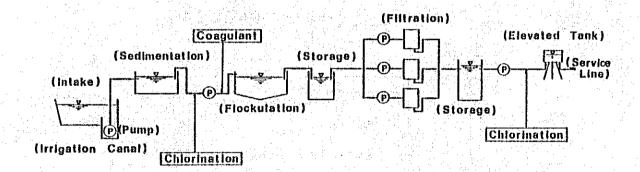
#### 3.3.3 Water Suply with Surface Water Source

The source of the urban water supply for Showak and Khashim el Gerba Towns is the surface water. A part of water source for Gedaref Town is piped for 70km from Showak.

The treatment plant at Khashim el Gerba Town was constructed by the Chinese Aid in 1977 taking the water from the Atbara Riber. The design capacity of the system is 2,160 m<sup>3</sup>/day and the design population is 20,000 people. Total length of service pipes is 15km with diameters ranging from 50mm to 200mm. The chemicals for water treatment are all imported from England. The system is illustrated in Fig. 16.

Fig. 16 Flow System of Khashim el Gerba Town Water Supply



## 3.4 Institutional System for Water Supply

#### 1. Office of Commissioner for Refugees (COR)

The COR is an official agent under the Ministry of Internal Affairs to undertake various activities to assist refugees in cooperation with UNHCR. At present the number of staff of the COR is 15,000 people.

There are five departments under the Commissioner and the Assistant Project Manager for Water Supply Undertakes Water Supply Projects for refugees. The budget in the last three years is a few million Sudanese pounds;

1983	S£6,949,000 (US\$2.8 mil)
1984	S£7,893,000 (US\$3.2 mil)
1985	S£2,989,000 (US\$2.0 mil)

Of these budgets about 26% was spent for the water supply for refugees.

## 2. National Water Cooperation

This cooperation is responsible for procurement, training and research, underground water development and construction of boreholes for the whole country. Operation and maintenance of boreholes in village water supplies is also under responsibility of the NWC, therefore, a close coordination with COR is necessary for the rural water supply project.

At present the drilling section of the NWC holds the following equipment for construction and rehabilitation of boreholes;

Ballerini ST6 5
Gardener Denver 2
Mayhuew 5
Percussion 22W 10
Sankyo T24 2
Sankyo Cable Tool 3
Farcom (Testing Unit) 3
Tone THR 300 2

Of these Sankyo Cable Tools and Farcom are the service machines and the percussion types are too old for drilling. Existing serviceable equipment are 19 units. Since the NWC is responsible for the whole country it is very much unlikely to save equipment for a certain project for a long period.

The drilling division of the NWC has been playing an important role in the underground water development and has a considerable experience. These more than 500 boreholes existing in the Eastern Region were also constructed by the NWC. Number of experts and technicians of the drilling section is listed as below;

Type of Field Number	of Staff
Chief Drilling Engineer	10
Chief Driller	30
Driller	80
Shift Driller	35
Geologist	24
Hydrologist	32

Usually a drilling crew consists of the following members;

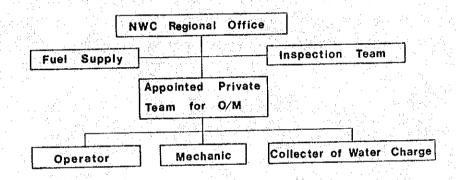
Type of Job	Number of Staff
Chief Driller	
Shift Driller	13 A 24 A 34 A 34 A 34 A 34 A 34 A 34 A 3
Hea Rig Man	41
	12
Driver	
Assistant Driver	2
Store Keeper	
Mechanic	1
Total	26

3. Operation and Maintenance of Village Water Supply

Operation and maintenance of village water supply is undertaken by three villagers appointed by the NWC. Major duties of these three operators are operation and maintenance of facilities and collection of water charge.

The organization chart of operation and maintenance is shown in Fig. 17.

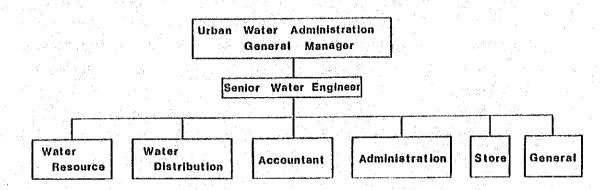
Fig. 17 Operation & Maintenance of Village Water Supply



4. Operation and Maintenance of Kassala Town Water Supply

The Urban Water Administration in the Eastern Region is responsible for operation and maintenance of the Kassala Town water supply. The organization chart of the Urban Water Administrationat Kassala is shown in Fig. 18.

Fig. 18 Organization Chart of the Urban Water Administration at Kassala



Since there are sufficient manpower with experience and the organization at present no difficulty in operation and maintenance will be expected. CHAPTER 4 FORMULATION OF THE PROJECT

# CHAPTER 4 FORMULATION OF THE PROJECT

## 4.1 Objectives

There are about 1.1 million rural population in the project area. Of these rural population about 400,000 people including refugees are suffering from shortage of domestic water. One of objectives of the Project is to ease this urgent problem serving suitable domestic water for 210,000 people in rural areas and refugee camps.

At the same time, Kassala Town, the Regional Capita is also suffering from the shortage of water; Due to rapid increase in the urban population about 75% of the urban area became the problematic area of urban water supply. Another objective of the project is to solve this problem and maintain the target supply capacity of 75 1/day/cap.

For this purpose, necessary equipment, machinery and materials are to be provided including construction of a part of facilities under the Grant Aid Cooperation of Japan.

- 4.2 Confirmation of the Project Background
- 4.2.1 Village Water Supply for Refugees and Sudanese Villagers
  Influenced by Refugees
  - 1. General Description

The following items were requested by the Government;

1) Construction of New Village Water Supply Facilities for 41 places including 6 refugee camps.

- 2) Rehabilitation of Boreholes of Existing Village Water Supply Facilities for 165 holes including 15 holes at Refugee Camps.
- 2. Construction of New Village Water Supply Facilities

Requested new village water supply facilities are to be constructed in the place near by existing facilities because capacities of the existing facilities are insufficient due to rapid increase in population.

Through the site investigation it is found that a total of 46 facilities in 42 sites are requested instead of 41 sites. For construction of the new boreholes it is critical to confirm the hydrogeological conditions at proposed drilling sites. Background information of existing boreholes are available at 31 sites of facilities (Table 5). It is, therefore proposed that the construction of new village water supply facilities will be made on 34 sets at 31 sites (Fig. 19).

 Rehabilitation of Existing Boreholes of Village Water Supply Facilities

In the Eastern Region the underground water has been playing an important role for supply of domestic water. However, many of these boreholes are very old and the machinery deteriorated.

At present 165 boreholes including 15 holes at refugee camps require rehabilitation. The request is the Government's intention to ease this problem. Of these boreholes, the necessary hydrological conditions are available at 73 holes (Table 6).

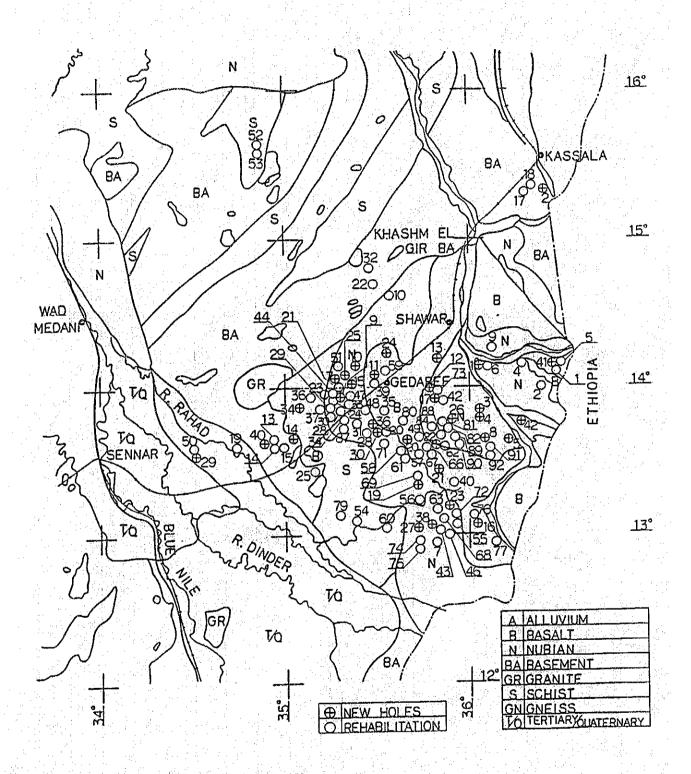


Fig. 19 Location of Boreholes

Table 5 BACKGROUND INFORMATION OF NEW BOREHOLES

Location	Type of Aquifer	Available Hydogeogical	Population to be Served	Present Water Source	No of Boreholes
EL SARAF EL AHRMER	Wibiangardetona	D=85m, S. W. L=20m	900		
	napi anadana	D= 120m, S. W. L=30m D= 725 0 /min 13.5	4.000		
ALLANA 55. 17. 35. 37. 37. 37.	Nuhian/Rasalt	D= 300m, S. W. L=50m 0= 230 0/min 13.8	2000 1		
19 EL AZAZ EL HATAR		D= 200m, S. W. L=20m Q= 115 g /min 6.9	3,000		
20 SHEIKHAIN 35 -43.		D= 150m, S. W. L=30m Q= 115.g/min 6.9	8,000	Borehole	
35° -46' 13° -09'		D= 160m, S. W. L=30m Q=90. g/min 5.4	000'2	Borehole Hafir	
TAGLI(1) - 30 5: 13° -33°		D= 150m; S. W. L=28m 0=65 0/min 3.9	3.000	Borehole	
	WIRTAN SANDSTONF	D= 150m; S. W. L=28m 0=75.9 / min 4.5	5.000		
	Minian Sandstone	0= 105m, S. W. L=43m 0=90 e/min 5. 4	3.000		
DOMAT(2)	Wilhian Sandstone	30	8.000	Boreho le	
ZERGA(1)	Subject of the subjec	D=95m S. W. L=22m G= 110 g/min 6:6	5,000		
UMM HRELAVIA	Nubjan/Sandstone		5,000	# 1 m	
UNIN MILEMA	Basalt Muhiap/Sandstone	370m, S. W.	5.000		
H(1)	Nubjan/Basalt	0= 150m, S. M. L=44m 0= 110.2/min 6:6	10,000	Hafile	
CALA EL NAHL(2) 34 ° -57' 13° -37'	Fractured Basement Complet	Om, S. W. L=57 5.2/min	2,000	8oreho1e	
	Wibian SandsTone	D= 180m, S. W. L=56m C=70 0 / min 4.2	3,000		

