

## APPENDIX Q HYDROLOGICAL ANALYSIS

### 1. Calculation for Probability distribution of daily rainfalls

Order	Daily rainfall $X_1$	Date	$E=2i - 1/2N$	$X = \log X_1$	$X_2 = (\log X_1)^2$
1	283.0	Aug. 9 1975	7.14	2.45179	6.01126
2	163.3	Oct. 3 1974	21.43	2.21299	4.89732
3	132.6	Jul.18 1970	35.71	2.12254	4.50518
4	120.0	Jul.31 1977	50.00	2.07918	4.32299
5	119.1	Aug.13 1971	64.29	2.07591	4.30940
6	110.3	Aug. 6 1978	78.57	2.04258	4.17213
7	100.3	Aug. 5 1973	92.86	2.00130	4.00520
Total				14.98629	32.22348

$$\bar{X} = \frac{\sum X}{N} = \frac{14.98629}{7} = 2.14090$$

$$\frac{\sum X^2}{N} = \frac{32.22348}{7} = 4.60335$$

$$= \sqrt{\frac{\sum X^2}{N} - (\bar{X})^2} = 0.14106$$

$$X = 2.14090 \quad X = 138.3$$

$$X + \sigma = 2.28196 \quad X = 191.4$$

$$X - \sigma = 1.99984 \quad X = 99.9$$

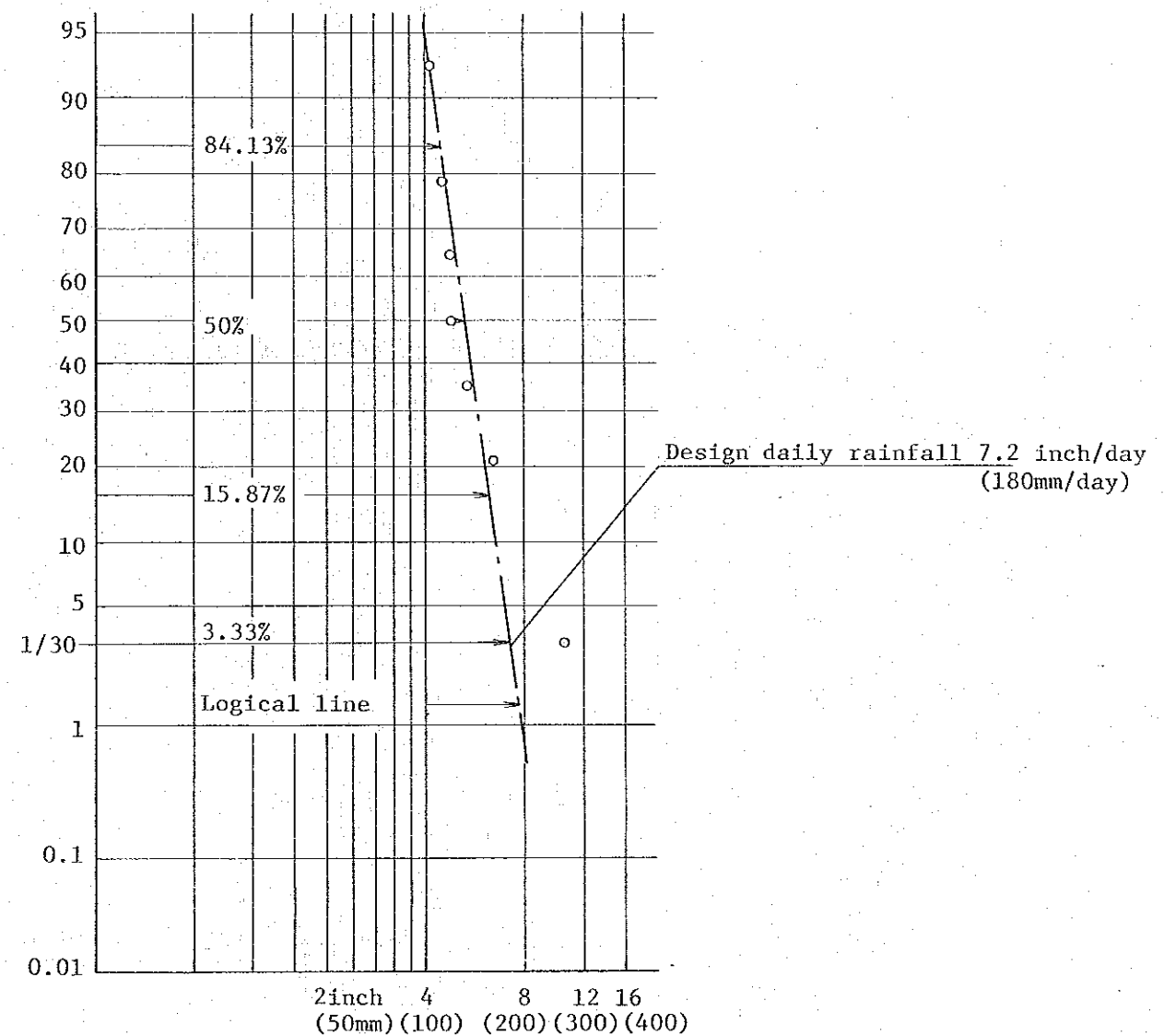


Fig. 1 Calculation of Probability Distribution for Daily Rainfall by Probability Paper at Makeni

## 2. Calculation for Time of Concentration

$$T = \frac{L}{W} \quad W = 72 \left( \frac{H}{L} \right)^{0.6}$$

where

T : Time of concentration (hr.)  
 L : Horizontal distance (Km)  
 H : Vertical Height (Km)  
 W : Velocity of flood (Km/hr.)

## 3. Calculation for Discharge Volume of Flood

$$Q = 0.2778 f R_t A \quad R_t = \frac{R}{24} \left( \frac{24}{T} \right)^{\frac{2}{3}}$$

where

Q : Assumed pick discharge volume (m<sup>3</sup>/Sec.)  
 f : Coefficient factor of discharge (0.2)  
 R<sub>t</sub> : Hourly rainfall (mm/h)  
 R : Daily rainfall (mm/h)  
 A : Catchment area (Sq.km)  
 T : Time of concentration (hr.)

## 4. Calculation for Assumed Discharge Volume on the basis of Opening

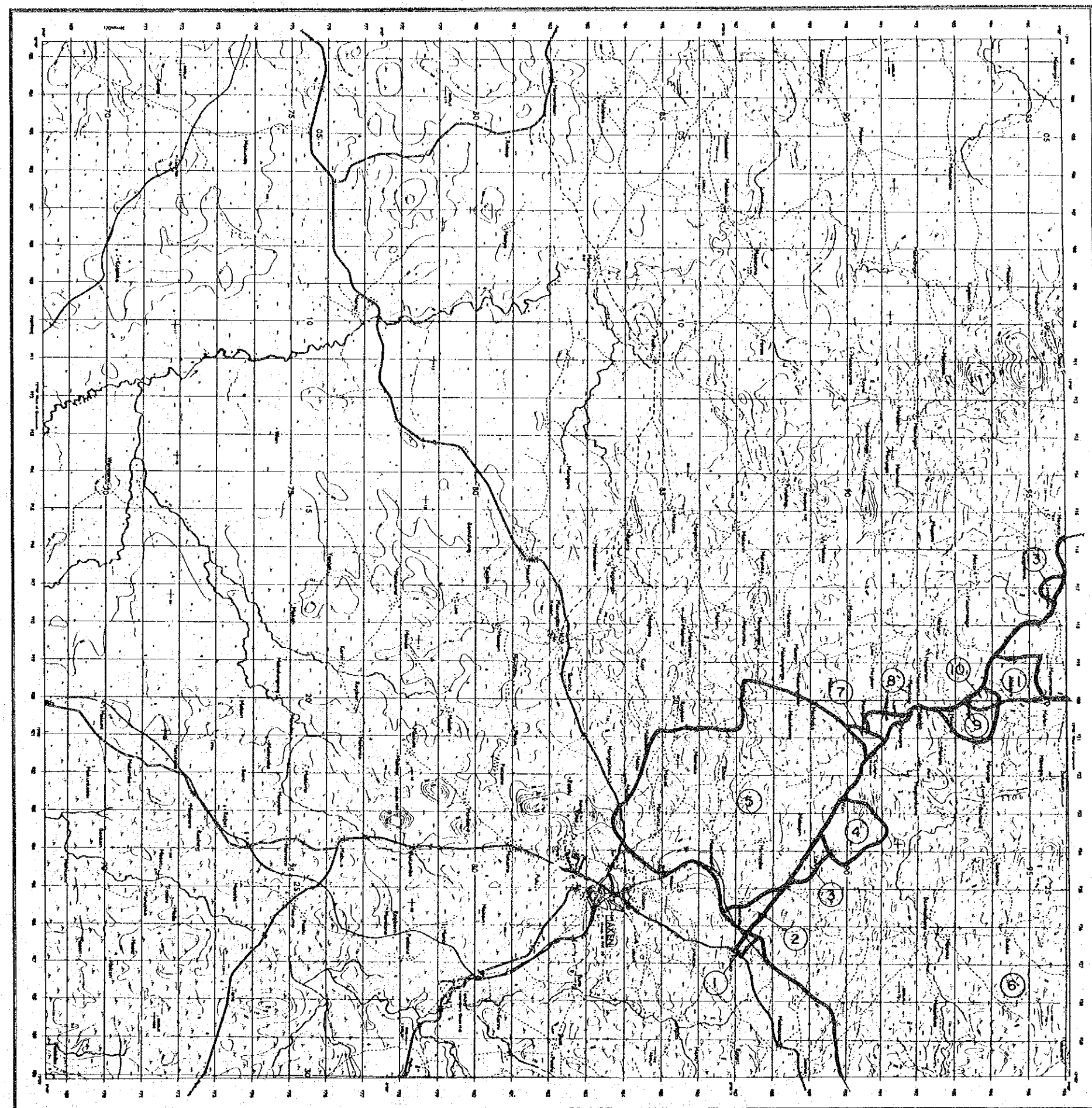
Area of Existing Structure

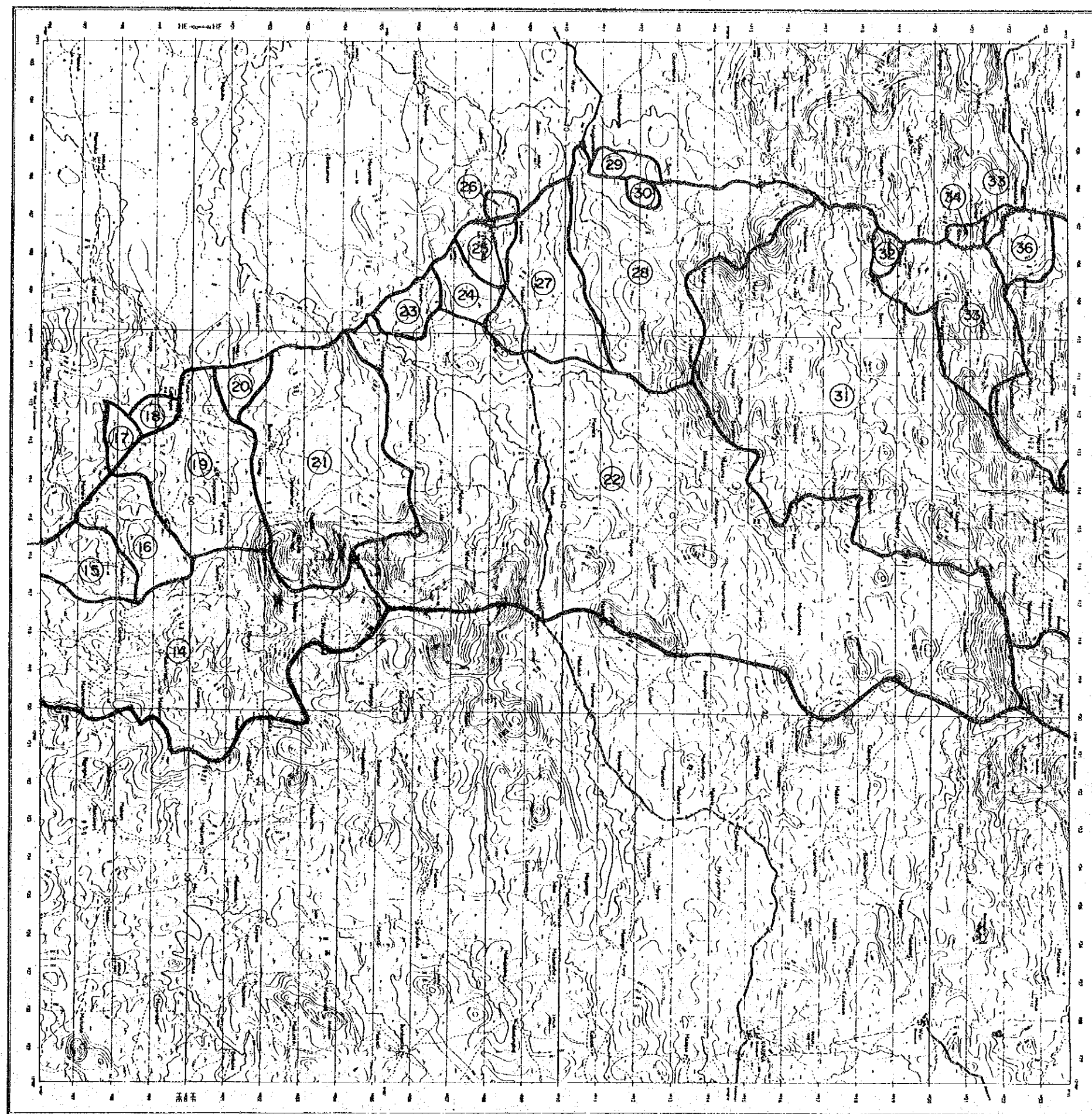
$$Q_c = A_c \cdot V \quad V = \frac{1}{n} \cdot R^{\frac{2}{3}} \cdot I^{\frac{1}{2}}$$

Q<sub>c</sub> : Assumed discharge volume (m<sup>3</sup>/sec.)  
 A<sub>c</sub> : Average velocity at existing structure (m<sup>2</sup>)  
 n : Coefficient factor by material of waterway  
 R : Discharge radius  
 I : Average gradient of river

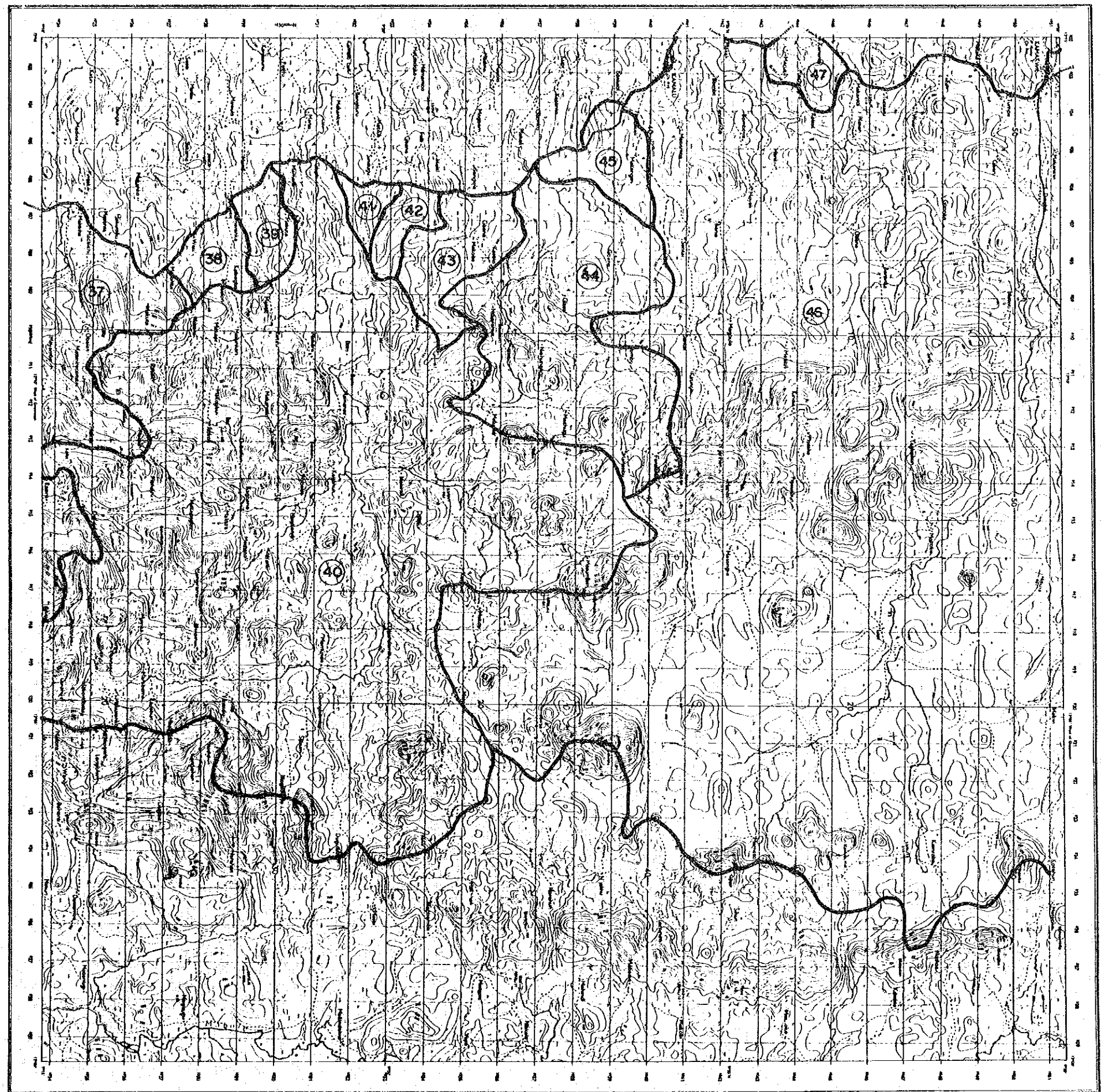
# 5. Calculation of Proposed Discharge Volume

