3.5.3 Plan of New Treatment Plant

The study of the future water supply development made in the preceding section requires construction of new water treatment plants.

The planning of the new plants will be studied in the following:

3.5.3.1 Areas and Required Capacity

The areas to be covered by the new treatment plants will be the municipalities of the Categories D, M and N as defined in the preceding section. Some municipalities belonging to Category M will have their-own water sources and the future water demand over city-owned water source will be supplemented by the new plant.

Municipalities to be supplied by the new plants are listed below:

Table-3.5.3 Areas and Required Capacity by the New Treatment Plants (Year: 2005)

 (m^3/day)

| | | | | () () |
|----------|------------------------------------|-------------------------|---------------------------------------|---|
| Category | Municipality | Water Demand in 2005 | City-Owned Water Source in 2005 | Balance to be Met by New Treatment Plant |
| | Zagazig City | 154,200 | 25,000 | 129,200 |
| М | Bilbeis City | 43,255 | 12,000 | 31,255 |
| M | Faqus City | 24,086 | 10,000 | 14,086 |
| | Abu Kebir City | 30,304 | 10,000 | 20,304 |
| N | Huseiniya City | 7,391 | _ | 7,391 |
| ** | Kafr Saqr City | 6,570 | - | 6,570 |
| | Villages in Kafr Saqr Markaz | 32,755 | - | 32,755 |
| D | Villages in Huseiniya Markaz | 31,568 | - | 31,568 |
| | Villages in Faqus Markaz | 43,047 x 83% | - | 3 5, 730 |
| | Total Capacity | of New Plants | | 308,859 |

3.5.3.2 Alternative Study of New Treatment Plant Construction

water source of the new treatment plants will be surface water of large canals which flow in the Shargiya Governorate. As alternative plans for new treatment plans, the following two major cases will be studied, namely, case A. Ismailiya canal which has a perennial large flow, and Case B, other canals which are located close to the water consuming municipalities. Further the Cases will be studied in plural sub-cases.

As the result of technical preliminary study, the following seven alternatives of construction sites of new treatment plants were planned.

They are:

- Case A-1: One integrated plant at Bilbeis (Ismailiya Canal water),
- Case A-2: One integrated plant at Abbasa (Ismailiya Canal water),

or,

- Case B-1: One integrated plant at Zagazig (Muweis Canal water),
- Case B-2: Two plants,
 - 1) Northeast site of the Governorate (Saidiya Canal water)
 - 2) Zagazig (Muweis Canal water)
- Case B-3: Three plants,
 - 1) Northeast (Saidiya Canal water)
 - 2) Zagazig (Muweis Canal water)
 - 3) Bilbeis (Ismailiya Canal water)
- Case B-4: Three plants,
 - 1) Kafr Saqr (Muweis Canal water)
 - 2) Zagazig (Muweis Canal water)
 - 3) Bilbeis (Ismailya Canal water)
- Case B-5: Four plants,
 - l) Kafr Saqr (Muweis Canal water)
 - 2) Northeast (Saidiya Canal water)
 - Zagazig (Muweis Canal water)
 - 4) Bilbeis (Ismailya Canal water)

Above plans are shown in the following pages.