Table 6 BACKGROUND INFORMATION OF BOREHOLE REHABILITATION

Population Served	2,000	3,000	900'9	2,980	3,000	4,000	5,000	2,000	3,000	10,000	6, 000	15,000	9,000	2,68	3,000	3,000	3,000
Yield (2/Bin)	889	89	166	54	9,	92	136	32	02	333	151	378	197	245	79	601	91
S.W.L.	L.	133	32	23	32		29	19	28	27	<u>0</u>	9	33	28	76	23	67
Tyep of Aquifer	Basalt/Nubian Sandstone		<b>#</b>	<b>F</b> .		Nubian Sandstone			•			Sandstone	Basalt/Nubian Sandstone				
Casing (mm)	6 5/8	ŧ	<b>=</b>	<b>.</b>	<b>1</b>	E	<b>L</b>	: : :	ŧ.	Ŀ	E	6 5/8	<b>=</b>	<b>*</b>	<b>+</b>	8 5/8	6 5/8
T-Depth (m)	183	163	212	340	194	179	211		118	92	22	37	275	187	205	114	237
Const. Year	1949	1967	1974	1967		1973	1973	1968	1969	1969	1970	1970	1969	1971	1972	1948	1947
Location	N. 36. 29 F. 14. 12	36. 19 14. 13	36.06 14.11	35.51 12 56	36.09 14.15	36.09 14.15	35.35 14.36	34.56 13.37	34.56 13.37	34.57 13.36	36.20	36.24	34.43	35.32 13.40	35.19	35.29 13.42	35.14 13.53
Code No.	5385	2287	5707	4248	6218	6181	6708	1415	2604	3529	2425	3024	3289	4303	5409	222	173
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Yield	( @/min)	257	64	<u> </u>	243		243	166	i	227	236	}	63		42	1	£	91		109	88		රිරි		Ç	50		605
S.W.L.	(m)	35			50		46	53		53	69	<u> </u>	85		159		5	48		23	67		32			<u>&amp;</u>	}	18
Tyep of Aquifer		Basalt/Nubian Sandstone			#		#	2.8			<b>1</b>		14		Basalt-Nubian Sandstone								Basalt-Nubian Sandstone		Mubian Sandstone	7-2		
Casing	(mm)	6 5/8		·	E.		=			Į.	8.4		ŧ		6 5/8		% 22/8	t		<b>=</b>			6 5/8					•
T-Depth	( <b>m</b> )	270	450	97	229		210	239	}	225	253	}	35		311		ß	53		<b>8</b>	153		370	ů.	116	371		88
Const. Year		1971	40.67	J461	1971		1969	1971		1968	1971	•	1971		1961		) SE	1947		1948	1956		1971			1971	<b>.</b>	1971
Location		* 35.11	t 13.25	3.5	35.18	13, 55	35, 18	35.25	13.43	35.27	35.70	13.53	35.21	13.45	35.34	13.49	35.11 13.54	35.14	13.53	35.29	35.30	14.03	35.55	13.19	35.52	35.51	13.02	35.18
No.											-				<u> </u>													
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	(6)期/67)	210	12	303	179	64	53	61	76	91	45	254	210	173	227	189	92	45
- 19 %	; (£)	45	20	10	89	53	38	15	50	30	21	20	14	88	34	32	66	22
Tven of Aniifer	io model	<b>8-8</b>			#	<b>#</b>	***		N. P. B.		<b>N-</b> 8		<b>7-</b>		<b>1</b>			
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Population Served	2,000	2,000	2, 000	5, 000	4,000	2,000	4.000	1.000	1,080	1,000	1,5000	<b>*</b>	2,000	4,000	3,000	3,000	5,000
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Yield (2/min)	76	91	68	236	145	84	151	52	62	53	454	151	J6	227	9/	129	151
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S.W.L. (m.)		16	16	31	28	88	40	23	76	72		59	102	8.2	72	29	87
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<u> </u>	262	336	8	260	363	211	143	301	312	171	267	136	169	171	137	185	179
Const. Year	1965	1968	1969	1971	1971	1972	1969	1971	1971		1966	1971	1965		1968	1968	1971
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Code No.	1788	2176	2923	3288	3297	5391	2903	3301	3302	3185	1924	4308	1787	4310	2489	2490	4254
No.	52	53	54	55	26	57	28	59	98	91	62	63	75	65	99	29	89

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The construction period of these existing boreholes is summarized as below;

Time	of Construction	Number of boreholes
	The 1940s	
	1951-1955	$oldsymbol{ ho}_{i_1,i_2,i_3,i_4,i_5}$ , where $oldsymbol{ ho}_{i_1,i_2,i_3,i_4,i_5}$
	1956-1965	
	1966-1070	.29
	1971-1975	29

Even the latest construction of boreholes was made in 10 years ago. Pumps and engines installed before 1965 appear to be worn out. Even these machinery installed after 1951 they are all old no more than safe use.

As for boreholes more than 15 years old need juvenilation and cleaning.

Most of these 73 boreholes distribute in the areas covered by Nubian Sandstones and Tertiary Basalts.

## 4. Standard Design

There are design criteria for major components of rural water supply facilities in Sudan.

Design Capacity

Type of Facility	Daily Consumption 1/day/cap
Water Point	10
Public Taps	15 - 30
With Pipeline	50 - 100
Large Animal	27
Small Animal	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

Standard design capacity 30% of the average daily consumption.

However, when water source is taken from the underground water, the available amount of water source is the key factor to determine the design capacity. For this reason a standard design as the standard unit of the village water supply facility is applied in practice. When one unit is insufficient additional unit is provided near by the existing unit.

The standard design of the village water supply facilities summarized as below;

1) Water Source : Borehole and pump house

2) Elevated Tank : FRP, 50 m<sup>3</sup>, 3 m

3) Public Taps : 1 1/sec, 4-8 pcs

4) Cattle Trough : 1-4 pcs

5) Water Yard : 50m x 50m

Under the project, the construction of new village water supply facilities requires all 5 items above and the rehabilitation of boreholes of existing village water supply requires only water source rehabilitation and replacement of pumps and engines.

### 5. Water Source

All the boreholes are to be constructed in the same area where existing boreholes are operated. Hydrogeological conditions of these new boreholes will be similar to the existing borehole conditions.

The relationship between available amount of water assuming 12 hrs pump operation and design population in each site of the proposed sites of construction of new village water supply facilities is shown in Fig. 20. Comparing these plots with the line of the standard water demand it is observed that all the sites are able to supply domestic water more than the minimum standard volume of 10 1/day/cap.

In this comparison an assumption is made that operation hours of pump and engine is 12 hrs. Therefore increasing operation hours from 12 hrs to 16 hrs the more supply capacity will be available.

These 6 sites where the available water source is insufficient for the demand even with the minimum supply level of 10 1/day/cap; these are sites No. 1, 3, 21, 29 and 30 as summarized below;

Hole No.	Yield	No. of B/H	Opera- tion Hour	Available Water Source	Popula- tion	Unit Consumption
	1/min	Б/П	hr hr	1/d		1/day/cay
					A Section	
1	75.5	2	12	108,720	12,000	9.1
	75.5		16	217,440	12,000	12.1 > 10
3	380	2 2	12	532,800	70,000	7.6
	380	2	16	729,600	70,000	10.4 > 10
21	90	1	12	64,800	7,000	9.3
	90	1	16	86,400	8,000	10.8 > 10
25	90	1	12	64,800	8,000	8.1
	90	1	16	86,400	8,000	10.8 > 10
29	110	1	12	79,200	10,000	7.9
	110	1	16	105,600	10,000	10.6 > 10
30	75	1	12	54,000	7,000	7.7
	75	1	1.6	72,000	7,000	10.3 > 10

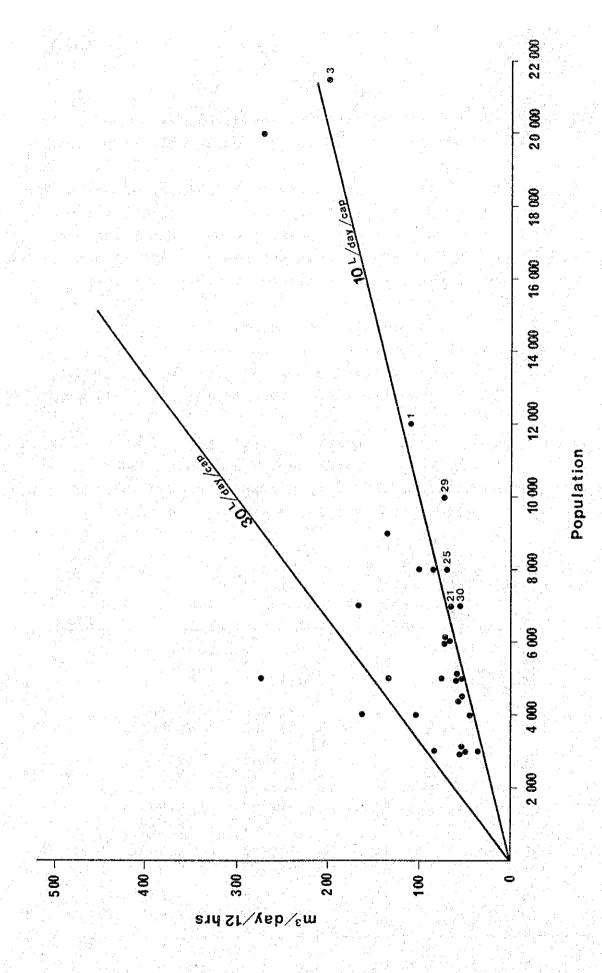


Fig. 20 Water Source/Population New Boreholes

However, all these boreholes can meet the minimum demand of 10 lit/day/cap by increasing the operation time of pump and engine from 12hrs to 16hrs. It has to be born in mind that most of these existing boreholes are installed with stout type screens which results in reduction of the borehole yield. Therefore, a proper construction borehole has to be applied for efficient usage of the underground water.

Also it has to be minded that the yields of these boreholes in themselves are quite favourable since the unit yield ranges between 70 1/min and 300 1/min.

Therefore, insufficiency of supplying domestic water is due to the demand of these sites. Accordingly, for future increase in water demand the construction of additional units of water supply facilities will be the solution.

In the same manner a composition of available water source and demand is made for 73 sites of the proposed rehabilitation of boreholes (Fig. 21).

As shown in Fig. 20 all the sites for rehabilitation of boreholes satisfy to supply the minimum supply level of 10 1/day/cap. However, these yields of borehole to be rehabilitated are the yields obtained from the data at the construction of these boreholes. Since a long span of time passed after construction these boreholes will require juvenilation and pump tests to confirm the actual safe yield.

#### 6. Elevated Tank

The standard design capacity of the elevated tank is  $50\text{m}^3$  of 3m high and the design criteria of water tanks is 30% of the daily consumption. These criteria are applicable where population is less than 7,000 and the daily consumption rate is less than 30 1/day/cap (Fig. 22). However, where the site population exceeds 15,000 people the volume of tank, 50 m³ is no more than 30% of the daily consumption of the unit. For these sites arrangement will be required to distribute the peak demand into certain duration of time to maintain equitable water supply (Fig. 23).

## 7. Pumping Facility

Three types of pumps are analyzed and it is recommended to use the vertical turbine pumps for the project.

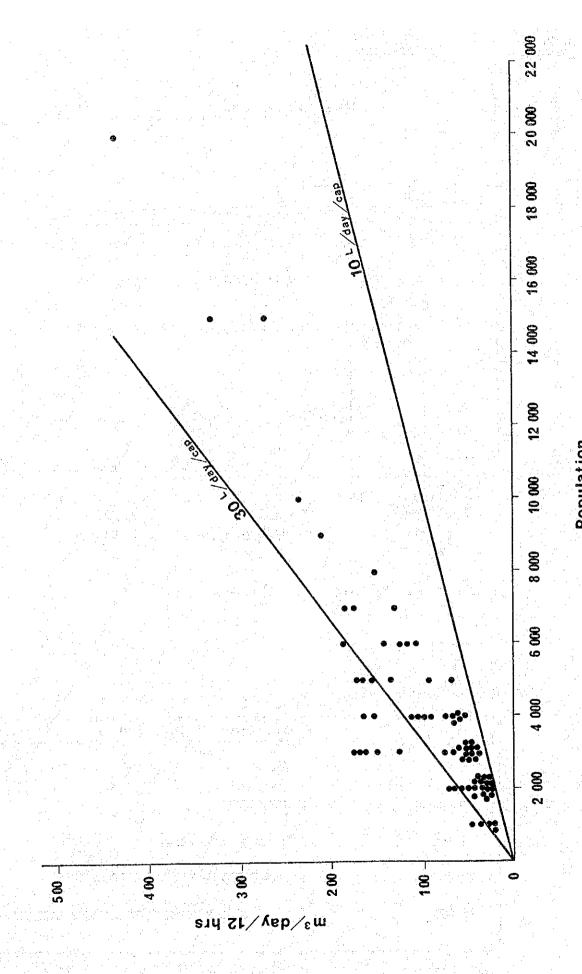


Fig. 21 Water Source / Population Rehabilitation of Boreholes

4 - 17

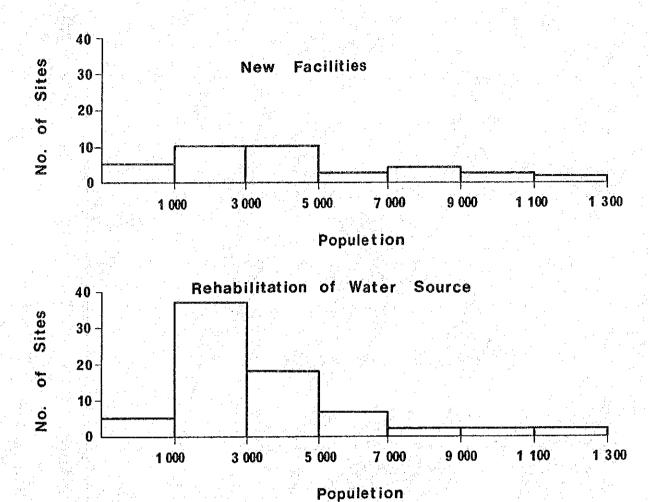


Fig. 22 Distribution of Population

		Storage Volum (m³)
Populetion		0 50 100
	5 000	<b>S</b> IIIIIIIIIIIIII
- Constitution of the Cons	7 000	vannamanninninna.
	10 000	
	15 000	

Fig. 23 Required Capacity of Storage Tank

## 4.2.2 Kassala Town Water Supply

### 1. Introduction

The urban area of Kassala Town is divided into two parts, the West and East by the Gash River. There is only one bridge between the two parts. The Gash River dries up during five months in the dry season.