Location Catchment Area				H	L	W	T	$(\frac{24}{T})^{2/3}$	R	$\frac{R}{24}$	Rt	A	f	Q	1/n	$R^{2/2}$	I	$I^{1/2}$	V	Ac	Qc	Proposed discharge volume
Km	Mile	No.	Km <sup>2</sup>	Km	Km	Km/hr	hr		mm		mm/hr	Km <sup>2</sup>		m <sup>3</sup> /sec		m	%		m/sec	M <sup>2</sup>	m <sup>3</sup> /sec	m <sup>3</sup> /sec
5+30	3.3	(4)	18.32	0.010	1.0	4.54	0.22	22.8	180	7.5	171.0	8.00	0.2	76.0	50.	1.02	0.5	0.07	3.57	8.6	30.7	76.0
6+0	3.7	(3)(4)(5)	45.17	0.020	4.0	3.00	1.33	6.9	180	7.5	51.8	45.17	0.2	130.0	50.	1.34	0.5	0.07	4.69	28.0	131.3	131.3
8+00	5.0	(6)(3)(4)(5)	695.02	0.015	15.0	4.54	3.30	3.8	180	7.5	28.5	750.0	0.2	1801.8	50.	2.00	0.5	0.07	7.00	300.0	2100.0	2100.0
9+40	5.9	(9)	0.96	0.003	0.5	3.34	0.15	29.5	180	7.5	221.3	0.96	0.2	11.8	66.6	0.70	0.5	0.07	3.26	2.80	7.3	11.8
10+90	6.8	(11)	1.90	0.004	0.7	3.25	0.22	22.8	180	7.5	171.0	1.90	0.2	18.1	66.6	1.02	0.5	0.07	4.76	8.84	33.7	33.7
13+20	8.3	(14)	32.60	0.015	3.0	3.00	1.00	8.3	180	7.5	62.3	32.60	0.2	112.8	50.	1.30	0.5	0.07	4.55	19.8	90.1	112.8
16+20	10.1	(15)	3.10	0.005	1.0	3.00	0.33	17.4	180	7.5	130.7	3.10	0.2	22.5	50.	0.96	0.5	0.07	3.36	7.2	24.2	24.2
17+00	10.6	(16)	5.72	0.006	1.2	3.00	0.40	15.3	180	7.5	114.8	5.72	0.2	36.5	50.	1.03	0.5	0.07	3.60	9.0	32.4	36.5
22+20	13.9	(17)(18)(19)	13.48	0.013	2.7	2.93	0.92	8.8	180	7.5	66.0	13.48	0.2	49.4	50.	1.08	0.5	0.07	3.78	13.5	51.0	51.0
23+40	14.5	(20)	1.68	0.005	0.8	3.43	0.23	22.2	180	7.5	166.5	1.68	0.2	15.5	76.9	0.28	1.0	0.1	2.15	0.28	2.4	15.5
26+30	16.4	(21)	21.46	0.006	1.5	2.62	0.57	12.1	180	7.5	90.8	21.46	0.2	108.3	50.	1.34	0.5	0.07	4.69	28.0	131.3	131.3
27+20	17.0	(22)	96.96	0.024	4.0	3.34	1.20	7.4	180	7.5	55.5	96.96	0.2	299.0	50.	1.45	0.5	0.07	5.08	56.0	284.5	299.0
27+70	17.3	(23)	1.82	0.003	0.8	2.52	0.32	17.8	180	7.5	133.5	1.82	0.2	13.5	50.	1.02	0.5	0.07	3.57	8.6	30.7	30.7
30+20	18.9	(24)	2.24	0.005	1.0	3.00	0.33	17.4	180	7.5	130.5	2.24	0.2	16.2	50.	1.04	0.5	0.07	3.64	9.0	32.8	32.8
32+00	20.0	(25)	1.84	0.004	0.8	3.00	0.27	19.9	180	7.5	149.3	1.84	0.2	15.3	50.	0.84	0.5	0.07	2.94	5.5	16.2	16.2
32+60	20.4	(26)(25)	2.50	0.005	1.0	3.00	0.33	17.4	180	7.5	130.5	2.50	0.2	18.1	66.6	0.93	0.5	0.07	4.34	6.6	22.9	22.9
34+00	21.3	(27)(26)(25)	11.70	0.010	2.0	3.00	0.67	10.9	180	7.5	81.8	9.20	0.2	59.9	50.	1.30	0.5	0.07	4.55	18.0	81.9	81.9
35+10	21.9	(28)(29)(30)	23.16	0.013	2.5	3.07	0.82	9.5	180	7.5	71.3	20.95	0.2	100.1	50.	1.28	0.5	0.07	4.48	17.28	77.4	100.1
35+90	22.4	(29)(30)	2.21	0.006	1.2	3.00	0.40	15.3	180	7.5	114.8	1.70	0.2	17.1	66.6	0.90	0.5	0.07	4.20	6.8	22.8	22.8
37+40	23.3	(30)	0.51	0.003	0.5	3.34	0.15	29.5	180	7.5	221.3	0.51	0.2	6.3	66.6	0.75	0.5	0.07	3.50	3.4	9.5	9.5
43+10	26.6	(31)	62.52	0.021	3.5	3.34	1.05	8.1	180	7.5	60.8	62.52	0.2	211.2	50.	1.55	0.5	0.07	5.43	42.5	230.8	230.8
43+90	27.4	(31)	0.30	0.0015	0.3	3.00	0.10	38.6	180	7.5	289.5	0.30	0.2	4.8	66.6	0.86	0.3	0.05	2.86	6.0	17.2	4.8
44+60	27.9	(32)	1.00	0.002	0.4	3.00	0.13	32.4	180	7.5	243.0	1.00	0.2	13.5	66.6	0.92	0.5	0.07	4.29	7.5	25.7	13.5
45+40	28.4	(33)(34)	11.32	0.007	1.5	2.88	0.52	12.9	180	7.5	96.8	10.86	0.2	65.8	50.	1.41	0.5	0.07	4.93	22.5	110.9	65.8
47+10	29.4	(34)	0.46	0.0015	0.3	3.00	0.10	38.6	180	7.5	289.5	0.46	0.2	7.4	66.6	0.61	0.5	0.07	2.84	1.8	4.1	7.4
49+00	30.6	(35)	0.54	0.001	0.2	3.00	0.10	38.6	180	7.5	289.5	0.54	0.2	8.7	66.6	0.72	0.5	0.07	3.36	3.2	8.6	8.7
49+60	31.0	(36)(35)	3.26	0.016	0.8	6.89	0.12	34.2	180	7.5	256.5	2.72	0.2	47.5	50.	1.00	0.5	0.07	3.50	8.1	28.4	47.5
51+20	32.0	(37)	24.70	0.009	1.6	3.22	0.50	13.2	180	7.5	99.0	24.70	0.2	135.9	50.	1.46	0.5	0.07	5.11	25.8	131.8	135.9
57+00	35.7	(38)	3.72	0.010	1.00	4.54	0.22	22.8	180	7.5	171.0	3.72	0.2	35.3	50.	0.89	0.5	0.07	3.12	6.6	20.6	35.3
58+10	36.1	(39)	3.10	0.005	0.80	3.43	0.23	22.2	180	7.5	166.5	3.10	0.2	28.7	50.	0.95	0.5	0.07	3.33	7.2	24.0	28.7
60+30	37.7	(40)	161.34	0.034	8.50	2.62	3.24	3.8	180	7.5	28.5	161.34	0.2	255.5	50.	1.36	0.5	0.07	4.76	49.5	235.6	255.5
61+50	38.4	(41)	2.08	0.005	1.00	3.00	0.33	17.4	180	7.5	130.5	2.08	0.2	15.1	50.	1.19	0.5	0.07	4.17	5.0	16.7	16.7
64+10	40.0	(42)	2.70	0.006	1.00	3.34	0.30	18.6	180	7.5	139.5	2.70	0.2	20.9	50.	0.81	0.5	0.07	2.84	5.4	15.3	20.9
64+80	40.5	(43)	7.08	0.008	1.60	3.00	0.53	12.7	180	7.5	95.3	7.08	0.2	37.5	50.	1.11	0.5	0.07	3.89	12.06	46.9	46.9
67+00	41.9	(44)	34.32	0.017	3.40	3.00	1.13	7.7	180	7.5	57.8	34.32	0.2	110.2	50.	1.36	0.5	0.07	4.76	30.0	142.8	142.8
67+40	42.1	(45)	5.60	0.006	1.20	3.00	0.40	15.3	180	7.5	114.8	5.60	0.2	35.7	50.	0.87	0.5	0.07	3.05	5.25	16.0	35.7
74+20	46.4	(46)	319.64	0.035	8.80	2.61	3.37	3.7	180	7.5	27.8	319.64	0.2	493.7	50.	1.67	0.5	0.07	5.85	80.0	468.0	493.7
76+30	47.4	(47)	3.22	0.005	1.00	3.00	0.33	17.4	180	7.5	130.5	3.22	0.2	23.3	66.6	0.61	0.5	0.07	2.84	1.92	4.4	23.3
53+30	33.3	(37)	1.00	0.001	0.20	3.00	0.10	38.6	180	7.5	289.5	1.00	0.2	16.1	66.6	0.84	0.5	0.07	3.92	5.28	16.6	16.6
3+40	2.1	(3)	0.44	0.0015	0.30	3.00	0.10	38.6	180	7.5	289.5	0.44	0.2	7.1	76.9	0.28	1.0	0.1	2.15	0.28	1.4	7.1









## APPENDIX R CALCULATION OF PAVEMENT THICKNESS

### 1. Future Traffic Volume between Panlap and Mabole

Unit: ADT

	1985		1995		2010	
	Number	rate	Number	rate	Number	rate
Cars	50	1.00	116	2.32	394	7.88
Pick-ups & Vans	99	1.00	164	1.66	326	3.29
Trucks & Buses	68	1.00	113	1.66	223	3.28
Extra heavy vehicles	23	1.00	39	1.70	52	2.26
Total	240	1.00	432	1.80	995	4.15

Source : JICA Mission.

### 2. Cumulative Axle Loads for ten years

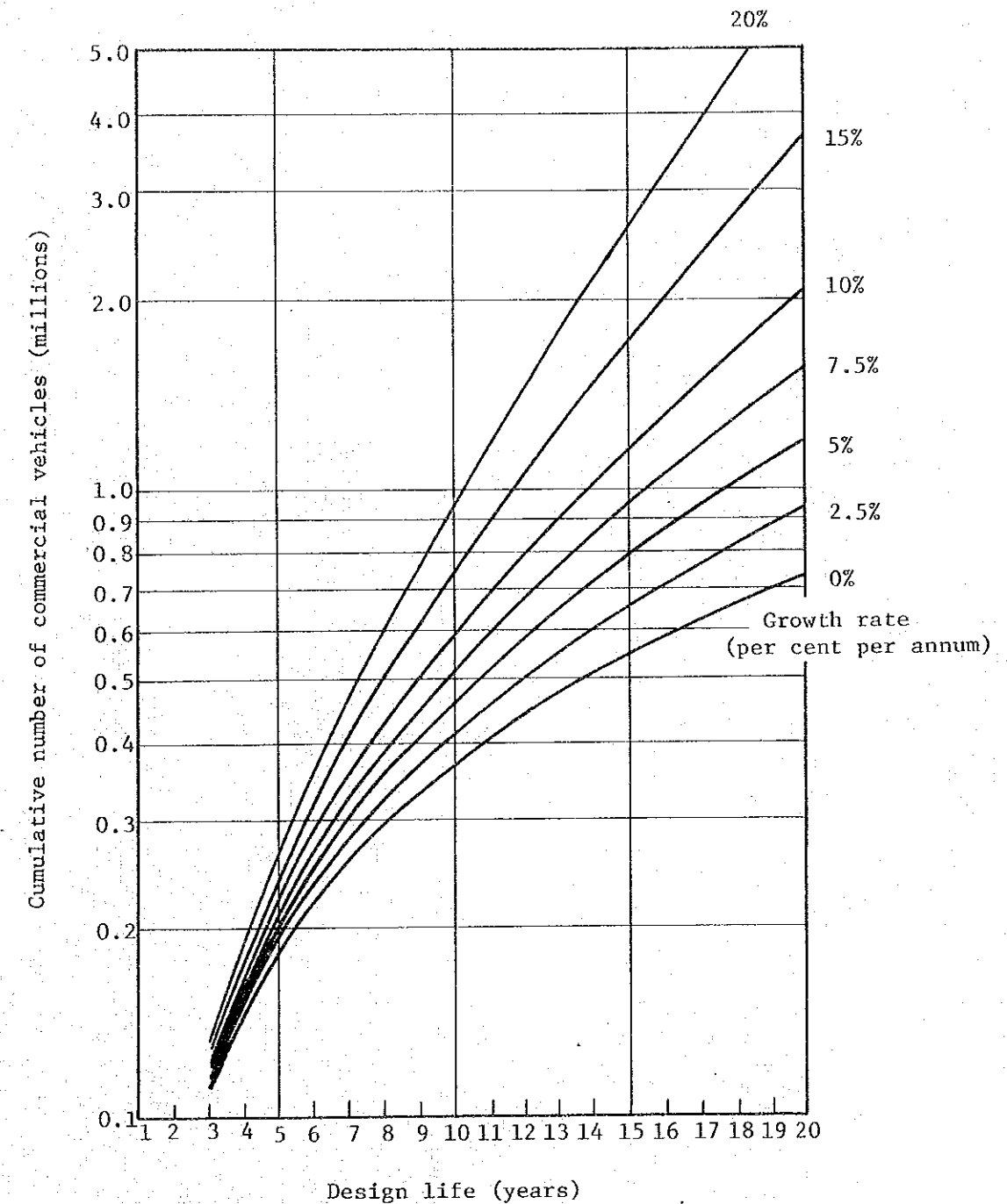
	Traffic Volume		One direction/ 100	Cumula- tive Vehicles No./100 L1	Actual Cumula- tive Vehicles Number	EQUIVA- lent Factor L2	Design Cumulative Number
	Both of Direction	One Direction					
	①	②=① x 2/3	③=②/100	④	⑤ = ③ x ④	⑥	⑦ = ⑤ x ⑥
Cars	50	33	0.33	0.65	0.2145	0.0002	0.00004
Pick-ups & Vans	99	66	0.66	0.49	0.3234	0.0025	0.00081
Trucks & Buses	68	45	0.45	0.48	0.2160	0.08	0.01728
Extra heavy Vehicles	23	15	0.15	0.50	0.0750	1.00	0.0750
Total	240	159	-				0.09313

Note : L1 is calculated by Fig. 1.

L2 is depended on "British Note 31 page 7".

Source : JICA Mission.

Fig. 1



RELATION BETWEEN TOTAL NUMBER OF VEHICLES USING A ROAD DURING THE DESIGN LIFE AND GROWTH RATE FOR AN AVERAGE DAILY TRAFFIC OF 100 COMMERCIAL VEHICLES PER DAY IN THE INITIAL YEAR

Source : British note 31

### 3. Calculation of design C B R Value

Sample No.	1	2	3	4	5	6	7	8	9	10	Average
C B R Value	43	160	130	98	100	74	87	26	46	50	65.5

Source : JICA Mission

$$\begin{aligned}
 \text{Design C B R} &= \text{Average C B R} - \frac{\text{Maximum C B R} - \text{Minimum C B R}}{d} \\
 &= 65.5 - \frac{100 - 26}{2.96} \\
 &= 40.5 > 25
 \end{aligned}$$

Note : "d" is a coefficient value on the basis of numbers of C B R Test Values.