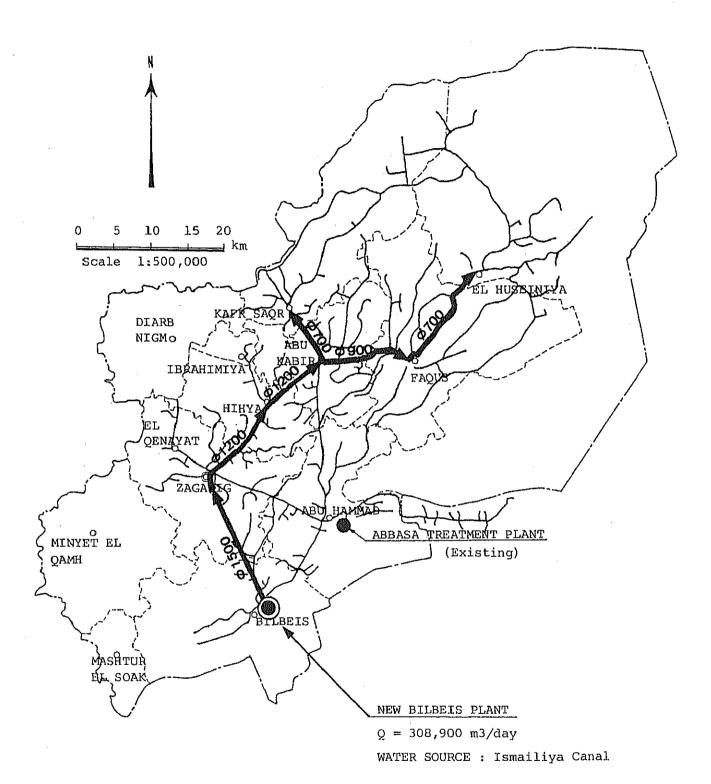


Fig-3.5.3 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, CASE A-1

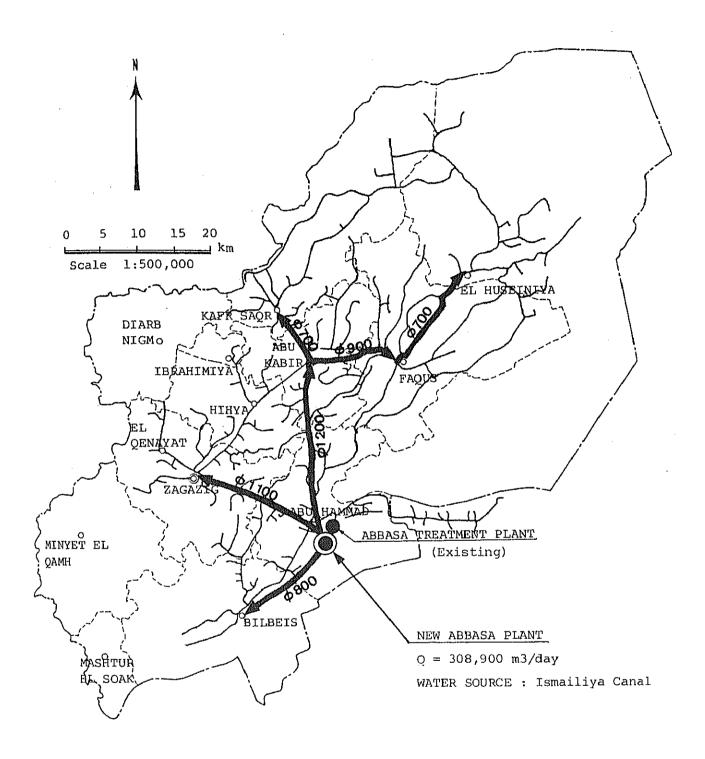


Fig-3.5.4 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, CASE A-2

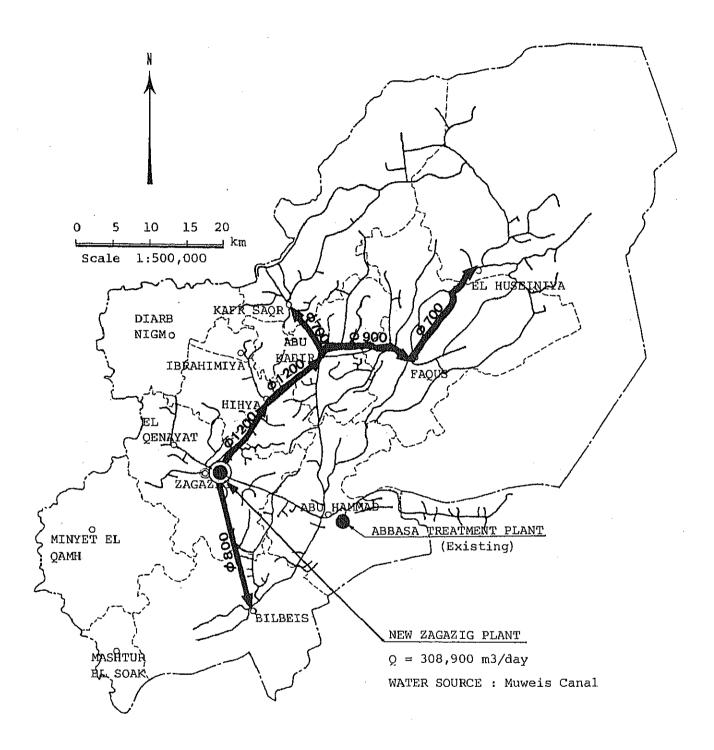


Fig-3.5.5 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, CASE B-1

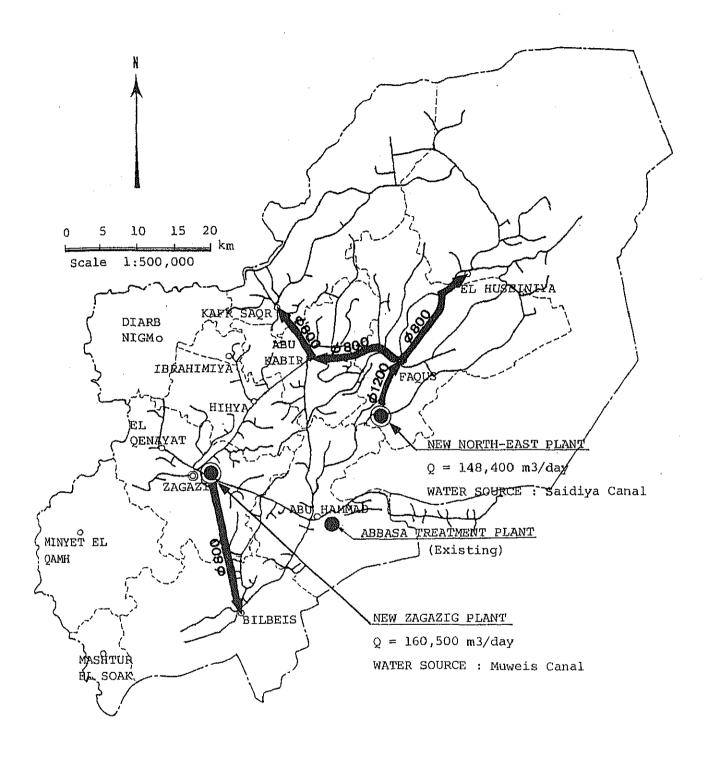


Fig-3.5.6 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, CASE B-2

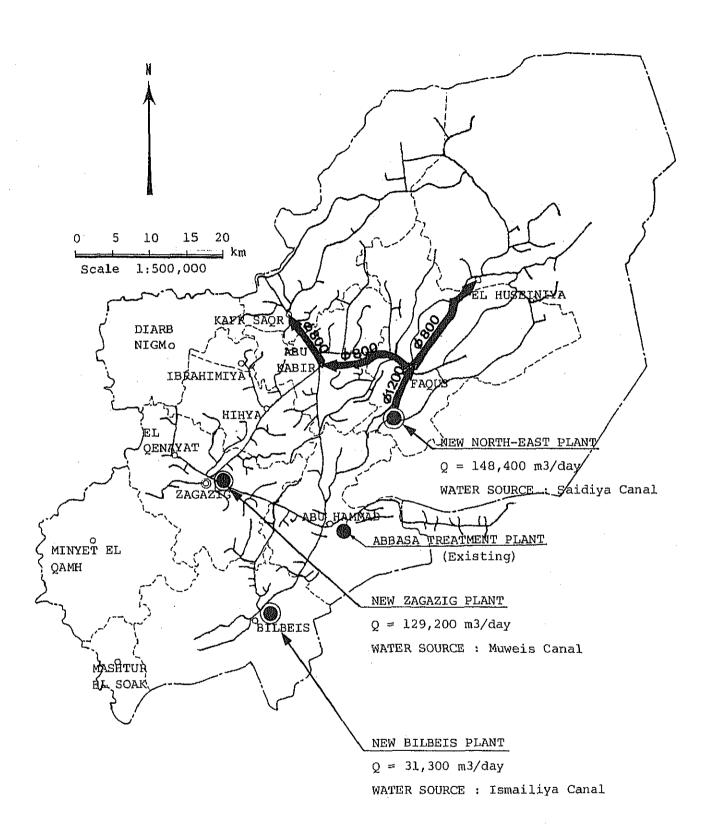


Fig-3.5.7 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, CASE B-3

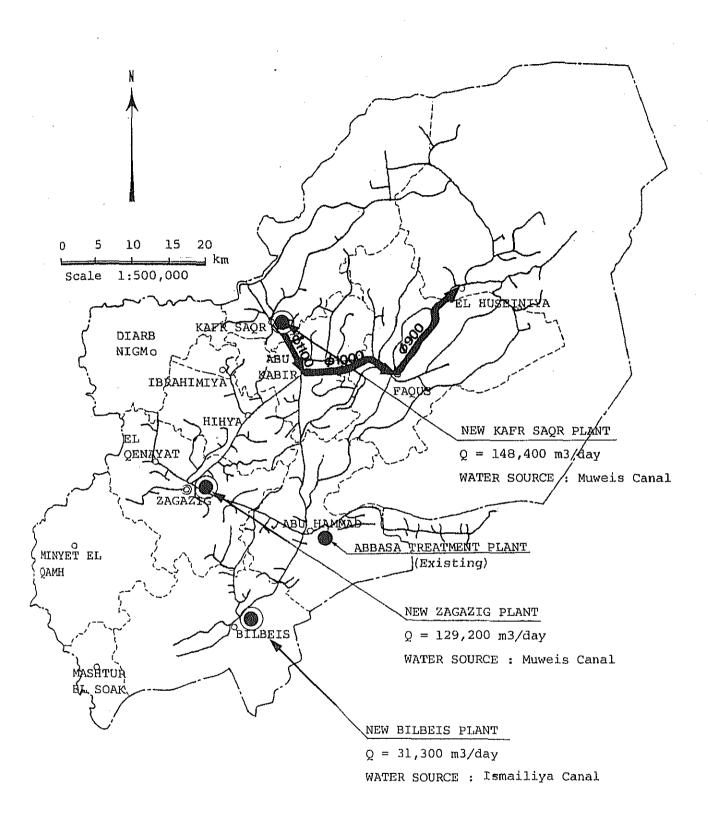


Fig-3.5.8 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, CASE B-4

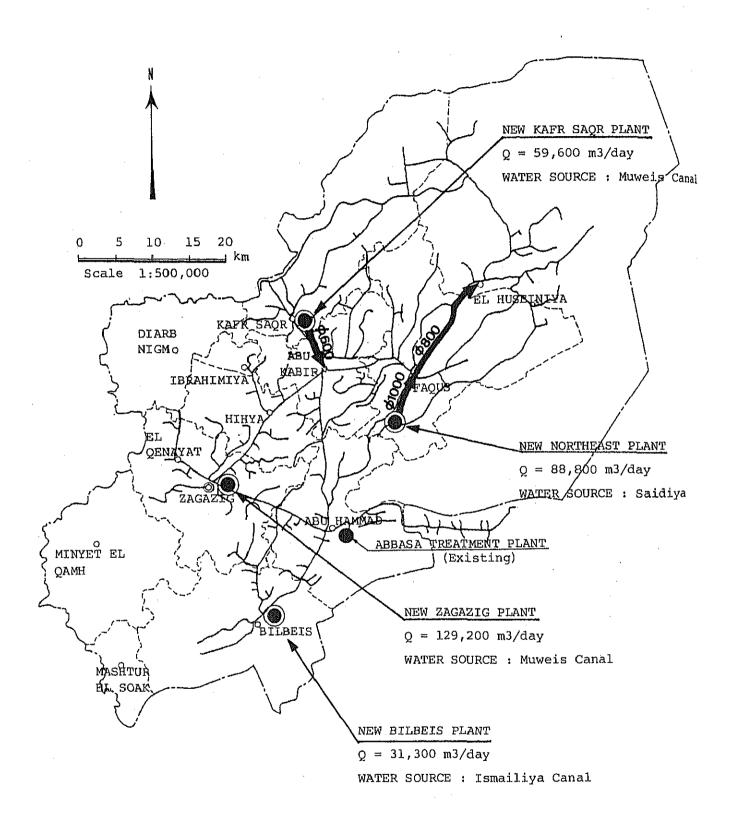


Fig-3.5.9 ALTERNATIVE PLAN OF NEW TREATMENT PLANT, B-5

3.5.3.3 Comparison of the Alternative Plans

From the general point of view, the seven alternative plans are preliminarily evaluated as follows:

Table-3.5.4 Comparison of the Alternative Plans

| Case | A-1 | A-2 | B-1 | B-2 | В-3 | B-4 | B-5 | | | |
|---|--------|----------------------|----------|--|-------|-------|-------|--|--|--|
| Construction cost (Million LE) 1/ | 151 | 150 | 125 | 120 | 117 | 123 | 115 | | | |
| Electric power required (kW) | | | | | | | | | | |
| Total | 11,730 | 10,046 | 8,632 | 6,593 | 6,593 | 3,509 | 4,958 | | | |
| Intake pumps | 522 | 522 | 522 | 531 | 504 | 255 | 258 | | | |
| Plant operation | 1,530 | 1,310 | 1,126 | 860 | 860 | 458 | 647 | | | |
| Distrubution pumps & booster pumps | 9,678 | 8,214 | 6,984 | 5,202 | 5,229 | 2,796 | 4,053 | | | |
| Number of personnel for plant operation | 109 | 108 | 105 | 198 | 273 | 271 | 365 | | | |
| Operation cost (Million LE) 2/ | 3.1 | 2.9 | 2.8 | 2.7 | 2.8 | 2.5 | 2.7 | | | |
| Present worth (Million LE) 3/ | 104.3 | 103.9 | 87.9 | 86.2 | 85.4 | 86.1 | 85.0 | | | |
| Phase-wise const- ruction method | | Difficult | t Easier | | | | | | | |
| Water source | | le throu- he year | | Approval of the canal authority is required. | | | | | | |

Note: 1/1984 prices, including transmission pipelines costs, and excluding distribution and service mains' costs.

^{2/} Operation cost at the final stage.

^{3/} Present worth for 20 years and 6% of discount rate.

3.5.3.4 Cost Comparison

The most important factors for evaluation of the alternative plans are considered construction cost and availability of water source. In Egypt, water right and availability of water of canal water are controlled by the authority concerned; therefore, for evaluation of the alternative plans construction cost shall be taken up as high priority.

Construction cost of each alternative plan is shown in the following page. The table of construction costs is prepared for cost comparison purpose and excludes costs for distribution pipelines and groundwater station construction, since their costs are common among the seven alternative plans.

The costs as of 1984 consisting of treatment plant construction, transmission installation and booster pumping station construction are taken up below:

Case A-1: LE 151 Million

Case A-2: LE 150 Million

Case B-1: LE 125 Million

Case B-2: LE 120 Million

Case B-3: LE 117 Million

Case B-4: LE 123 Million

Case B-5: LE 115 Million

Note: LE: Egyptian Pounds

(Exchange rate: 1 US\$ = 0.82 LE)

Table-3.5.5 COMPARISON OF CONSTRUCTION COSTS

| _ | | Cost | 35,500 | 27,500 | 20,800 | 13,232 | | 936 | | | | 936 | | 16,497 | | 1,023 | 17,520 | | Total 115,488 |
|----------------------------------|----------|-------------|--------------------------------|----------------------------------|----------------------------------|--------------|---|------------------------|----------------------|----------------------|--------------------------|-----------|---|--------------------------------|---------------------|------------------------------------|-----------|-----------|---------------|
| Unit : Thousand Egyptian Pounds) | Case 9-5 | Description | Zagazig Plant (129,200m3/d) | Northeast Plant (88,800m3/d) | Kafr Sagr Plant (59,600m3/d) | (31,300m3/d) | | fluseiniya Sta- | | | | Sub-Total | | Pipeline #1000-#600x40, | Pipe Bridge | Valve, Air valve Drain & others | Sub-Total | | Total |
| nd Egy | | Cost | 35,500 | 40,300 | 13,232 | 89,032 | | 4,102 | | | | 4,102 | | 779, 72 | | 1,716 | 29,393 | | 122,527 |
| it : Thousa | Case B-4 | Description | Zagazig Plant (129,200m3/d) | Kafr Sagr Plant (148,400m3/d) | Bilbeis Plant (31,300m3/d) | Sub-Total | | Fagus Station | | | , | Sub-Total | : | Pipeline km #1100-#900x44.5 | Pipe Bridge | Valve, Air valve Drain & others | Sub-Total | | Total |
| (G | _ | Cost | 35,500 | 40,300 | 13,232 | 89,032 | | 1,375 | 936 | | | 2,311 | | 24,175 | | 1,578 | 25,753 | | 117,096 |
| | Case 8-3 | Description | Zagazig Plont (129,200m3/d) | Hortheast Plant (148,400m3/d) | Bilbeis Plant (31,300m3/d) | Sub-'rotal | | Abu Kebir Station | Huselniya Statlon | | | Sub-fotel | | Pipeline s1200-8800x54.5m | Pipe Bridge L.S. | Valve, Mfr valve Drain & others | Sub-Total | - | Total |
| | | Cost | 43,771 | 40,300 | | 84,071 | | 1,375 | 936 | | | 2,311 | | 31,595 | 1 | 2,045 | 33,640 | | 120,021 |
| | Case B-2 | Description | Zagazig Plant (160,500m3/d) | Hortheast Plant (148,400m3/d) | | Sub-Total | | Abu Kebir Station | Huseiniya Station | | | Sub-Total | - | Pipeline | Pipe Bridge | Valve, Air valve Orain & Others | Sub-Total | | Total |
| | | Cost | 70,400 | | | 70,400 | | 2,776 | | 936 | | 5,530 | | 46.131 | | 2,841 | 48,972 | | 124,902 |
| | Case B-1 | Description | Zagazig Plant (308,900m3/d) | | | Sub-Total | | Abu Kebir Station | Faqus Station | Nusejniya Station | | Sub-Total | | Pipeline | Pipe Bridge | Value, Air valve Orgin 6 others | Sub-Total | | Total |
| | | Cost | 70,400 | | | 70,400 | 1 | 2,948 | 2,776 | 1,618 | 936 | 8,478 | | 67,089 | | 4,105 | 71,194 | | 150,072 |
| 4 | Case A-2 | Description | Abbasa Plant (308,900m3/d) | | | Sub-Total | | S, 257 Zagazig Station | Abu Kebir Station | 1,818 Fayus Station | Insciniya 936 station | Sub-Total | | Pipeline 81200-d700x115.7 | Pipe Bridge | Valve, Air valve Orain & others | Sub-Total | | Total |
| | | Cost | 70,400 | | | 70,400 | | 5,257 | 2,785 | 1,018 | 936 | 10,796 | | 65.615 | | 3,997 | 69,632 | | 150,828 |
| | Case A-1 | Description | Bilbeis Plant (308,900m3/d) | | | Sub-Total | | Zagazig Station | Abu Kebir Station | Faque Station | Huseiniya Station | Sub-Total | | Pipeline km | Pipe Bridge | Valve, Air valve Drain 6 others | Sub-Total | | Total |
| | | | Treatment | | | | | Booster Pump- | | | | | | Transmission | | | | | |

3.5.3.5 Conclusion

The plan of Case B-5 shows smallest in the construction cost and present worth as estimated in the preceding Table-3.5.4. In addition it has the largest number of treatment plants, by which it will be capable of diffusing unforeseeable risk in operation. Considering these matters, the plan of the Case B-5 is recommended for implementation.

It should be noted that water sources of the above plan (Case B-5) are surface water of Muweis Canal, Saidiya Canal and Ismailiya Canal. Therefore, year-round-continuous flow of the canals shall be assured by the authorities concerned.

3.5.4 Groundwater Development Plan

3.5.4.1 Existing Groundwater Production

In the Sharqiya Governorate, there exist numerous groundwater stations which can be classified into three categories.

They are:

- 1) Housing Department System
 82 stations (Total production = 27,211 m³/day)
- 2) Abbasa System's groundwater stations 15 stations (Total production = $64,109 \text{ m}^3/\text{day}$)
- 3) City-owned groundwater stations 43 stations (Total production = 62,512 m³/day)

In principle, above existing groundwater stations will be used continuously in the future years, by periodical rehabilitation or replacement work.

3.5.4.2 Groundwater to be Developed

In addition to the above, new groundwater stations will be developed to meet water demand increase in the future, together with construction of new treatment plants.

In the whole Sharqiya Governorate, by the year 2005 production to be newly developed by groundwater is estimated at $151,243 \text{ m}^3/\text{day}$. To this end, about 117 new groundwater stations will be constructed (Refer to the following section.).

Note: Total water demand in 2005 is 687,018 m³/day.

It will be supplied by:

| i) | New treatment plants | 308,859 r | n³/day |
|------|-------------------------------|-----------|--------|
| i.i) | Existing treatment plants | 73,094 | •• |
| iii) | Existing groundwater stations | 153,832 | •• |
| iv) | New groundwater development | 151,233 | 11 |

Total = 140 stations with 153.832 m³/day

3.5.4.3 Unit System of a Groundwater Station

As a result of the field survey and hydrogeological study, a representative groundwater well is planned as below:

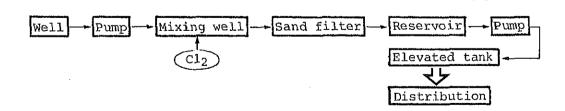
- Production: $30 \text{ l/sec} \times 12 \text{ hours} = 1,296 \text{ m3/day}$

- Depth : 60 m

- Diameter : 10 inches

Concerning the development of groundwater, it is taken into consideration that groundwater sampled and analyzed in the field contains high value of iron and manganese quite often. The substances will cause coloring trouble in the water, specially in the case of execution of chlorination due to chemical reaction. Currently chlorination is not practiced to groundwater and no trouble of coloring has happened. In other words, chlorination has not been made in order to avoid coloring trouble. From the view point of public health control, however, application of chlorination to groundwater for public water supply should be considered. In this study, groundwater stations will be planned to be equipped with chlorination facilities. Simulteneously with chlorination treatment, treatment facility for iron/ manganese removal will be installed. To this purpose, rapid sand filters with manganese-coated sand will be installed. Field survey reveals that about 70 % in number of groundwater stations require the iron/manganese removal system.

Flow diagram of a groundwater station will be as follows:



A typical layout of the sand filtration facility is shown in Fig-3.5.8.

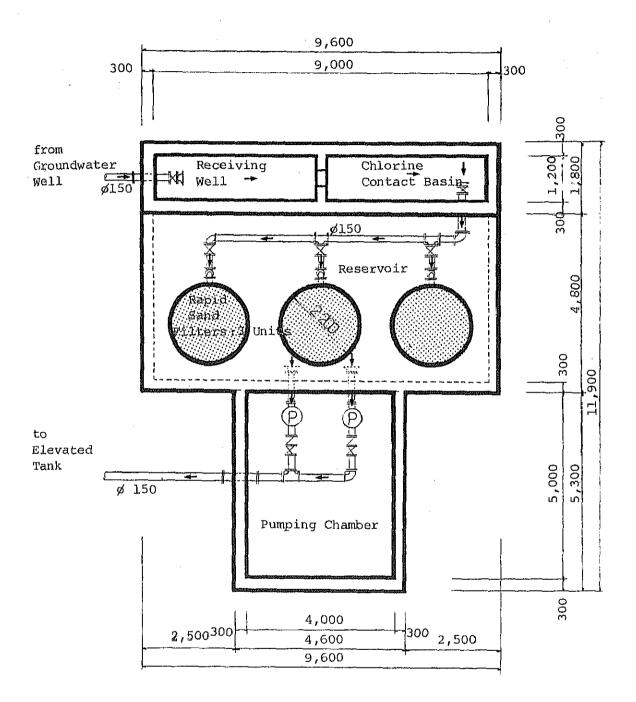


Fig-3.5.10 TYPICAL LAYOUT OF SAND FILTRATION FACILITY FOR GROUNDWATER (Capacity ; Q = 30 l/sec)

3.5.5 Design Criteria

Taking account of the feature of the Sharqiya Governorate, design criteria for water supply facilities are planned as below. The criteria are applied for the Long-term Program and First Priority-phase Program of Sharqiya Water Supply System.

