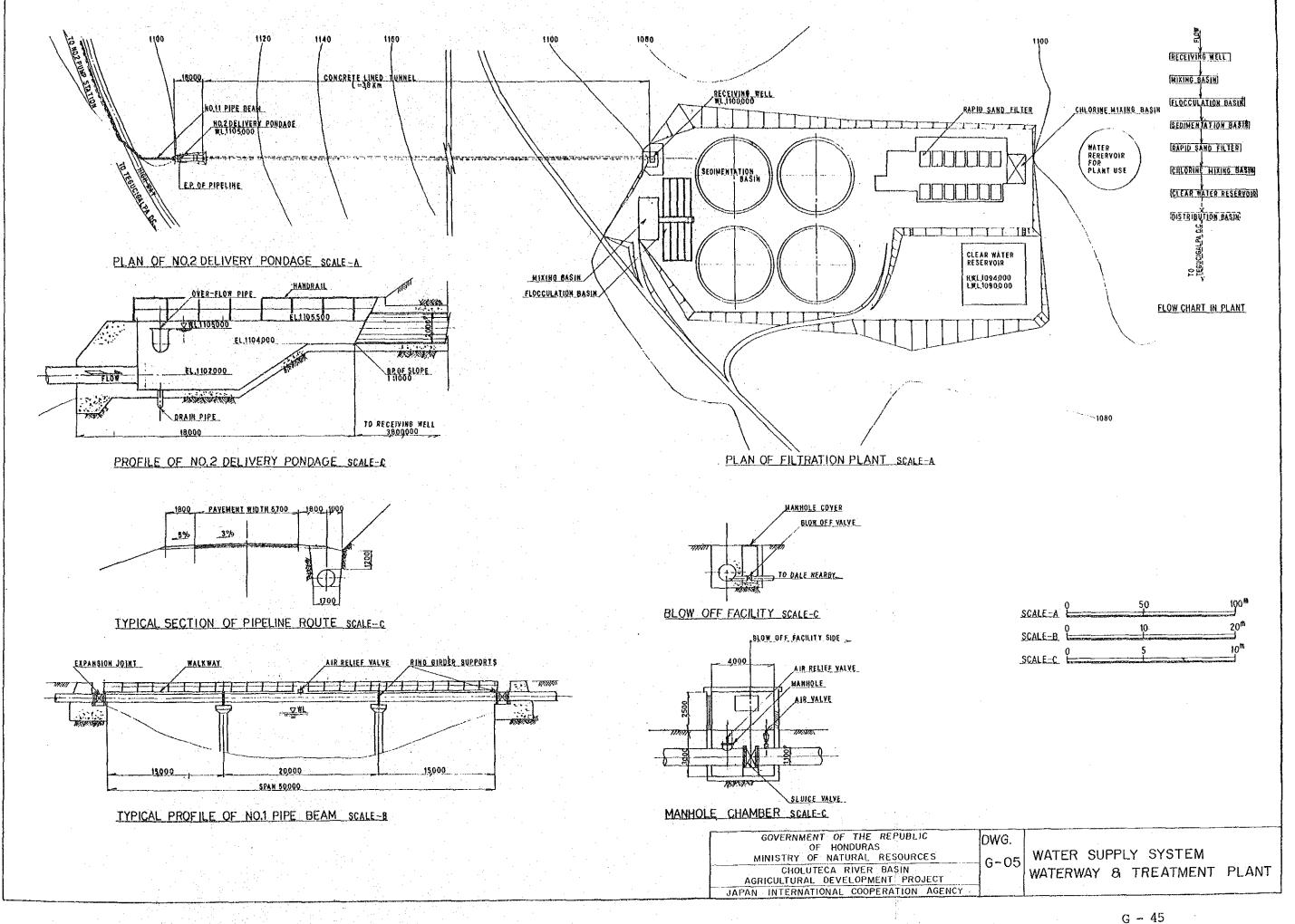
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ANNEX H IRRIGATION AND DRAINAGE PLAN

ANNEX - H

IRRIGATION AND DRAINAGE PLAN

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H. IRRIGATION AND DRAINAGE PLAN

H.1 IRRIGATION WATER REQUIREMENT

H.1.1 General

Irrigation water requirement is basically dependent on the cropping patterns, climatic conditions and soil conditions. The cropping patterns proposed for each irrigable area are as discussed in Annex F. Since the irrigable areas will extend largely in the Choluteca river basin, their climatic conditions are different from each other. Under such situations, the proposed irrigation areas were categorized into the following groups, for estimation of the irrigation water requirement and for the evaluation of the project:

1.	Western plain area in Choluteca plain	16,000 ha
2.	Eastern plain area in Choluteca plain	(9,800 ha)
· ·	a) Eastern plain-A	4,600 ha
	b) Eastern plain-B	5,200 ha
3.	Middle Reach area in Orocuina-Choluteca (existing)	680 ha

Net Irrigable Area

4. San Juan de Flores area (existing) 2,680 ha

In addition to the above schemes, the water requirement for the alternative scheme of supplemental irrigation during the rainy season on the Western plain was also estimated in a case study.

