

UNITED NATIONS

COMMITTEE FOR COORDINATION OF INVESTIGATIONS
OF THE LOWER MEKONG BASIN

SECOND PHASE REPORT
ON

NONG KHAI / VIENTIANE BRIDGE PROJECT
LAOS AND THAILAND

APPENDICES

NOVEMBER 1968

OVERSEAS TECHNICAL COOPERATION AGENCY
TOKYO JAPAN

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PREFACE

Nong Khai/Vientiane Bridge Project was taken up as one of the first priority projects in the Ten-Year Development Program for the Lower Mekong Basin at the 29th Session of the Committee for Coordination of Investigations of the Lower Mekong River Basin.

The feasibility investigation on the Nong Khai/Vientiane Bridge Project was carried out in two phases, in accordance with the Plan of Operation signed in April 1967 between the Government of Japan and the Mekong Committee.

The first phase investigation was undertaken for about two months from August to October 1967 and the second phase investigation for about four months from February to June 1968.

The results of the field investigations and the subsequent studies such as topographic survey, soil survey, material survey, analysis of meteor- and hydrological data, economic survey and so forth, are compiled in Appendices.

The report therefore constitutes an integral part of the Second Phase Report of the Nong Khai/Vientiane Bridge Project.

NONG KHAI/VIENTIANE BRIDGE PROJECT

APPENDICES

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APPENDIX I

TOPOGRAPHIC SURVEY

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1.1 Survey Operations

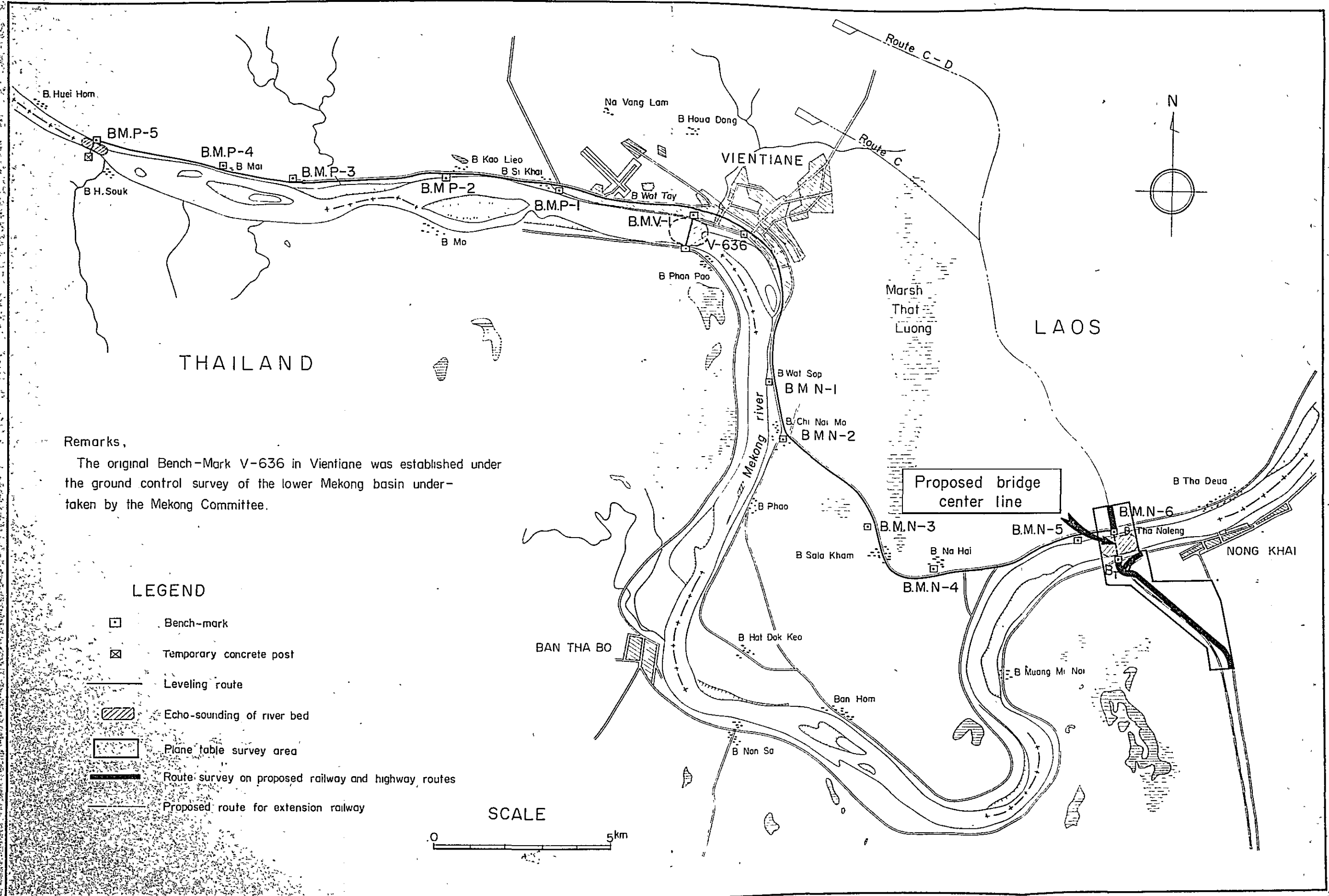
Survey Operations carried out
in the First and the Second Phase Investigations

Items	Locations
<u>1st phase investigations</u>	
1. Spot leveling (Double-run leveling)	From Vientiane to each of the three bridge sites, Pa Mong, Vientiane and Nong Khai.
2. Echo-sounding (Accuracy of the machine: 1/100)	In the Mekong river channel based upon the bench-mark established at the three sites.
3. Simple triangulation to measure the width of the Mekong	Three bridge sites: Pa Mong, Vientiane and Nong Khai
<u>2nd phase investigations</u>	
1. Spot leveling to unify the elevation of the topography of the project area extending over both countries (Double-run leveling)	Bench-marks B.M.N-6 to B ₁ via the bench-mark established by N.E.A in the site of Hydrographic Office
2. Plane table survey including echo-sounding (Scale: 1/2,000, 1m contour)	Whole project area of the Nong Khai bridge site
3. Route survey (Transverse survey: 50 m long each on both sides of the route at intervals of 100 m along the route)	Routes for the access railway and the access highway on both the countries
4. Triangulation to measure the exact width of the Mekong	Nong Khai bridge site

Remarks: -

- 1) The results of survey operations executed during the first phase investigation regarding Pa Mong and Vientiane bridge sites are not compiled in Appendices. As for Nong Khai bridge site, only the results of spot leveling are given in Appendices.
- 2) The map on a scale of 1/2,000 obtained from the plane table survey is not listed in Appendices. Reference, however, is made to PLATE 2 titled "GENERAL LAYOUT" scaled down to 1/10,000 from the above map.
- 3) The results of transverse survey operations in the route survey are not compiled in Appendices.

SURVEY OPERATIONS



Remarks.

The original Bench-Mark V-636 in Vientiane was established under the ground control survey of the lower Mekong basin undertaken by the Mekong Committee.