3.5.5.1 Per Capita Consumption

The average daily per capita consumptions are projected in detail in the preceding chapter based on the study during field surveys. The average daily per capita consumptions are summarized as follows:

PER CAPITA CONSUMPTION

(Unit : lcd)

| Ite | em | 1985 | 1995 | 2005 |
|-----|--|------|------|------|
| 1) | Domestic | | | |
| | Individual House Connection Consumers | | | |
| | | | 105 | 205 |
| | Class A | 185 | 195 | 205 |
| | Class B | 120 | 130 | 150 |
| | Class Cl | 90 | 90 | 90 |
| | Class C2 | 65 | 65 | 65 |
| | Standpipe Consumers | | | ٠ |
| | Class D | 48 | 55 | 55 |

2) Non-domestic (Urban)

Commercial : 10 1cd on served population

Institutional: 15 - 10% of domestic and commercial demands

Industrial : 10% of domestic and commercial demands

3) Non-domestic (Rural): Included in domestic consumption

3.5.5.2 Peak Factor

As data to estimate maximum daily and peak hourly demands are hardly available, both factors of demands are assumed as follows:

Average Daily Demand: 1.00

Maximum Daily Demand: 1.25 x Average Daily Demand Peak Hourly Demand : 1.20 x Maximum Daily Demand

3.5.5.3 Conception of Facilities

All facilities shall be designed taking into consideration hygienical safty, technical adequacy and practical economy.

Structure and material of facilities shall have sufficient durability and watertightness.

It is desirable that facilities are installed with standby, bypass and partition wall to be divided into plural parts.

3.5.5.4 Capacity of Facilities

The capacity of the intake water source facility, transmission facility, purification facility shall be determined based on Maximum Daily Demand.

The distribution facility shall be designed based on Peak Hourly Demand.

3.5.5.5 Water Pressure

Maximum static pressure of pipelines (Operating Pressure) shall not exceed 6 kg/cm^2 .

Minimum water pressure at pipe ends of the distribution system shall not be less than 15 m in head, as far as practicable.

3.5.5.6 C Value

C Value of Hazen and William's Formula to be used for hydraulic calculation of pipelines shall be as follows:

C VALUE OF PIPELINES

| Type of Pipe | C Value |
|--------------------------------|---------|
| Mortar lining cast iron pipe | 110 |
| Lining steel pipe | 110 |
| Asbestos cement pipe | 110 |
| Rigid polyvinyle chloride pipe | 110 |

Remark: Where bend loss and others are calculated separately, 130 of C Value can be used.

C Value can be adjusted according to the condition of pipeline.

3.5.5.7 Pipe Material

Pipe materials shall be selected from the following: ACP, CIP, DCIP, Steel Pipe and PVC.

In selecting pipe materials, the following shall be taken into consideration:

- 1) Maximum static pressure and water hammer impact,
- 2) Conditions of the road under which the pipeline is to be laid,
- Corrosiveness of the soil in which the pipeline is to be burried.

3.5.5.8 Valves, Air Valves and Drain Pipe

Valves shall be installed at the following points:

Transmission pipelines: strategic operating points and at

about 2 km intervals.

Distribution mains : all main crosses and branches and

at about 500 m intervals.

Air valves shall be installed at the top of vertical curves of pipelines.

Drain pipes shall be installed at the bottom of vertical curves of pipelines, where draining from the pipeline is possible.

Pressure reducing valves shall be installed before standpipes, in case water pressure exceeds 4 kg/cm2.

Fire hydrants shall be installed at proper intervals.

3.5.5.9 Meters

All production of facilities and distribution shall be metered. For this purpose, meters shall be provided at appropriate and convenient places to measure.

House meters shall be installed at all service connections.

3.5.6 Cost Estimation

3.5.6.1 Construction Cost

Construction cost of water supply facilities for new construction and rehabilitation/replacement by year 2005 is shown below:

Table-3.5.6 Construction Costs (1984 Prices)

| | Item | Cons | tructi | on Cost | |
|-----|--|------|--------|---------|---|
| 1) | Emergency Works | LE | 11.8 | Million | |
| 2) | New Kafr Sagr Treatment Plant | LE | 13.7 | u , | |
| 3) | New Northeast Treatment Plant | LE | 18.2 | , II | |
| 4) | New Zagazig Treatment Plant | LE | 35.5 | u | |
| 5) | New Bilbeis Treatment Plant | LE | 13.2 | Ħ | |
| 6) | New booster pumping stations | LE | 2.3 | u | |
| 7) | Transmission pipelines | LE | 17.5 | 11 | |
| 8) | Extension of distribution pipelines | LE | 143.4 | n | |
| 9) | New groundwater stations equipped with iron/maganese removal facility, 77 stations | LE | 70.8 | 11 | |
| 10) | - ditto - without above facility, 35 stations | LE | 20.4 | ** | |
| 11) | Rehabilitation/replacement of existing groundwater stations, 140 stations | LE | 60.9 | 11 | |
| 12) | Rehabilitation/replacement of existing pipelines | LE | 22.3 | 11 | |
| | Total Costs | LE | 430.0 | Million | _ |

Note : The above costs are estimated on the basis of Case $B-5\ plan$.

3.5.6.2 Operation and Maintenance Cost

Costs for operation and maintenance per annum are estimated below:

Table-3.5.7 Operation and Maintenance Cost (1984 Prices)

(Unit: Million Egyptian Pounds) 1985 1990 1995 2000 2005 Year A) Personnel cost 2.83 3.17 3.33 3.54 1.23 0.75 0.84 Power cost 0.28 0.44 0.61 B) C) Chemical cost 0.20 0.42 0.65 0.86 0.89 D) Repair cost 0.52 1.24 1.81 2.13 1.71 4.21 5.67 6.75 7.40 Total

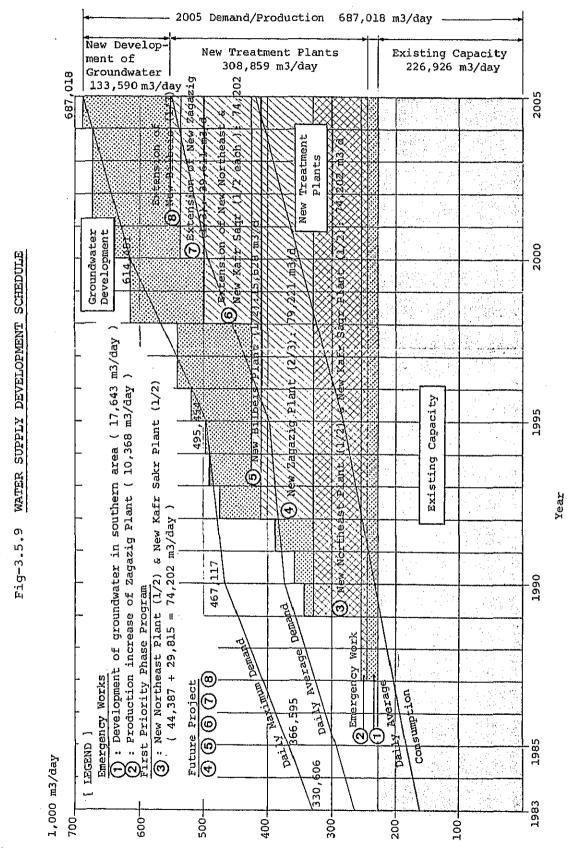
Note: The above costs are estimated on the basis of Case B-5 plan.

3.5.7 Implementation program

Implementation program for water supply development is shown in Table-3.5.8 and Fig-3.5.9. Construction of facilities to meet fully the daily maximum water demand is targeted at the year 1995.

Table-3.5.8 IMPLEMENTATION PROGRAM

| Work | Item Year | 1985 | 98 | 87 | 88 | 83 | 1990 | 91 | 92 | 93 | 94 | 1995 | 96 | 97 | 98 | 66 | 2000 | 0] | 05 | 03 | 2005 |
|------|---|------|-----|---------------|------|-----|------|-----|----|----|----|------|----|-----|------------|-------|------|----|----|----|------|
| 1) | Emergency Works | | | 8 | | | | | | | | | | | | | | | | | |
| ź) | New Northeast Treatment Plant | | | | | | | | | | | | | *** | | | | | | | |
| 3) | New Kafr Saqr Treatment Plant | | | ***** **** | | | | | | | | ĺ | | | | 1 | | ĺ | | ŀ | İ |
| 4) | New Zagazig Treatment Plant | | | | | | | *** | | | | | | | | | | | | | |
| 5) | New Bilbeis Treatment Plant | | | | | | | | | ** | | | | | | | | | | | |
| 6) | Booster pumping station | | | | | | | | | | | | | *** | | ! | | | | | |
| 7) | Transmission pipelines | | 8 | | | | | | | | | | | | | | | | | | |
| 8) | Extension of distribution pipe- lines/service mains | | 888 | | | *** | | | | | | | | | | | | | | | |
| 9) | New groundwater stations equipped with iron/manganese removal facility; 77 stations | | | | 1000 | | | | | | | | | | *** | | | | | | |
| 10) | New groundwater stations without iron/manganese removal facility; 35 stations | | | | 2003 | | | | | | | | | | | | | | | | |
| 11) | Rehabilitation/replacement of existing groundwater stations; 140 stations | - | | | 888 | | | | | | | | | | | | | | | | |
| 12) | Rehabilitation/replacement of existing pipelines | | | | 888 | | | | | | | | | | | | | | | | |



Water Demand and Production

3.6 Institution and Management

3.6.1 Proposed Organizational Framework

The organizational setup has been studied to redress the present deficiencies as presented in the assessment of the existing organization and management in Section 2.6.2.

In this respect the in-depth study has already been completed in early 1981 by British consulting firm under the IDA's technical assistance program. The recommendation for the organization development presented in above study report has been agreed by the concerned authorities of Egyptian government to be implemented and their new organizational evolution has been progressed to the considerable extent.

Under such situation it has been considered adequate to streamline the institutional framework recommended in the previous study in comparison with the present status so that a practical proposal can be concluded.

1) Institutional Framework Recommended by Previous Study

The institutional framework recommended for the overall water supply works in the previous study is composed of four key entities as;

- a) Ministry of State for Utilities
- b) High Council for Utilities
- c) National Water Authority
- d) Public Water Companies (Local Water Undertakings)

The establishment of Ministry of State for Utilities was proposed to undertake the overall responsibility for utilities in the country including water utilities which are assigned in the present Ministry of Housing and local Governorates in order to meet the growing size and complexity of the utility sector and to facilitate the coordination between national policy making and planning and local initiatives.

The High Council for Utilities was proposed to support the Ministry of State for Utilities undertaking the responsibility mainly for policy making matter for water resources and utilities development and coordination between different utilities entities.

The National Water Authority is proposed to develop policies and regulatory frame work as a supervisory agency for all local water supply undertakings. This organization is proposed to maintain and observe the standard and technical and financial objectives of local water undertakings. The capital resources required for the local water undertakings are sought through this agency. This agency was proposed to replace the existing General Organization for Potable Water (GOPW) amplifying its original powers and increasing flexibility and freedom in dealing with organizational, financial and personnel system and practice. The close contacts and liaisons of this agency with local water undertakings were emphasized extending its specialist services including technical and training assistance until local undertakings become virtually self-sufficient.

The Public Water Companies were proposed to be instituted under each local governorate for respective local water undertakings unifying fragmented responsibilities between three entities of regional, municipal and village groundwater supply system. The Company was recommended as a semi-autonomus public water supply executing entities dealing with implementation of provincial water supply scheme and its day-to-day operation.

Prior to the recommendation of PWC, three other alternatives were evaluated as follows.

- a) Functional amplification of NWA (present NOPWASD) to undertake the local water undertakings
- b) Direct undertakings of the central Ministry
- c) Establishment of private water supply company

The first and second alternatives had been discarded because they have a disadvantage of inconsistency in the momentous decentralization policy and have the restricted freedom for efficient operation of the system. The third alternative was considered having advantage of complete freedom from the central government but it was considered inadequate due mainly to the public nature of the water supply system.

2) Present Progress

The evolution of the institutional frame work has progressed since with the basic objectives proposed as above.

The proposed concept of National Water Authority has been embodied in the National Organization for Potable Water and Sanitary Drainage (NOPWASD) with added functions for sewerage merging previous GOPW (General Organization for Potable Water) and GOSSD (General Organization for Sewerage and Sanitary Drainage) under the new presidential Decree No. 197 issued in 1981.

The creations of Public Water Companies (PWCs) proposed have also been progressed for the provincial governorates where water supply schemes of significant scale are implemented. The Beheira Public Water Company was established under Beheira Governorate by Decree No.198 in 1981 for the provincial water supply operation. Further the establishment of another Public Water Company (PWC) for Kafr El Sheikh is presently under the process with same objectives of Beheira PWC.

3) Recommendation by Present Study

The present study will identify a most appropriate institutional framework for the Sharqiya water supply project, taking due account of the previous recommendations and the present progress of relevant reforms.

The previous concept to create Ministry of State for Utilities and High Council for Utilities has not been embodied but the concept to create NWA has been embodies by creation of NOPWASD. Such formation of NOPWASD as a single national agency to set up general guidelines and

standards would result to more uniform nationwide policies, and at the same time, having sole agency dealing with international lending and aid agencies would facilitate loan and grant negotiations. Further problems and conflicts normally associated with creating new organizations could be minimized by withholding the setup of above two extra agencies.