### 5. Pavement Structure

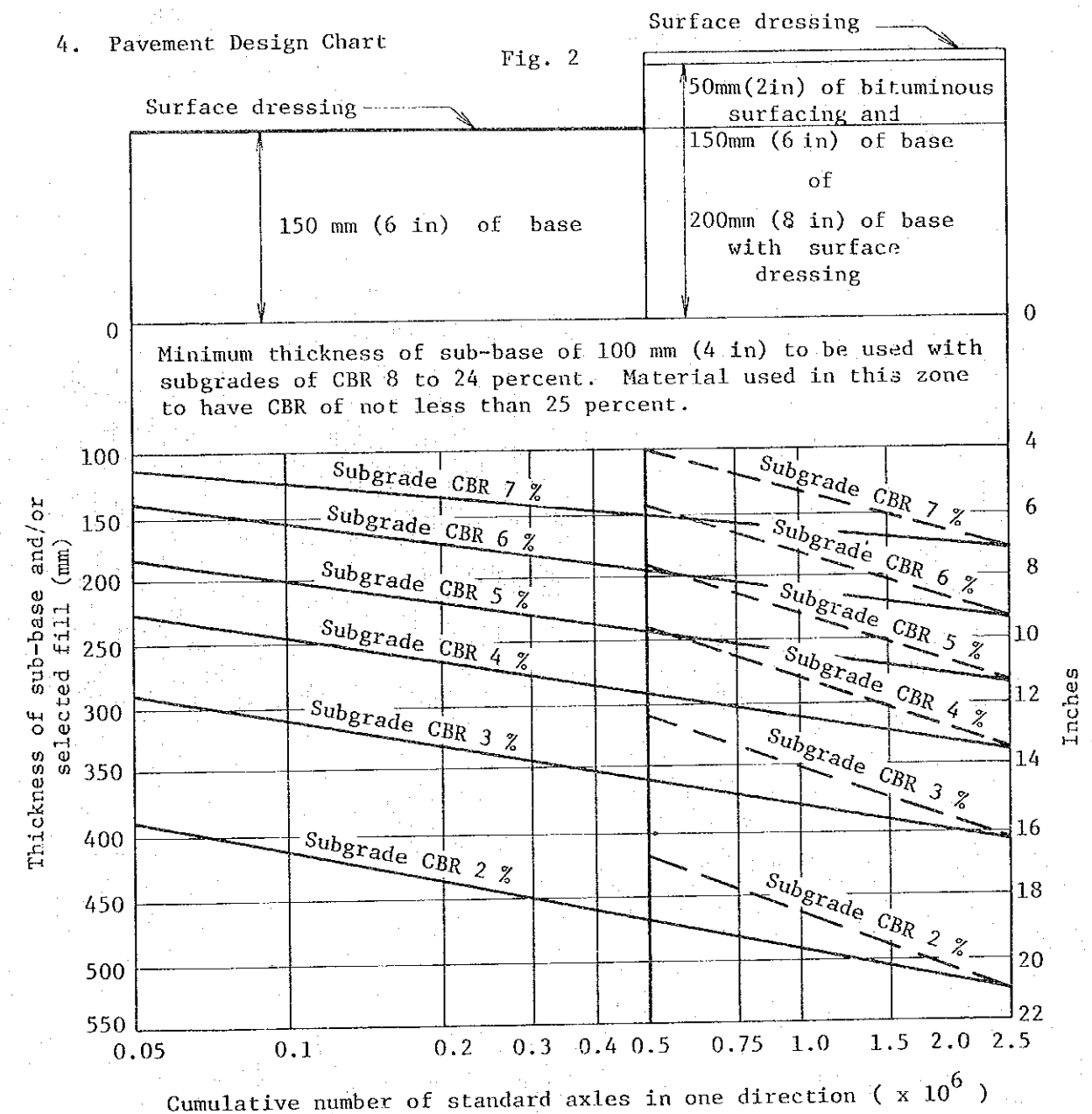
The thickness of each layer is calculated on the basis of Design C B R and Design cumulative number of standard axles from Fig. 2 Pavement Design Chart for flexible pavement.

The thickness of each layer is as follows.

Sub base course	4 inch
Base course	8 inch
Surface dressing	2-coat 3/4 inch + 1/2 inch chipping.

### 4. Pavement Design Chart

Fig. 2



If it is desired to provide at the time of construction a pavement capable of carrying more than 0.5 million standard axles the designer may choose either a 150 mm (6 in) base with a 50 mm (2 in) bituminous surfacing or a 200 mm (8 in) base with a double surface dressing. For both of these alternatives, the recommended sub-base thickness is indicated by the broken line.

Alternatively, a base 150 mm (6 in) thick with double surface dressing may be laid initially and the thickness increased when 0.5 million standard axles have been carried. The extra thickness may consist of 50 mm (2 in) of bituminous surfacing or at least 75 mm (3 in) of crushed stone with double surface dressing. The largest aggregate size in the crushed stone must not exceed 19 mm (3/4 in) and the old surface must be prepared by scarifying to a depth of 50 mm (2 in). For this stage construction procedure, the recommended thickness of sub-base is indicated by the solid line.

Source : British Note 31



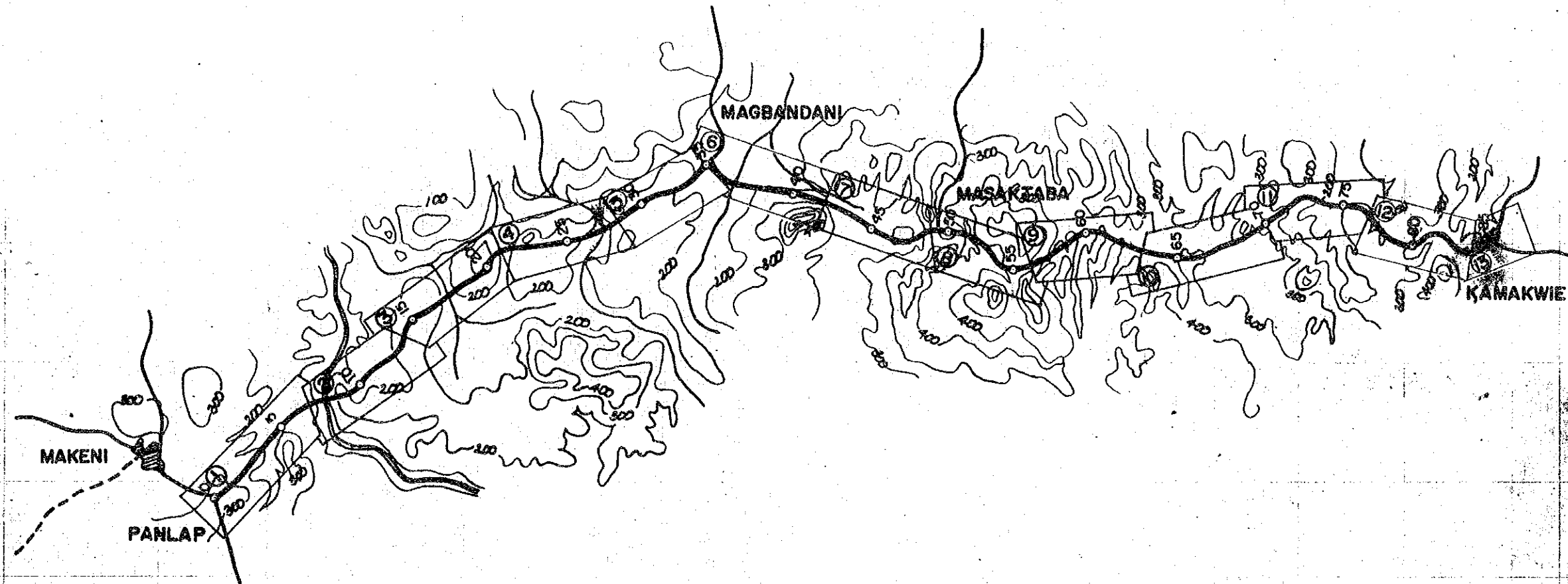
# APPENDIX S-I / PLAN AND PROFILE OF ALTERNATIVE PLAN A

MAKENI - KAMAKWIE ROAD PROJECT  
FEASIBILITY STUDY

COVER SHEET (A)

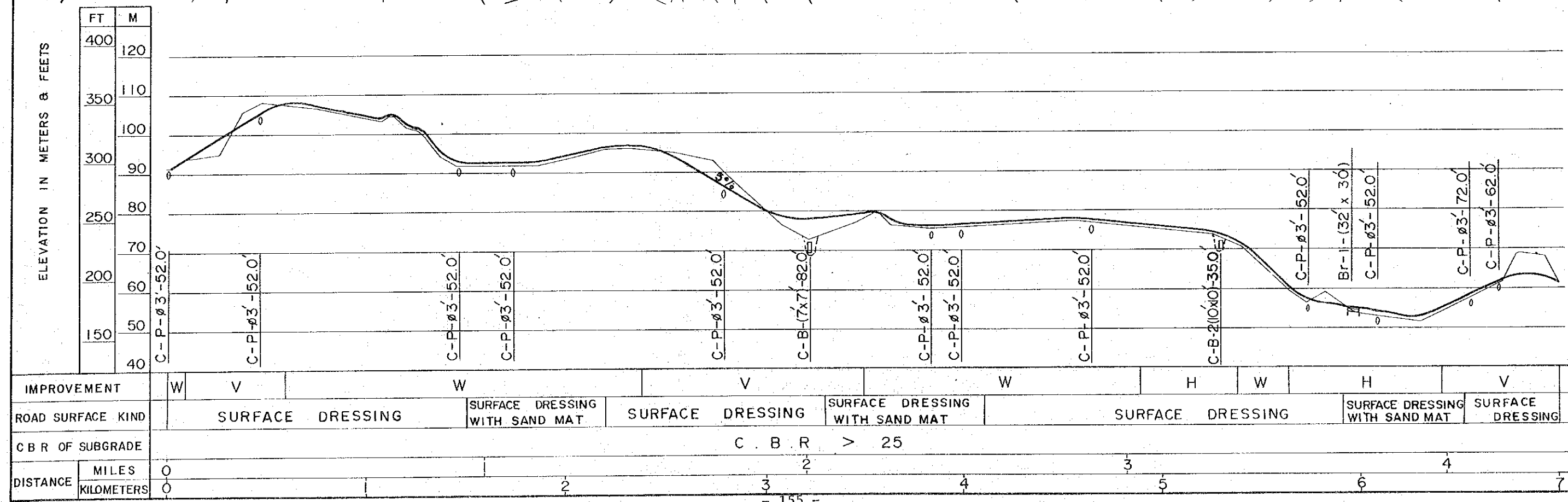
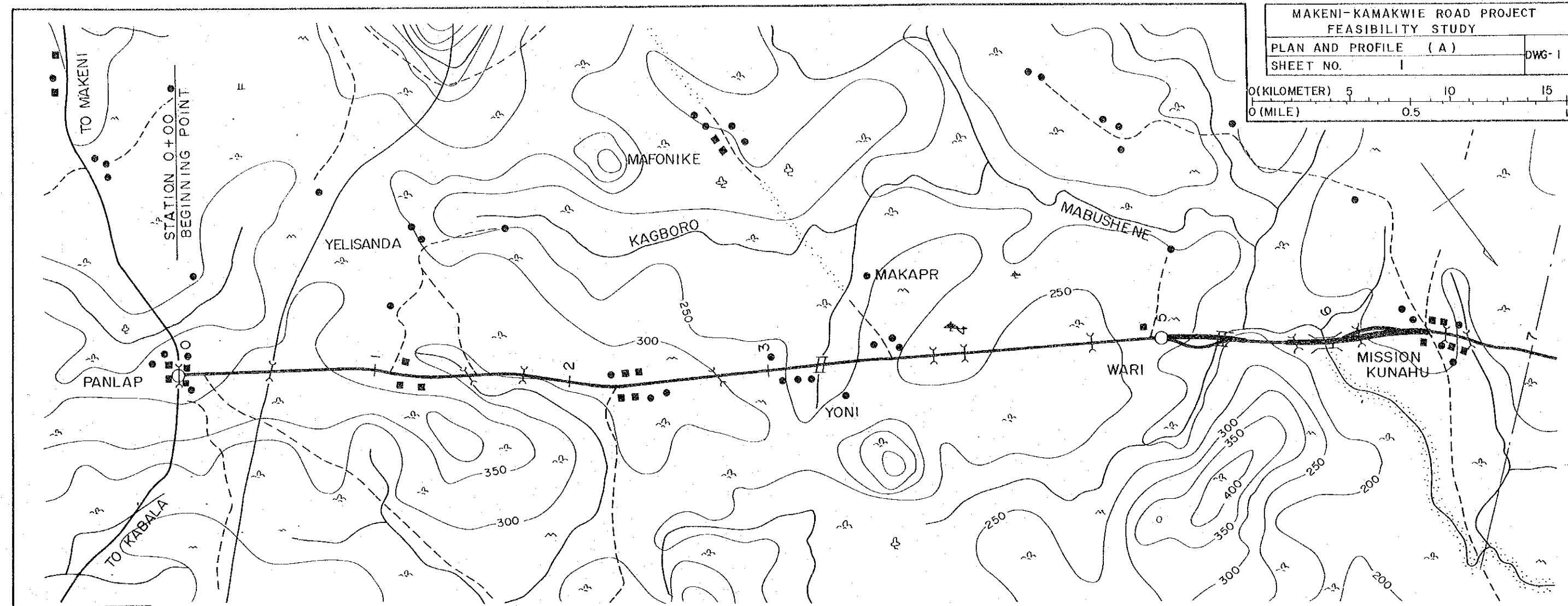
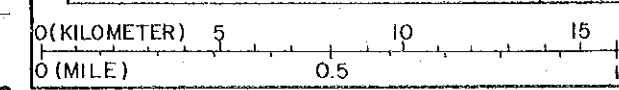
SHEET NO.

DWG-



## ABBREVIATIONS

H	RE-ALIGNMENT, HORIZONTAL FOR ROAD IMPROVEMENT
V	RE-ALIGNMENT, VERTICAL FOR ROAD IMPROVEMENT
W	WIDENING OF ROAD WIDTH FOR ROAD IMPROVEMENT
C - P - $\phi a - l$	PROPOSED PIPE CULVERT, $\phi a$ (DIAMETER, FOOT), $l$ (LENGTH, FOOT)
C - B (a x b) - $l$	PROPOSED BOX CULVERT, a x b (WIDTH x LENGTH ALONG THE ROAD), $l$ (CULVERT LENGTH)
C - B - n(a x b) - $l$	PROPOSED BOX CULVERT, n (ROW), a x b (WIDTH x LENGTH ALONG THE ROAD), $l$ (CULVERT LENGTH)
Br - n - (a x b)	PROPOSED PRESTRESSED CONCRETE BRIDGE, n (NOS. OF SPAN), a x b (WIDTH x SPAN LENGTH)