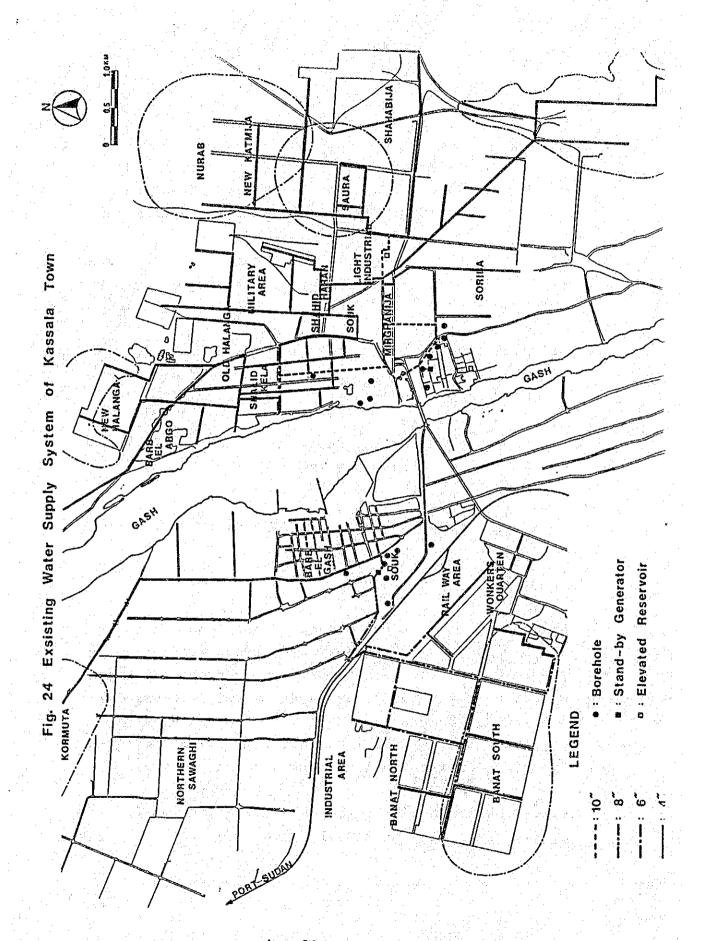
At present there are 500,000 refugees in Kassala Town, which resulted in serious water shortage of the town together with deteriorated water supply facilities.

Existing distribution pipes are 10", 8", 6", 4" and 3" of asbestocement pipes and 2" and 1/2" of P.V.C. pipes as summarized below;

Pipes	East Bank	West Bank
10"	5.4 km	
8 "		1.0
6 <b>"</b>	18.2	6.1
4 "	33.8	8.3
3"	20.5	2.5
2"	68.5	38.7

Since small size pipes occupy rather long length in the piping network, there would be low hydraulic pressure zone in the network. These problematic area of 25% of the urban areas where hydraulic pressure is too low for 24 hrs and other 50% of urban areas receive water only during night time. The remaining 25% of urban areas only receives normal services.

It is therefore urgent to rehabilitate the water supply facilities of Kassala Town (Fig. 24).



## 2. Target Year

At present no future projection of the town water supply is made, however the growth rate of the town population is only 5.8% in the past 10 years from 1983 which corresponds to less than 1% of the annual population growth rate. The increase in the future water demand may be mainly caused by increase in numbers of private connections and increase in refugee population if any, however not in a significant level. Therefore the design target is to meet the present demand with the target design capacity of 75 1/day/cap.

#### 3. Service Area

The service area consists of two parts, the east bank area and the west bank area. The water supply systems on the both bank sides are completely independent. The size of the service area is  $14~{\rm km}^2$  and  $8~{\rm km}^2$  on the east and the west bank sides respectively.

### 4. Water Source

#### 1) General

The only prospecting water source for the Kassala Town Water Supply is the underground water from the alluvial deposits of the Gash River since the near by Gash River dries up during the dry seasons and the perenial rivers, the Nile and the Atbare Rivers, are available at 70 km away from the Town.

## 2) Boreholes

The safe yield of existing boreholes varies from  $35 \text{ m}^3/\text{hr}$  to  $80 \text{ m}^3/\text{hr}$ . Therefore,  $40 \text{ m}^3/\text{hr}$  is applied as the design capacity of boreholes. Available amount of the underground water from the alluvial deposits of the Gash River is estimated at  $78 \text{ million m}^3/\text{year}$  and only a part of it is utilized at present. The draw down is estimated to be 5 m and the dynamic water level is about 35 m.

The standard design of the boreholes is proposed as below;

Diameter : 10 inch

Yield: 40 m<sup>3</sup>/hr/hole

Draw Down : 5.0 m

Dynamic Water Level : 35.0 m

Operation Time : 16 hrs

Distance Among Holes : More than 100 m

## Water Quality

The quality of the underground water is quite potable and meet the WHO standard as shown below;

Table 7 Water Quality

Items	Ground Water	WHO- Recommended	WHO- Tolerable	Japanese Standards
1. Turbidity mg/l	2.7	5	25	2
2. Color mg/l	Nil	5	50	50
3. pH	7.7	7.0-8.0	6.5-9.2	5.8-8.5
4. Electric conductivity uv/cm	750			. #14
5. Hardness mg/l	165	100	500	300
6. Alkalinity mg/l	55	80	400	
7. Cl mg/l	128	200	600	200
8. Fe mg/l		0.7	0.8	0.8
9. Fe mg/ $1$	Nil	0.1	1.0	0.3
10. Mn mg/1				0.3
11. Hg mg/l				- 11 <b>0</b> pm
12. Cu mg/l	-		<b>-</b>	1.0
13. Zn mg/l	eranî û de. Parî de de		- A - A	1.0
14. Pb mg/l	- ·			0.1
15. As mg/l	- V			0.05
16. Cd mg/l	· · · · · · · · · · · · · · · · · · ·		u jak	
17. Total Solid	460	500	1,000	500

## 5. Population To Be Served

The design population to be served is determined based on the existing conditions of the service system. There are 11,700 private connections; 7,590 in the east bank and 3,750 in the west bank. An average size of one family is 9 people. The design population is summarized as below;

Table 8 Population of Kassala Town

	East Bank	West Bank	<u>Total</u>
Total (including refugee)	112,000	53,000	165,000
Private Connection	72,000	34,000	106,000
Communal Taps	40,000	19,000	59,000

## 6. Water Consumption

Due to insufficient water sources of the both systems on the east and the west banks an average water consumption is 65 1/day/cap and 36 1/day/cap in the east bank and the west bank respectively.

The target daily consumption of the local town water supply for private connection is 75 1/day/cap - 100 1/day/cap in Sudan.

Daily water consumption rates of the similar countries are summarized in Table 9.

Table 9 Common Value of Daily Water Consumption (lit/day/cap)

of the profit and state of the following section of the state of the s	Privage Connection	Communal <u>Tap</u>
Developing countries	50-150	20-40
Kenya	75	25
Sri Lanka	180	45
Indonesia	100-125	40

Considering the similarity of environment and socioeconomic conditions between Sudan and Kenya, the
target of 75 1/day/cap of the water consumption in
local town private connection appears to be quite
reasonable and is applied for the design criteria of
the project. As the design criteria for the daily
water consumption at public taps, 25 1/day/cap, is
proposed in the same reason.

## 7. Water Consumption for Public Purposes and Losses

Since Kassala Town is the Regional Capital there are many public institutions like government offices, schools and hospitals. The design criteria for the water consumption of public purposes is 1/3 of the total design capacity in Sudan. Considering the high rate of losses due to old asbestocement pipes used in the system, losses are estimated to be about 20% of the total design capacity. Therefore, remaining 13% out of 1/3 of the total design capacity is considered to be quite reasonable rate of the net public purpose water use considering the size of the town. Therefore the water consumption for public purposes and losses is determined to be 1/3 of the total design capacity.

#### 8. Seasonal Variation of Water Demand

In Kassala Town water supply, the seasonal variation is determined to be 130% of an average consumption.

## 9. The Peak Hour Demand

The peak hour demand is one of the design basis to determine the design capacity of water supply facilities. For the water supply system of this size of capacity the ratio between an average demand and a peak hour demand is 1.5.

## 10. Water Demand

The water demands as the design basis are summarized in Table 10.

Table 10 Design Capacity of Kassala Water Supply

	East Bank	West Bank
	72,000	34,000 75 persons 1/day/cap = 2,550 m <sup>3</sup>
Average Daily (1) Consumption	40,000 25 persons * 1/day/cap	19,000 25 persons x 1/day/cap
	= 1,000 m3 Total 6,400 m <sup>3</sup>	= 475 m <sup>3</sup> Total 3,000 m <sup>3</sup>
Public Purpose (2) and Losses (1) x 0.33	$6,400 \text{m}^3 \times 0.33 = 2,100 \text{m}^3$	$3,000 \text{m}^3 \times 0.33 = 1,000 \text{m}^3$
Average Daily (3) Demand (1) + (2)	8,500 m <sup>3</sup>	4,000 m <sup>3</sup>
Max. Daily (4) Demand (3) x 1.3	11,050 m <sup>3</sup>	5,200 m <sup>3</sup>
Max. Hourly Demand (4) x 1.5	16,600 m <sup>3</sup> /day = 692 m <sup>3</sup> /hr = 5.3 m <sup>3</sup> /min	7,800 m <sup>3</sup> = 315 m <sup>3</sup> /hr 11.5 m <sup>3</sup> /hr
	And the control of th	

The water distribution is designed based on the size of the service area;

East Bank Area  $16,600\text{m}^3$   $1,400\text{ha} = 11.86\text{m}^2/\text{ha}$ West Bank Area  $7,800\text{m}^3$   $800\text{ha} = 9.75\text{m}^2/\text{ha}$ 

## 11. Service Time

At present the water supply services are made for 24 hrs and the same service level will be maintained in the project.

#### 12. Hydraulic Pressure

Since the existing hudraulic pressure is approximately 2.0 m. Considering friction losses of pipe network of the smallest diameter necessary hydraulic pressure in the service line network is estimated to be 5.0 m at the level of the pipe diameter of 100 mm.

## 13. Service Line Network

The service line network is already installed in Kassala Town water supply system. However, existing capacity of the pipes is insufficient to meet the demand with the target water consumption rates. Therefore, it is proposed to design the pipe line system to utilize the existing system as much as possible with installation of additional distribution mains to allow the proposed design capacity so as to maintain the hydraulic pressure in the existing pipe line network in the same level as present conditions to prevent leakage from existing asbestocement pipes due to increased hydraulic pressure.