The efforts and resources otherwise required should be directed to amplify and strengthen the functions of the existing Ministry of Housing so that responsibility for the water utilities and coordination with central policy making would fully be achieved.

The present structure of NOPWASD as indicated by Fig-2.1.1 and their functions described in Section 2.5.1 Existing Organization Framework are based on the organization criteria instituted for NOPWASD which are dictated mostly by the proposed organization structure in the previous study. This new structure is distinguished, however, by the added divisions specifically provided for the designing and implementation for the water supply projects all over the Egypt.

The existing status of NOPWASD is considered appropriate reflecting the current transitional decentralization period. The project control functions still retained by NOPWASD will be required until the decentralization of water undertaking authority are fully achieved and local water supply organization is capacitated to undertake the sizable water supply schemes.

The direct control and administration of the project should, however, be transferred to local water authority progressively in future so that such authority can be afforded much greater managerial autonomy.

The establishment of Public Water Company for the local water undertaking at governorate level, as recommended after comparative evaluation among four alternatives in the previous study, is considered effective and reasonable proposition also for the Sharqiya water supply project.

In the present study, however, another alternative have been considered to set up a unified water supply division in Sharqiya Governorate combining existing separately operated three water supply systems on the ground that this would conform to the recent decentralization policy to empower local governorate and at the same time may avoid potential problems associated with the establishment of a new public company.

Although this alternative has a significant advantage as above, it is not necessarily considered an effective and recommendable solution because the present progress of the decentralization is too slow to expect the proposed water supply division in the Governorate to be free from the central government's restrictions.

After evaluation of above additional alternative the establishment of Sharqiya PWC is considered to be conclusive solution for the present project.

- 4) Proposed Sharqiya PWC
- a) Objectives

The principal objectives of proposed Sharqiya PWC is to:

- Unify and consolidate the fragmented responsibilities dispersed in the existing separate water supply units and achieve control of water supply operation with well organized functions' distribution, defined responsibilities and efficient personnel management
- improve coordination between individual water supply schemes
- define the responsibilities for water supply undertaking between NOPWASD, Sharqiya Governorate and PWC
- provide more freedom for the management of water supply with their own accounting, budgeting and financial systems as well as personnel procedures

- permit to operate water supply undertakings in accordance with more commercially oriented objectives and practices with effective use of resources and ensure sound financial capability with new tariff system
- motivate productivity of the staff for the water supply services with adequate allowances and incentive payments
- ensure safe potable water supply throughout the Sharqiya Governorate
- establish efficient maintenance and operation system to reduce water
 leakage and loss
- let the water consumer to be well informed about the activities of Company especially for the necessity of new tariff system to be required in future

b) Powers and Functions:

The Company will be established by joint investments of initial foundation capital of NOPWASD and Sharqiya Governorate with allocation of shares to both parties in an appropriate proportion. The general assembly is to be provided comprising shareholders, representatives of other Ministries concerned, the Company employees and other nominated outside experts presided by the Governor to perform general supervisory duties for the Company's activities.

The chief executive power is vested with Chairman as the head of the Board of Director and responsible for management, control and external representation of the Company. The Board as the executive body of the Company comprises General Managers and selected employees of the Company and perform all tasks needed to fulfill the Company's objectives and policy makings. The main powers of the Company are:

- to develop, operate and maintain all public water supplies
- to prepare capital investment plan for the extension and improvement of the water supply in the service area
- to borrow funds from local and foreign financial institution in accordance with NOPWASD's quidelines
- to establish water supply charges for the revenue required to implement the project and maintain the water supply operation.

The functions of PWC to achieve required objectives largely fall in administration of financing, personnel and training, planning and design, operation and maintenance of the system and water quality control.

The organization structure of PWC is therefore recommended to be composed based on functions required to achieve principal objectives.

c) Management Structure

The followings are keynote of the proposed management structure dictated by the principal objectives.

(1) The company activities are to be performed based on three systemized leveles of headquarter, regional offices, and district offices.

Headquarter is to be located in Capital City of Zagazig and perform central control and supervision through several regional offices.

Regional Offices are to be located in some focal points strategically advantageous to control the region covering two or three Marakaz depending on magnitude of population and area, through district offices in Marakaz.

<u>District Offices</u> are to be located in every Marakaz and perform day to day operation of water supply and customer services.

- (2) Financial and accounting functions are to be strengthened by well coordinated activities of headquarter for central finance control, and regional offices for accounting supervision of bill collection of district offices.
- (3) The central laboratory is to be provided in the headquarter to perform monitoring and analysis of water quality sampled from all water sources in addition to water quality monitoring units to be provided at every treatment plants.
- (4) The water leakage and loss reduction units are to be provided in every district offices to perform routine inspection and repair.
- (5) The public relations unit is to be provided to enhance public awareness of the value and cost of safe water.

The organization charts of the proposed organization framework are indicated by Figs-3.6.1 and 3.6.2.

Some functions of respective organizational units are self-evident from the organization chart. The major roles of departments and distributed offices are summarized as follows:

d) Administrative & Finance

- develop and administer the internal personnel policies and procedure as approved by the Board
- review the company's organization and management structure and remodel the system if changes are required in compliance with the guidelines and policies set by the Board
- control the staff performance to consider incentive payments and promotions as well as career development maintaining personnel records and preparing reports for the Board

- prepare manpower planning and administer the recruitment and staff training program
- prepare annual company accounts and the regular financial reports for the Board and external authorities
- prepare the company's annual operating and capital expenditure budgets and medium term financial plan and provide the data base to seek for the funding resources
- perform financial adjudication of tenders for procurement and capital works

Technical Department:

This Department performs central control over technical aspects of water supply operation coordinating Sub-Divisions of (1) Plan & Design, Construction, (2) Central Laboratory and (3) Operation and Maintenance.

(1) Plan & Design, Construction

- prepare future estimation of the level of the service and demand for the potable water within the company's service area
- identify potential sources of water supply and appropriate means of extending distribution networks in coordination with operation and maintenance department
- develop medium term investment program showing the scale, timing and order of priority of new projects to meet future demand within the available investment resources in consultation with Finance and Administration Department
- develop the design of the project determined to be implemented
- prepare the tender documents, invitation and awards of the contracts
- supervise the implementation of the capital project

(2) Central Laboratory

- perform monitoring and analysis of water quality samples collected from treatment plants, distribution systems, wells and standpipes
- study acceptability of the water in conformance with water quality standard maintaining contacts with Ministry of Health

3) Operation and Maintenance

- supervise and coordinate the day-to-day water supply operations of the company in each Regional Office
- resolve disputes or conflicts of priority between Regional Offices
- advise Regional Offices on appropriate technical performance standards
- prepare reports on technical performance

Regional Office:

Under the Operation and Maintenance Sub-Division several core Regional Offices are recommended to be provided to control the grouped District Offices so that day-to-day operation and maintenance can be achieved locally in an efficient manner. The grouping of District Offices to be under each Regional Office should preferably reflect the geographical location and magnitude of population, water supply flow and the features of the water supply systems.

The Regional Offices will be responsible for:

- the operation and maintenance activities of Districts within the Division including major treatment plants and transmission mains
- the provision of specialist for electrical and mechanical services to
 District Offices and major treatment plants and buildings

- provision of divisional stores and procurement services, workshop facilities for the maintenance of the plant, vehicles and meters
- provision of engineering services such as designing for minor extensions and improvements throughout the District Offices
- routine accounting practices including billing, payments and payroll
- routine personnel administration including recruitment of junior staff
- ensuring that adequate training is provided by NOPWASD for the staff

District Office:

The District Offices are the basic day-to-day operational units to be under Regional Offices. The appropriate size of District Office will be dependent on the number of consumers and geographical extent. The administrative boundaries of existing Marakaz will provide a base to form the District Office providing sub-divided zones in the District Office where size of population and area is too large to administer.

The principal functions of District Office are:

- to maintain safe and continuous water supply in the District Office
- to lay, operate, and repair all distribution pipes other than trunk mains
- perform routine inspections and minor maintenance of trunk mains and reporting major faults to Regional Office
- to perform routine operation and maintenance of wells, public stand pipes and minor treatment plants
- to perform water leakage and loss detection and prevention,
 including routine reading and reporting of distribution meters

- to install customer connections after receiving their application and perform routine inspection and programmed meter maintenance and replacement thereafter
- to perform customer's meter reading and revenue collection
- d) Coordination with Ministries of Central Government:

The coordination with Ministries related to the water supply operation is recommended to be embodied by the proposed general assembly of Sharqiya PWC which involves representative members of respective Ministries such as Ministry of Irrigation, Ministry of Health, Ministry of Finance and Ministry of Planning.

Ministry of Irrigation is responsible for the development and maintenance of national irrigation and drainage canals to provide source of water for irrigation purpose. The irrigation canals provide also source for surface water treatment plants to be utilized for potable water, and the approval from the Ministry is required for abstraction of water for such purpose. Therefore close liaison and communication between the Ministry and Sharqiya PWC would be required to ensure safe and sufficient water sources.

Ministry of Health is responsible to control water for various purposes by preparing guidelines and national standard for water quality as one of the political measures for the enhancement of national public health standard. The close coordination would be required between Ministry and Sharqiya PWC through the Company's Central Laboratory which is to be responsible for the monitoring and analysis of water quality throughout the Sharqiya Governorate.

The Ministry of Finance is largely responsible for overall budget administration including budget for capital investment and recurrent expenditure of the government agencies. The support of this Ministry would be required for the political decision on the funding of the central government for the water supply development and operation of proposed PWC.

The Ministry of Planning deals with national and regional plan and assessment of the priority of the various projects and sets the guideline for capital budget of the projects. The close coordination among this Ministry, Ministry of Finance and Sharqiya PWC would be required to formulate authoritative proposals to the decision-making body of the central government for the water supply development project.

3.6.1.1 Staffing and Training:

In accordance with the staged development of the water supply extensions projects the staffing of Sharqiya PWC should also be scheduled particularly for the qualified and skilled personnel. The roughly estimated staffing plan up to the year 2005 has been made as shown in Table-3.6.1 providing that more detailed schedule would be made in the subsequent feasibility study.

The existing staff in the Sharqiya governorate appears normal in terms of number. There is an acute shortage, however, of the professional staff especially for the skilled laborers such as welders, fitters, plumbers, mechanics and electricians and a serious misallocation of staff with some functions overmanned and others neglected resulting in a low labor productivity.

The level of staffing should aim to increase the efficiency of water operation represented by the increase of potential water output per employee which in turn requires reduction of overmanning and redundancy of unskilled laborers. The adequate staffing for routine works such as leak detection and preventive maintenance should be emphasized in order to maintain the volume of water available for consumption and achieve improvements in output per employee.

The shortage of skilled laborers and technical staff should progressively be reduced through vigorous training activities. The basic strategy for the training is considered primarily to meet the immediate need for the improvements of the existing level of water operation and maintenance and secondly to meet the future manpower requirements in the development of the systems and new Public Water company to be established.

The rapid introduction of training for the existing key staff using presently available training facilities are desirable for the initial stage of the training program concentrating on practical on-the-job training in the field and employment of highly experienced expertise or foreign professional consultants until the trained key staff can sufficiently be available to train and instruct their subordinate and build up future permanent training program.

Presently the regular training courses are provided in NOPWASD in coordination with Ministry of Housing to meet the ever increasing requirement for the professional staff for the water supply and sewerage operation. The intensification and amplification of such training activities will be required to motivate the early implementation of training program for the proposed Sharqiya water supply extension project.

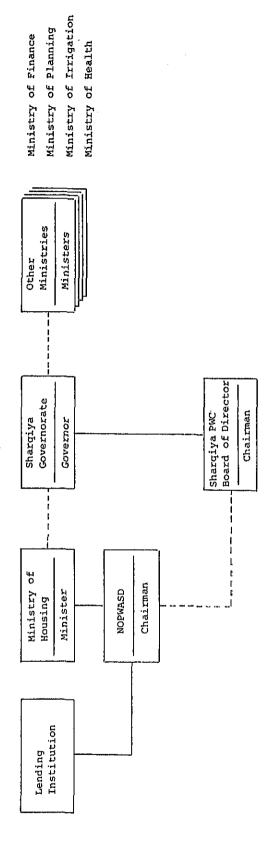


Fig - 3.6.1 PROPOSED ORGANIZATION FRAMEWORK (1)

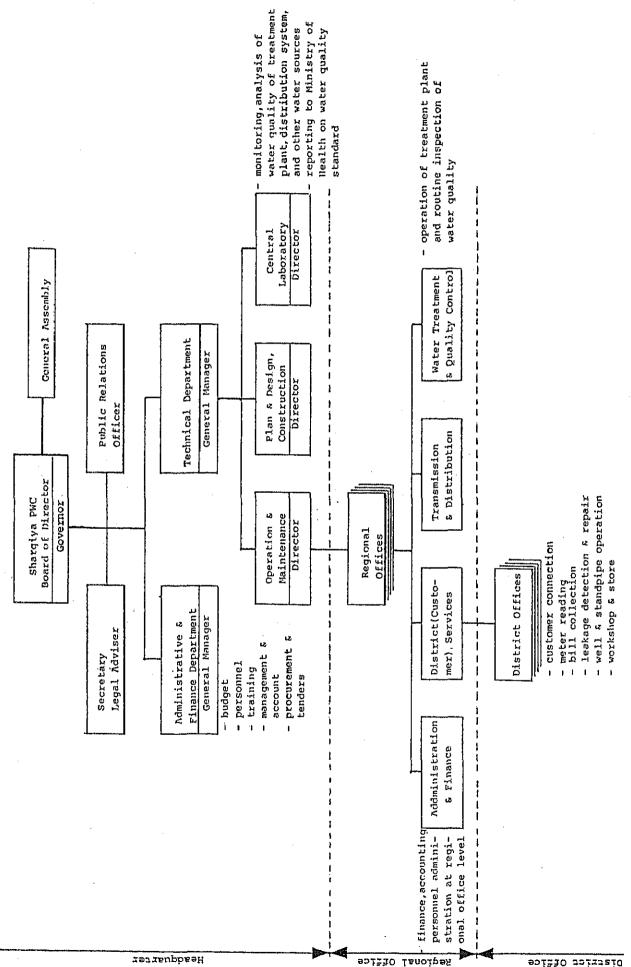


Fig - 3.6.2 PROPOSED ORGANIZATIONAL FRAMEWORK(2)