MAKENI-KAMAKWIE ROAD PROJECT

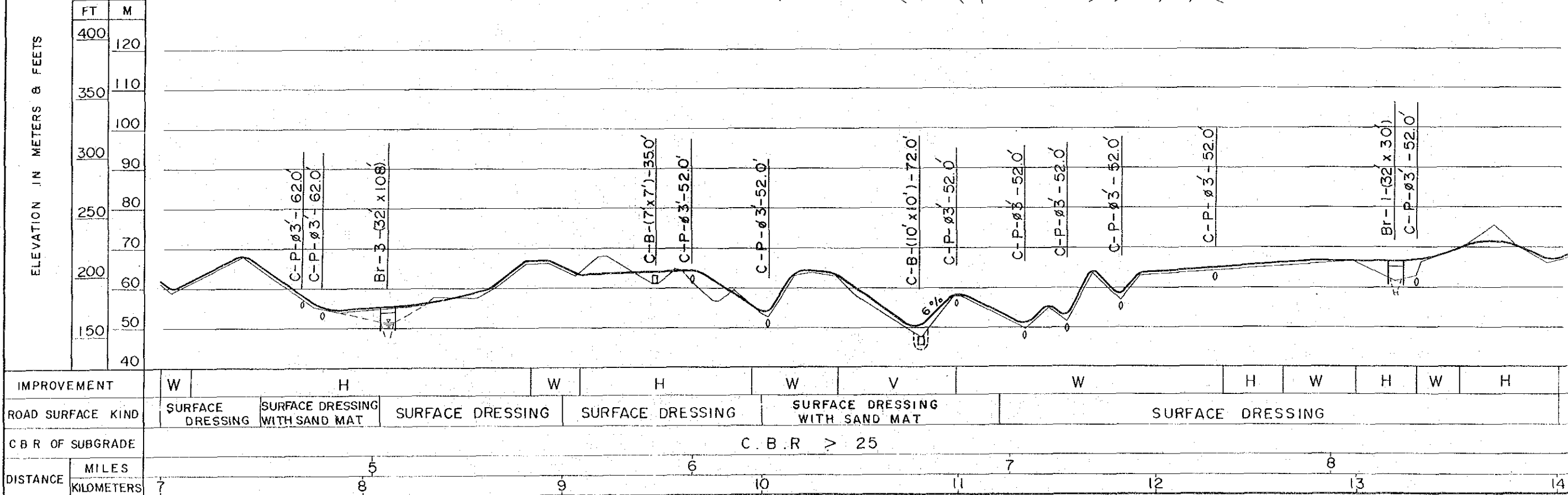
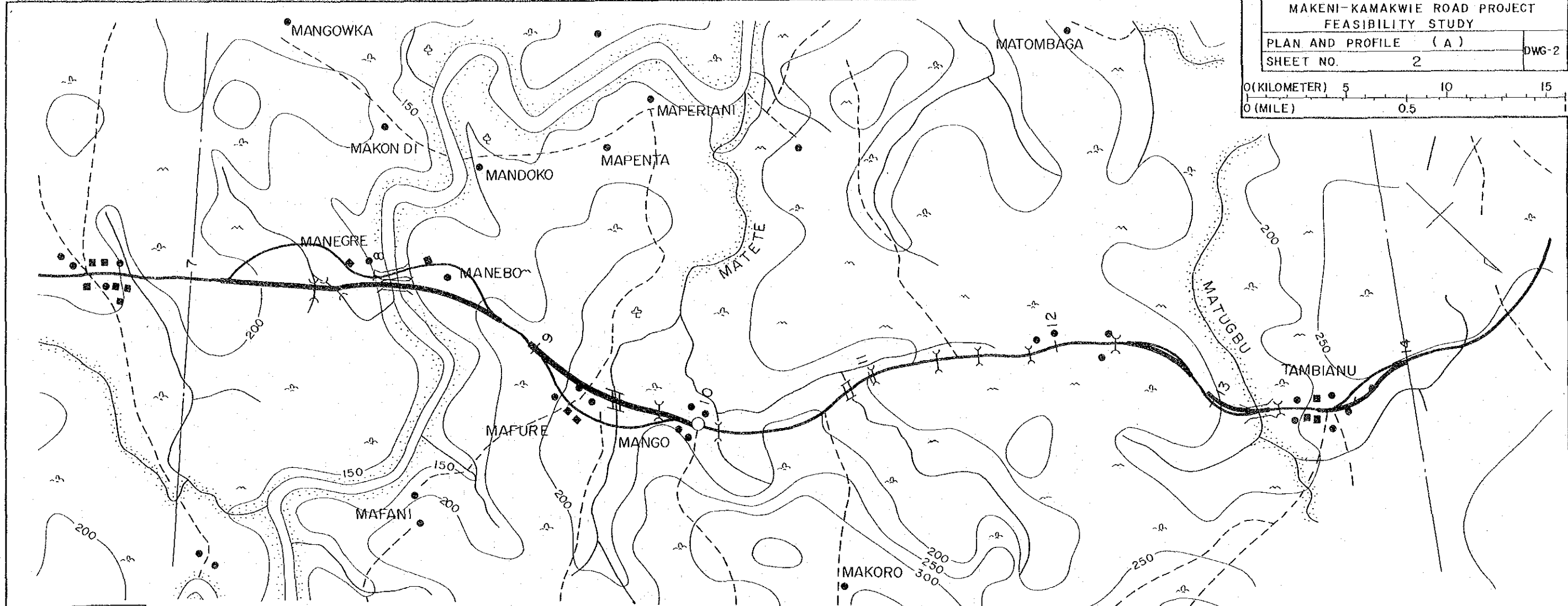
FEASIBILITY STUDY

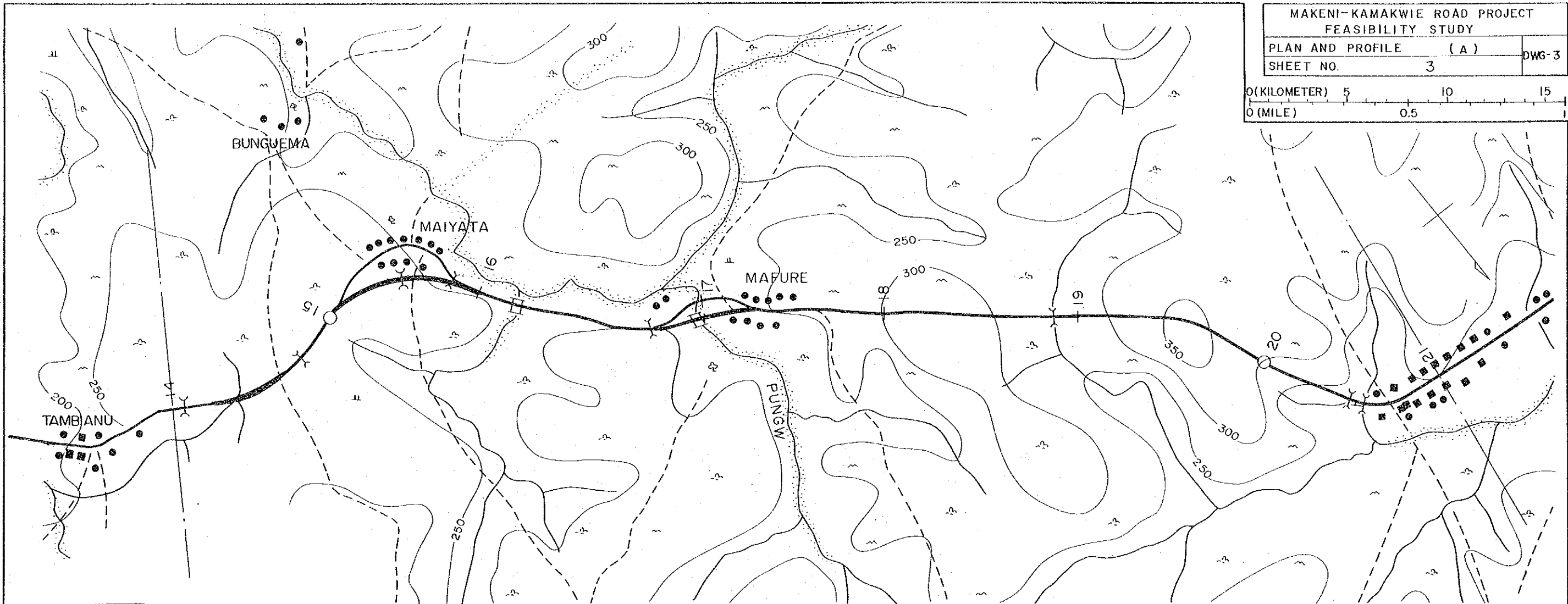
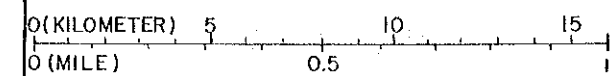
PLAN AND PROFILE (A)

SHEET NO. 2

DWG-2

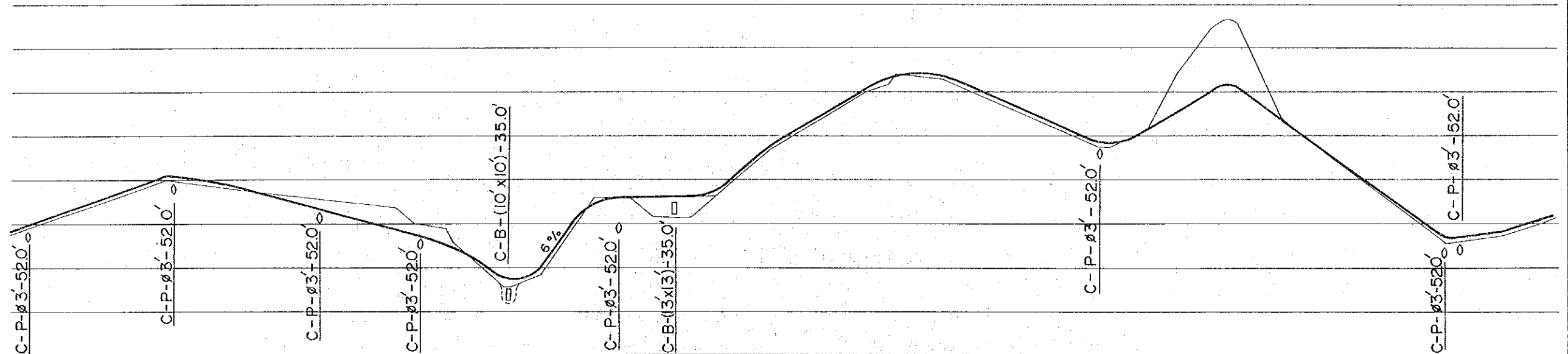
0 (KILOMETER) 5 10 15  
0 (MILE) 0.5 1



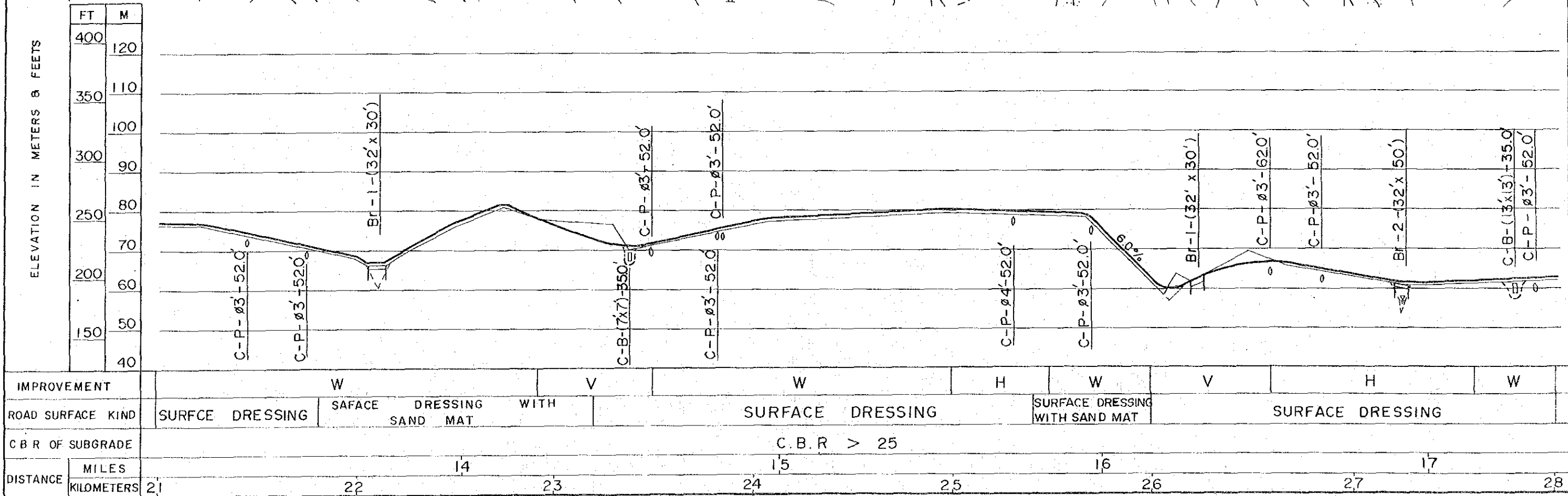
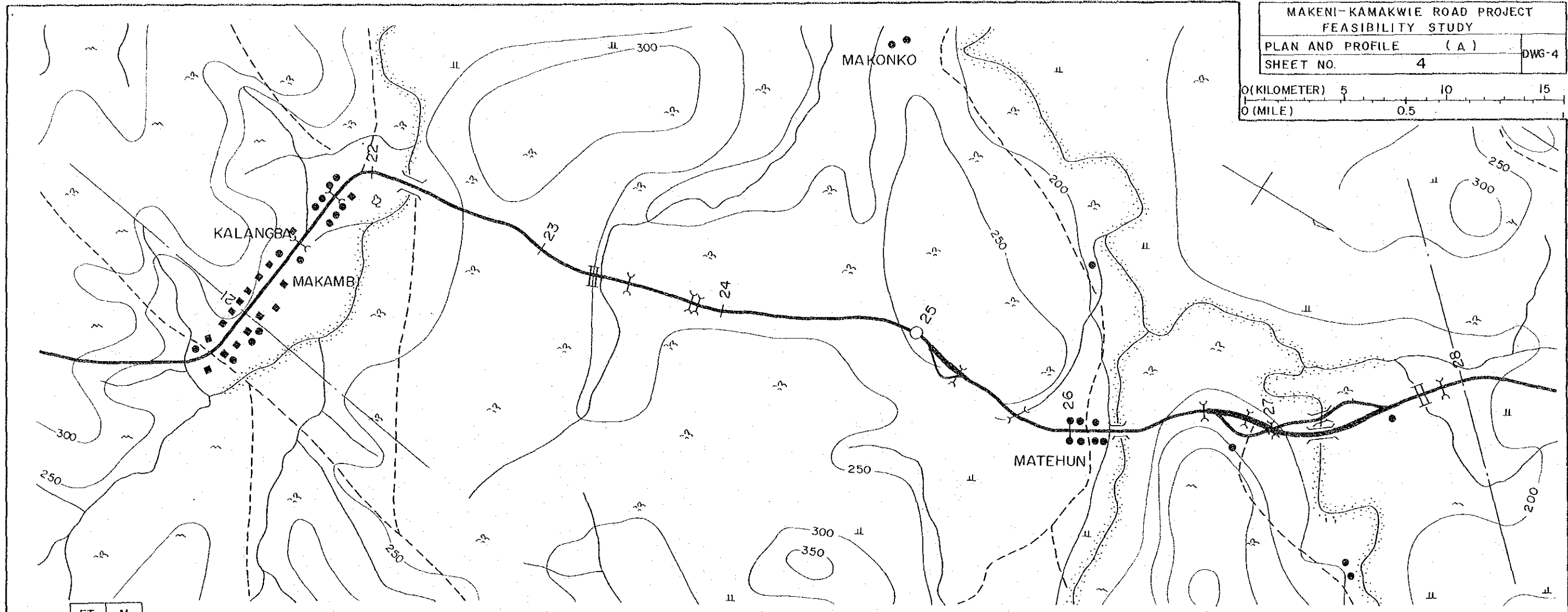
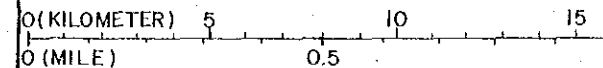


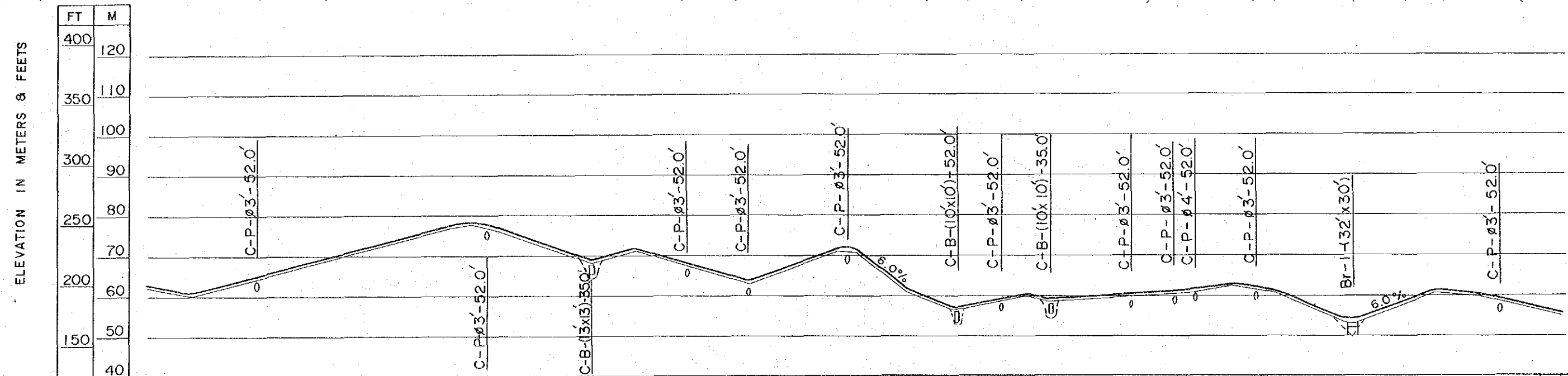
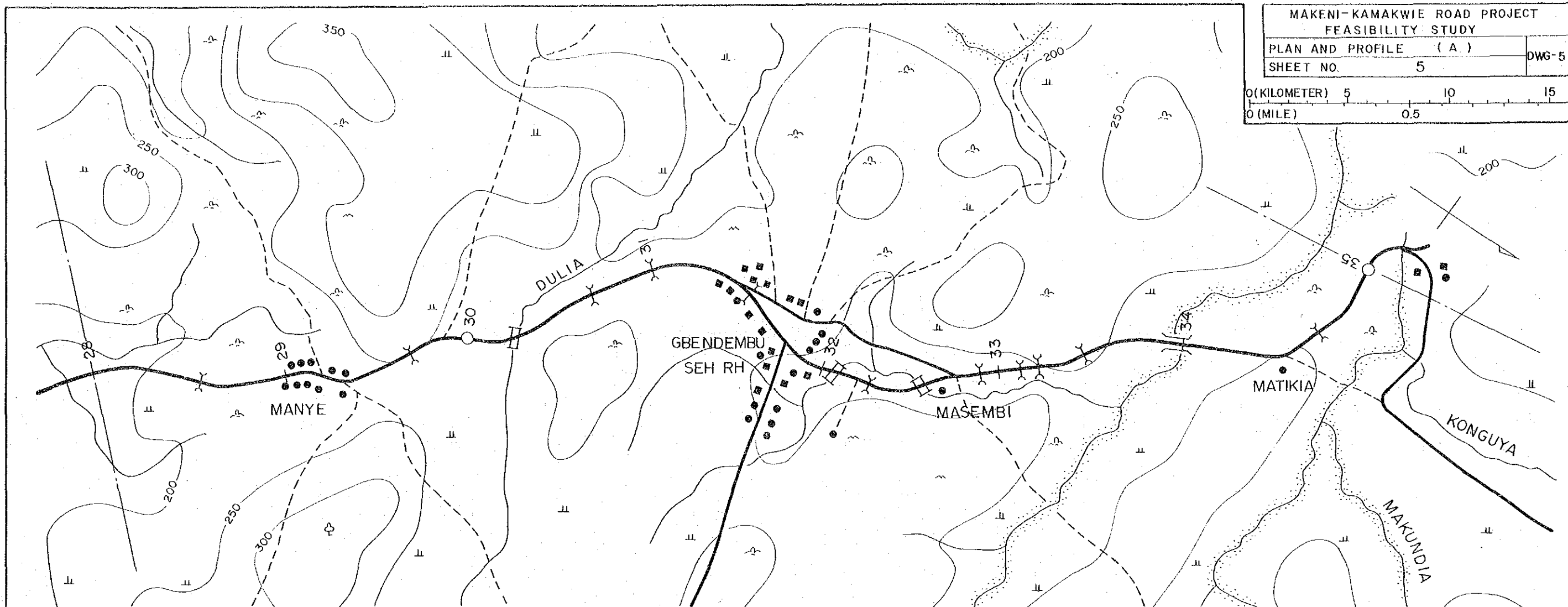
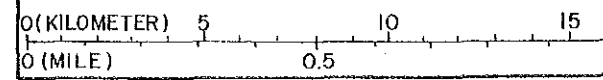
ELEVATION IN METERS & FEET