For sure to prevent leakage in the existing asbesto cement pipes, additional pipelines are to be installed to form loops among terminal points to maintain an even hydraulic pressure. For this purpose polyvinyle chloride pipe is recommended because of its durability and the price.

14. Function of the Existing Facilities in the Proposed Design

The proposed design basis is to utilize existing facilities as much as possible in the basic design of the project.

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These 10 boreholes on the east bank shall be used as they are at present. Five boreholes out of seven on the west bank are also to be used in the project.

The elevated tanks on the both bank sides will be discarded due to required hydraulic pressure in the proposed design.

The booster pump facility on the west bank will be used only for a supplementary function of the project.

The existing service line networks will be utilized in the proposed design as much as possible.

## 15. Future Extension

At resent, no significant future increase in water demand is expected and minor increase of future demand will be absorbed by addition of required units of facilities.

#### 16. Storage Capacity

The design capacity of the storage capacity for distributing function is determined by two factors; the capacity of water sources and the design capacity of the whole system. Usually available water source is limited so that the storage capacity is determined to be the capacity equivalent to the daily maximum demand for 8 to 12 hrs with the design capacity of the daily maximum for the water source.

In Kassala Town, however, the design capacity of the storage tank is equivalent to the hourly maximum demand for 3 hrs. Therefore, the storage capacity tends to be small compared with the whole design capacity of the system. For this reason it is

proposed to take the hourly maximum demand for the design capacity of the water source since sufficient underground water is available.

Advantage of this design consideration is to minimize construction costs of the reservoir in the project and to be able to reserve allowance in the design capacity of water sources for future extension of the water supply system by providing additional storage capacity. Accordingly, the proposed design capacities of the storage tanks are determined as shown below included additional 500m<sup>3</sup> to the storage capacity of the west bank system.

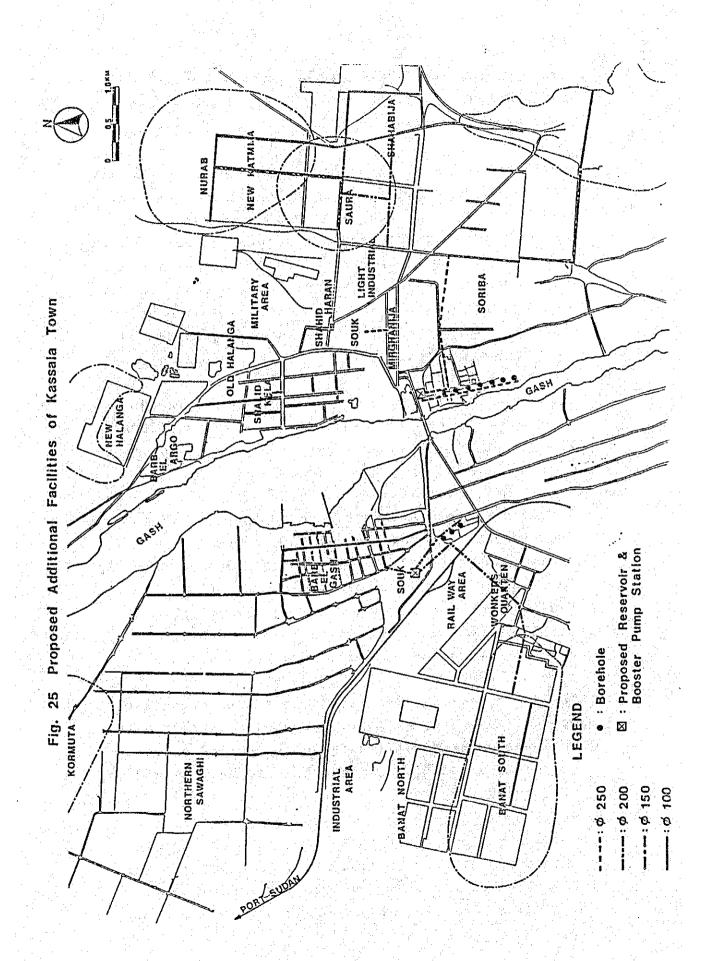
East Bank System  $16,600 \text{m}^3$  24 x 3 = 2,000 m<sup>3</sup> West Bank System  $7,800 \text{m}^3$  24 x 3 + 500 = 1,500 m<sup>3</sup>

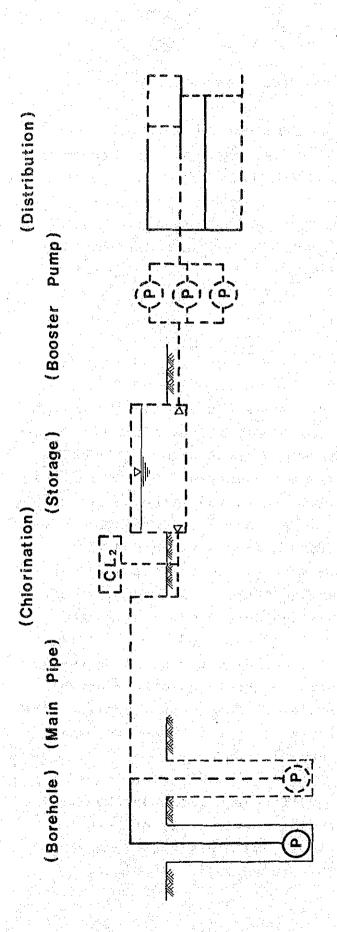
## 17. Service System

The water supply systems on the both bank sides are independent. In the both systems a booster pump station is provided. The design basis of each facility is the hourly maximum demand (Fig. 25 and Fig. 26).

#### 18. Metering System

There are 11,700 water meters installed at private connections, however most of them are out of order which results in difficulty in collection of water charge. No additional meters are to be provided in the project.





Exsisting	Proposed	Total
East Bank		e de la companya de La companya de la co
57.4Km	19.0Km	76.4Km
West Bank		
15.5Km	8.0Km	23.4Km
East Bank		
Non	0ne	One
	11.5m³/min	11.5m³/min
West Bank		
Non	One	One
	2.7m³/min	2.7m³/min
East Bank		
150m³	2,000m <sup>3</sup>	2, 000m³
WEST Bank		
75m³	1,500m³	1,500m³
East Bank		
abandoned	5	5
		2,076gr/hr
West Bank		
abandoned	3	3
		945gr/hr
East Bank		
412m³/hr	280m³/hr	692m³/hr
West Bank		
193m³∕hr	120m³/hr	313m³/hr
1	The second secon	and the second s

Fig. 26 Water Supply System of Kassala 4 - 31

#### 19. Supply of Electricity

The power supply at Kassala Town is made by 4 generators at 4,000 kW capacity. Power source of the town water supply is obtained from the same source.

Since there often occurs power supply failure it is proposed to provide the stand by generator for the Kassala Water Supply System.

#### 4.3 Description of the Project

### 4.3.1 Village Water Supply

It is requested by the Government to construct 42 village water supply facilities and to rehabilitate 165 boreholes of existing village water supply facilities. All these sites requested require urgent measures. Of these sites, 34 village water supply facilities and 73 rehabilitation of existing boreholes are proposed in the project due to various reasons. The water shortage problems will be solved at 107 sites. Accordingly the remaining 100 sites which are not proposed in the project will be provided for an urgent measures by the Government.

For construction of new boreholes a few drilling team will be required considering urgency and extent of the distribution of sites of the project. For rehabilitation of existing boreholes also a few service machine teams will be required by the same reasons.

At present there are 19 drilling equipment in the NWC, however, they have to provide services all over the nation. It is, therefore very much unlikely to save drilling equipment for the project.

By all reasons above it is proposed to provide a few sets of drilling equipment and also a few sets of service machine for rehabilitation. For this reason sufficient technology transfer will be required for drilling and rehabilitation of boreholes. This is advantageous in two The number of the proposed sites is not small. aspects. Therefore providing necessary technology transfer through construction of new village water supply facilities and rehabilitation of existing boreholes, the remaining work of the project will be able to be completed by the Sudanese counter parts. Even after completion of the project there are 100 sites where urgently require water in the region. For this purpose the equipment and crew with sufficient technology to be provided by the project can serve the solution of the wter problem.

There are two major type of aquifers in the area; the alluvial deposits and the Nubian Sandstones. Therefore construction of 10 boreholes in each type of aquifer will be sufficient to make necessary technology transfer of drilling. For rehabilitation of existing boreholes, different conditions are expected in each borehole. For this reason the rehabilitation of 30 to 40 numbers of existing boreholes will be required to make sufficient technology transfer.

The technology transfer is to be given to the staff from the NWC which holds sufficient numbers of staff for the project.

These equipment to be provided by the project will be transferred to the NWC after completion of the project.

Under the project, the village water supply includes following two items;

1.	Construction of Village Water Supply facilities:	34 places
2.	Rehabilitation of Boreholes of	
;	Existing Village Water Supply Facilities:	73 places

Above two items include water supply schemes at refugee camps as summarized below;

	Site Name	No. of Boreholes
New Village	Wad El Hellow	2
Water Supply	Wad Sheref	2
	Helet Hcoma	1.
	Wad Kole	2
	Um Ali	1
Rehabilitation	Um Gargour	2
of Boreholes	Karkorer	2
	Wad El Helow	<b>3</b>
	Wad Sharef	4
	El Suki	2
	Tawewa	2

The rural population to be served by these schemes is estimated to be 210,000 people including refugees.

# 4.3.2 Kassala Town Water Supply

Rehabilitation of the Kassala Town water supply facilities includes construction of civil works and pipe laying for distribution main, as summarized below;

1.	Boreholes	•	10 holes
2.	Submersible Motor Pumps	•	10 units
3.	Storage Tank, 2,000 m <sup>3</sup>		
	and 1,500 m <sup>3</sup>	:	one each

4. Chlorination Facilities

8 units

5. Booster Pump Stations

2 places

6. Distribution Main

27 km

For drilling boreholes, the equipment to be provided for the village water supply facility will be utilized. Further details of the drilling equipment is discussed in 5.5 Equipment in Chapter 5 BASIC DESIGN.

A significant extent of the problematic area in the Town results in problems of the water revenue of Kassala UWA, however, this situation will be much improved by the project.

## 4.3.3 Operation and Maintenance

Responsibility for operation and maintenance of the village water supply falls on the branch office of NWC at Kassala. On the other hand, UWA at Kassala is responsible for operation maintenance of the Kassala Town Water Supply.

CHAPTER 5 BASIC DESIGN

# CHAPTER 5 BASIC DESIGN

## 5.1 Basic Considerations for Design

Since this is an urgent project, easy implementation with the reasonable design is required. The standards and criteria officially used in Sudan are to be applied unless there found any difficulties.

In addition, existing climatic conditions and transportation are another importance for determination of the work schedule.

## 5.2 Village Water Supply

#### 5.2.1 Introduction

The project includes 34 new village water supply facilities and 74 rehabilitation of existing boreholes. For this purpose the standard specifications are applied for the Basic Design. For the Detail Design further determination of specifications will be required. The proposed village water supply system is shown in drawing No. 7.

# 5.2.2 Standard Design for New Boreholes

# 1. Boreholes (Drawing No. 1)

The standard specification of the borehole depth is proposed to be 180m since this depth is an average depth of boreholes in the project. The finish diameter of the borehole is proposed to be 6 5/8 inches which is the most common diameter of the

existing boreholes aiming at easiness of construction and high exchangeability of material and machinery. Accordingly drilling diameter is required in 9 5/8 inches.

The length of screens is 10% of total depth of the borehole on an average referring to the existing borehole design.

Construction of new boreholes are required for 34 sites and the standard design of the proposed borehole is shown in the drawing No. 1.