Table-3.6.1 Estimated Staffing Schedule 1985 - 2005, Sharqiya PWC

| Job Title | 1985 | 1990 | 1995 | 2000 | 2005 |
|---------------------------------|----------|------|------|------|------|
| PWC Headquarter | | • | | | |
| Top Management | 8 | 8 | 8 | 8 | 8 |
| Engineer | . 7 | 11 | 16 | 17 | 18 |
| Chemist | 7 | 10 | 12 | 12 | 13 |
| Legal Officer | 1 | 1 | 1 | 1 | 1. |
| Personnel & Training Officer | 5 | 5 | 6 | 6 | 6 |
| Personnel Specialist | 4 | 5 | 6 | 6 | 6 |
| Financial Officer | 3 | 4 | 5 | 5 | 5 |
| Accountant | 2 | 5 | 6 | 6 | 6 |
| Administrative Officer | 5 | 8 | 9 | 9 | 9 |
| Procurement & Supply Officer | 10 | 12 | 13 | 13 | 13 |
| Clerk | 18 | 35 | 43 | 48 | 51 |
| Operation & Maintenance |) | | | | |
| Divisions | - | | | | |
| Engineer | 18 | 28 | 33 | 35 | 44 |
| Technician | 24 | 31 | 43 | 46 | 60 |
| Chemist | 3 | 6 | 8 | 8 | 4 |
| Superintendent | 9 | 18 | 21 | 24 | 29 |
| Water Operator | 15 | 41 | 48 | 53 | 76 |
| Craftman | 20 | 43 | 51 | 55 | 38 |
| Administrative Staff | 25 | 28 | 31 | 35 | 39 |
| Clerk | 30 | 76 | 92 | 110 | 121 |
| Laborer | 80 | 210 | 225 | 236 | 235 |
| Distribution District | | | | | |
| Superintendent | 21 | 43 | 48 | 51 | 55 |
| Inspector | 45 | 93 | 104 | 112 | 121 |
| Craftman | 22 | 49 | 54 | 61 | 68 |
| Water Operator | 135 | 361 | 409 | 451 | 492 |
| Skilled Laborer | 80 | 194 | 203 | 215 | 221 |
| Laborer | 178 | 371 | 382 | 391 | 401 |
| Administrative Staff | 25 | 27 | 31 | 36 | 41 |
| Clerk | 23 | 159 | 163 | 168 | 173 |
| Total | 823 | 1882 | 2071 | 2218 | 2354 |

3.6.2 Long Term Financing

The water supply improvement and extension proejct in Sharqiya Governorate will require total investment of LE 430.0 million at 1984 price up to the year 2005 as estimated in previous section 3.5 and shown in Table-3.5.6 This investment is estimated based on the long term target to provide every household with satisfactory access to public water by the year 2005.

The investment program for such large scale construction scheme will be required to be implemented progressively by several stages in accordance with the available found resource and government policy for the water supply development. Such staged development should also be schemed in accordance with urgency of the water supply requirement.

In this context the initial stage of the construction from 1985 to 1989 has been identified, as described in subsequent Chapter 4 in order to facilitate the early implementation and satisfy the urgent requirement of water supply extension which will require approximately LE 100.1 million at 1984 price.

Capital Source:

The capitals for the construction composing the foreign currency, approximately 40 % of total costs and remaining local currency portions will be funded from the various sources. The international and bilateral funding agencies would provide the loan or even grant if such is available for the foreign currency portion and the central government agency is required to provide the loan, equity and subsidy for the local currency requirements if the local funds are not sufficiently available in the local water utilities.

Under the existing financial situation and practices of the water supply entities in Sharqiya Governorate where investment resources are scarce with low revenue-earning capacity mostly ascribable to the extremely low water tariff, a subsidiary funding such as soft loan with low interest and long term repayment and the government subvention will be required for the early stage of the development plan until the financial capability of water supply utilities are sufficiently strengthened.

Revenue Sources:

The substantial increase of the level of the water tariff and introduction of appropriate charge for the present free tap of public standpipe will be necessary to generate sufficient revenues to sustain a long term investment program which accounts for large utilities' current operating costs and debt-service obligation for the substantial external borrowings.

The introduction of the water tariff to raise sufficient revenue or at least break even for the operating costs of water supply will be required to reinforce the financial capability of proposed Sharqiya PWC, reducing its dependency on the scarce government subsidy. The water tariff reasonable imposed based on the economic cost of the water supply would make the water consumers aware that water is not costless and hence discourage the waste of water. Further the progressive increase of the water tariff will be necessary to ensure internal cash generation so that future heavy investment program can be sustained.

The new tariff systems, however, should be designed taking account of affordability of consumers to pay charges and implemented with support of authorized power of the Company to set their own water tariff accompanied by improved management, accounting system and effective billing and collection procedures.

3.6.3 Rough Economic Evaluation of Long Term Program

As mentioned in previous section the financial measurements of the prospective investment of proposed project is a key factor and it will be assessed in the subsequent financial feasibility study to provide means to determine investment. The evaluation of the water supply project depending on accounting measurement only sometimes gives rise to distorted conclusion since the water supply services normally intended to provide safe potable water which is the essential social requirement and not based on commercial profitability.

The evaluation including the socio-economic factors will therefore be necessary as a helpful tool to determine the desirability of the project. Although the most of the economic benefits derived from the water supply investment is difficult to quantify, the simplified benefit-cost analysis has been explored based on the following assumptions.

- 1) The incremental capital investment and maintenance and operation costs for the period of project life from 1985 to 2034 at constant price of 1984 have been assumed for the outputs of the project.
- 2) The inputs of the project have been assumed based on the incremental water consumptions derived from the implementation of long range water supply improvement and potential water tariff projection.

The present water tariff under the government strict control is considered not representing real economic price of the water. The water tariff representing realistic value of water is normally assessed by the price which the potential consumers will pay for the safe potable water. The questionary surveys have been made to assess consumers' willingness to pay. It was found, however, difficult to obtain reliable results because they are presently provided with water of extremely low tariff under the government subvention which likely distorts their realistic evaluation of water.

In some rural areas where the safe water is in accutely short supply, the water is sold by private water vendors at approximately 20 pts/20 litres (equivalent to LE 10/m³). This may be an extreme example which is not appropriate to apply for estimation of the realistic value of water but it provides a certain implication for the appreciation of the water. Such being the case, the amount of water charge which the potential consumers afford to pay has been considered representing value approximate to the realistic water price and it was estimated to be 30 pts/m³ based on the incremental average income level and water consumption.

3) The discount rate of 6% has been assumed to reflect the real opportunity cost of capital because the economic rates of return in developing countries generally range 4% to 6% and the water supply project is socially-desirable public welfare project of the country.

Based on the assumptions outlined above a flow of incremental costs and benefits discounted at 6% has been projected as shown in Table-3.6.1 and the benefit-cost ratio has been computed to be 1.0006. The ratio of more than one indicates that long term benefits of the project outweigh its long term costs justifying the project which provides positive net benefits to the economy.

Further above ratio would increase if the unquantifiable socio-economic benefits are compounded. On notable benefits among others are to be derived from the health improvement. The provision of safe and clean potable water will contribute to the improvement of public health and hygene and reduction of water-borne diseases such as bilharzia, gastroenteritis, cholera, typhoid, stomach illness which are widely spread in the project area. In addition to above, other benefits can be expected such as better quality of life and amenities, employment increase during construction period of the project, and decrease of fire incidence by improved fire fighting capacities.

Table-3.6.2 Discounted Flow of Incremental Costs and Benefits

| Pounds) |
|----------|
| Egyptian |
| Million |
| (UNIT: |

4. Project Identification

In the preceding chapter, a long term program up to the year 2005 was studied. In this chapter, among a number of serial construction/ rehabilitation works to be involved in the long term program, the project which shall be implemented in the earlier stage as the First-Phase Project will be identified.

4.1 Selection of the Project to be Implemented

Considering urgent necessity of water supply and present situation of water shortage in the northern area of the Governorate, the priority of the project implementation will be placed on new construction of Northeast Treatment Plant and Kafr Saqr Treatment Plant; and their transmission/distribution pipelines.

In addition to the above new construction works, the emergency works will be proposed in order to relieve present condition of water supply to a certain extent.

4.1.1 Construction of New Treatment Plants

Treatment plants to be newly constructed in the frame of the First-Phase Project will be located both at Kafr Saqr and Northeast. The two plants will have a final capacity of 150,000 m3/day in total by the year 2005. Supply area and capacity of the plants is summarized in Table 4.1.1.

In the First-Phase Project, the plants will aim to have supply capacity up to the year 1995. According to the water demand study, the capacity to be newly developed by 1995 is about a half of the 2005 capacity.

Therefore, capacity of each plant in the First-Phase will be:

- Northeast Plant : $90,000 \text{ m}3/d \times 1/2 = 45,000 \text{ m}3/day$
- Kafr Saqr Plant : $60,000 \text{ m}^3/\text{d} \times 1/2 = 30,000 \text{ m}^3/\text{day}$

Total : $150,000 \text{ m } 3/d \times 1/2 = 75,000 \text{ m } 3/day$

Table 4.1.1 New Treatment Plants and Supply Area

| Plant | Suppl | .у А | rea and | Capac | city by 2005 |
|---------------|-----------------------------------|------|----------|-------|--------------------------------|
| | - Faqus City | : | 14,086 | m3/d | |
| New Northeast | - Huseiniya City - Villages in | : | 7,391 | 73 | Total = 88,775 m 3/6 |
| | Huseiniya Markaz - Villages in | : | 31,568 | u . | Say, $90,000 \text{ m} 3/6$ |
| | Faqus Markaz | : | 35,730 | 11 | |
| | - Abu Kebir City | : | 20,304 | m 3/d | |
| New Kafr Saqr | - Kafr Saqr City - Villages in | : | 6,570 | 11 | Total = $59,629 \text{ m} 3/6$ |
| | Kafe Saqr Markaz | : | 32,755 | II | Say, 60,000 m3/6 |
| | | Gr | and Tota | al | = 150,000 m3/d |

Simulteneously with construction of the two new plants, transmission pepelines to the above areas will be installed and booster pumping stations on the way of the transmission pipelines will be constructed. In addition, distibution pipelines and elevated tanks will be expanded/supplemented to the existing distribution systems.

4.1.2 Emergency Works

1) Production Increase of Zagazig Treatment Plant

In order to relieve present poor water supply condition in Zagazig City, the existing treatment plant (200 l/sec = 17,280 m 3/day) will be preferably expanded to possible extent.

Field survey revealed that production of the existing Zagazig plant would be increased by 120 1/sec (= 10,368 m³/day) with additional construction works. The work for production increase will be included in the emergency works.

2) Rehabilitation of Existing Treatment Plants

As studied in Working Paper No.4 "Existing Water Supply Systems", a number of mechanical equipment/devices such as pumps, flow meters, valves, chemical dosing equipment, etc, of the old existing treatment plants have been out of services or do not keep their original normal function, owing to deterioration by their age and lack of their spare parts.

All of the existing treatment plants will be rehabilitated by the emergency works, in order to recover the original design capacity.

They are:

- i) Abbasa Treatment Plant
- ii) Zagazig Treatment Plant
- iii) Faqus Treatment Plant

3) Development of Groundwater in Southern Area

Water supply systems in the following cities/villages which are located in the southern area and will not be benefited by new treatment plants

will be strengthened by new construction/reconstruction of groundwater stations to some extent in the frame of the emergency works.

They are:

| - | Bilbeis City | Three | groundwater stations |
|-----|----------------------|-------|----------------------|
| _ | Ibrahimiya City | One | ¥ |
| ••• | Hihiya City | One | 11 |
| - | Diarb Nigm City | One | п |
| - | Mashtul el Souk City | One | п |
| - | Minyet el Qamh City | One | |
| | 77 1 | - | 3 |

- Housing Department System ... Seven groundwater stations

4) Procurement of Machines/Vehicles/Spare Parts for Maintenance

At present machines and vehicles are insufficient. The following will be purchased in order to take prompt action for repair of existing facilities, and for regular maintenance work:

| Four-wheel Drive Cars | : | 3 cars |
|------------------------------------|---|--------|
| Trucks | : | 4 cars |
| Backhoe (0.05 m3) | : | l unit |
| Pumps (Engine 3.5 PS x 0.6 m3/min) | : | 5 sets |
| Portable Chlorinator (50 g/hour) | : | 2 sets |
| Leak Detector | : | 3 sets |
| Overseas Training for | | |
| Local Engineers | : | L.S. |
| | | |

Others : L.S.

4.2 Project Cost

Rough project cost is tabulated below:

Table 4.1.2 COST FOR FIRST PRIORITY-PHASE PROJECT

| | Item | | Cost |
|-----|---|-----|--------------------|
| (A) | Construction of New Northeast Plant and New Kafr Saqr Plant; installation of transmission/distribution pipelines, booster stations and elevated tanks | LE | 71.1 million |
| (B) | Emergency works | LΈ | 11.8 million |
| (C) | Engineering services (Detail design and construction supervision) | LE | 4.1 million |
| (D) | Physical contingency $1/$ | LE | 8.7 million |
| (E) | Price contingency 2/ | LE | 30.3 million |
| | Total project cost = | LE | 126.0 million |
| | <pre>(Broken down into: Foreign currency portion : LE 54.3 million and Local currency portion : LE 71.7 million</pre> | (or | US\$66.2 million), |

Note: 1/ Physical contingency: 10% of (A+B+C)

- LE : Egyptian Pounds (LE 0.82 = US\$ 1.00)

 The cost described above is denoted in 1984 price.