LEGEND

- Bench-mark
- Temporary concrete post
- Leveling route
- Echo-sounding of river bed
- Plane table survey area
- Route survey on proposed railway and highway routes
- Proposed route for extension railway

SCALE



1.2. Spot Leveling

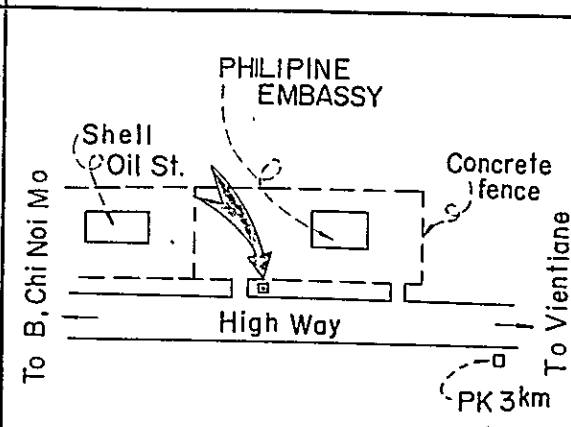
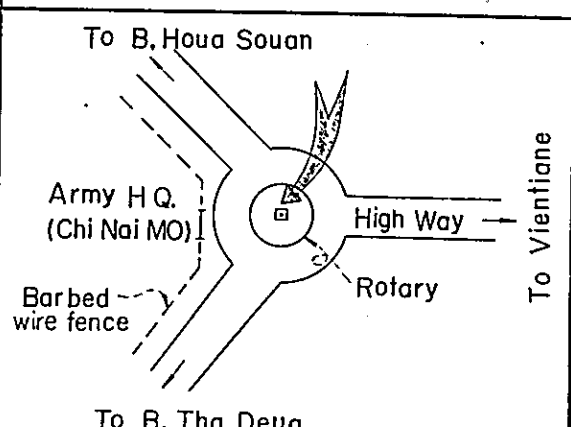
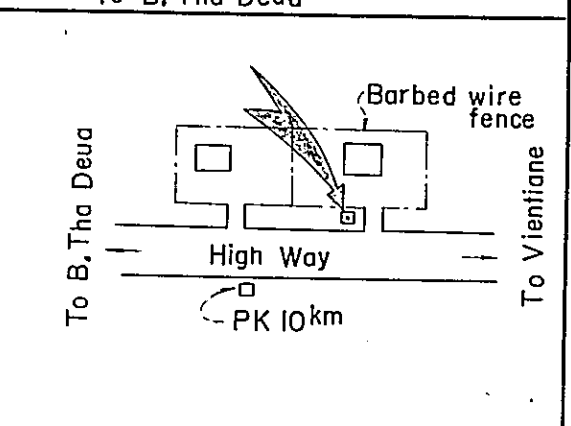
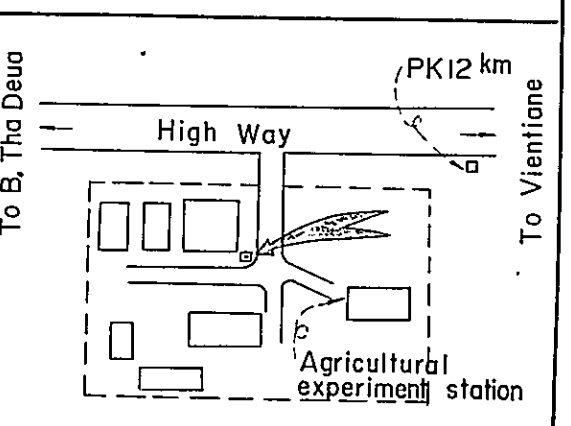
- 1) The original bench-mark V636 is represented by a spike into a wall of the building of the Ministry of Foreign Affairs in Vientiane with the approval of the Mekong Committee.
- 2) Leveling between the bench-marks V636 and B.M.N-6 was carried out in the first phase investigation, and that between the bench-marks B.M.N-6 and B.M.N-7 was carried out in the second phase investigation via the bench-mark established by N.E.A. in the site of Hydrographic Office.
- 3) The bench-marks B.M.N-6 and B.M.N-7 were set on the bridge center line proposed in the First Phase Report. The mark B.M.N-7 is called the mark B₁ in the Second Phase Report for the sake of convenience.
- 4) The elevation of the bench-mark in the site of Hydrographic Office is EL. 166.044 above the mean sea level at Ko Lak datum. On the other hand, according to the result of leveling made in the second phase investigation, the elevation of this bench-mark becomes EL. 165.861. The difference of the elevation between the two is 18.3 centimeters. Concerning this matter, please refer to the Second Phase Report, Chapter II.

RESULTS OF LEVELING

Elevation of the bench-marks set on the leveling route

T. P. No.	DISTANCE	DIFFERENCE OF ELEVATION (m)			ADJUST	ADJUSTED DIFFERENCE	ELEVATION (m)	REMARKS
		1	2	MEAN				
TPN-3		-	-	-			171.500	
BMN-1		+2.878	+2.888	+2.883			174.383	
BMN-2		-	-	-			170.972	TPN-6
TPN-10		-	-	-			174.350	
BMN-3		+0.088	+0.087	+0.087			174.437	
TPN-13		-	-	-			167.573	
BMN-4		+0.218	+0.214	+0.216			167.789	
TPN-18		-	-	-			167.655	
BMN-5		-0.190	-0.191	-0.190			167.465	
Checking midway on the leveling								
TPN-4		-	-	-			170.641	
Zero point of staff gage in the Wat Sop G.S.		-12.643	-	-12.643			157.998	
Remarks:								
1) This leveling was of single-run.								
2) Since the elevation of the zero point of staff gage in the Wat Sop gaging station is EL.158.040 above the mean sea level at Ko Lak datum, the difference of the elevations is 4.2 centimeters.								

Location of Bench-Marks (1)

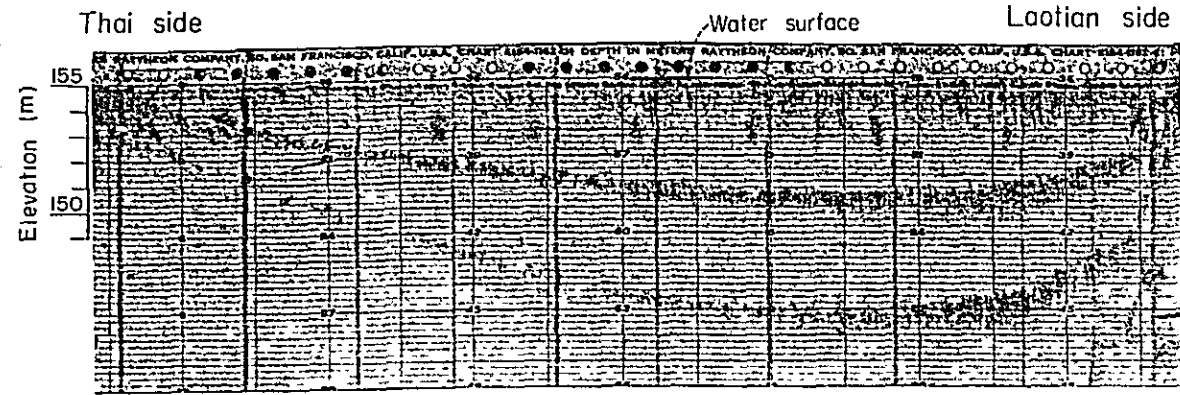
B. M. No.	DESCRIPTIONS		SKETCH
N-1	ELEVATION	174.383	
	LOCATION	Ban Wat Sop	
	ESTABLISHED ON	26 Sep. 1967	
	CARVED ELEVATION		
	Concrete precast post		
N-2	ELEVATION	170.972	
	LOCATION	Ban Chi Nai Mo	
	ESTABLISHED ON	26 Sep. 1967	
	CARVED ELEVATION		
	Concrete precast post		
N-3	ELEVATION	174.437	
	LOCATION	Ban 10 Km	
	ESTABLISHED ON	28 Sep. 1967	
	CARVED ELEVATION		
	Concrete precast post		
N-4	ELEVATION	167.789	
	LOCATION	Ban Na Hai	
	ESTABLISHED ON	28 Sep. 1967	
	CARVED ELEVATION		
	Concrete precast post		

Location of Bench-Marks (2)