2. Pump and Engine (Drawing No. 2)

There are three types of pumps for boreholes;

- 1) Submersible Motor Pump
- 2) Reciprocal Pump
- 3) Vertical Turbine Pump

Considering availability of power source and operation and maintenance requirements submersible motor pump shall be discarded in the project. Vertical turbine pumps are commonly used in a wide areas over the East African countries. Boreholes recently constructed at refugee camps in Sudan are also equipped with this type of pumps. However, there used to be installed reciprocal pumps at older boreholes in the Eastern Region. In order to determine the type of pumps to be used under the project comparison between the vertical pump and the reciprocal pumps is made as below;

Comparison	Reciprocal Pump	Vertical Turbine Pump
Efficiency		higher
Installation	same	same
Durability	same	same
Maintenance	easier	
Cost	much higher	
Running cost		lower

Although reciprocal pumps are installed on the old boreholes in the project sites, vertical turbine pumps are common in boreholes recently constructed and are familiar to the local people. In addition the advantage of vertical pumps are the high efficiency and the small price. Therefore the vertical turbine pumps are applied for the project. A standard capacity of Q=180 l/min and H=100m is applied for this basic design since almost of 60% of required pump capacity of the project sites falls on the range of this capacity.

The standard specifications of the pump and engine are summarized as below;

#### 1) Pumps

Q = 180 1/min

H = 100 m

HP = 11 HP

 $\phi = 142 \text{ mm}$ 

34 units

#### 2) Engines:

Vertical, Water Cooling Four cycles

HP = 15 HP, 1,800 rpm

34 units

Pipes and Accessories

34 sets

3. Pump House (Drawing No. 3)
Prefabrication Type

34 units

4. Storage Tanks (Drawing No. 4)

The standard specification of the NWC is applied;

FRP Panel type  $Q = 50 \text{ m}^2$ 

Prefabricated Steel Holder; 3 m Dimension; 5m x 5m x 2m

34 units

5. Pipings (Drawing No. 5)

34 units

6. Public Taps, Q = 1 lit/sec.

34 sets

7. Fences, 50m x 75m (Drawing Nos. 8 & 9)

34 units

5.2.3 Standard Design of Existing Boreholes

The same standard designs as new village water supply boreholes, and pump and engine are proposed.

Pumps
 Engines
 Piping Connection

73 units

73 units

73 units

5.3 Rehabilitation of Kassala Town Water Supply

5.3.1 Water Source

The standard design of the boreholes for Kassala Town water source is shown in the drawing No. 1 and No. 2.

#### 1. Design Capacity

The design basis is the hourly maximum demand; 16,600 m<sup>3</sup>/day for the east bank system and 7,5000 m<sup>3</sup>/day for the west bank system. Therefore, it is determined as below;

East Bank System;  $16,600\text{m}^3/\text{day}$  24 hrs =  $1,038\text{m}^3/\text{hr}$  West Bank System;  $7,500\text{m}^3/\text{day}$  24 hrs =  $469\text{m}^3/\text{hr}$ 

#### 2. Number of Boreholes

The safe yield of boreholes at the aquifer of alluvial deposits of the Gash River is estimated to be  $40~\text{m}^3/\text{hr}$ . Therefore the required number of boreholes is determined as below;

East Bank System;  $692m^3/hr$   $40m^3/hr = 17$ West Bank System;  $313m^3/hr$   $40m^3/hr = 8$ 

Considering the number of existing boreholes the number of additionally required boreholes is determined as below;

East Bank System; Existing 10 holes + New 7 holes West Bank System; Existing 5 holes + New 3 holes

## Depth of Boreholes

Referring to the depth of existing boreholes at Kassala Town it is proposed to be  $50\ \text{m}$ .

#### 4. Diameter of Boreholes

The diameter of boreholes is proposed to be 250 mm (10 inches) considering the capacity of aquifer and the size of pump.

#### 5. Length of Strainer

The required length of the strainer is obtained by the following formula;

$$L = \frac{Q}{A \times N \times V}$$

where,

A: area of aquifer facing to strainer; m<sup>2</sup>

N: opening ratio of strainer; %

V: critical velcity; m/sec

and,

 $A = 2 r = 2 \times 3.14 \times 0.125 = 0.875 \text{m}^2$ 

N = 15%

V = 1.5 cm/sec = 54 m/hr

Therefore, L is determined to be 8m and the length of strainer is proposed to be 10m.

### 6. Location of Boreholes

Location of boreholes is on the banks of the Gash River.

#### 7. Drilling Diameter

Considering necessary spacing allowance for casing, the drilling diameter is proposed to be 450 mm.

#### 8. Pumps

Considering availability of the power source and the efficiency of pumps, the submersible motor pump is proposed:

$$Q = 40 \text{ m}^3/\text{hr}$$
  
 $H = 60 \text{ m}$   
 $\phi = 150 \text{ mm}$   
 $P = 22 \text{ kW}$ 

10 units

#### 5.3.2 Chlorination

The hypochlorous potadium acid is recommended for chlorination considering existing type of equipment.

Required capacity is 3ppm injection to maintain more than 0.4ppm residual chlorine for the hourly maximum demand;

East Bank System; 
$$692m^3/hr \times 3ppm = 2,076kg/hr$$
  
 $500gr/hr \times 5$  units  
West Bank System;  $315m^3/hr \times 3ppm = 945kg/hr$   
 $500gr/hr \times 3$  units

#### 5.3.3 Storage Tanks

The basic designs of the proposed storage tanks are shown in the drawing No. 6.

Required capacities of the storage tanks are;

East Bank System; 2,000m<sup>2</sup>
West Bank System; 1,500m<sup>2</sup>

The reinforced concrete structure is proposed in the dimension below;

East Bank System; 3m x 29m x 3m West Bank System; 3m x 18m x 3m

### 5.3.4 Distribution Mains

Asbesto cement pipes are installed in the existing pipe network, however polyvinyl chloride pipes are recommended for additional pipes considering the durability, workability and costs. Proposed piping network is summarized as below;

Proposed Piping Network

		East Bank				
Diameter	Existing	Proposed	<u>Total</u>	Existing	Proposed	<u>Total</u>
10"	5.4km	3.5km	8.9km		•••	-
8"	-	3.0km	3.0km	1.0km	2.5km	3.5km
6 "	18.2km	6.1km	24.3km	6.1km	1.9km	8.0km
4 "	33.8km	6.4km	40.2km	8.3km	3.6km	11.9km
Total	57.4km	19.0km	76.4km	15.4km	8.0km	23.4km

The piping density will be much improved by the proposed additional pipe lengths of 27 km (Fig. 27).

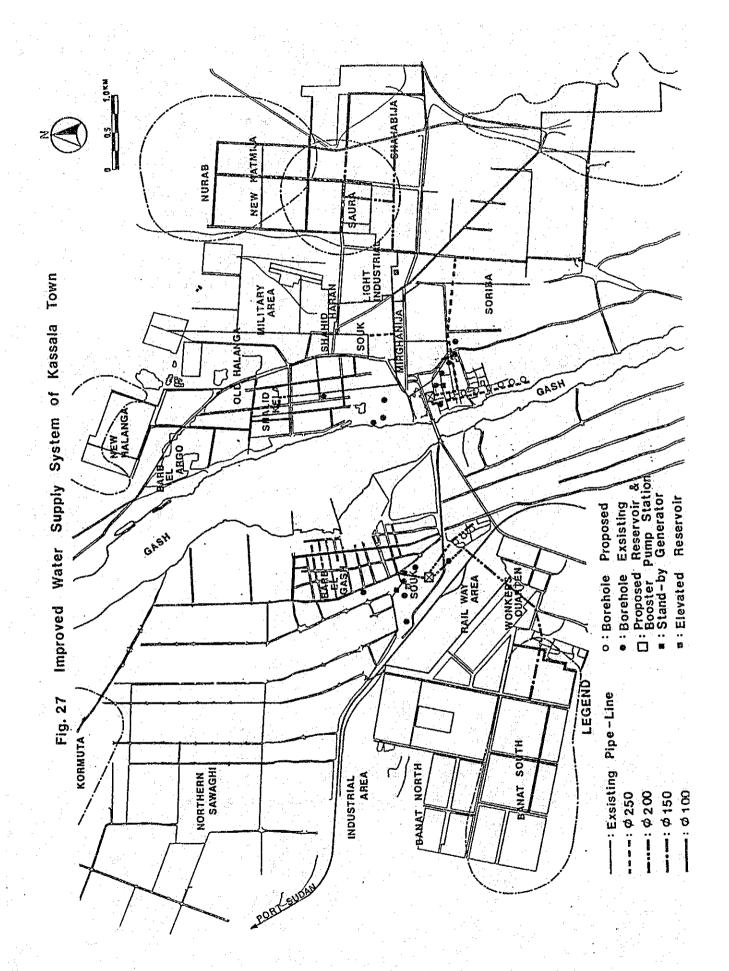
## 5.3.5 Booster Pump Station

Design basis of the booster pump station is the hourly maximum demand.

East Bank System; 5.8m<sup>3</sup>/min x 3 units West Bank System; 2.7m<sup>3</sup>/min x 3 units Specifications of pumps are proposed as below;

$\frac{1}{2} \frac{1}{2} \frac{1}$	East Bank	West Bank	
Type;	Volute Pump	Volute Pump	
Diameter:	ø 300mm	ø 200mm	
Head;	40 m	4 0 m	
Power;	75kW	30kW	
Number	3 units	3 units	

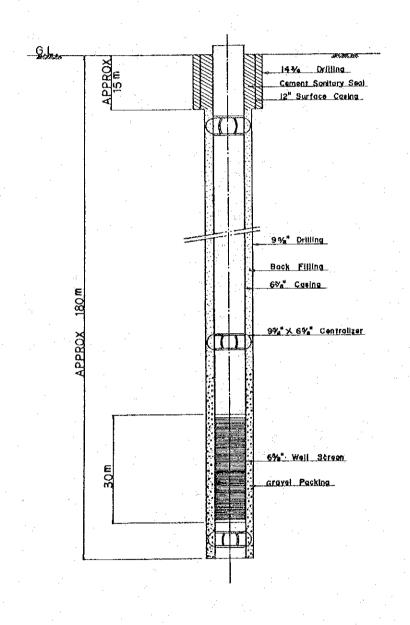
The Basic Design of the booster pump station is shown in the drawing No. 7.

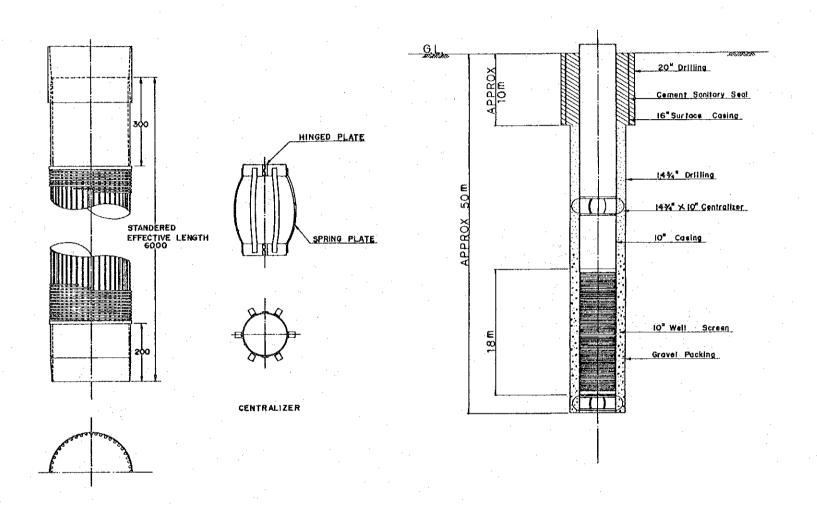


# NEW - DRILLING WELLS

# CONTINUOUS V-SLOT WELL SCREEN

# NEW-DRILLING WELLS FOR KASSALA





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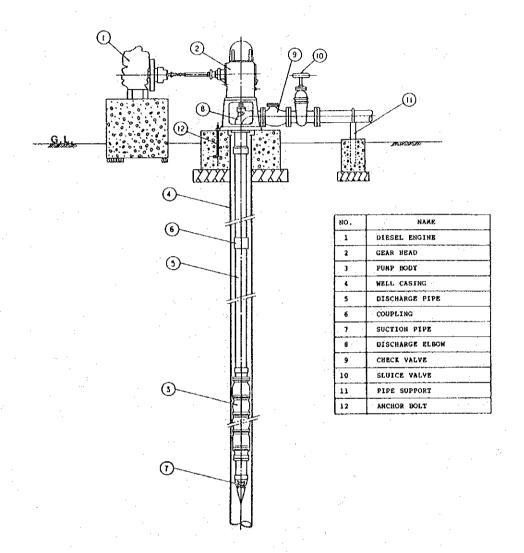
WATER SUPPLY PROJECT RELATED
TO ICARA II