4.3 Project Implementation Schedule

Implementation schedule for the First Priority-Phase Project will be :

- 1) Loan application: Early 1985 Middle 1985 (1/2 year)
- 2) Detail design : Middle 1985 Middle 1986 (one year)
- 3) Construction of New Northeast Plant and New Kafr Saqr Plant : Middle 1986 - Middle 1988 (two years)
- 4) Emergency works : Middle 1986 Middle 1987 (one year)

FEASIBILITY STUDY ON
SHARQIYA WATER SUPPLY SYSTEM
IN
THE ARAB REPUBLIC OF EGYPT

PART THREE FIRST PRIORITY-PHASE PROGRAM



FEASIBILITY STUDY ON SHARQIYA WATER SUPPLY SYSTEM IN THE ARAB REPUBLIC OF EGYPT

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1. GENERAL

1.1 Scope of First Priority-Phase Program

The long term program which formulates a long range plan of Sharqiya Water Supply up to the year 2005 is studied in the preceding Part II. Consequently the most urgently necessitated project is identified as the First Priority-Phase Program in the last chapter of the Part II, from the technical and financial standpoints.

The selected project consists of a) New Northeast Plant and New Kafr Sagr Plant Systems which are to supply the northern part of the Governorate aiming at 1995, and b) imminent rehabilitation for existing plants and strengthening works for the southern area and densely populated Zagazig city which are called Emergency Works collectively.

1.2 Major Works of the Study

To cover the study for the scope stated above, the following works have been undertaken:

- a) Preliminary Design
- On the basis of the estimated population and water demand, two water supply systems supplying the northern Governorate are planned together with their distribution system and the emergency works. The construction cost for each of the stated above is estimated.
- b) Implementation Program

 The yearly disbursement schedule is planned from the planned implementation schedule, and the project cost is estimated on each of the local and foreign currency.
- c) Institution, Organization and Financial Feasibility
 Necessary setup of institutional and organizational matters for
 managing the Sharqiya Water Supply is proposed and a financial plan,
 involving the funding arrangement such as equity and water tariff is
 presented.

- 2. Preliminary Design
- 2.1 Distribution System
- 2.1.1 Design Criteria

Based on the estimated per capita consumption, quoted in 3.5.5, Part II, the design criteria for the distribution system are selected as follows:

- i) Minimum residual pressure:
 - 10 m for rural areas; and
 - 20 m for urban areas.
- ii) Distribution pipelines consist of:
 - Trunk mains: Transmission/distribution mains; and
 - Service mains: Distribution mains within cities and villages.
- iii) Peak factors (Refer to 3.5.5, Part II) :
 - Daily max. demands =
 - 1.25 x Daily average demands
 - Peak hour demands =
 - 1.5 x Daily average demands; or
 - 1.2 x Daily max. demands
- . iv) Distribution method:
 - Pumping system with elevated tanks
 - v) Capacity of pipeline:

The trunk mains for the present project will be one of the following:

- Existing mains meeting the 1995 demands;
- Existing plus proposed mains meeting the 2005 demands; or
- Proposed mains meeting the 2005 demands
- vi) Elevated tanks:

- Capacity: 300 m3;

- Height: 25 m above ground;

- Effective depth: 4 m

- Purpose: Backstopping supply at power failure

to meet 1 hr equivalent of the daily

maximum demands.

2.1.2 Service Area and Water Demands

presently the whole Governorate is covered by the existing system, while the new assets are to serve the following areas:

- i) New Northeast System:
 - Huseiniya Markaz (City and rural area);
 - Fagus City; and
 - Part of Fagus Markaz (rural area).
- ii) New Kafr Sagr System:
 - Kafr Sagr Markaz (City and rural area);
 - Part of Abukebir City; and
 - One village of Fagus Markaz.

Water demands are summarized in Table 2.1.1. by each Markaz. Fig. 2.1.1 shows the proposed service areas of the project, and the schematic plan of the distribution systems is presented on Fig. 2.1.2.

The village level water demands are estimated from 1984 through 1995, 2000, and 2005 based on the study undertaken in 1983 by the team.

Table-2.1.1 Water Demands

(m3/day)

| | | FIRST | PRIORITY PROGRAM | -PHASE | | LONG-TERM PROGRAM | |
|------------|---------|--------|--|---------------------------------------|---------|--|---------|
| Area | | AVG | XAM | PEAK | AVG | MAX | PEAK |
| New Northe | ast Sys | tem | | · · · · · · · · · · · · · · · · · · · | | , direction field | |
| Huseiniya | U | 3,106 | 3,883 | 4,660 | 5,913 | 7,391 | 8,869 |
| | R | 14,022 | 17,527 | 21,032 | 25,292 | 31,616 | 37,939 |
| | Т | 17,128 | 21,410 | 25,692 | 31,205 | 29,007 | 46,808 |
| Faqus | U | 9,142 | 11,428 | 13,714 | 19,269 | 24,086 | 28,903 |
| | R *1 | 16,379 | 20,474 | 24,569 | 29,417 | 36,771 | 44,125 |
| | T | 25,521 | 31,902 | 38,283 | 48,686 | 60,857 | 73,028 |
| Sub-Tota | al | 42,649 | 53,312 | 63,975 | 79,891 | 99,864 | 119,836 |
| New Kafr S | agr Sys | tem | | | | | |
| Kafr Saqr | Ü | 2,790 | 3,487 | 4,184 | 5,256 | 6,570 | 7,884 |
| | R | 13,748 | 17,185 | 20,622 | 25,637 | 32,047 | 38,456 |
| | Т | 16,538 | 20,672 | 24,806 | 30,893 | 38,617 | 46,340 |
| Abu Kebir | U | 11,850 | 14,813 | 17,776 | 24,243 | 30,304 | 36,365 |
| Faqus | R *2 | 268 | 335 | 402 | 510 | 638 | 766 |
| Sub-Total | | 28,656 | 35,820 | 42,984 | 55,646 | 69,559 | 83,471 |
| Total | | 54,177 | 89,132 | 106,959 | 135,537 | 169,423 | 203,307 |
| Existing F | aciliti | es | and the second s | | | NIB-20-88-1-11-7-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1 | |
| Faqus City | | 9,504 | 9,504 | 9,504 | 10,000 | 10,000 | 10,000 |
| Abu Kebir | City | 5,103 | 5,103 | 5,103 | 10,000 | 10,000 | 10,000 |

^{*1 (}Faqus Rural Demands) x 87 % - (*2)

^{*2} Kafr El-Ashgam Village.

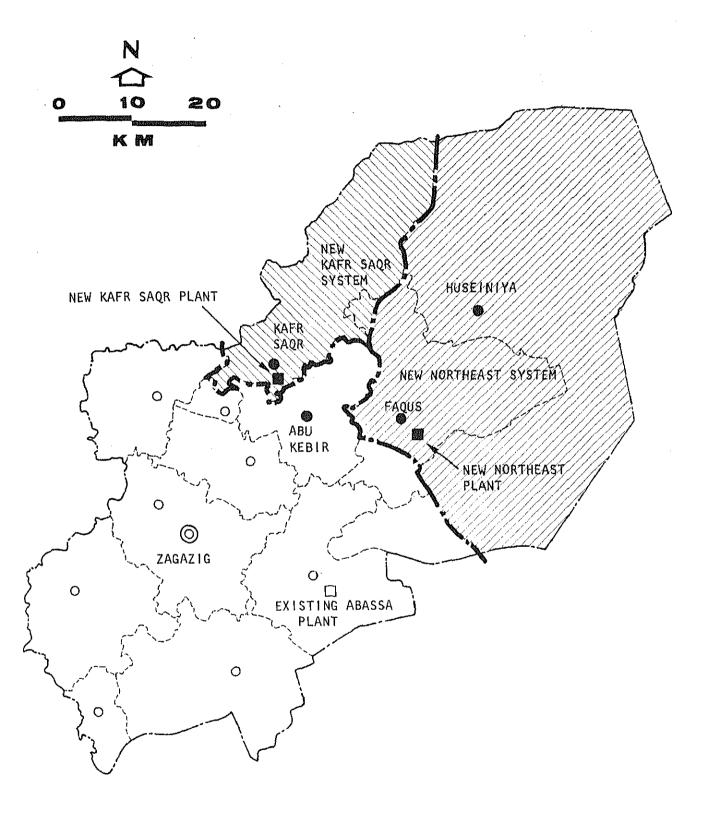
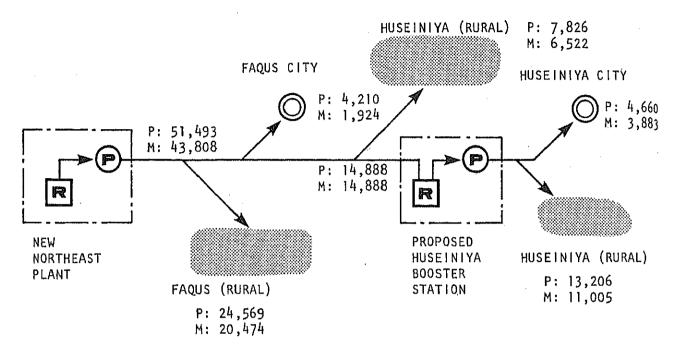


Fig -2.1.1 Proposed Service Areas

(UNIT: m3/day)

NEW NORTHEAST SYSTEM



DEMANDS

P: PEAK HOUR

M: DAILY MAXIMUM

NEW KAFR SAQR SYSTEM

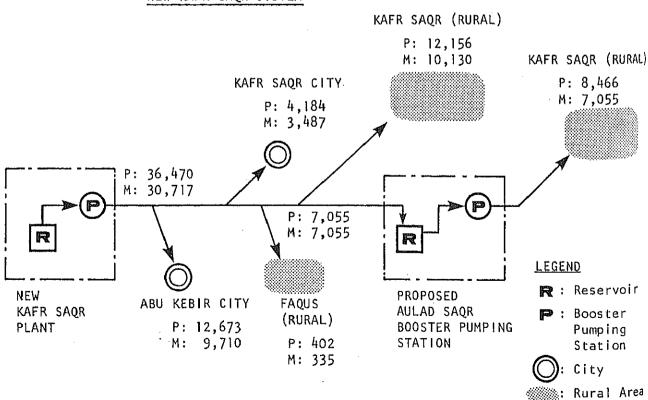


Fig -2.1.2 Schematic Plans of Distribution Systems

2.1.3 Trunk Mains

This section deals with the transmission and distribution pipelines in the service areas of the present project. Diameters of the proposed mains range from 1,100 mm to 100mm.

The existing transmission and distribution mains were initially installed with cast iron pipes in 1959 and since periodically expanded or reinforced with asbestos cement pipes. The existing mains are to be used for the project to full extent.

New Northeast System

Water from the proposed New Northeast Water Treatment Plant will be distributed to Faqus City, rural areas of Faqus Markaz, and part of Huseiniya Markaz. Water will be also conveyed to the proposed Huseiniya Booster Pumping Station. The trunk main from the proposed plant - Faqus City - Huseiniya will have capacity of 2005 peak hour demands.

New Kafr Sagr System

The trunk main, of which diameter ranges from 900 mm to 450 mm, functions as the distribution main to Kafr Saqr City and the southern rural area of Kafr Saqr Markaz, and at the same time, as a transmission main to the proposed booster pumping station to be located in Aulad Saqr Village. This station will serve the northern rural area of the Markaz.

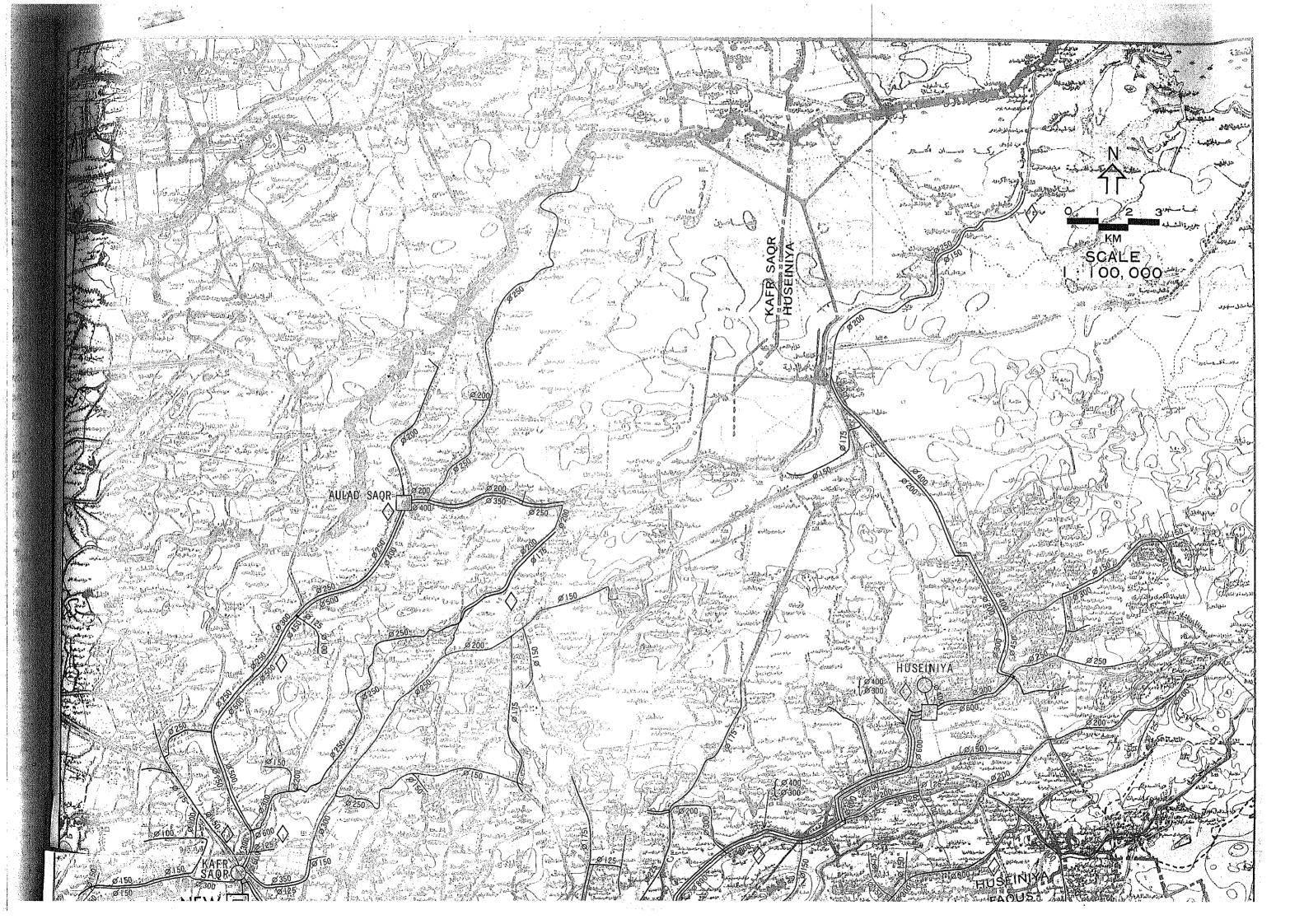
Another trunk main of 700 mm diameter branched from the abovementioned main at the railway crossing bounds south to supply Abu Kebir City.