FT	M
400	120
350	110
300	100
250	90
200	80
150	70
100	60
50	50
40	40



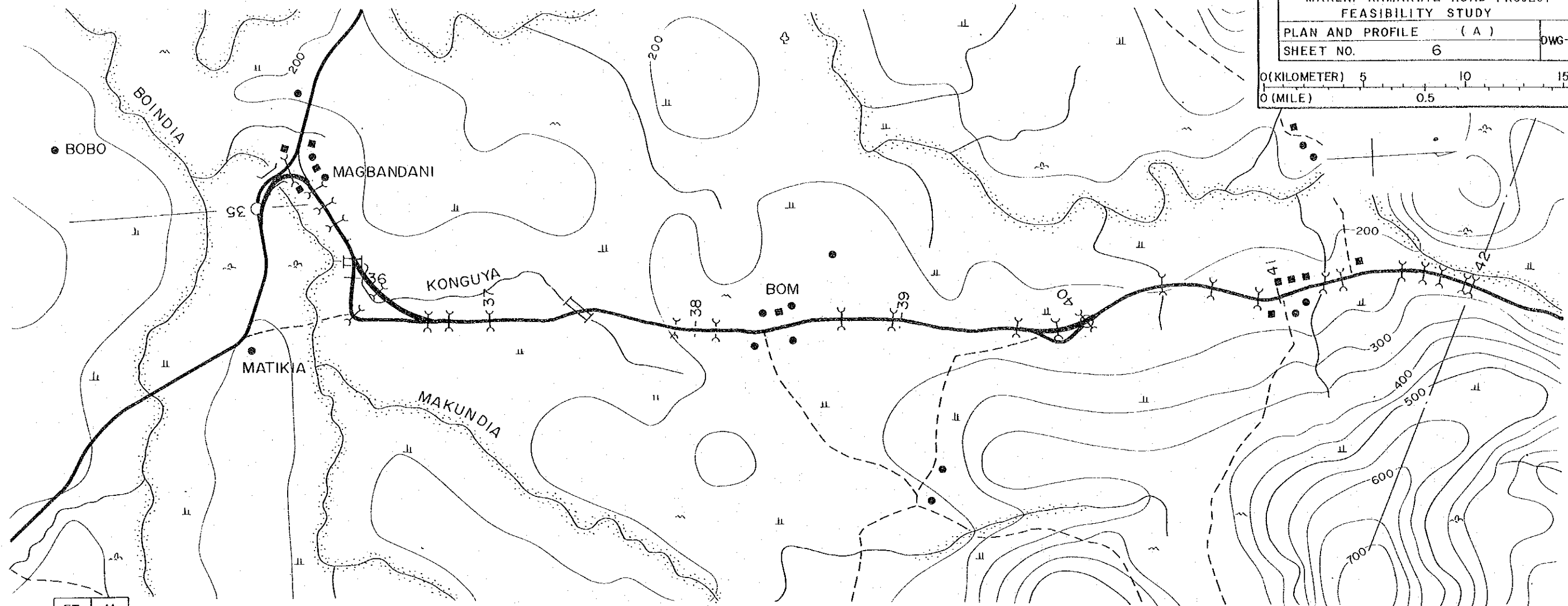
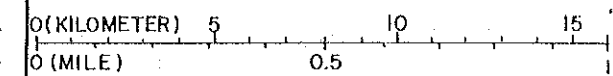
IMPROVEMENT		W	H	W	H	W	H	W	V	W	V	W				
ROAD SURFACE KIND		SURFACE DRESSING			SURFACE DRESSING WITH SAND MAT		SURFACE DRESSING				SURFACE DRESSING WITH SAND MAT	SURFACE DRESSING	SURFACE DRESSING WITH SAND MAT	SURFACE DRESSING		
C B R OF SUBGRADE		C . B . R > 25														
DISTANCE	MILES	9			10			11			12			13		
	KILOMETERS	14	15			16			17			18			19	20





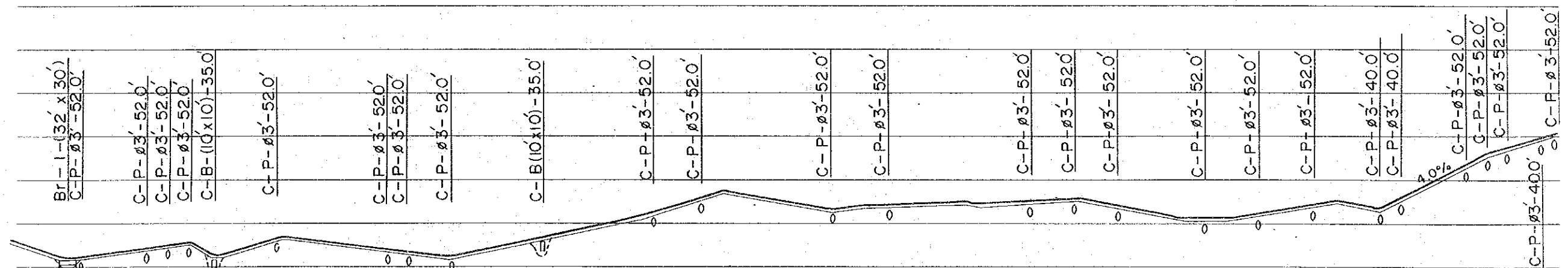
IMPROVEMENT		W						H
ROAD SURFACE KIND		SURFACE DRESSING	SURFACE DRESSING WITH SAND MAT	SURFACE DRESSING	SURFACE DRESSING WITH SAND MAT	SURFACE DRESSING	SURFACE DRESSING WITH SAND MAT	SURFACE DRESSING
C.B.R. OF SUBGRADE		C.B.R. > 25						
DISTANCE	MILES	18	19	20	21	22	23	24
	KILOMETERS	28	29	30	31	32	33	34



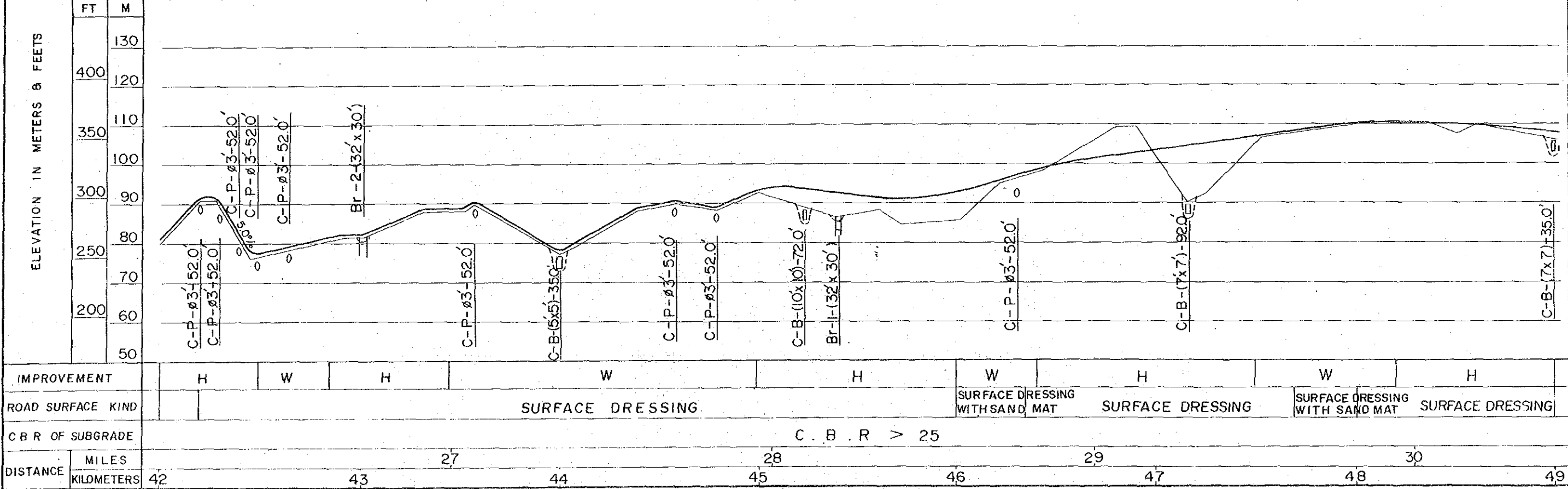
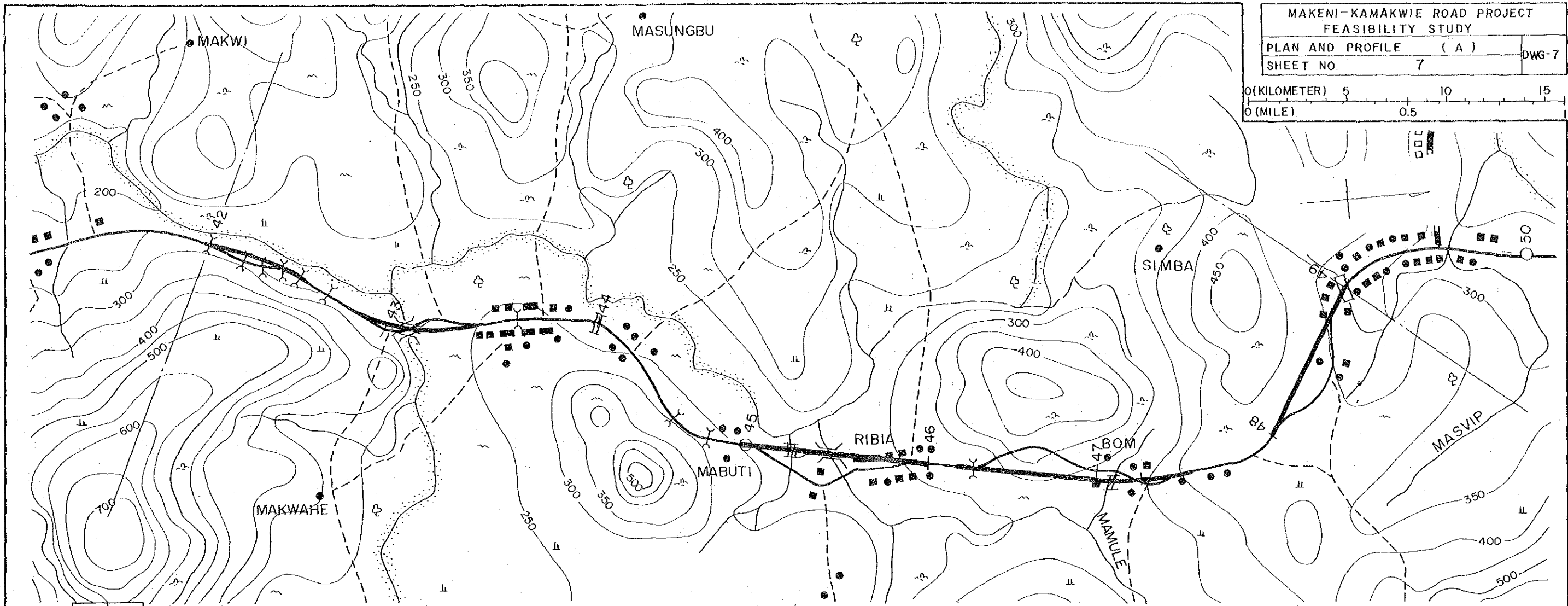
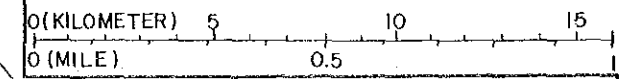


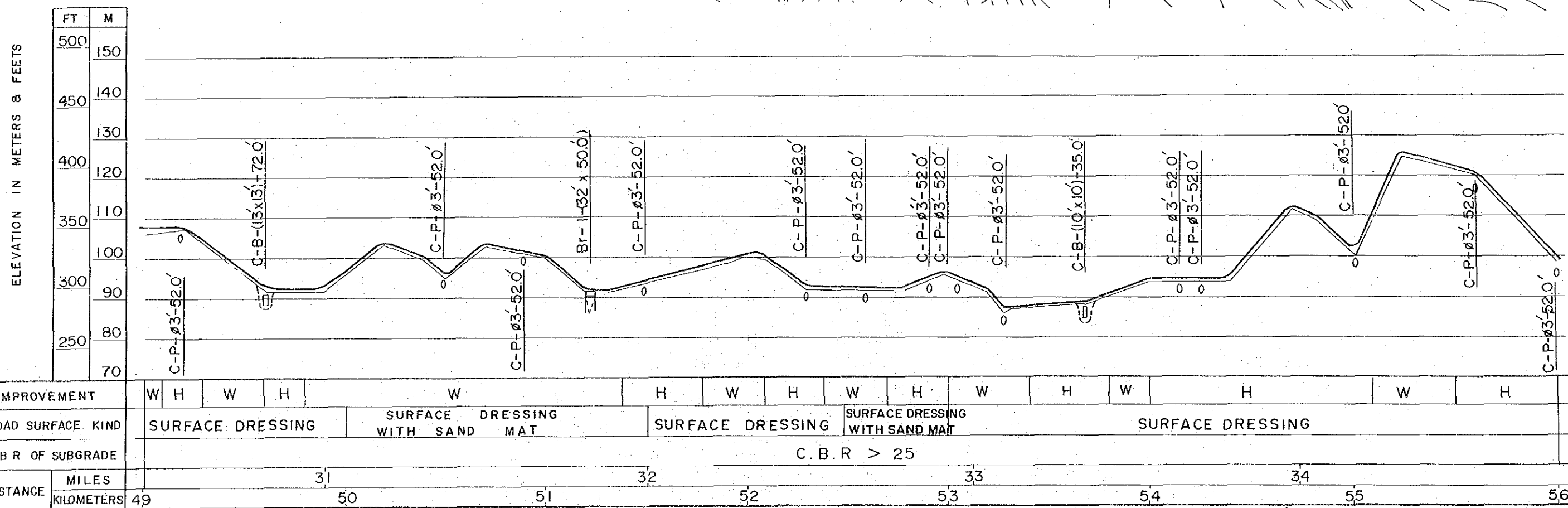
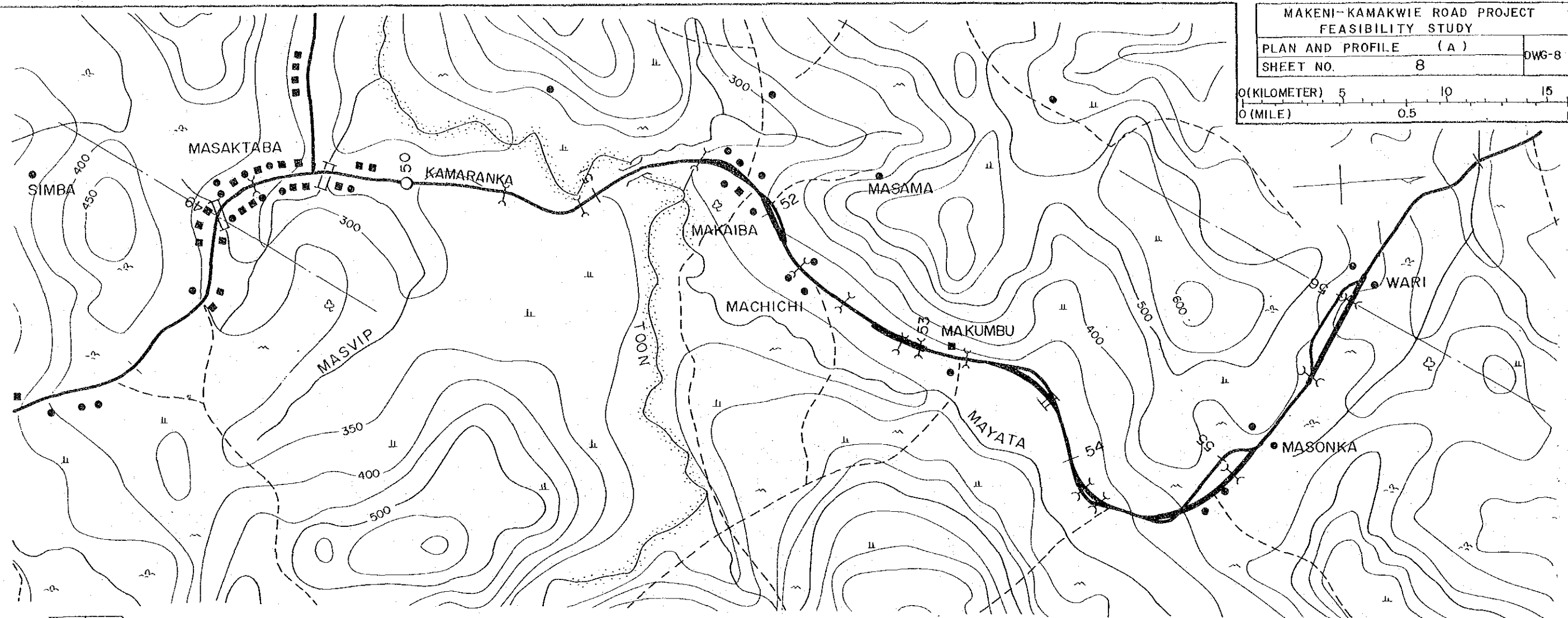
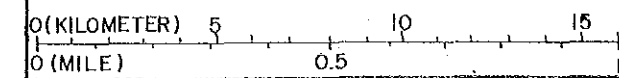
ELEVATION IN METERS & FEET