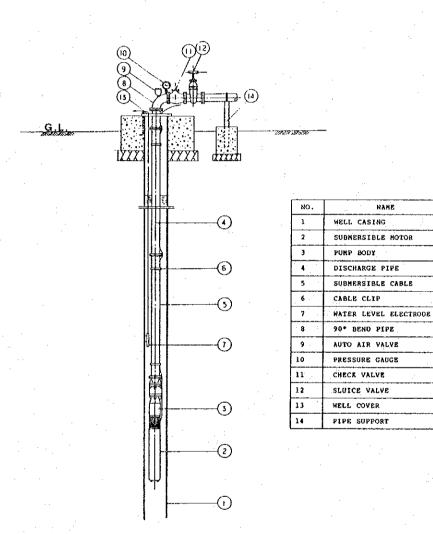
STANDARD DESIGN OF BOREHOLE

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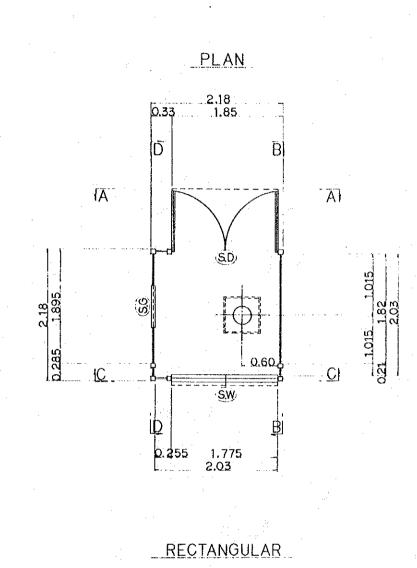
JAPAN INTERNATIONAL COOPERATION
AGENCY (JICA)

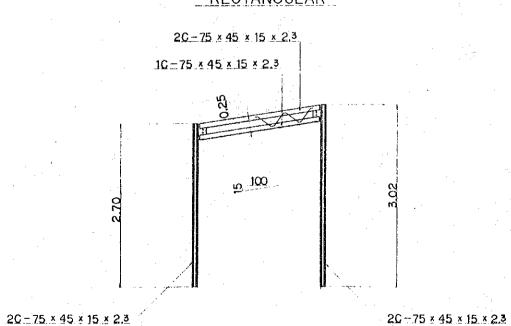
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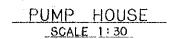


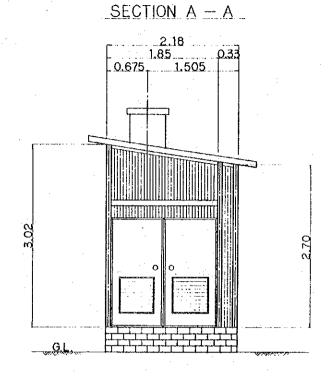


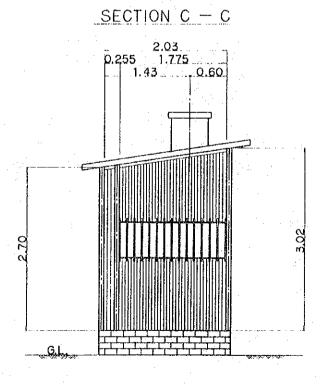
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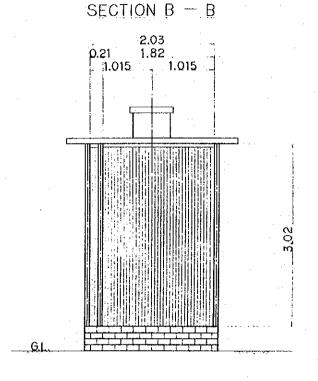


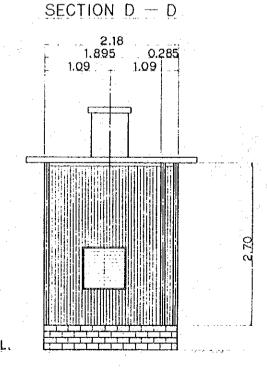








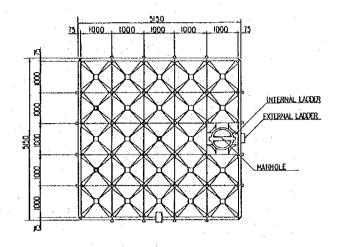


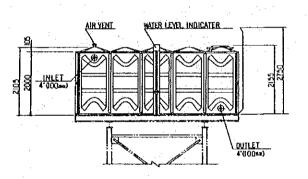


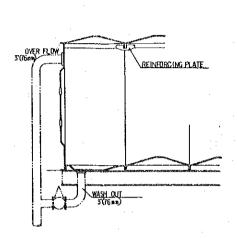


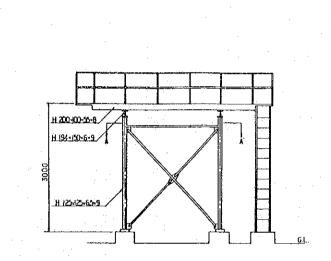
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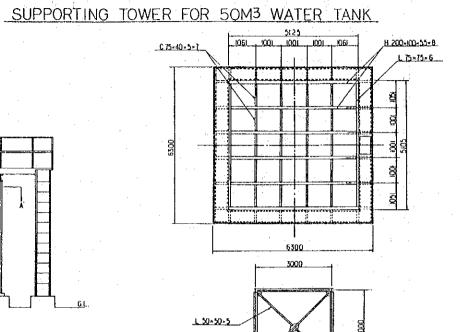
## PANEL TYPE WATER TANK 50M3







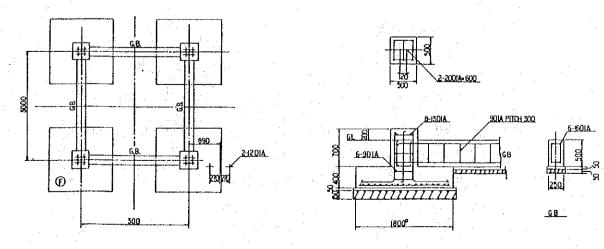




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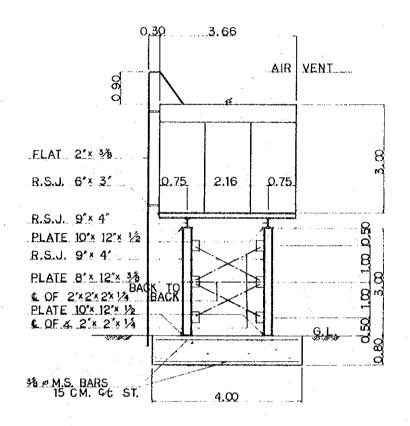
# CONCRETE FOUNDATION FOR 50M3 WATER TANK

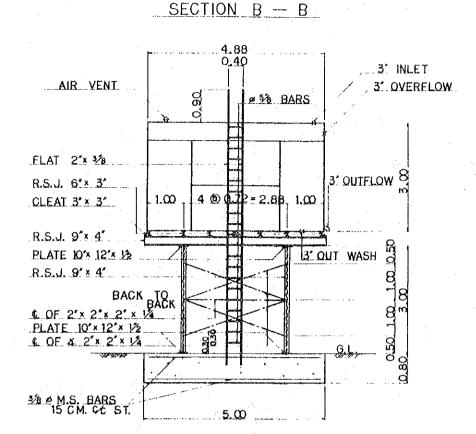


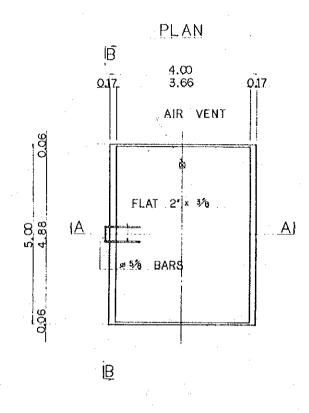
REPUBLIC OF THE SUDAN COMMISSIONER FOR REPUGEES ICOR)
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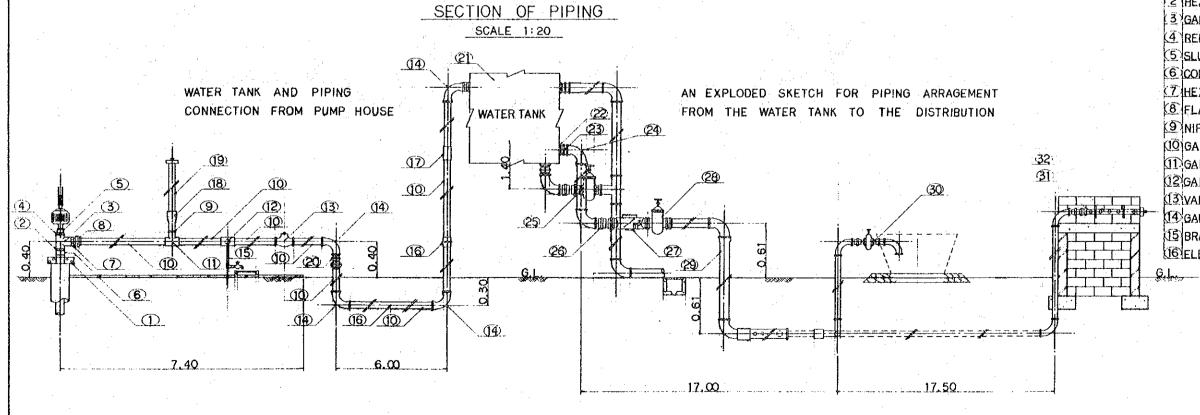
# INDICATIVE DRAWING SCALE 1:50