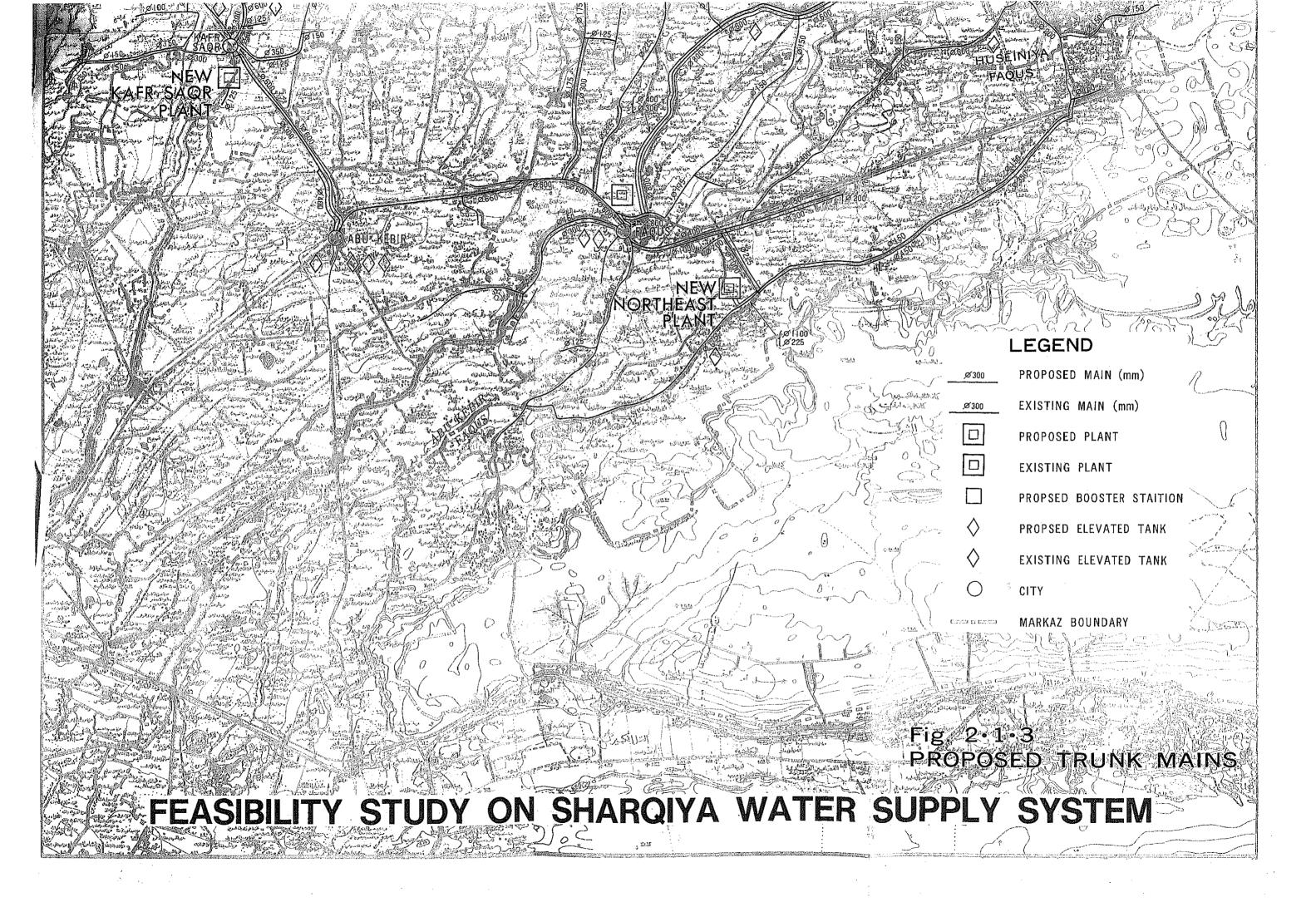
Both proposed trunk mains will have capacities to meet the 2005 peak hour demands.

Table-2.1.2 summarizes the proposed trunk mains. The proposed trunk main plan is shown on Fig -2.1.3.

Table-2.1.2 Proposed Trunk Mains

| DIAMETER (mm) | NEW NORTHEAST (km) | NEW KAFR SAQR (km) | TOTAL (km) |
|------------------|--------------------|-----------------------|---------------|
| 1,100 | 0.7 | - | 0.7 |
| 1,000 | 6.8 | - | 6.8 |
| 900 | 0.2 | 0.8 | 1.0 |
| 800 | 10.4 | 0.2 | 10.6 |
| 700 | 0.9 | - | 0.9 |
| 600 | 15.0 | 7.9 | 22.9 |
| 500 | - | 11.7 | 11.7 |
| 450 | 12.0 | 3.8 | 15.8 |
| 400 | 18.5 | 4.5 | 23.0 |
| 350 | 8.4 | 8.1 | 16.5 |
| 300 | 28.2 | 5.9 | 34.1 |
| 250 | 10.2 | 29.6 | 39.8 |
| 200 | 7.2 | 17.3 | 24.5 |
| 150 | 1.8 | 19.7 | 21.5 |
| 100 | 2.2 | - | 2.2 |
| Total | 122.5 | 109.5 | 232.0 |
| | | | |





2.1.4 Booster Pumping Stations

The present project will include two booster pumping stations: the Huseiniya station and the Aulad Sagr station. (Refer to P. A-1)

The Huseiniya station will be constructed to meet the 1995 water demands of Huseiniya City and the northern rural area of Huseiniya Markaz. The facilities will include:

- 2 sets of 7.5 m3/min pumps;
- 2 sets of 4 m3/min pumps;
- 3,500 m3 storage reservoir; and
- 300 m2 pump house

The station will be augmented by 2 sets of 7,5 m3/min pumps and expansion of the reservoirs to 7,000 m3 by 2005.

Aulad Sagr station will be constructed to meet the 1995 water demands in the northern part of Kafr Sagr Markaz. The station will consist of:

- 2 sets of 4 m3/min pumps;
- 2 sets of 2 m3/min pumps;
- 1,800 m3 storage reservoir; and
- 200 m2 pump house

To furnish the long-term program's water demands, additional 2 sets of 4 m3/min pumps will be installed and storage capacity will be expanded to 3,600 m3 by 2005.

2.1.5 Elevated Tanks

There are 11 elevated tanks in the proposed service area of the project totalling to 4,550 m3 in storage capacity. (See Table 2.1.3). The analysis reveals that the existing tanks in the rural area suffice the demands up to 1995 but the urban area requires construction of five tanks; namely, one in Huseiniya City, one in Faqus City, and three in Abu Kebir City. (See Table 2.1.4). The location of the tanks will be studied and determined during the detailed design. Fig. 2.1.4 shows typical elevation of the tank.

Table -2.1.3 Existing Elevated Tanks

| | Location | | | |
|------|-----------------|-----------|----------|-----------------------|
| City | y/Village | Markaz | Volume | Height above gound |
| 1) | San El Hagar | Huseiniya | 300 m3 | 25 m |
| 2) | Tell Rak | Kafr Saqr | 300 m3 | 25 m |
| 3) | Kafr Saqr City* | | 400 m3 | 20 m |
| 4) | Natora | 16 | 400 m3 | 20 m |
| 5) | Kahboona | Faqus | 1,000 m3 | 25 m |
| 6) | Kanteer | 19 | 500 m3 | 25 m |
| 7) | Faqus City | 77 | 350 m3 | ** |
| 8) | El Roda | 11 | 400 m3 | 32 m |
| 9) | Abu Kebir City | Abu Kebir | 100 m3 | ** |
| 10) | Hanut | Kafr Saqr | 400 m3 | ** |
| 11) | Aulad Sagr | " | 400 m3 | ** |
| | Total Capacity | | 4,550 m3 | |

^{*} Operated by the present Abbasa System

Table -2.1.4 Elevated Tanks Construction Program

| Area | No. of tanks to be constructed | | Present capacity |
|----------------|--------------------------------|---------|---------------------|
| | Ву 1995 | Ву 2005 | (m3) |
| Huseiniya City | 1. | - | - |
| Kafr Sagr City | •• | - | 400 * |
| Faqus City | 1 | 1 | 350 |
| Abu Kebir City | 3 | 3 | 100 |
| Rural Area | - | 3 | 3,700 |
| Total | 5 | 7 | 4,550 |

^{*} Operated by the present Abbasa System

^{**} Data unavailable

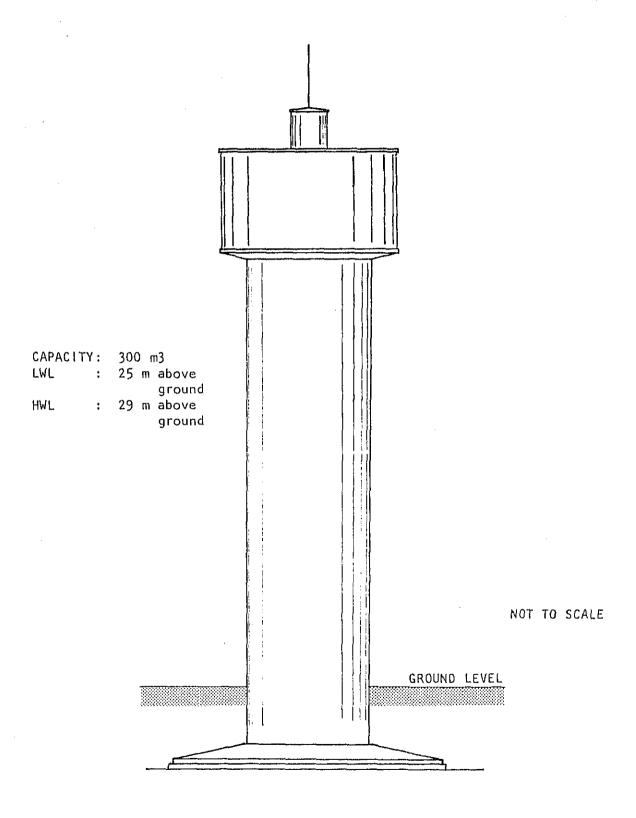


Fig-2.1.4 Typical Elevation of Proposed Elevated Tank

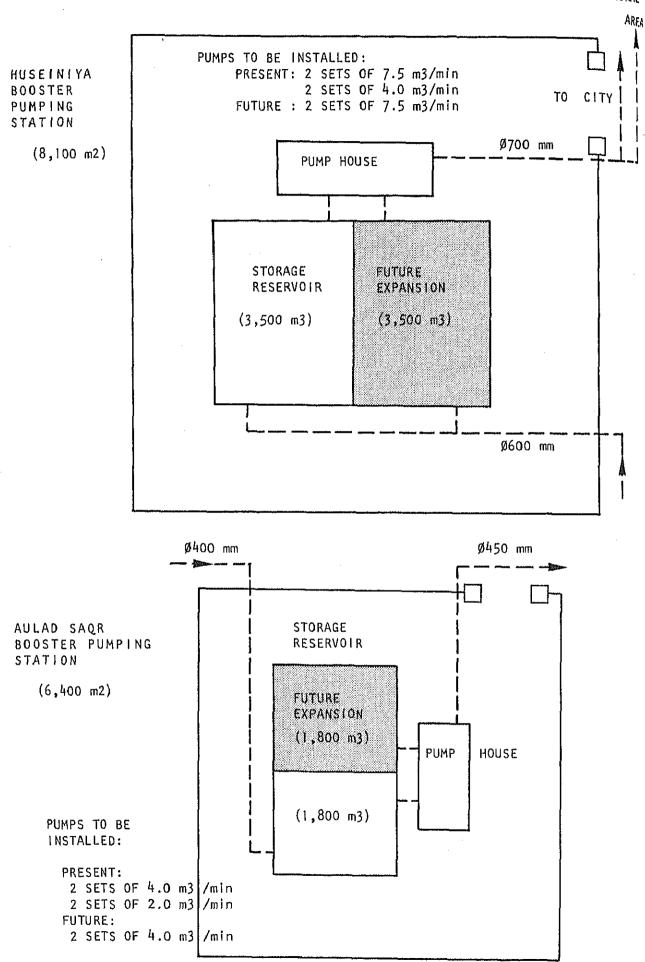


Fig-2.1.5 Schematic Layouts of Booster Pumping Stations

2.1.6 Service Mains

Service mains are defined for the present project as the distribution pipes within the cities or villages.