The estimate of irrigation water in this updating study, in principle, followed the procedures adopted in the study made in 1977-78. However, the updated climatological and other data were applied and the irrigation water requirement was re-estimated. The design irrigation water requirement was estimated through the following procedures:

H - 1

a) Estimate of potential evapotranspiration

b) Estimate of consumptive use

- c) Estimate of effective rainfall
- d) Estimate of percolation (for paddy field)
- e) Estimate of net irrigation water requirement
- f) Estimate of diversion water requirement.

H.1.2 Potential Evapotranspiration and Consumptive Use

In the previous study in 1977-78, several methods of estimating the potential evapotranspiration have been compared, including the Modified Blaney-Cliddle, Modified Penman, Hargreaves, Christiansen-Hargreaves, and it was found that the Christiansen-Hargreaves method explained below was found to be most suitable:

 $E_{p} = 17.4 \text{ x d x To x Fh x Fw x Fs x Fe}$ Fh = 0.59 - 0.55 Hn²
Fw = 0.75 + 0.0255 x $\sqrt{\text{Wkd}}$ Fs = 0.478 + 0.58 S
Fe = 0.950 + 0.0001 E

where,

Hn :	Mean noon humidity in decimal (%) $0.40 \text{ Hm} + 0.60 \text{ Hm}^2$
Wkd:	Mean wind velocity (km/day) at 2 m above ground surface
s :	Mean monthly sunshine hour (%)
Hm :	Mean daily relative humidity (%)

E : Elevation above the sea level

For the Western plain area and Eastern plain area, as well as for the Middle Reach area of Orocuina - Choluteca which is located relatively close to Choluteca city, the A-pan evaporation record at Choluteca meteorological station and the potential evapotranspiration calculated by the Christiansen - Hargreaves method were reviewed. As shown in Annex C, Table C-14, the annual mean A-pan evaporation was around 2,898 mm. On the other hand, the potential evapotranspiration at Choluteca station was calculated at around 2,405 mm/year, as shown on Table H-01.

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On the other hand, in the "Monthly Precipitation Probabilities for Moisture Availability for Honduras" by G. Hargreaves it was estimated at around 2,020 mm/year. In view of the accuracy of observation and conditions in the Choluteca plain, the updating study followed the calculation by the Christiansen-Hargreaves method. The design monthly potential evapotranspiration ranges from 112.9 mm in September to 274.6 mm in March. During the driest months from December to April, the potential evapotranspiration would always exceed 240 mm/month.

The design potential evapotranspiration in the San Juan de Flores area is different from that on the Choluteca plain. The potential evapotranspiration calculated by the Christiansen-Hargreaves method seemed to be substantially high. On the other hand, A-pan evaporation recorded at El Porvenir in the San Juan de Flores valley was around 1,670 mm/year, as shown in Annex C, Table C-15. As applied in the previous study in 1977-78, the A-pan evaporation was selected for the design potential evapotranspiration in the San Juan de Flores area.

Consumptive use was estimated by multiplying potential evapotranspiration by crop consumptive use coefficient (Kc) in different growth stages of crops. The crop coefficient curve and Kc values were set in accordance with the study and recommendation by G. Hargreaves.

H.1.3 Effective Rainfall

Probable rainfall on the Choluteca plain (Western and Eastern) and Middle Reach valley of Orocuina-Choluteca was estimated on the basis of monthly rainfall records at Choluteca station, at Azucarela Central (ACENSA) and at Los Encuentros station, which are reproduced in Table H-02 to H-04. For the calculation of probable rainfall on the Western plain and Eastern plain, and average of rainfall record at Choluteca station (1974-83) and at ACENSA station (1978-83) was applied. While, the probable rainfall in the Middle Reach valley of Orocuina-Choluteca was estimated using an average of the record at Choluteca station and Los Eucuentros station (1974-83). On the other hand,

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the probable rainfall in the San Juan de Flores area was estimated on the basis of rainfall record at El Porvenir station.

In this updating study, 80% probable rainfall has been applied instead of the probable drought rainfall of 90% recurrence adopted in the previous study in 1977-78. The 80% probable rainfall was calculated as shown on Table H-05 to H-07.

Effective rainfall was estimated separately for upland crop cultivation and paddy field as noted hereunder:

a) Effective Rainfall for Upland Crops:

A relationship among average monthly effective rainfall, mean monthly rainfall and averate monthly consumptive use has been developed by US Department of Agriculture, Soil Conservation Services (USDA-SCS) and it is applicable to the estimate of effective rainfall in the project area. The effective rainfall values are shown on Table H-08. The values were calculated for a 75 mm net irrigation application, and adjustment factors were worked out for application to other net depth soil and crop characteristics, as shown on Table H-09. The monthly effective rainfall should not exceed the rate of consumptive use.

b) Effective Rainfall for Paddy Field:

Effective rainfall for paddy field was estimated by means of daily water balance method. The effective rainfall at Choluteca station, ACENSA station and Los Encuentros station was calculated as shown on Table H-10 to H-12. Further, the average effective rainfall at Choluteca and ACENSA stations, as well as at Choluteca and Los Encuentros stations, was calculated as shown on Table H-13 and H-14. The effective rainfall for paddy field was applied in the following manner:

> Daily Rainfall (R=mm) Effective Rainfall (mm) 80 < R 5 < R < 80R < 5

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