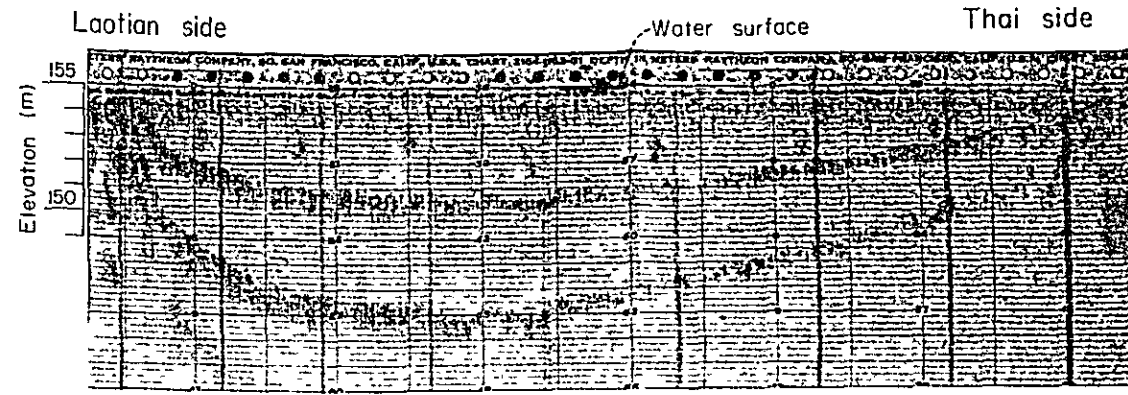
B. M. No.	DESCRIPTIONS		SKETCH
N-5	ELEVATION	167.465	
	LOCATION	Ban Tha Naleng	
	ESTABLISHED ON	28 Sep. 1967	
	CARVED ELEVATION		
	Concrete precast post		
N-6	ELEVATION	168.224	
	LOCATION	Ban Tha Naleng (No.1 proposed site)	
	ESTABLISHED ON	28 Sep. 1967	
	CARVED ELEVATION		
	Concrete precast post		
N-7	ELEVATION	166.574	
	LOCATION	Wat Chommane (No.1 proposed site)	
	ESTABLISHED ON	9 Mar. 1968	
	CARVED ELEVATION		
	Concrete precast post		
	ELEVATION		
	LOCATION		
	ESTABLISHED ON		
	CARVED ELEVATION		

1.3 Echo - sounding

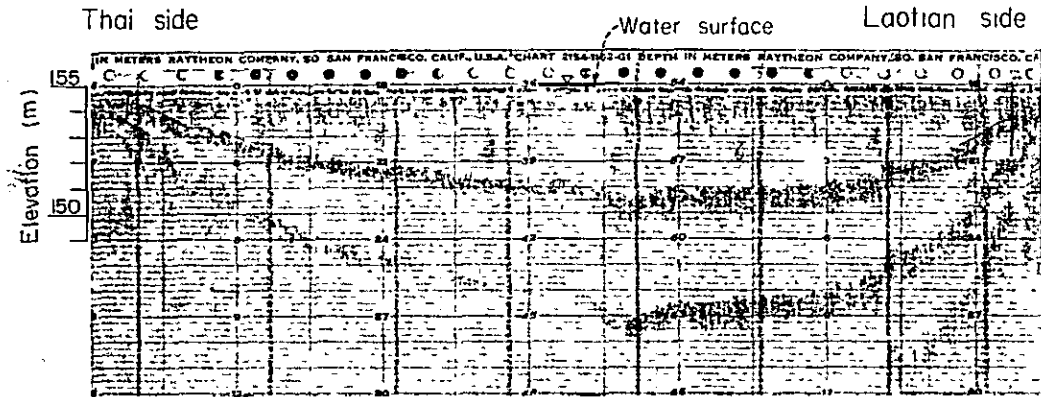
ECHO - SOUNDING RECORDS AT THE BRIDGE SITE