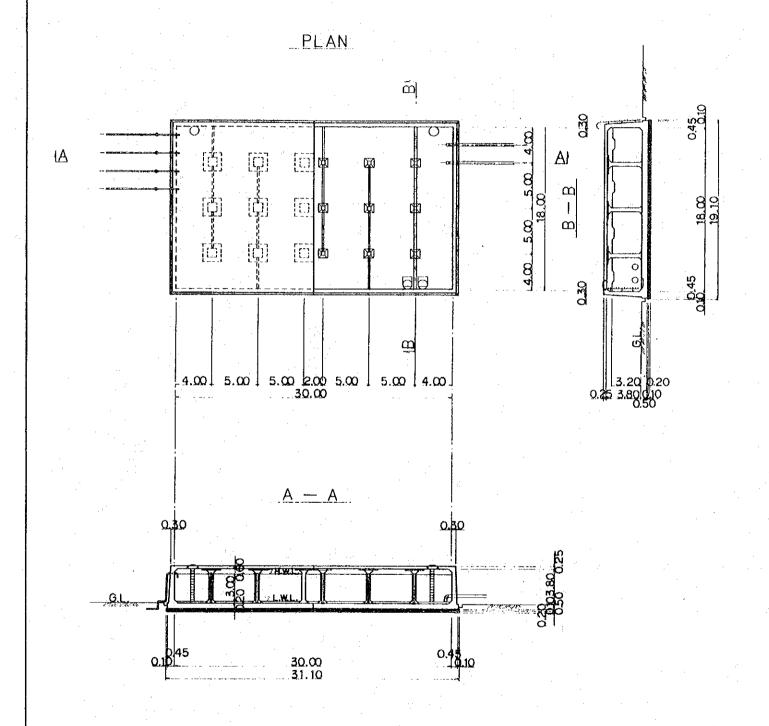


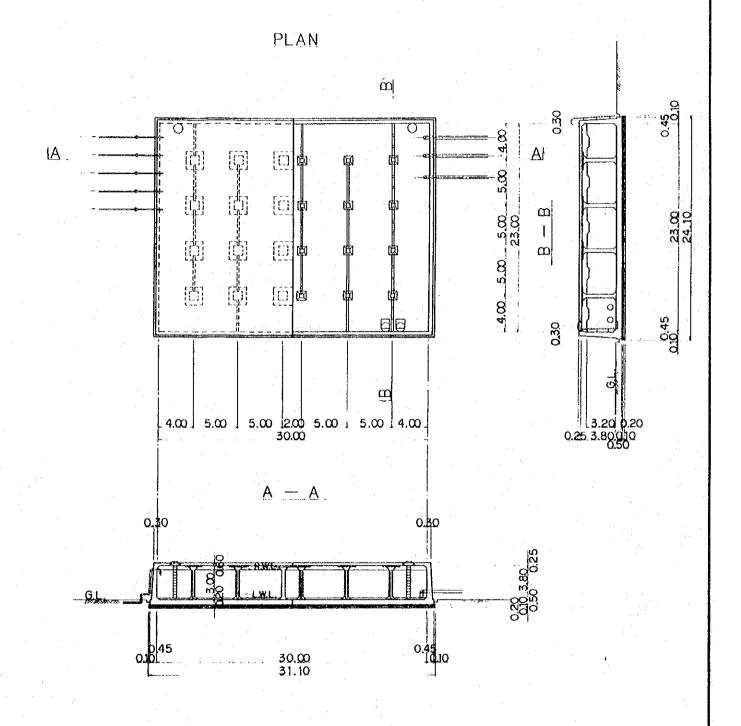
1 COLUMN PIPE CLAMP417 GALU. SOCKET 3 2 HEXAGONAL NIPPLE 4 18 REDUCING SOCKET 4'x 3 3 GALU. IEE 4' 19 HEXAGONAL NIPPLE 3 4 REDUCING BUSH 4'x 3' 20 NIPPLE 3' 21 WATER TANK 5 SLUTTING BOX 3 6 COLUMN PIPE 4' 22 NIPPLE 4' THEXAGONAL NIPPLE 3 23 ELANGE 4 8 FLANGE 3" 24 BEND 4' 25 GALU. PIPE 4 26 NIPPLE 4' (10) GALU, PIPE 3' 11 GALU. TEE 3' x 3' 27 WATER METER 4' x 12 GALU. TEE 3' x 2' 28 VALVE 4'\_ (3) VALVE 3' 29 PIPE 4" (4) GALU. BEND 3" 30 NIPPLE 2' 15 BRASS BIPCOCK 1' 3) TEE 2'x 1' 16 ELBOW 3" 32 GALU. PIPE 2'

REPUBLIC OF THE SUDAN
COMMISSIONE FOR REPUGEIS KON
MINISTRY OF INTERIO AFFAIRS
WATER SUPPLY PROJECT RELATED
TO ICARA II

DATE FEB. '80 1:50, 1:20 DWG No. 1

5 - 15 JAPAN INTERNATIONAL COOPERATION
AGENCY (JICA)





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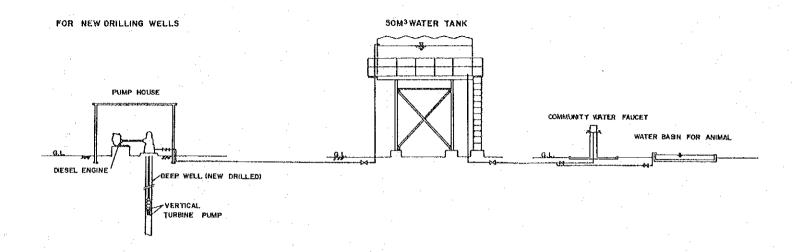
TO ICARA II

BASIC DESIGN OF STORAGE TANK

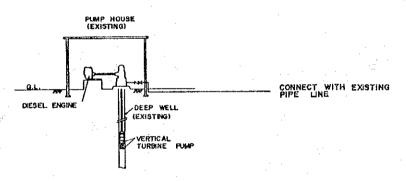
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JAPAN INTERNATIONAL COOPERATION
AGENCY (JICA)

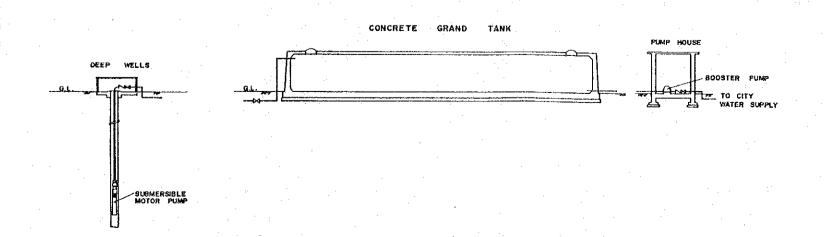
## TYPICAL DIAGRAM OF WATER SUPPLY SYSTEM



#### FOR REHABILI TATING WELLS

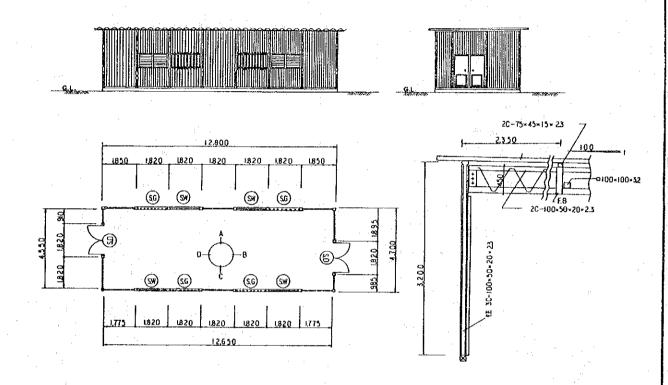


#### TYPICAL DIAGRAM OF WATER SUPPLY SYSTEM (FOR KASSALA CITY)



## PUMP HOUSE FOR BOOSTER PUMP

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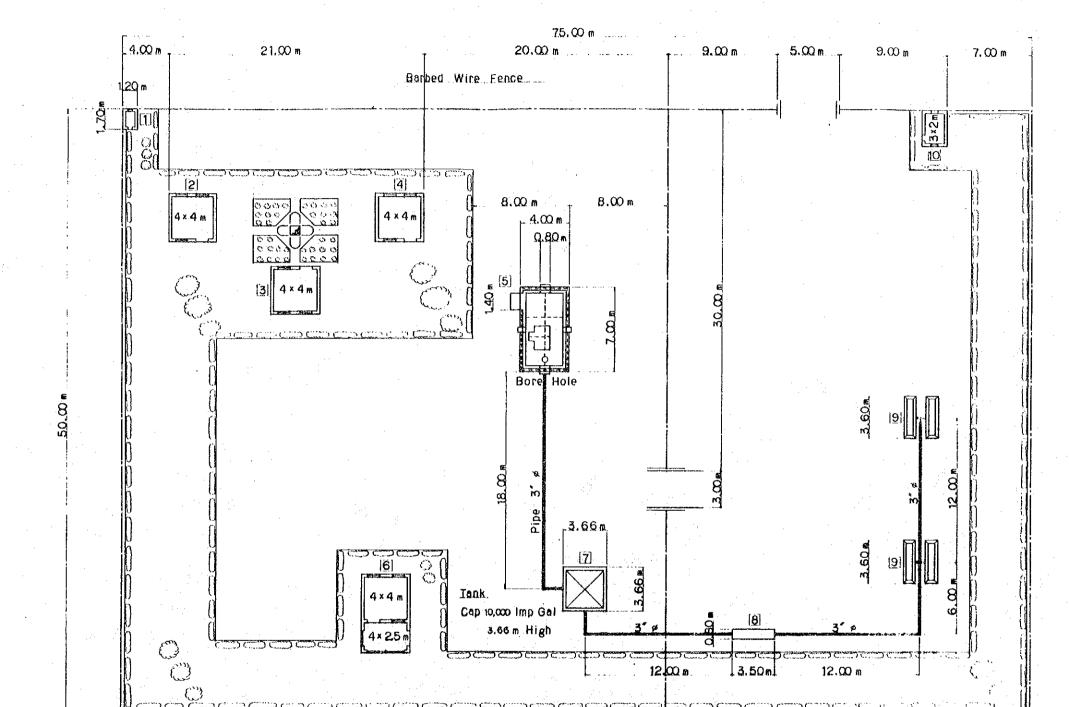


REPUBLIC OF THE SUDAN COMMISSIONER FOR REFUGEES ICOR)
MUNISTRY OF INTERIO AFFAMS WATER SUPPLY PROJECT RELATED TO ICARA II BOOSTER PUMP AND TYIPICAL DIAGRAM

OF WATER SUPPLY SYSTEM

DATE FES. '86 NON- SCALE DWG. No. 7 JAPAN INTERNATIONAL COOPERATION
AGENCY (JICA)

5 - 17



[1] Pitlatrine

on the control of the

- [2] Clerk Room
- [3] Operators Room
- [4] Keepers Room
- [5] Pumping House
- [6] Rest House
- [7] Tank
- 8 Dis Plat Form
- [9] Trough
- 10 Ticket Office

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WATER SUPPLY PROJECT RELATED

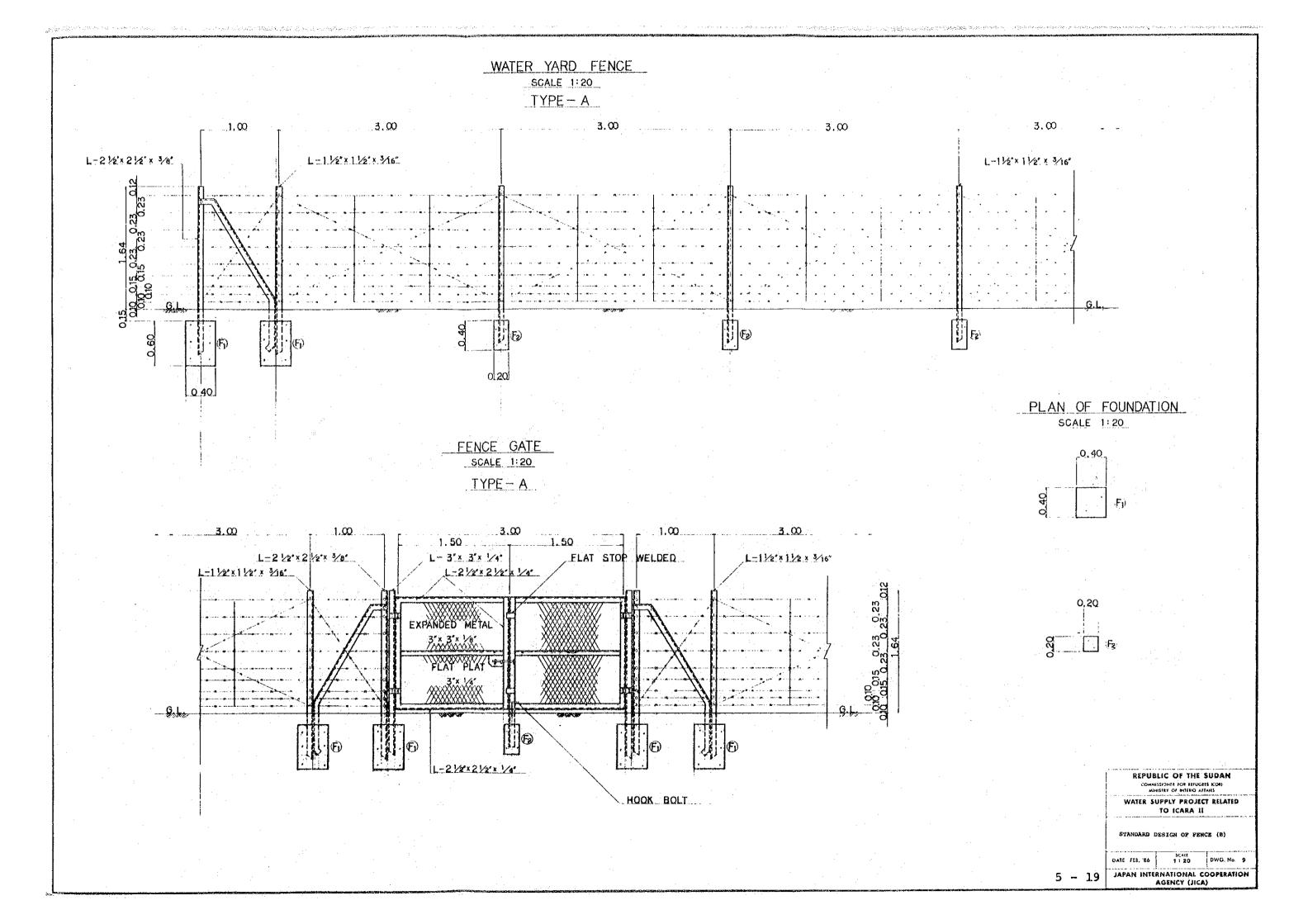
TO ICARA II

STANDARD DESIGN OF FENCE (A)