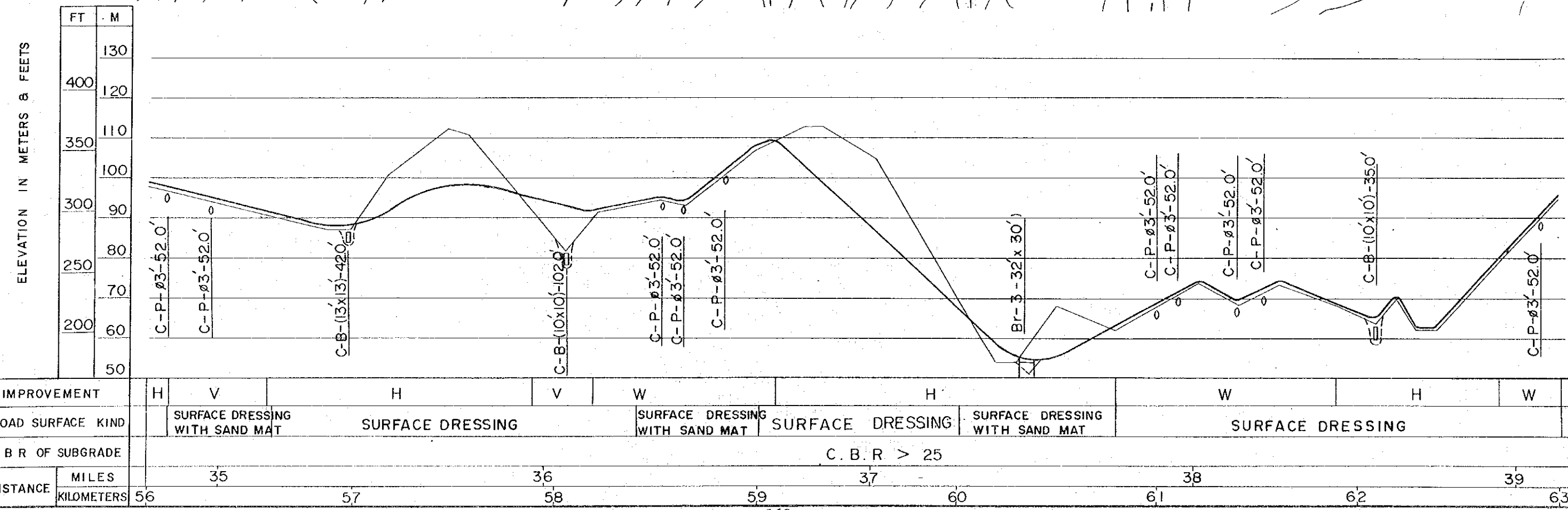
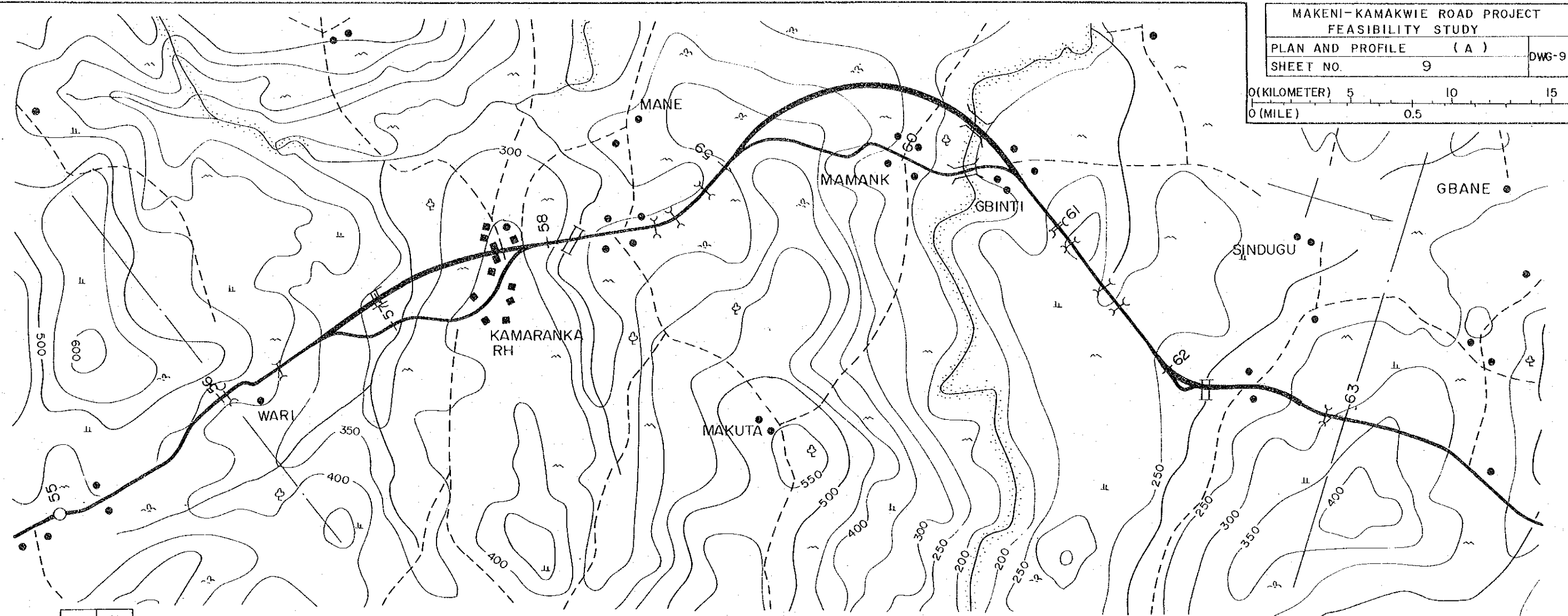
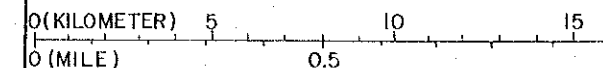
FT	M
400	120
350	110
300	100
250	80
200	60
150	50
40	

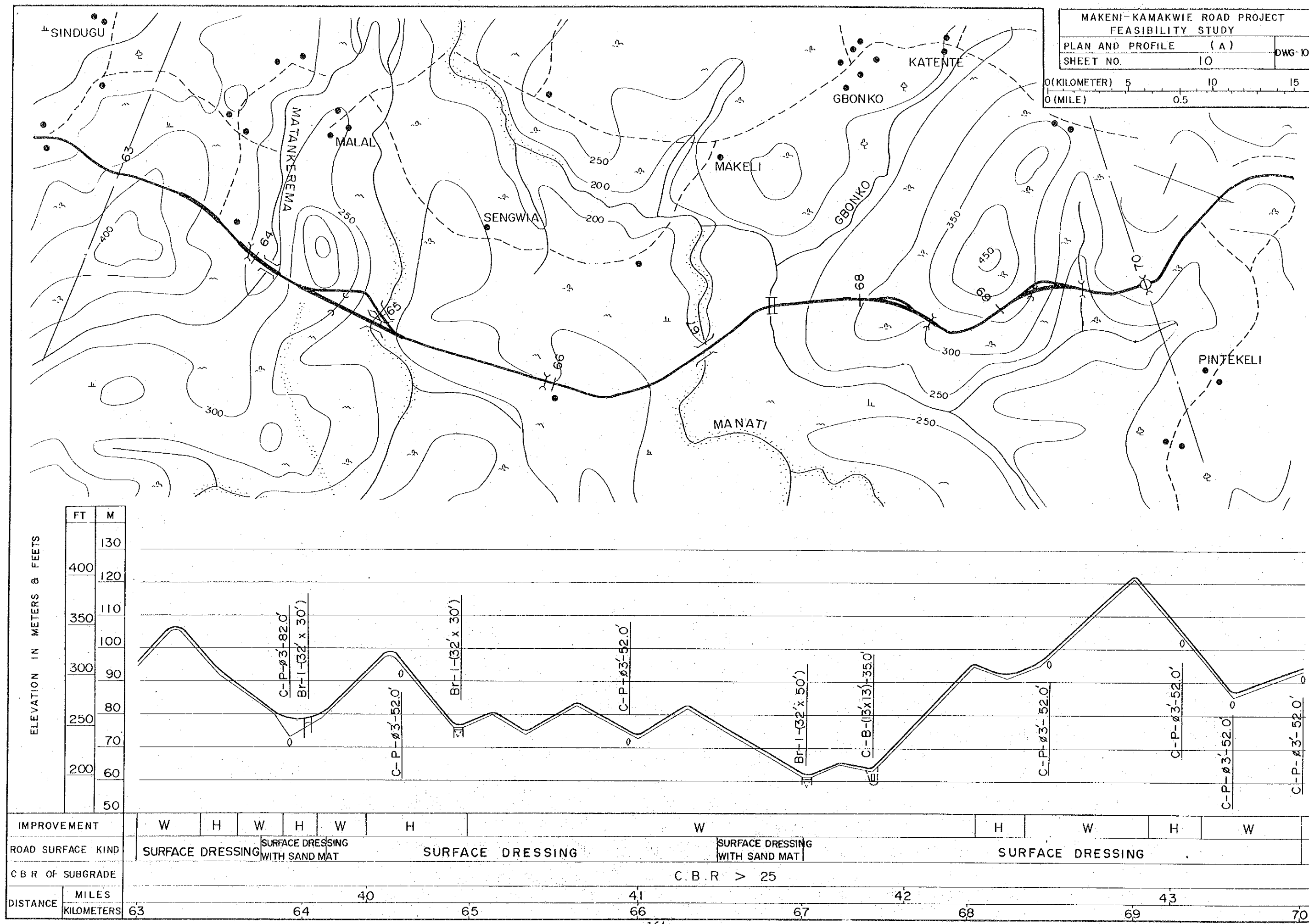


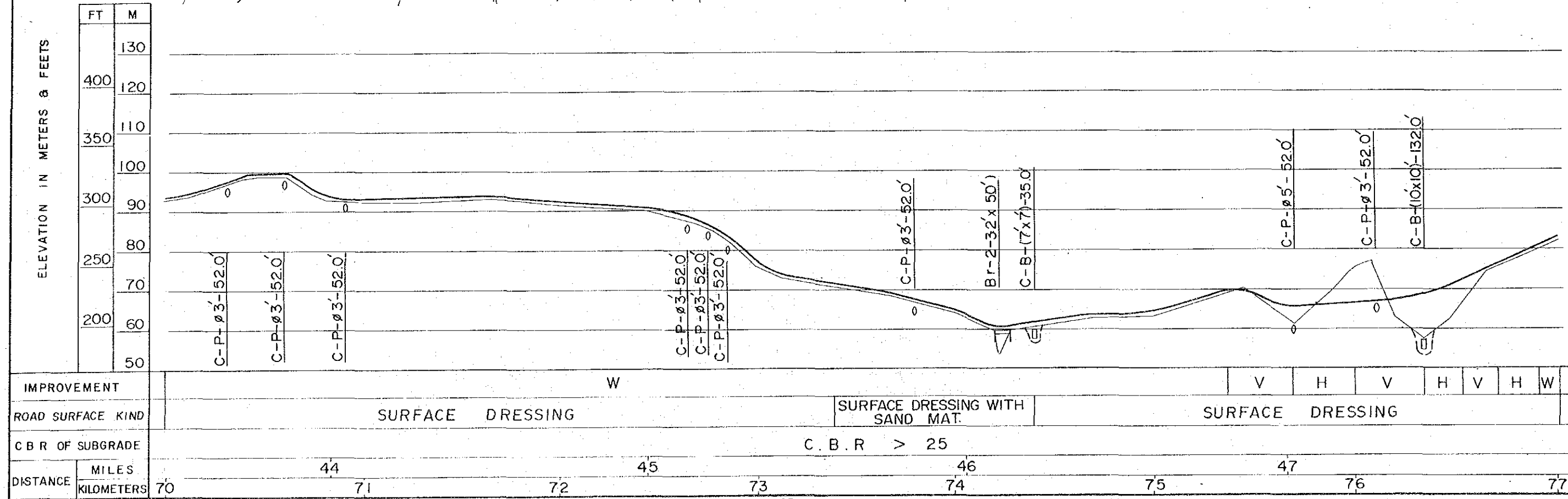
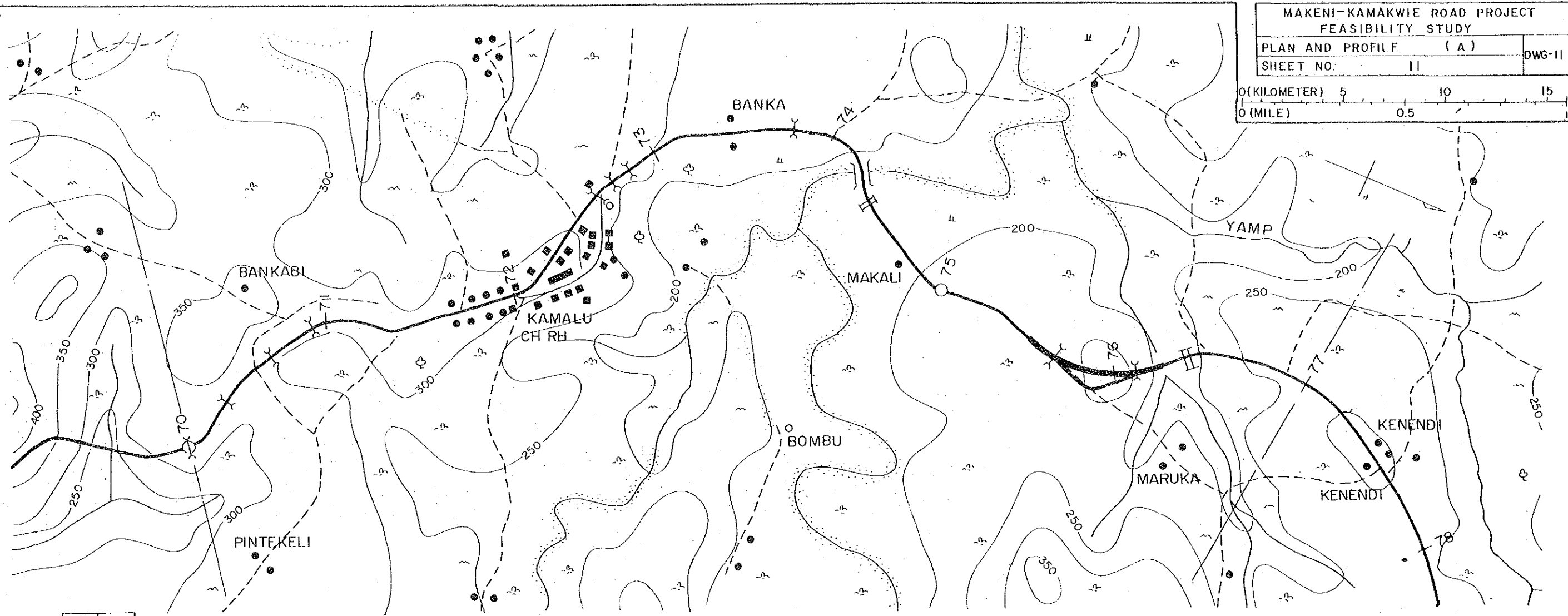
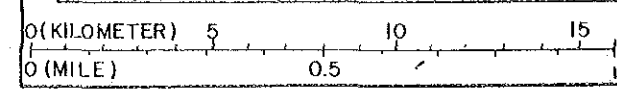
IMPROVEMENT		H	W	H	W			H	W											
ROAD SURFACE KIND		SURFACE DRESSING			SURFACE DRESSING WITH SAND MAT		SURFACE DRESSING			SURFACE DRESSING WITH SAND MAT		SURFACE DRESSING								
C B R OF SUBGRADE		C . B . R    >    25																		
DISTANCE	MILES	22			23			24			25			26						
	KILOMETERS	35	36			37			38			39			40			41		42