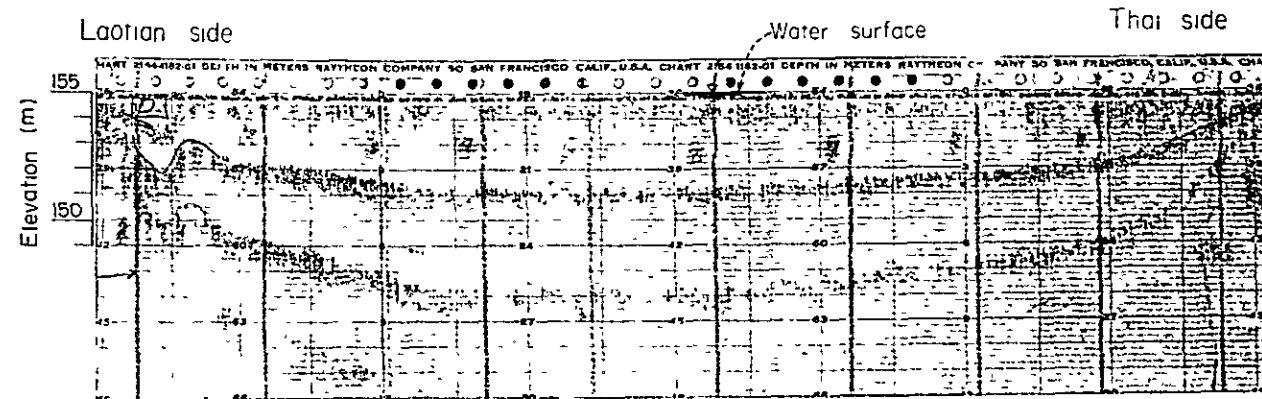
COURSE - A



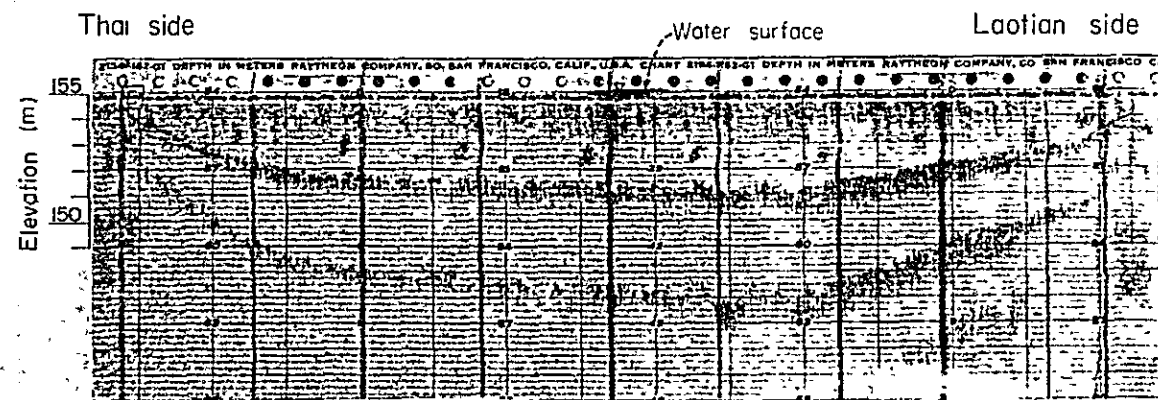
COURSE - B



COURSE - C



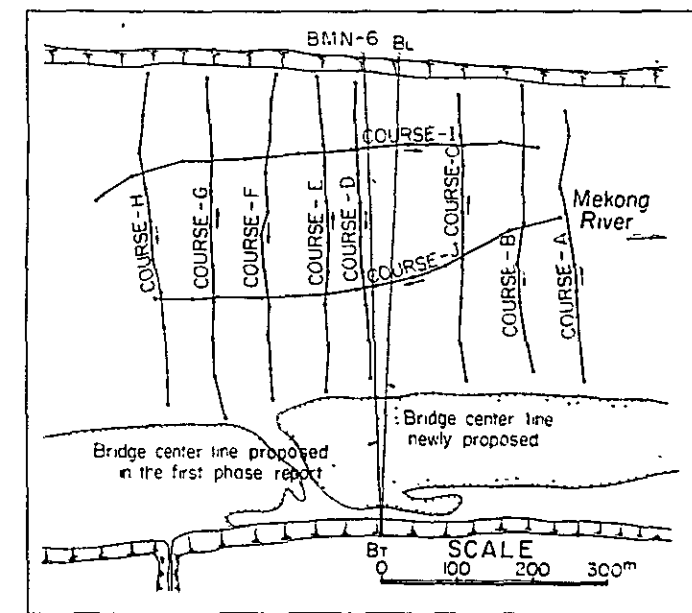
COURSE - D



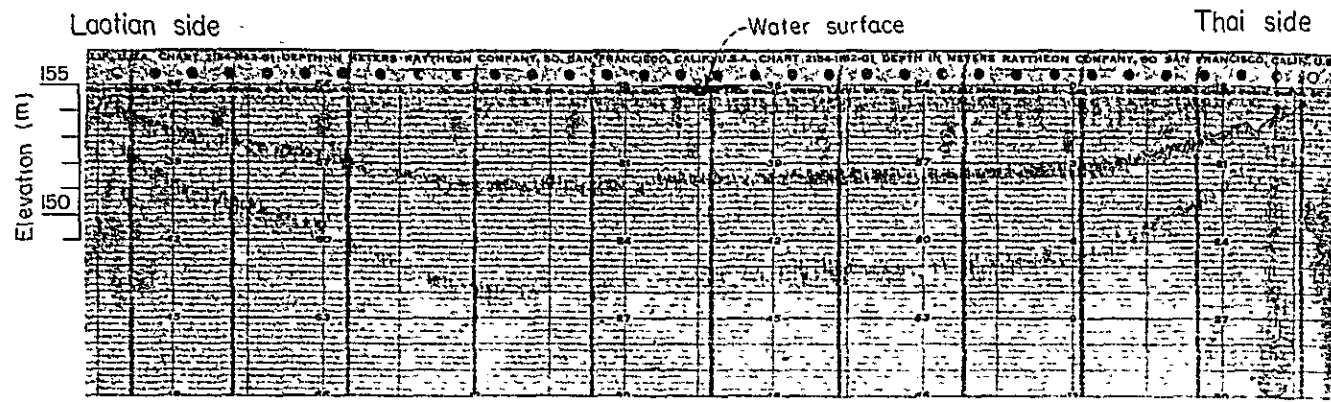
COURSE - E

Remarks :

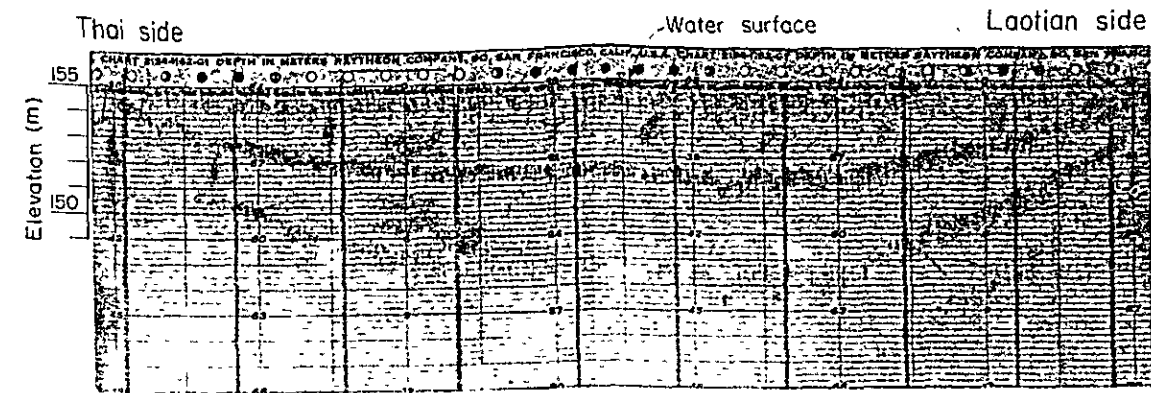
- 1) These records were taken on April 5 1968 in the second phase investigation
- 2) Water surface on April 5, 1968, EL. 155.0^m



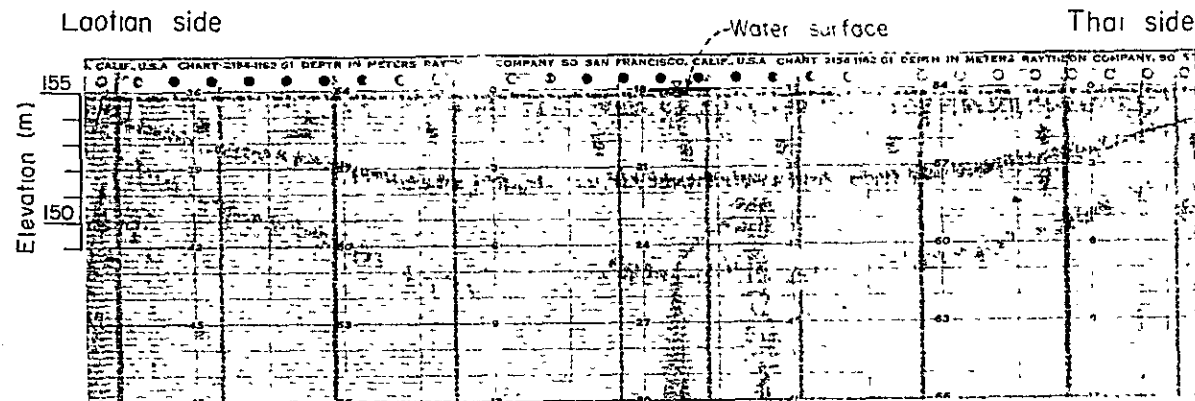
ECHO - SOUNDING RECORDS AT THE BRIDGE SITE - CONTINUED



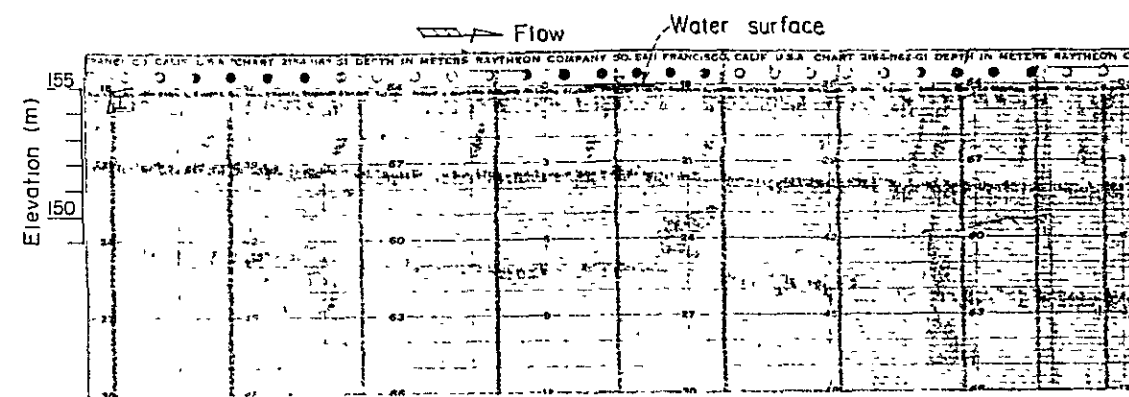
COURSE - F



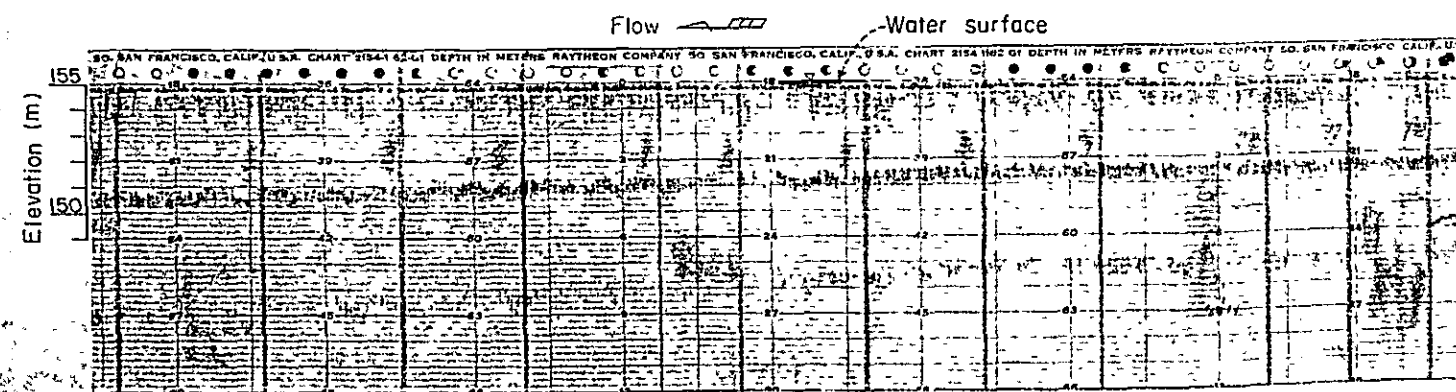
COURSE - G



COURSE - H



COURSE - I



COURSE - J

Remarks

- 1) These records were taken on April 5, 1968 in the second phase investigation
- 2) Water surface on April 5, 1968, EL. 155.0^m