DATE FEB. 86 1:160 DWG. No. 8

5 - 18

JAPAN INTERNATIONAL COOPERATION
AGENCY (JICA)



# 5.4 Summary of Proposed Material

Different kinds of equipment, machinery and material are required for the project. Of these items proposed machinery and materials are summarized as below;

1. New Village Water Supply Facilities: 34 sites

				0
	Item	Specifications	Unit	Quantity
1. 1	Material for New Boreholes: 34 sites			
1)	Casing and Screen			
	(1) Casing	12" x 5.5m	pcs	102
	(2) Casing	6 5/8 <sup>11</sup> x 6m	pcs	850
	(3) Screen	6 5/8" x 6m	pcs	170
	(4) Centralizer	6 5/8" x 9 5/8"	pcs	408
2)	Mud Material			
	(1) Bentnite		ton	100
	(2) Chemicals		kg	1,000
3)	Bits			
	(1) Tricon Bit	14 3/4" S	pcs	17
	(2) Tricon Bit	9 5/8"	pcs	17
	(3) Tricon Bit	9 5/8" S	pcs	17
	(4) Air Hammer Bit	9 5/8"	pcs	17
2.	Related Facilities (34 holes)			
1)	Vertical Turbine Pump	1801it/n x 100m	set	34
2)	Spare Parts		set	34
3)	Material for Pump House		set	34
4)	Elevated Tank	50m <sup>3</sup> , 3mH	set	34
5)	Pipe Material		set	34

# II. Rehabilitation of Existing Boreholes

Item	Specification	Units	Quantity
1. Material and Machinery (73 holes)			
1) Vertical Turbine Pump	180 lit/m x 100m	set	73
2) Spare Parts		set,	73
3) Pipe Material	set	73	
2. Rehabilitation Chemical (73 holes	3)		
1) Well Cleaner	22,000	kg	1,100

## III. Rehabilitationof Kassala Water Supply

· .				
	Item	Specifications	Unit	Quantity
. Water	Source Machinery			
1) Subm	ersible Motor Pump			4
(1)	•	$40 \text{ m}^3/\text{h} \times 60\text{m}$	set	10
(2)	Spare Parts		set	10
2) Casi	ng and Screen			
. (1)	Casing	16" x 5.5m	pcs	20
	Casing	10: x 5.5m	pcs	60
(3)		10" x 6.0m	set	30
(4)	Centralizer	10" x 14 3/4"	set	33
3) Bent	nite	:	ton	15
4) Bits				
(1)	Tricon Bit	20" S	pcs	2
(2)	Tricon Bit	14 3/4" MH	pcs	5
(3)	Tricon Bit	14 3/4" S	pcs	5
2. Stora	ge Tank and Booster Pump St	ation	:	
**	age Tank			
	Generator	30 kVA	unit	10
(2)	•		set	10
	Pipe Material	∮125mm x 5.5m	pcs	1,820
	Accessory		pcs	. 1
(5)	Piping Connection		pcs	2
			<u> </u>	
	ster Pump			•
(1)	Pump House	350m <sup>3</sup> /h x 40m	unit set	2
(7)	Pumps Sparts	330m2/H x 40m		3
	Spare Parts	3	set	3
(3)		150m2/h v //0m		
	Pumps Spare Parts	$150 \text{m}^3/\text{h} \times 40 \text{m}$	set set	3

Item	Specifications	Unit	Quantity	
3) Chlorination Equipment	500/h	set	8	
4) Pipe and Valves		<u> </u>	<del>ره دره سره ال</del> استندان مسترسط <u>و سوم برق بو و سوم میشد.</u> د	
(1) Pipes	∮100A x 50m	pcs	2,000	
(2) Pipes	ø150A	pcs	1,600	
(3) Pipes	ø200A	pcs	1,100	
(4) Pipes	ø250A	pcs	700	
(5) Stopper	100A	pcs	34	
(6) Stopper	150A	pcs	27	
(7) Stopper	200A	pcs	19	
(8) Stopper	250A	pcs	12	
(9) Air Valves	25mm	pcs	28	
(10) Joints	•	set	1	

#### 5.5 Equipment

#### 5.5.1 Introduction

It is proposed to provide equipment for drilling and rehabilitation of boreholes which will be utilized for further underground water development in the Region even after completion of the project.

For determination of the specification and the quantity of these eqipment, considerations were given to various aspects of the project; the guideline of the Grant Aid Cooperation, urgency of the project, necessary technology transfer, work volume required for the project and existing conditions in Sudan.

# 5.5.2 Equipment for Construction of New Boreholes

Drilling equipment has to be a complete unit of all necessary items including camping facility, pump test equipment and well logging apparatus.

For selection of the drilling equipment, type of aquifer and required depth and available construction period have to be taken into account.

Major types of aquifers in the project area are alluvial deposits and the Nubian Sandstone. Most of sites distribute in the area of the Nubian Sandstone and drilling depth required is 180m. Considering these conditions and urgency of the project it is proposed to provide for the rotary type drilling machine with the air hammer. The proposed specifications for the drilling equipment are summarized as below;

Type: Track mounted rotary machine

Capacity:  $4 \frac{1}{2} \operatorname{inch} \times 250 \text{ m}$ 

Method: Mud circulation and Air Hammer

Mast Load: More than 40 ton

Mud Pump: 1,500 1/min, 20 kg/cm<sup>2</sup>

Truck:  $6 \times 6$ 

Spare Parts: One set

Considering required period of purchasing the drilling equipment and duration of the rainy season, it takes approximately two years for the project period. The first year is for procurement of the drilling equipment and the dry season in the second year is for construction.

There are three factors to determine the required number of the drilling equipment; the drilling capacity of the equipment, the available time for the project and the necessary work volume to complete necessary technology transfer.

Since the major types of aquifers in the project area are alluvial deposits and Nubian Sandstones, construction of ten boreholes in each aquifer will be sufficient to transfer necessary technology to complete the remaining construction of 24 boreholes by Sudanese counterparts.

The necessary period for construction of one borehole is estimated considering the drilling capacity of the proposed equipment and the nature of aquifer;

Alluvial deposits; 23 days for 50m Nubian Sandstones; 39 days for 180m

Assuming working days in a month are 25 days discarding Friday, total duration required for drilling is determined as below;

 $\frac{39 \text{ days}}{25 \text{ days}} \times 10 \text{ holes} + \frac{23 \text{ days}}{25 \text{ days}} \times 10 \text{ holes} = 24.8 \text{ months}$ 

On the other hand, the available time for construction during the dry season is 8 months. Therefore, required units of the drilling equipment is determined as below;

 $\frac{24.8 \text{ months}}{8 \text{ months}} = 3.1 \text{ units}$ 

Therefore it is proposed to provide 3 units of the drilling equipment.

5.5.3 Equipment for Rehabilitation of Existing Boreholes

The same considerations to determine the required number of the drilling equipment units are necessary to determine the number of the rehabilitation equipment. However, rehabilitation of existing borehole requires different kinds work depending on the required nature of rehabilitation. For this reason it is proposed to rehabilitate 36 holes for technology transfer.

Considering required work volume for rehabilitation of boreholes, 13 days will be required for rehabilitation of one borehole on an average.

Available time for rehabilitation is the same duration of construction of new boreholes; 8 months. Therefore, required number of rehabilitation equipment is determined as below;

 $\frac{13 \text{ days}}{25 \text{ days}} \times 36 \text{ holes} \div 8 \text{ months} = 2.3 \text{ units}$ 

Therefore it is proposed to provide 2 units of rehabilitation equipment. The proposed specifications for rehabilitation equipment are summarized as below;

Truck Mounted Cable Tool Туре Single Line Main Drum 5 ton Capacity 19mm x 240m Rope Speed Max. 50 m/min 25 ton Sand Reel 9 - 10 m Mast Height Mast Load 25 ton  $500 \text{ 1/min, } 22 \text{ kg/cm}^2$ Water Pump Truck 4 x 4 Spare Parts One set

### 5.5.4 Other Equipment

Since the construction work will be made by 6 working groups vehicles and communication facilities will be required. The working groups and required vehicles and wireless radio sets are summarized as below;

	Working Group	8-ton <u>Truck</u>	Pick- up	Station Wagon	Wireless Radio
1.	Head Office	2	1	2	1
2.	Drilling Team R-1	1	1	e e	1
	R-2 R-3	1 1	1		1 1
3.	Rehabilitation Team S-1 S-2	1	1	in de la seconda de la sec La seconda de la seconda d	1
4.	Construction Team for Village Water Supply C-1 C-2 C-3		1 1 1		1
5.	Pipe Laying at Kassala T-1 T-2	a 1	1		- 1

... cont'd

Working Group	8-ton Truck	Pick- up	Station Wagon	Wireless Radio
6. Construction Team for Kassala Reservoir and Booster Pumps	:			
P-1 P-2	1	1		
TOTAL	9	11	2	7

# 5.5.5 Summary of The Proposed Equipment

]	item	Unit	Quantity
1. Dri	lling Equipment	set	3
1)	Drilling Machine	set	3
2)	Compressor	set	3
3)	Test Apparstus	set	3
4)	Camping Facility	set	3
2. Dri	illing Material		
1)	Attachments	set	1
2)	Cementing Tools	set	1
3)	Measurement Apparatus	set	1
4)	Completion Material	set	1
5)	Water Tanks	set	. 1
6)	Fuel Tanks	set	1
7)	Electric Tools	set	1
8)	Welder	set	1
9)	Mechanical Tool	set	1
10)	Earth Work Tools	set	1
11)	Wires	set	1

	Item	U	nit	Quantity
3.	Construction of Water Supply Facilities			
	1) Machinery	S	et	1
	2) Watr Tank	· · · · · · · · · · · · · · · · · · ·	et	1
	3) Fuel Tank	S	et	· 1
	4) Welder	· · · · · · · · · · · · · · · · · · ·	et	1
	5) Mechanical Tool	S	et	1
	6) Earth Work Tool	S	et	1
	7) Frames	· _ 8	et	1
	8) Wires	s	et	, 1
4.	Rehabilitation Equipment			
	1) Service Machine		et	2
	2) Testing Apparatus	5	set	2
	3) Camping Facility	5	et	2
5.	Piping Material			
	l) Measuring Apparatus	·. ·	et	- 1
	2) Water Tank	S	et	1
	3) Fuel Tank	ş	et	. 1
	4) Electric Apparatus	· s	et	1
	5) Welder	· · ·	et	1
	6) Earth Work Tools	s	et	1
	7) Piping Tools		et	. 1
6.	Vehicle			
	1) 8 ton truck	3 ton crane u	ınit	9
	2) Spare Parts	5	et	9
	3) Station Wagon	Ü	ınit	2
	4) Spare Parts		set	2
	5) Pick Up	Ü	ınit	11
	6) Spare Parts	٤	set	11
7.	Wowkshop Tools	<u> </u>	set	1

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