The project includes installation of the following service mains:

Table -2.1.5 Proposed Service Mains

| Proposed pipe length (km) | | | | | |
|---------------------------|--------|--------|-----------|---------|-------|
| Markaz | | City | | Rural | Total |
| | (ø250) | (ø150) | Sub-total | (ø 150) | |
| New Northeast | System | | | | |
| Huseiniya | 3.1 | 2.4 | 5.5 | 35.5 | 41.0 |
| Faqus | 4.2 | 3.1 | 7.3 | 48.0 | 55.3 |
| | | | | | |
| Sub-total | 7.3 | 5.5 | 12.8 | 83.5 | 96.3 |
| New Kafr Saqr | System | | | | |
| Kafr Saqr | 2.2 | 1.6 | 3.8 | 36.5 | 40.3 |
| Aby Kebir | 11.3 | 8.4 | 19.7 | - | 19.7 |
| Sub-total | 13.5 | 10.0 | 23.5 | 36.5 | 60.0 |
| Total | 20.8 | 15.5 | 36.3 | 120.0 | 156.3 |

2.2 Treatment Plants

2.2.1 General Concept

1) Design Capacity

Taking account of the water to be used for the plant operation, 110 % of the maximum day demand is employed as the design capacity for the new treatment plants.

- Northeast Plant (2005): $90,000 \text{ m}3/d \times 1.1 = 99,000 \text{ m}3/d (1.15 \text{ m}3/s)$
- Kafr Saqr Plant (2005) : $60,000 \text{ m}3/d \times 1.1 = 66,000 \text{ m}3/d (0.76 \text{ m}3/s)$

In the First Priority-Phase Program, the plants will aim to have supply capacity up to the year 1995. According to the water demand study, the capacity to be newly developed by 1995 is about a half of the 2005 capacity. In the first stage, therefore, each plant will be constucted with half capacity.

2) Plant Site

Proposed sites for the treatment plants are shown in Fig-2.2.1. The sites were selected taking account of the quality and quantity of the canal water, the distance from the served area, and the convenience of the operation, maintenance and supply. Northeast Plant is proposed to be constructed on the left bank of Saidiya Canal, about one kilometer upstream far from the Abu Shalabi Gate. Kafr Saqr Plant is proposed to be constructed on the right bank of Muweis Canal, about one kilometer upstream of the core area of Kafr Saqr City.

3) Treatment Process

Canal is the sole available source for potable water in the northern half of the study area to meet the demand for the present project. Saidiya Canal for Northeast Plant and Muweis Canal for Kafr Sagr Plant are selected as water sources. For the canal water, the treatment method by rapid sand filtration is employed. The treatment process is schematically shown on Fig-2.2.2.

The treatment process and layout of the plants are planned in consideration of the following matters:

1) Grit Chamber and Junction Well

In the near future canals will be operated uninterruptedly all year round by means of dredging without stoppage of canals. Taking account of possibly increasing turbidity of the canal water due to the dredging, enough space for the Grit Chamber is arranged beside the Raw Water Pumping Well. The Junction Well is planned to install valves/gates of the Grit Chamber for the convenience of the future construction and operation.

2) Wastewater Treatment

Although the wastewater of the plant is to be directly discharged to the canal, it will become necessary in future to treat it because of pollution control. For the coming wastewater treatment as well, the space is considered in the plant site.

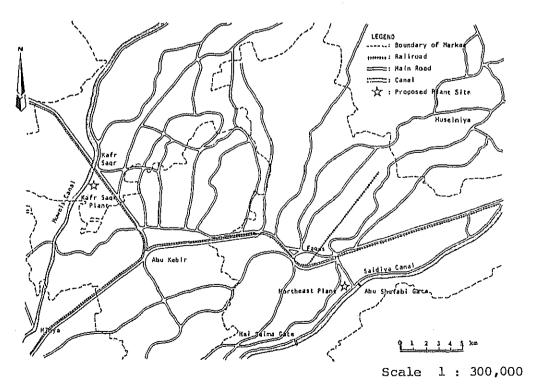


Fig-2.2.1 LOCATION PLAN OF PROPOSED PLANT SITES

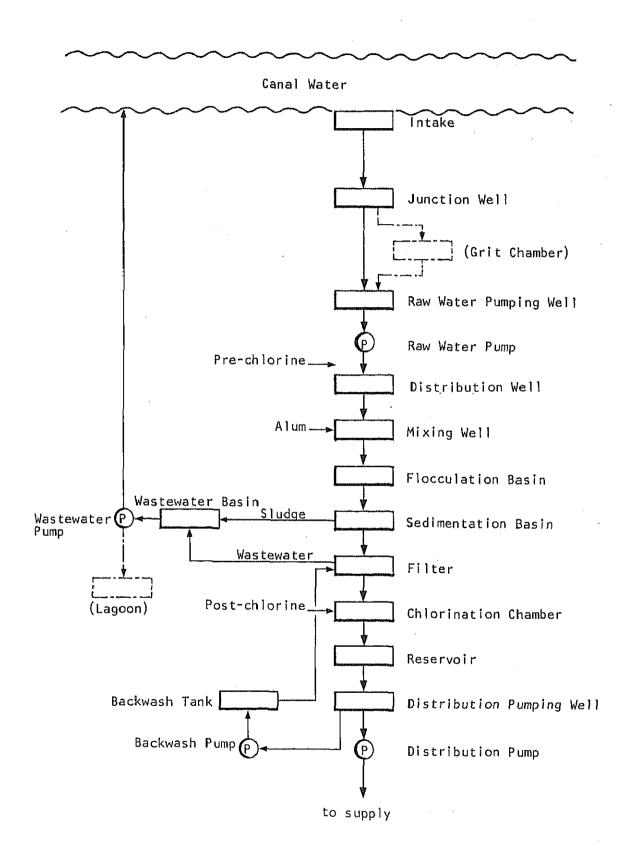


Fig-2.2.2 FLOW DIAGRAM OF TREATMENT PROCESS

2.2.2 Comparison of Facilities

1) Mixing Well

The widely employed rapid-mixing types for water treatment are as follows:

- a) Hydraulic mixing by waterfall,
- b) Hydraulic mixing by baffle plates/walls,
- c) Mechanical mixing by rotary mixer(s), and
- d) Mechanical mixing by pump (s)

Main features of them are listed in Table-2.2.1. The main feature of the mechanical system will be an adjustable intensity of mixing according to qualitative and/or quantitative fluctuation of raw water. As a matter of course, an adequate periodical and routine maintenance become indispensable for mounted machinery.

In case of hydraulic types which utilize water energy itself, any mechanical facilities are not necessary to get an appropriate mixing condition, while sludge is liable to be heaped up on a bottom of the mixing basin.

However, devices to adjust mixing intensity of mechanical type are scarcely operated everywhere in practice after mounting mixers, due to insensibility for mixing.

From the viewpoint of easy maintenance, hydraulic waterfall mixing type is employed as rapid mixing.

2) Flocculation Basin

Basic matters of slow mixing system in other words flocculation system are quite similar to rapid mixing's stated above in substance, and flocculation system as well can broadly be classified into two types; hydraulic type and mechanical. Although both types have a vertical and horizontal types respectively, there are no basic differences between them.

Table-2.2.2 shows a comparison for main matters of flocculation system. Hydraulic type generally necessitates rather high head loss comparing with mechanical system and desludging is rather difficult. In addition, as stated in the preceding section, hydraulic type is unable to carry out an adjustment by means of rotation of mixers according to changing conditions of water.

The most characteristic matter of mechanical system, adjustment, is not always utilized practically, and stepless variable speed reducers which are a type of devices are usually operated in a fixed speed.

Taking account of such practical condition and its easy maintenance, hydraulic type flocculation system is employed in spite of rather difficult desludging and $_{\rm much}$ loss head.

3) Sedimentation Basin

In Table-2.2.3, the most popular types of sedimentation basis are compared in point of conspicuous features, that is,

- a) Horizontal flow type,
- b) Horizontal flow type with settling devices, and
- c) Up-flow type

The type of the shortest detention time will generally be a horizontal flow type with settling devices as shown in the table. However, the construction cost does not always follows the order of their detention time in general because of the expensive devices' cost. The most costly one will be a horizontal flow type and then a horizontal flow type with settling devices, and the cheapest one an up-flow type. Their maintenance costs do not have much difference.

On the other hand, a size of sedimentation basin equipped with settling devices can be made small because of effective activity of devices. However, an appropriate attention for the devices have to be paid to the following:

- a) Sludge lying on settling devices which may cause a serious damage of the devices, and
- b) Deterioration of devices' material made of plastics due to mainly sunbeam.

Generally desludging becomes rather difficult because of the devices by which sludge space is covered.

The main feature of up-flow type of sedimentation basin is its compactness in size, and therefore the economization is derived. At the same time, it arises out of the compactness that the type is weak against fluctuation, qualitative and quantitative, of water, especially rapid fluctuation.

Horizontal flow type is most stable for fluctuation stated above, and operation and maintenance of this type are quite easy in general on account of big size. From the standpoint of stability for fluctuation, and easy operation and maintenance, horizontal flow type is employed for the project.

4) Wastewater Treatment

The wastewater treatment facilities are not constructed in the beginning of the First Priority-Phase Program stage for the time being, and only enough space for the facilities is maintained for the future necessity.

In Table-2.2.4, natural drying treatment and mechanical treatment systems are compared broadly. The necessary area is estimated based on the following precondition; wash water of filters is planned as drawing into canal directly, and drain water from sedimentation basins is treated by the treatment system.

For the preparation of the necessary area for treatment system, natural drying system is employed from the viewpoint of easy operation and maintenance, and little precipitation, although the system necessitates rather wide space.

Table-2.2.1 COMPARISON OF MIXING WELL

| Type Item | a. Hydraulic Type | b. Mechanical Type |
|---|---|---|
| | | →———————————————————————————————————— |
| | The above figures show represen | tative types. |
| l. Facilities | Basin Weir Chemical feeding equipment | Basin Mechanical mixer Chemical feeding equipment |
| 2. Head loss | about 50 cm | Negligible |
| 3. Reliability for mixing | Good | Good |
| 4. Adjustability for water mixing | Unadjustable | Adjustable |
| 5. Maintenance and repair | Not necessary | Necessary |
| 6. Service life period | Long | Short |
| 7. Economical aspects 7-1 Construction cost | LE 39,200.~ | LE 40,600.— |
| 7-2 Operation cost | LE 0 | LE 190/Year |
| 8. Land area | 40 m2 (First Stage) | 30 m2 (First Stage) |
| | | |
| | | |

Table-2.2.2 COMPARISON OF FLOCCULATION BASIN

| Туре | a. Hydraulic Type | b. Mechanical Type |
|-------------------------------------|--------------------------------|---|
| Item . | a. Hydradric Type | S. Mechanical Type |
| · | | |
| | The above figures show represe | ntative types. |
| l. Facilities | Basin Baffle walls | Basin Flocculators and motors Motor room/space Compartment walls |
| 2. Head loss | About 40 cm | Negligible |
| 3. Reliability for flocculation | Good | Good |
| 4. Adjustability for water mixing | Unadjustable | Adjustable |
| 5. Maintenance and repair | Not necessary | Necessary |
| 6. Desludging | Difficult | Easy |
| 7. Service life period | Long | Short |
| 8. Economical | | |
| aspects 8-1 Construction cost | LE 150,000 | LE 314,600 |
| 8-2 Operation cost | LE O | LE 320/Year |
| 9. Land area | 300m2 (First Stage) | 300 m2 (First Satge) |
| | | |
| | | |
| | | |

Table-2.2.3 COMPARISON OF SEDIMENTATION BASIN

| Туре | a. Horizontal Flow | b. Horizontal Flow | |
|--|---------------------------|--|---|
| Item | Type | Type with settl- ing plates/tubes | C. Up-flow Type |
| | | | → 1 → 1 → |
| | The above figures sh | pw representative type | es. |
| l. Facilities | Basin Sludge scraper | Basin Settling plates/ tubes Sludge scraper | Basin Mechanical equip- ment Chemical feeding equipment |
| 2. Detention time | 3 - 5 hrs | 0.5 - 1 hrs | 1.5 - 2 hrs |
| 3. Adaptability to fluctua- tion of water quality and quantity | Very good | Good | Good |
| 4. Desludging | Easy | Rather difficult | Rather easy |
| 5. Operacion | Easy | Easy | Rather difficult |
| 6. Maintenance and repair | Easier | £asy | Rather difficult |
| 7. Service life period | Long | Long | Short |
| 8. Economical aspects | | | |
| 8-1 Construction cost | LE 986,100 | LE915,100 | LE 630,900 |
| 8-2 Operation cost | IE 370/Year | LE 930/Year | LE 590/Year |
| 9. Land area | 2,800 m2 (First Stage) | 700 m2 (First Stage) | l,100 m2 (First Stage) |

Table-2.2.4 COMPARISON OF FUTURE TREATMENT FOR SETTLED SLUDGE AND WASH WASTE

| | | <u> </u> |
|---|---|---|
| Type | Natural Treatment | Mechanical Treatment |
| 1. Flow diagram | Settled Sludge Wash Waste Sludge Basin 400m3 x 2 Basin 400m3 x 2 Lagoon or Drying Bed 3,200m2 x lm | Settled Sludge Wash Waste Sludge Basin 400m3 x 2 Basin 400m3 x 2 P Thickener 100m2 x 4m x 2 Canal Mechanical Dewatering 86m3/d Cake Yard 8.6t/d |
| 2. Characteristics | Efficiency affected by precipitation. Easy operation and main-tenance | A number of necessary equipments High technical level operation & maintenance |
| 3. Economical aspects 3-1 Construction cost | LE 750,700 | LE 6,020,700 |
| 3-2 Operation cost | LE 3,300/Year | LE 1,600/Year |
| 4. Land Area | G,000 m2 Wastewater Basin Lagoon or Drying Bed Sludge Basin | 3,200 m2 Wastewater Basin Sludge Basin Mechanical Dewatering System Cake Yard |