1.4 Triangulation

Triangulation Computation

1. General

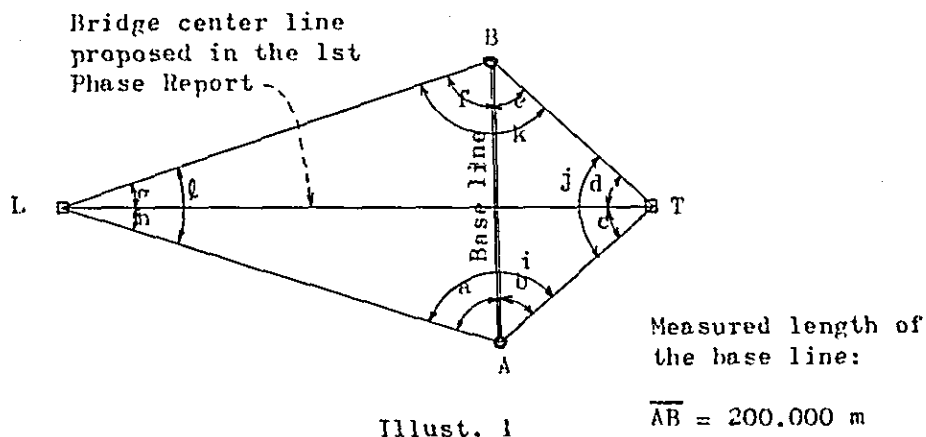
Triangulation was carried out in the second phase investigation as well as the first phase investigation to obtain the exact width of the Mekong along the proposed bridge center line that had been determined in the First Phase Report.

The base line for triangulation is desirable to be provided on either Laotian or Thai bank. But, it takes much time to do so because of jungle clearing. By this reason the base line was provided on the sandbar in the Mekong, and then measured and adjusted so that the errors resulting from high temperature and others could be minimized.

The exact length between the two temporary bench-marks established on both banks of the Mekong for triangulation was computed by means of the electronic computer on the basis of the results of the triangulation, as mentioned below.

2. Computation

The base line \overline{AB} and the angles of the quadrilateral BLAT shown in the following Illust. 1. were observed.



Each angle was observed as follows.

a. 75°43'15.0"	g. 11°14'47.5"
b. 54°19'10.0"	h. 10°42'00.0"
c. 39°15'55.0"	i. 130°02'00.0"
d. 37°15'05.0"	j. 76°31'10.0"
e. 49°10'00.0"	k. 131°30'05.0"
f. 82°19'50.0"	l. 21°56'32.5"

2.1. Base Line Correction

The measured length of the base line was corrected in the three items expressed in the following equation. The slope and sea-level corrections were omitted because these are negligibly small.

$$D = D_N + C_t + C_s + C_p$$

where, D_N : measured length of the base line = 200,000 m
 D : corrected length
 C_t : temperature correction
 C_s : sag correction
 C_p : tension correction

(1) Temperature correction

$$C_t = D_N \cdot \alpha (T_m - T_o)$$

where, T_o : standard temperature = 15°C
 T_m : mean temperature = 29°C
 α : coefficient of expansion of steel tape
 = 0.0000117 m/°C

$$\therefore C_t = 200 \times 0.0000117 \times (29 - 15) \\ = 0.03276 \text{ m}$$

(2) Sag correction

$$C_s = -\frac{D_N}{24} \cdot \frac{w \cdot d}{P}^2$$

where, w : weight of steel tape per meter = 0.02158 kg/m
 d : supported length = 10 m
 P : mean tension = 10 kg

$$\therefore C_s = -\frac{200}{24} \times \left(\frac{0.02158 \times 10}{10} \right)^2$$

$$= -0.00388 \text{ m}$$

(3) Tension correction

$$C_p = D_N \cdot \frac{(P - P_0)}{E \cdot S}$$

where, P : mean tension = 10 kg
 P_0 : standard tension = 7 kg
 E : modulus of elasticity for steel = $2.1 \times 10^6 \text{ kg/cm}^2$
 S : cross sectional area of steel tape = 0.02749 cm^2

$$\therefore C_p = 200 \times \frac{(10 - 7)}{2.1 \times 10^6 \times 0.02749}$$

$$= 0.01040 \text{ m}$$

As a result of the above calculation, the corrected length of the base line is obtained as follows.

$$D = 200 + 0.03276 - 0.00388 + 0.01040$$

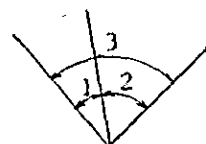
$$= 200.03928 \text{ m}$$

Accordingly the base line is 200.0393 meters long.

2.2. Angle Adjustment2.2.1. Condition

The quadrilateral BIAT in Illust. 1. has to be adjusted so as to satisfy the following three conditions.

- i. The sum of Angles 1 and 2 shall be Angle 3.
(see Illust. 2)
- ii. The sum of the interior angles of a triangle shall be exactly 180°.
- iii. The length of a side-line in a triangulation scheme is the same regardless of the conceivable courses of the computation.



Illust. 2

The above three conditions are expressed by the following eight equations.

Station equations

$$\begin{aligned}
 a_0 + b_0 &= i_0 \quad \dots\dots (1) \\
 c_0 + d_0 &= j_0 \quad \dots\dots (2) \\
 e_0 + f_0 &= k_0 \quad \dots\dots (3) \\
 g_0 + h_0 &= l_0 \quad \dots\dots (4)
 \end{aligned}$$

Angle equations

$$\begin{aligned}
 a_0 + b_0 + c_0 + d_0 + e_0 + f_0 \\
 + g_0 + h_0 &= 360'' \quad \dots\dots\dots (5) \\
 a_0 + h_0 &= d_0 + e_0 \quad \dots\dots\dots (6) \\
 b_0 + c_0 &= g_0 + f_0 \quad \dots\dots\dots (7)
 \end{aligned}$$

Side equations

$$\frac{\overline{AB}}{\sin (c_0+d_0)} = \frac{\overline{TB}}{\sin b_0}, \quad \frac{\overline{TB}}{\sin g_0} = \frac{\overline{LT}}{\sin (e_0+f_0)}$$

$$\frac{\overline{AB}}{\sin (g_0+h_0)} = \frac{\overline{AL}}{\sin f_0}, \quad \frac{\overline{AL}}{\sin c_0} = \frac{\overline{LT}}{\sin (a_0+b_0)}$$

$$\therefore \frac{\sin (a_0+b_0) \cdot \sin (c_0+d_0)}{\sin (e_0+f_0) \cdot \sin (g_0+h_0)} \cdot \frac{\sin f_0 \cdot \sin g_0}{\sin b_0 \cdot \sin c_0} = 1 \quad \dots\dots\dots (i)$$