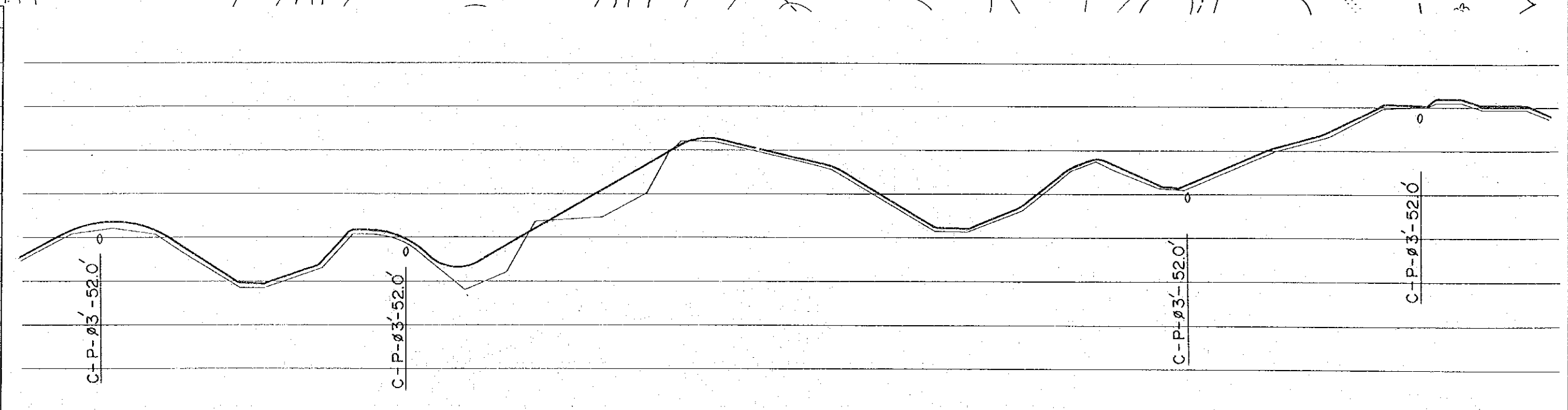
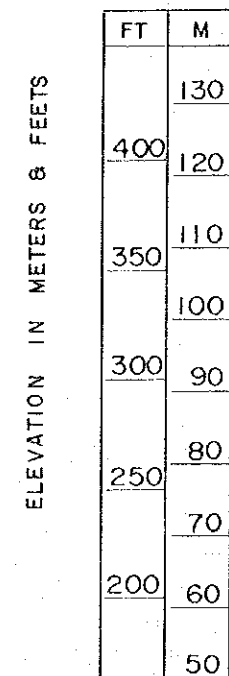
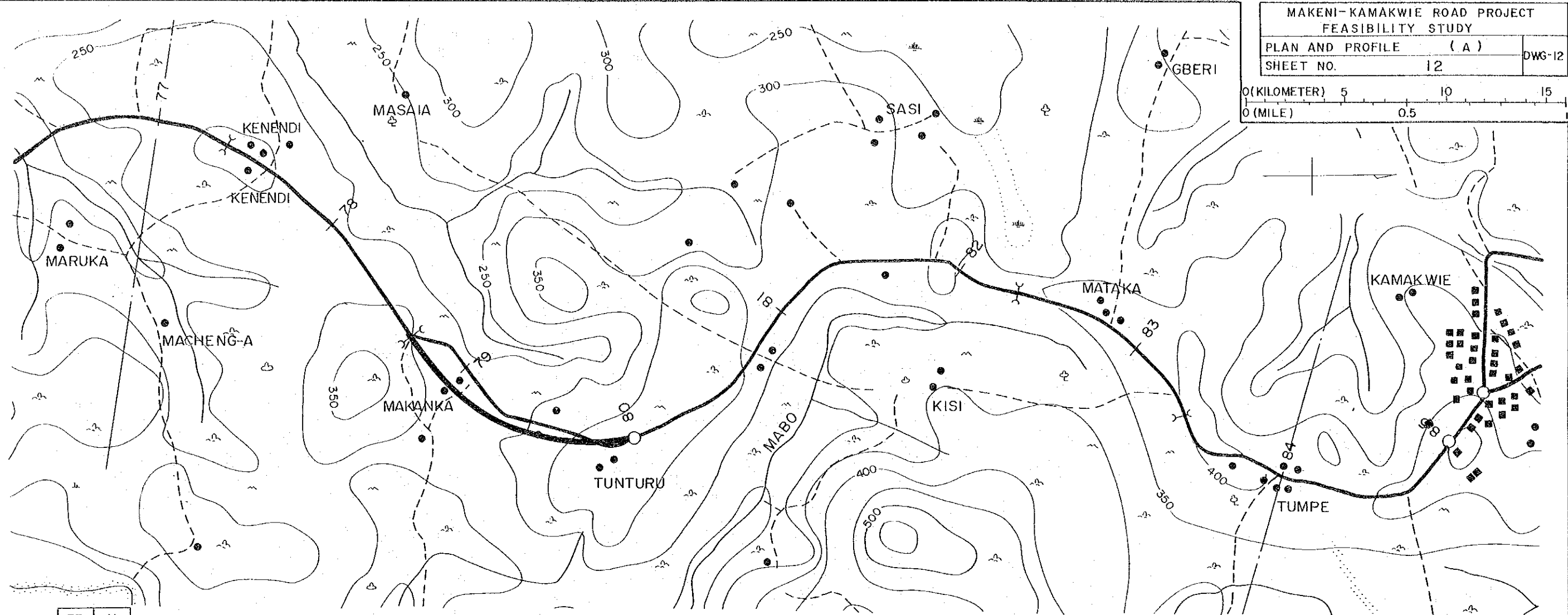
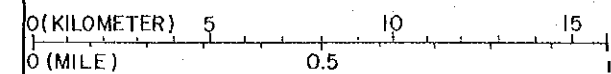






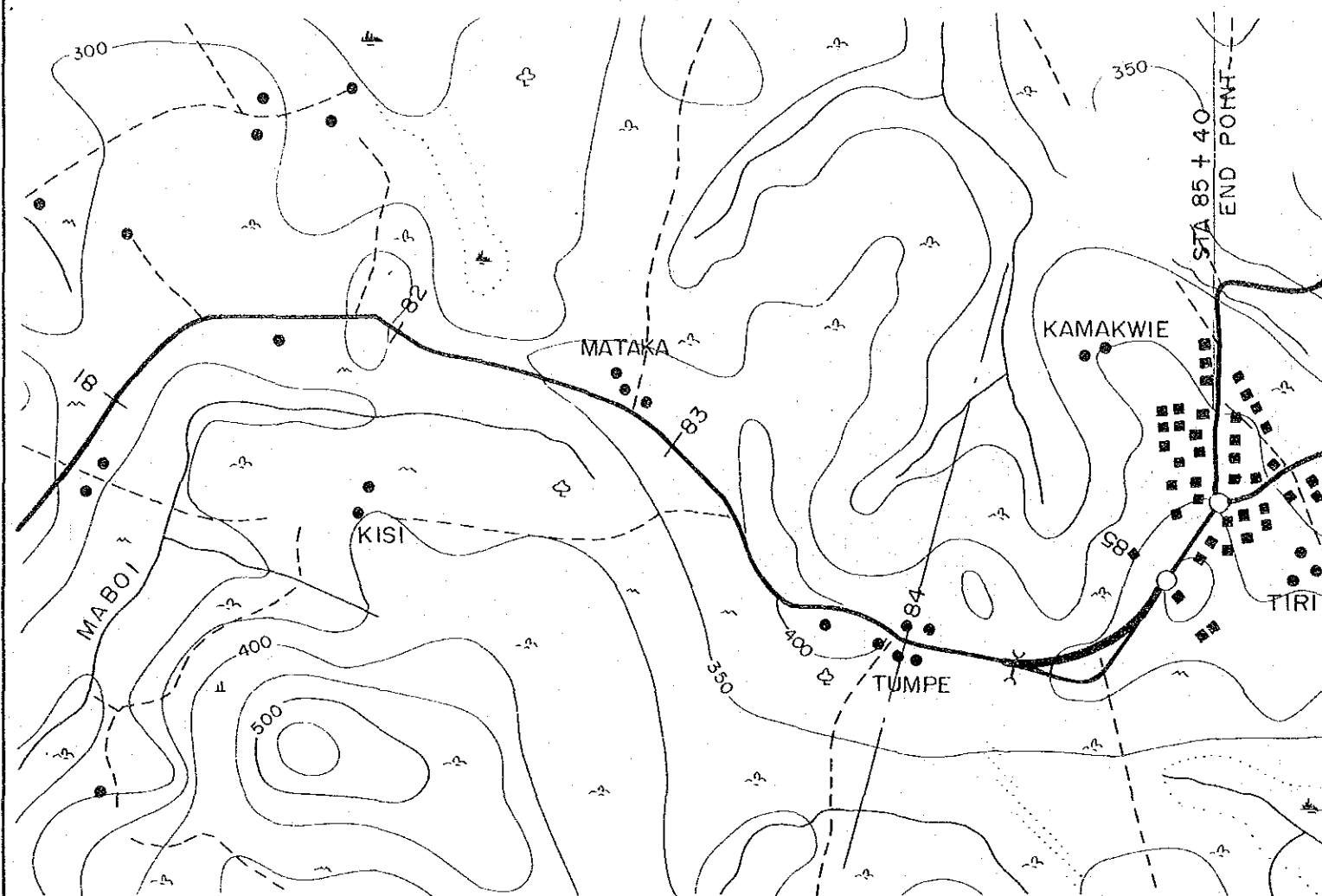
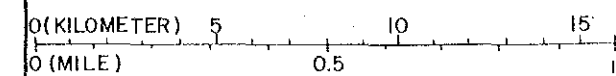




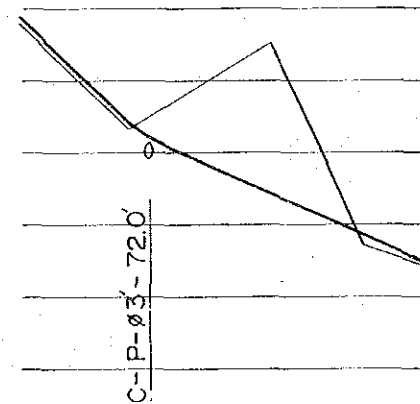


IMPROVEMENT		W	H	W	H	W
ROAD SURFACE KIND		SURFACE DRESSING		SURFACE DRESSING WITH SAND MAT	SURFACE DRESSING	
C B R OF SUBGRADE		C . B . R > 25				
DISTANCE	MILES	48	49	50	51	52
	KILOMETERS	77	78	79	80	81
					82	83
						84

MAKENI-KAMAKWIE ROAD PROJECT FEASIBILITY STUDY		
PLAN AND PROFILE	(A)	DWG-13
SHEET NO.	13	



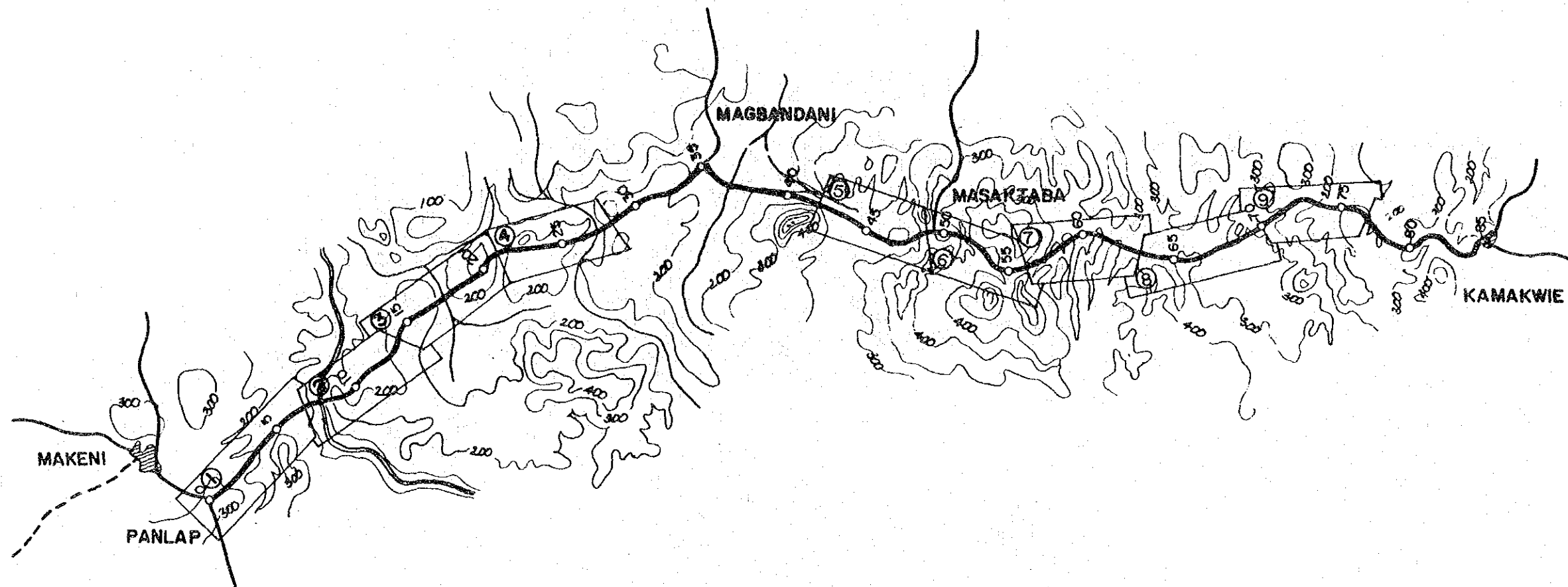
ELEVATION IN METERS & FEET	
FT	M
400	120
350	110
300	100
250	80
200	60
150	45



IMPROVEMENT	W	H	W
ROAD SURFACE KIND	SURFACE DRESSING		
C.B.R. OF SUBGRADE	C.B.R. > 25		
DISTANCE	MILES		
	KILOMETERS	84	85