Likewise

$$\frac{\overline{AB}}{\sin (c_0+d_0)} = \frac{\overline{AT}}{\sin e_0}, \quad \frac{\overline{AT}}{\sin h_0} = \frac{\overline{LT}}{\sin (a_0+b_0)}$$

$$\frac{\overline{AB}}{\sin (h_0+g_0)} = \frac{\overline{BL}}{\sin a_0}, \quad \frac{\overline{BL}}{\sin d_0} = \frac{\overline{LT}}{\sin (e_0+f_0)}$$

$$\therefore \frac{\sin (a_0+b_0) \cdot \sin (g_0+h_0)}{\sin (c_0+d_0) \cdot \sin (e_0+f_0)} \cdot \frac{\sin d_0 \cdot \sin e_0}{\sin a_0 \cdot \sin h_0} = 1 \quad \dots\dots\dots (ii)$$

The following equation is derived from the above two equations (i) and (ii).

$$\frac{\sin^2 (g_0+h_0) \cdot \sin b_0 \cdot \sin c_0 \cdot \sin d_0 \cdot \sin e_0}{\sin^2 (c_0+d_0) \cdot \sin a_0 \cdot \sin f_0 \cdot \sin g_0 \cdot \sin h_0} = 1 \dots\dots\dots (8)$$

2.2.2. Computation of Adjustment Value

Observation equations

$$\begin{array}{ll} a_0 = a_1 + v_a & g_0 = g_1 + v_g \\ b_0 = b_1 + v_b & h_0 = h_1 + v_h \\ c_0 = c_1 + v_c & i_0 = i_1 + v_i \\ d_0 = d_1 + v_d & j_0 = j_1 + v_j \\ e_0 = e_1 + v_e & k_0 = k_1 + v_k \\ f_0 = f_1 + v_f & l_0 = l_1 + v_l \end{array}$$

where a_0, b_0, \dots, l_0 = Most probable values of angles
 a_1, b_1, \dots, l_1 = Observed angles
 v_a, v_b, \dots, v_l = Probable errors (= adjustment values)

Condition equations

Equations (1) to (8) are rewritten from the above observation equations as follows.

$$\begin{array}{l} 1) (a_1+v_a) + (b_1+v_b) = i_1 + v_i \\ \therefore v_a+v_b-v_i+w_1 = 0 \equiv \varphi_1 \quad (\text{where } w_1 = a_1+b_1-i_1) \dots\dots\dots (9) \end{array}$$

$$\begin{array}{l} 2) (c_1+v_c) + (d_1+v_d) = j_1 + v_j \\ \therefore v_c+v_d-v_j+w_2 = 0 \equiv \varphi_2 \quad (\text{where } w_2 = c_1+d_1-j_1) \dots\dots\dots (10) \end{array}$$

$$\begin{array}{l} 3) (e_1+v_e) + (f_1+v_f) = k_1 + v_k \\ \therefore v_e+v_f-v_k+w_3 = 0 \equiv \varphi_3 \quad (\text{where } w_3 = e_1+f_1-k_1) \dots\dots\dots (11) \end{array}$$

$$\begin{array}{l} 4) (g_1+v_g) + (h_1+v_h) = l_1 + v_l \\ \therefore v_g+v_h-v_l+w_4 = 0 \equiv \varphi_4 \quad (\text{where } w_4 = g_1+h_1-l_1) \dots\dots\dots (12) \end{array}$$

$$5) (a_1+v_a) + (b_1+v_b) + (c_1+v_c) + (d_1+v_d) + (e_1+v_e) + (f_1+v_f) + (g_1+v_g) + (h_1+v_h) - 360^0 = 0$$

$$\therefore v_a+v_b+v_c+v_d+v_e+v_f+v_g+v_h+w_5 = 0 \equiv \varphi_5 \dots \dots \dots (13)$$

where $w_5 = a_1+b_1+c_1+d_1+e_1+f_1+g_1+h_1 - 360^0$

$$6) (a_1+v_a) + (h_1+v_h) = (d_1+v_d) + (e_1+v_e)$$

$$\therefore v_b+v_h-v_d-v_e+w_6 = 0 \equiv \varphi_6 \text{ (where } w_6 = a_1+h_1-d_1-e_1) \dots \dots \dots (14)$$

$$7) (b_1+v_b) + (c_1+v_c) = (g_1+v_g) + (f_1+v_f)$$

$$\therefore v_e+v_c-v_g-v_f+w_7 = 0 \equiv \varphi_7 \text{ (where } w_7 = b_1+c_1-g_1-f_1) \dots \dots \dots (15)$$

$$8) 2 \log \sin (g_0+h_0) - 2 \log \sin (c_0+d_0) + \log \sin b_0 + \log \sin c_0 + \log \sin d_0 + \log \sin e_0 - \log \sin a_0 - \log \sin f_0 - \log \sin g_0 - \log \sin h_0 = 0$$

The logarithmic sine can be expanded to the form of Taylor's series.

$$\log \sin (M + v) = \log \sin M + \mu \cot M \frac{v}{\rho} + \dots \dots \dots$$

$$\approx \log \sin M + d \cdot v$$

in which v is a very small angle as compared with M and d is tabular difference of 1" for $\log \sin M$.

$$(d = \mu/\rho \cot M = 21.055 \times 10^{-7} \cot M)$$

Thus,

$$\{2 \log \sin (g_1+h_1) + d_{gh}(v_g+v_h)\} - \{2 \log \sin (c_1+d_1) + d_{cd}(v_c+v_d)\} + (\log \sin b_1 + d_b v_b) + (\log \sin c_1 + d_c v_c) + (\log \sin d_1 + d_d v_d) + (\log \sin e_1 + d_e v_e) - (\log \sin a_1 + d_a v_a) - (\log \sin f_1 + d_f v_f) - (\log \sin g_1 + d_g v_g) - (\log \sin h_1 + d_h v_h) = 0$$

$$\therefore d_{gh}(v_g+v_h) - d_{cd}(v_c+v_d) + d_b v_b + d_c v_c + d_d v_d + d_e v_e - d_a v_a - d_f v_f - d_g v_g - d_h v_h + w_8 = 0 \equiv \varphi_8 \dots \dots \dots (16)$$

where $w_8 = 2 \log \sin (g_1+h_1) - 2 \log \sin (c_1+d_1) + \log \sin b_1 + \log \sin c_1 + \log \sin d_1 + \log \sin e_1 - \log \sin a_1 - \log \sin f_1 - \log \sin g_1 - \log \sin h_1$

Correlate equation

The most probable values of adjustment angles will be determined so that the following value Ω becomes minimum according to the least square method.

$$\Omega = [vv] - 2 \lambda_1 \varphi_1 - 2 \lambda_2 \varphi_2 - \dots - 2 \lambda_8 \varphi_8$$

where $[vv]$ = sum of squares of probable errors
 λ = undetermined coefficient

The condition to satisfy the above equation is

$$\frac{\partial \Omega}{\partial v_\lambda} = 0 \quad (\lambda = a, b, c, \dots, l) \dots\dots\dots (17)$$

By differentiating the above equation (17), the following equations are given.

$$v_a - \lambda_1 - \lambda_5 - \lambda_6 + \lambda_8 d_a = 0 \dots\dots\dots (18)$$

$$v_b - \lambda_1 - \lambda_5 - \lambda_7 - \lambda_8 d_b = 0 \dots\dots\dots (19)$$

$$v_c - \lambda_2 - \lambda_5 - \lambda_7 + \lambda_8^{d_{cd}} - \lambda_8^{d_c} = 0 \dots\dots\dots (20)$$

$$v_d - \lambda_2 - \lambda_5 + \lambda_6 + \lambda_8^{d_{cd}} - \lambda_8^{d_d} = 0 \dots\dots\dots (21)$$

$$v_e - \lambda_3 - \lambda_5 + \lambda_6 - \lambda_8^{d_e} = 0 \dots\dots\dots (22)$$

$$v_f - \lambda_3 - \lambda_5 + \lambda_7 + \lambda_8^{d_f} = 0 \dots\dots\dots (23)$$

$$v_g - \lambda_4 - \lambda_5 + \lambda_7 - \lambda_8^{d_{gh}} + \lambda_8^{d_g} = 0 \dots\dots\dots (24)$$

$$v_h - \lambda_4 - \lambda_5 - \lambda_6 - \lambda_8^{d_{gh}} + \lambda_8^{d_h} = 0 \dots\dots\dots (25)$$

$$v_i + \lambda_1 = 0 \dots\dots\dots (26)$$

$$v_j + \lambda_2 = 0 \dots\dots\dots (27)$$

$$v_k + \lambda_3 = 0 \dots\dots\dots (28)$$

$$v_l + \lambda_4 = 0 \dots\dots\dots (29)$$

From the equations (9) to (16) and (18) to (29), the following correlate equations are obtained.

$$\begin{aligned}
 1) & (\lambda_1 + \lambda_5 + \lambda_6 - \lambda_8^{d_a}) + (\lambda_1 + \lambda_5 + \lambda_7 + \lambda_8^{d_b}) + \lambda_1 + w_1 \\
 & = 3\lambda_1 + 2\lambda_5 + \lambda_6 + \lambda_7 + (d_b - d_a) \lambda_8 + w_1 = 0 \dots\dots\dots (30)
 \end{aligned}$$

$$\begin{aligned}
 2) & (\lambda_2 + \lambda_5 + \lambda_7 - \lambda_8^{d_{cd}} + d_c \lambda_8) + (\lambda_2 + \lambda_5 - \lambda_6 \\
 & - \lambda_8^{d_{cd}} + \lambda_8^{d_d}) + \lambda_2 + w_2 \\
 & = 3\lambda_2 + 2\lambda_5 - \lambda_6 + \lambda_7 + (d_c + d_d - 2d_{cd}) \lambda_8 + w_2 = 0 \dots\dots (31)
 \end{aligned}$$

$$\begin{aligned}
 3) & (\lambda_3 + \lambda_5 - \lambda_6 + \lambda_8^{d_e}) + (\lambda_3 + \lambda_5 - \lambda_7 - \lambda_8^{d_f}) + \lambda_3 + w_3 \\
 & = 3\lambda_3 + 2\lambda_5 - \lambda_6 - \lambda_7 + (d_e - d_f) \lambda_8 + w_3 = 0 \dots\dots\dots (32)
 \end{aligned}$$

$$\begin{aligned}
 4) & (\lambda_4 + \lambda_5 - \lambda_7 - \lambda_8^{d_g} + \lambda_8^{d_{gh}}) + (\lambda_4 + \lambda_5 + \lambda_6 \\
 & + \lambda_8^{d_{gh}} - \lambda_8^{d_h}) + \lambda_4 + w_4 \\
 & = 3\lambda_4 + 2\lambda_5 + \lambda_6 - \lambda_7 + (2d_{gh} - d_g - d_h) \lambda_8 + w_4 = 0 \dots\dots (33)
 \end{aligned}$$

$$\begin{aligned}
 5) & (\lambda_1 + \lambda_5 + \lambda_6 - \lambda_8^{d_a}) + (\lambda_1 + \lambda_5 + \lambda_7 + \lambda_8^{d_b}) \\
 & + (\lambda_2 + \lambda_5 + \lambda_7 - \lambda_8^{d_{cd}} + \lambda_8^{d_c}) + (\lambda_2 + \lambda_5 - \lambda_6 \\
 & - \lambda_8^{d_{cd}} + \lambda_8^{d_d}) + (\lambda_3 + \lambda_5 - \lambda_6 + \lambda_8^{d_e}) \\
 & + (\lambda_3 + \lambda_5 - \lambda_7 - \lambda_8^{d_f}) + (\lambda_4 + \lambda_5 - \lambda_7 - \lambda_8^{d_g} \\
 & + \lambda_8^{d_{gh}}) + (\lambda_4 + \lambda_5 + \lambda_6 + \lambda_8^{d_{gh}} - \lambda_8^{d_h}) + w_5 \\
 & = 2\lambda_1 + 2\lambda_2 + 2\lambda_3 + 2\lambda_4 + 8\lambda_5 + (-d_a + d_b + d_c \\
 & + d_d + d_e - d_f - d_g - d_h - 2d_{cd} + 2d_{gh}) \lambda_8 + w_5 = 0 \dots\dots\dots (34)
 \end{aligned}$$

$$\begin{aligned}
 6) & (\lambda_1 + \lambda_5 + \lambda_6 - \lambda_8^{d_a}) + (\lambda_4 + \lambda_5 + \lambda_6 \\
 & + \lambda_8^{d_{gh}} - \lambda_8^{d_h}) - (\lambda_2 + \lambda_5 - \lambda_6 - \lambda_8^{d_{cd}} \\
 & + \lambda_8^{d_d}) - (\lambda_3 + \lambda_5 - \lambda_6 + \lambda_8^{d_e}) + w_6 \\
 & = \lambda_1 - \lambda_2 - \lambda_3 + \lambda_4 + 4\lambda_6 - (d_a + d_d + d_e \\
 & - d_{gh} - d_{cd}) \lambda_8 + w_6 = 0 \dots\dots\dots (35)
 \end{aligned}$$

$$\begin{aligned}
7) & (\lambda_1 + \lambda_5 + \lambda_7 + \lambda_8^{d_b}) + (\lambda_2 + \lambda_5 + \lambda_7 - \lambda_8^{d_{cd}} + \lambda_8^{d_c}) \\
& + (-\lambda_4 - \lambda_5 + \lambda_7 + \lambda_8^{d_g} - \lambda_8^{d_{gh}}) + (-\lambda_3 \\
& - \lambda_5 - \lambda_7 + \lambda_8^{d_f}) + w_7 = \lambda_1 + \lambda_5 - \lambda_3 - \lambda_4 + 4\lambda_7 \\
& + (d_b + d_c + d_f + d_g - d_{cd} - d_{gh}) \lambda_8 + w_7 = 0 \dots\dots\dots (36)
\end{aligned}$$

$$\begin{aligned}
8) & d_{gh} (\lambda_4 + \lambda_5 - \lambda_7 - \lambda_8^{d_g} + \lambda_8^{d_{gh}} + \lambda_4 + \lambda_5 + \lambda_6 \\
& + \lambda_8^{d_{gh}} - \lambda_8^{d_h}) - d_{cd} (\lambda_2 + \lambda_5 + \lambda_7 - \lambda_8^{d_{cd}} + \lambda_8^{d_c} \\
& + \lambda_2 + \lambda_5 - \lambda_6 - \lambda_8^{d_{cd}} + \lambda_8^{d_d}) + d_b (\lambda_1 + \lambda_5 \\
& + \lambda_7 + \lambda_8^{d_b}) + d_c (\lambda_2 + \lambda_5 + \lambda_7 - \lambda_8^{d_{cd}} + \lambda_8^{d_c}) \\
& + d_d (\lambda_2 + \lambda_5 - \lambda_6 - \lambda_8^{d_{cd}} + \lambda_8^{d_d}) + d_e (\lambda_3 + \lambda_5 \\
& - \lambda_6 + \lambda_8^{d_e}) - d_a (\lambda_1 + \lambda_5 + \lambda_6 - \lambda_8^{d_a}) - d_f (\lambda_3 \\
& + \lambda_5 - \lambda_7 - \lambda_8^{d_f}) - d_g (\lambda_4 + \lambda_5 - \lambda_7 - \lambda_8^{d_g} \\
& + \lambda_8^{d_{gh}}) - d_h (\lambda_4 + \lambda_5 + \lambda_6 + \lambda_8^{d_{gh}} - \lambda_8^{d_h}) + w_8 \\
& = (d_b - d_a) \lambda_1 + (d_d + d_e - 2d_{cd}) \lambda_2 + (d_e - d_f) \lambda_3 \\
& + (2d_{gh} - d_g - d_h) \lambda_4 + (-d_a + d_b + d_c + d_d + d_e \\
& - d_f - d_g - d_h - 2d_{cd} + 2d_{gh}) \lambda_5 - (d_a + d_d + d_e \\
& - d_{gh} - d_{cd}) \lambda_6 + (d_b + d_c + d_f + d_g - d_{cd} - d_{gh}) \lambda_7 \\
& + (d_a^2 + d_b^2 + d_c^2 + d_d^2 + d_e^2 + d_f^2 + d_g^2 + d_h^2 \\
& + 2d_{cd}^2 + 2d_{gh}^2 - 2d_g d_{gh} - 2d_h d_{gh} - 2d_c d_{cd} \\
& - 2d_d d_{cd}) \lambda_8 + w_8 = 0 \dots\dots\dots (37)
\end{aligned}$$

The undetermined coefficients $\lambda_1, \lambda_2 \dots \lambda_8$ are the solutions which simultaneously satisfy the above correlate equations (30) to (37).