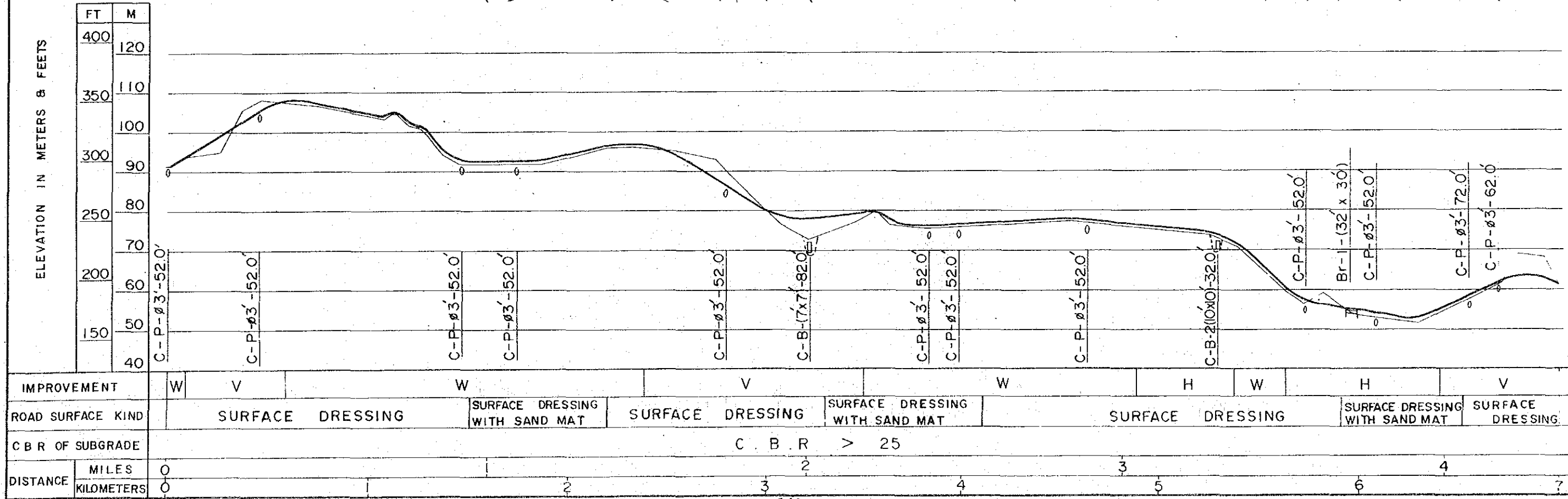
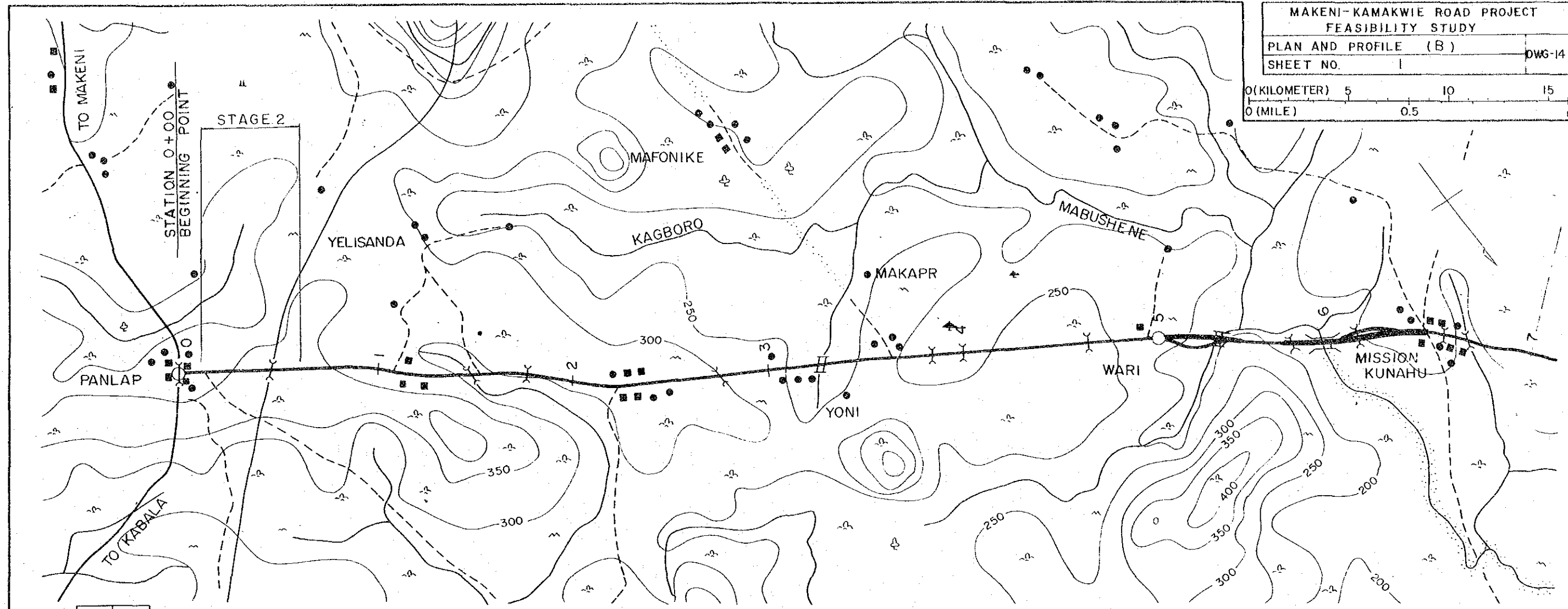
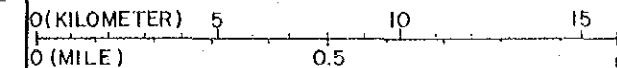
# APPENDIX S-2 PLAN AND PROFILE OF ALTERNATIVE PLAN B

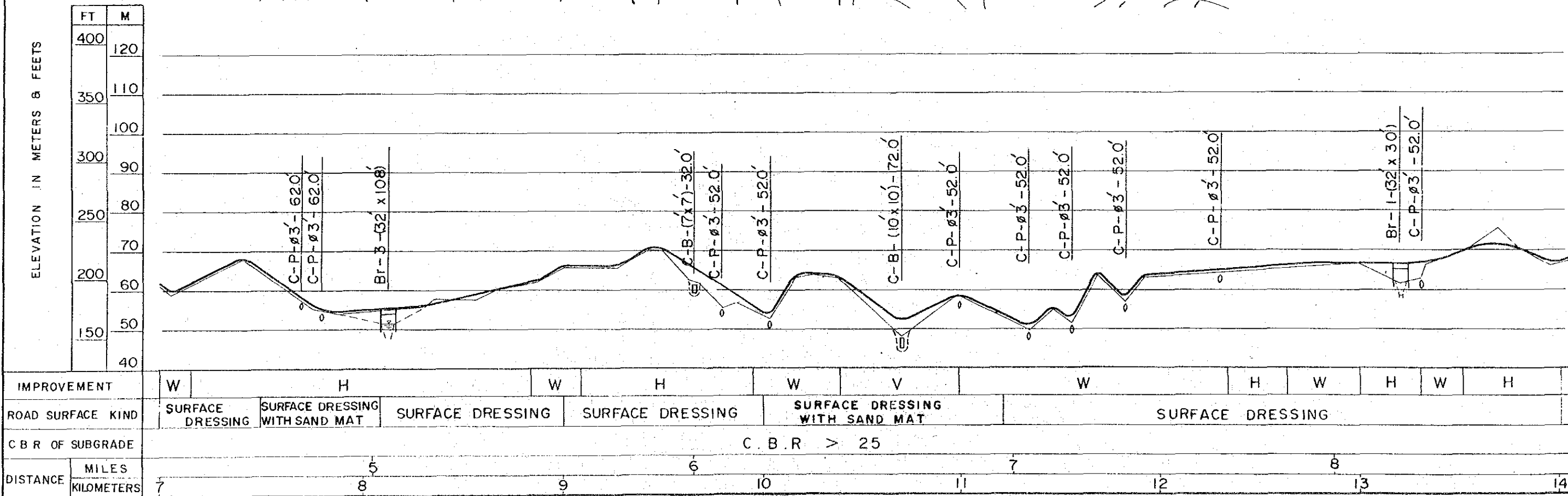
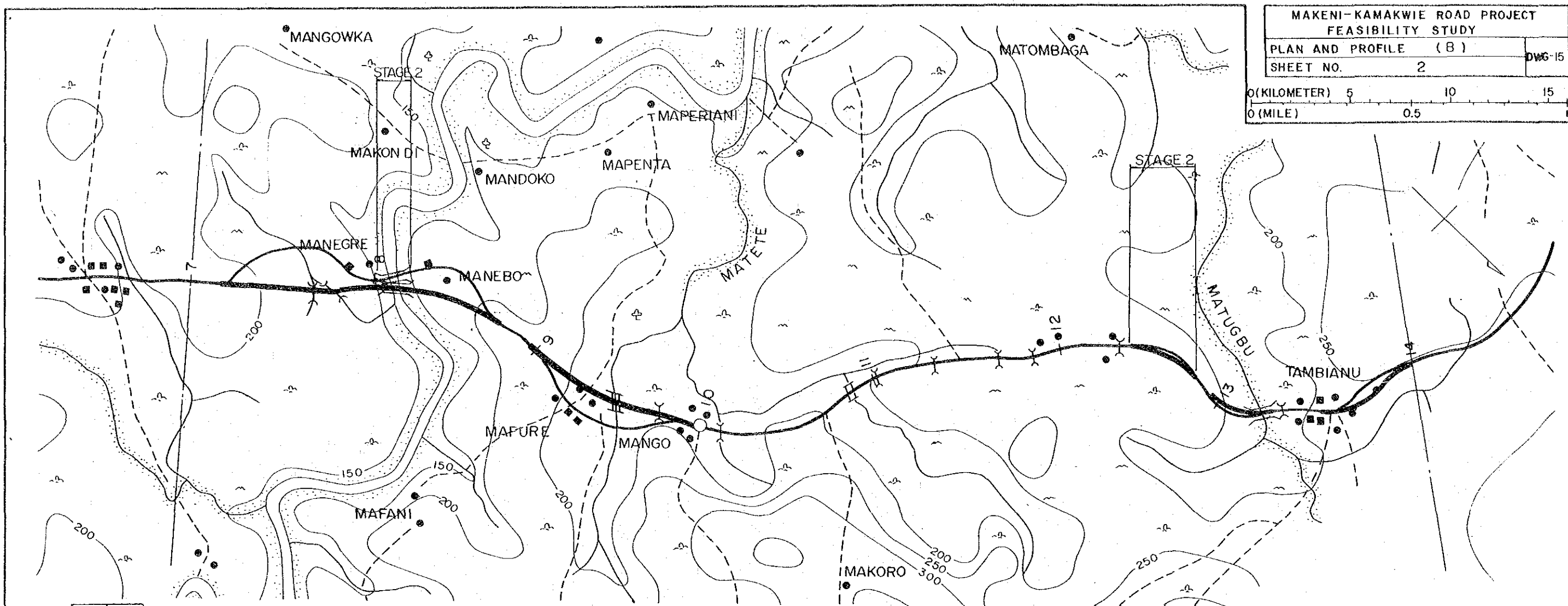
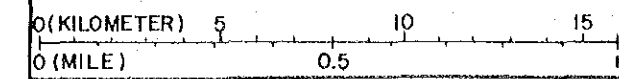
MAKENI - KAMAKWIE ROAD PROJECT FEASIBILITY STUDY	
COVER SHEET (B)	DWG-
SHEET NO.	

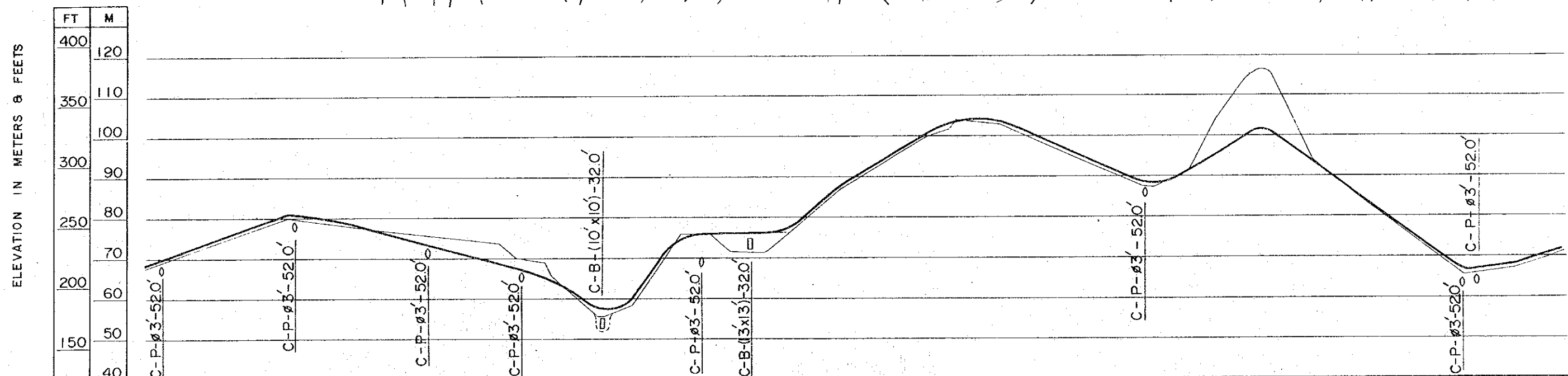
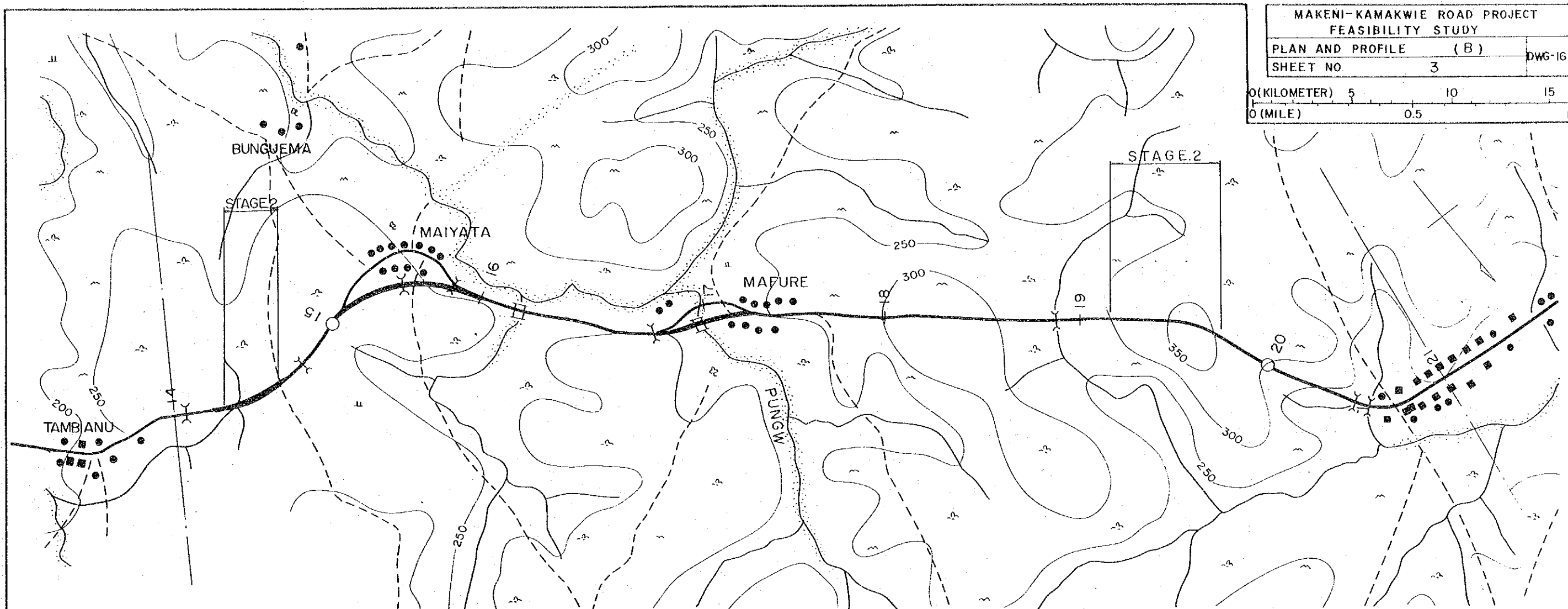
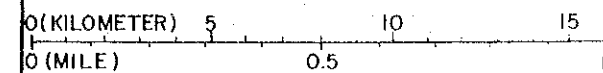


## ABBREVIATIONS

H	RE-ALIGNMENT, HORIZONTAL FOR ROAD IMPROVEMENT
V	RE-ALIGNMENT, VERTICAL FOR ROAD IMPROVEMENT
W	WIDENING OF ROAD WIDTH FOR ROAD IMPROVEMENT
C - P - $\phi a - l$	PROPOSED PIPE CULVERT, $\phi a$ (DIAMETER, FOOT), $l$ (LENGTH, FOOT)
C - B (a x b) - l	PROPOSED BOX CULVERT, a x b (WIDTH x LENGTH ALONG THE ROAD), $l$ (CULVERT LENGTH)
C - B - n(a x b) - l	PROPOSED BOX CULVERT, n (ROW), a x b (WIDTH x LENGTH ALONG THE ROAD), $l$ (CULVERT LENGTH)
Br - n - (a x b)	PROPOSED PRESTRESSED CONCRETE BRIDGE, n (NOS. OF SPAN), a x b (WIDTH x SPAN LENGTH)







IMPROVEMENT		W	H	W	H	W	H	W	V	W	V	W																	
ROAD SURFACE KIND		SURFACE DRESSING			SURFACE DRESSING WITH SAND MAT		SURFACE DRESSING			SURFACE DRESSING WITH SAND MAT		SURFACE DRESSING																	
C B R OF SUBGRADE		C.B.R > 25																											
DISTANCE	MILES	9				10				11				12				13											
	KILOMETERS	14	15				16				17				18				19				20				21		