Consequently, the most probable values of the angles can easily be computed from both the equations (18) to (29) and the observation equations.

2.3. Computation of length \overline{IT}

The side equations can be arranged as follows.

$$\overline{IT} = \frac{\sin (a_o + b_o) \cdot \sin f_o}{\sin (g_o + h_o) \cdot \sin c_o} \overline{AB}$$

Therefore, from the above equation the length between the two bench-marks set on both banks is determined based on the length of the base line \overline{AB} and the most probable values of angles obtained in the preceding paragraph.

These triangulation computations were made by a electronic computer requiring only a final result. As a result, the length \overline{IT} was figured out at 641.722 meters.

APPENDIX II

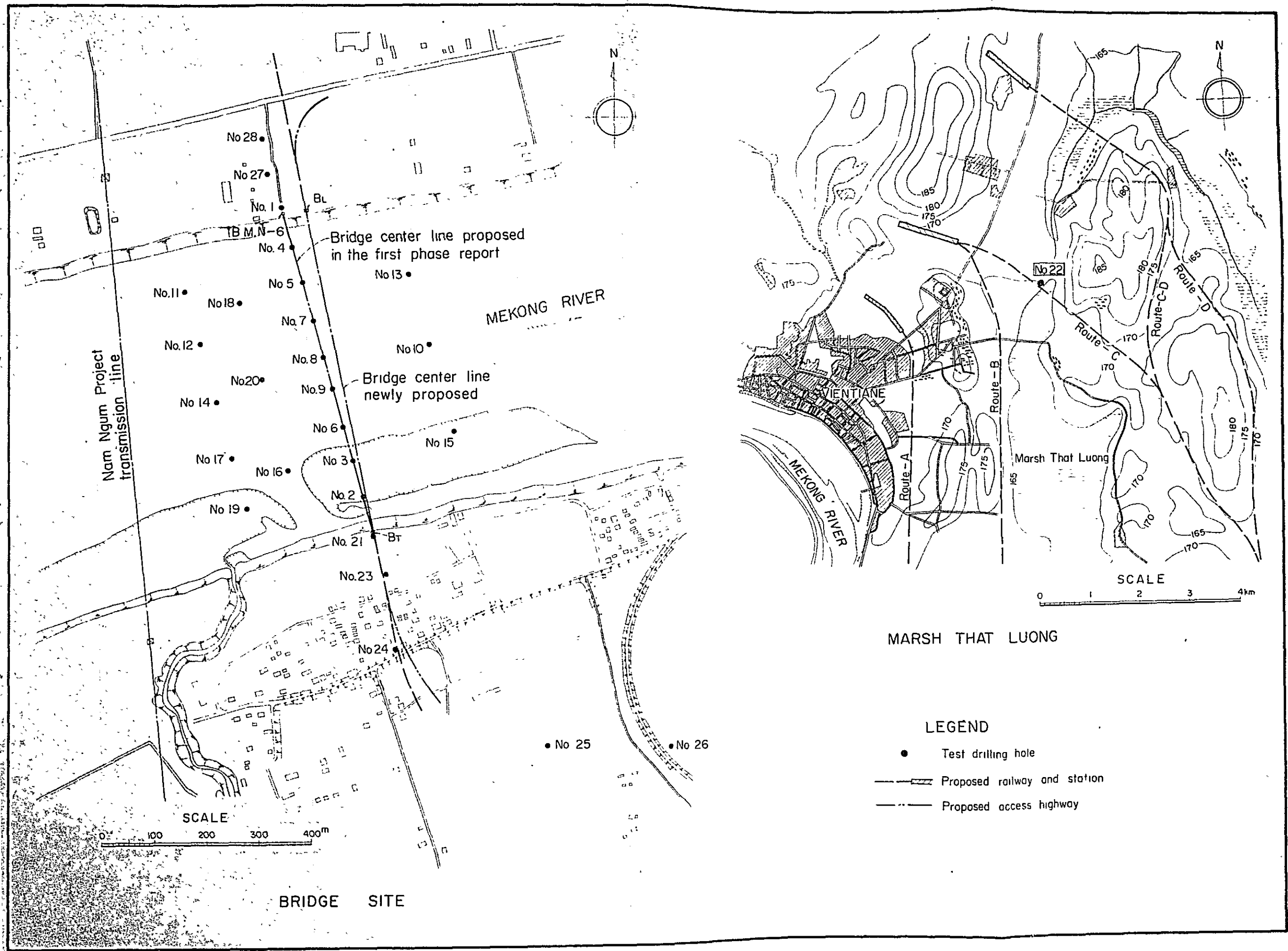
SOIL SURVEY

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2.1 Test Drilling

LOCATION OF THE TEST DRILLING HOLES



Summary of Test Drilling Holes

	Hole No.	Depth (m)	Elevation of ground surface(m)	Diameter of hole (mm)	Nos. of penetration test	Drilling expert	Operation period (year:1968)
Drilling of the nearby land	1	24.00	168.33	65 - 56	18	T.Onoue	Feb.23-Mar. 1
	21	22.00	166.59	65 - 56	13	K.Shirayama	Apr.29-May 5
	22	44.30	Unobserved	65 - 56	43	T.Onoue	Apr.27-May 28
	23	23.40	163.81	65 - 56	14	K.Shirayama	May 6-May 11
	24	26.00	165.80	65 - 56	13	"	May 12-May 16
	25	25.00	164.71	85 - 65	11	"	May 18-May 26
	26	35.00	165.41	85 - 65	23	"	May 27-Jun. 7
	27	23.00	168.17	65 - 56	15	T.Onoue	May 30-Jun. 4
	28	21.30	167.90	65 - 56	14	"	Jun. 5-Jun. 8
	Total	244.00			164		
Drilling of the Mekong river-bed	2	16.60	155.73	65 - 56	11	K.Shirayama	Feb.28-Mar. 2
	3	17.15	155.73	65 - 56	7	"	Mar. 5-Mar. 7
	4	13.00	151.92	65 - 56	3	T.Onoue	Mar. 2-Mar. 8
	5	13.00	150.96	65 - 56	4	"	Mar. 9-Mar.13
	6	13.60	153.58	65 - 56	6	K.Shirayama	Mar.11-Mar.16
	7	16.21	149.84	65 - 56	3	T.Onoue	Mar.14-Mar.18
	8	13.00	150.73	65 - 56	3	"	Mar.19-Mar.25
	9	12.80	151.61	65 - 56	4	K.Shirayama	Mar.21-Mar.23
	10	17.10	151.13	65 - 56	3	"	Mar.26-Mar.30
	11	12.50	152.16	65 - 56	1	T.Onoue	Mar.26-Mar.29
	12	12.00	150.95	65 - 56	1	"	Mar.30-Apr. 1
	13	7.20	149.82	65 - 56	1	K.Shirayama	Apr. 2-Apr. 3
	14	13.50	151.35	65 - 56	4	T.Onoue	Apr. 2-Apr.10
	15	16.30	156.11	65 - 56	5	K.Shirayama	Apr. 5-Apr. 9
	16	23.80	155.28	65 - 56	3	"	Apr.11-Apr.18
	17	15.00	152.36	65 - 56	6	T.Onoue	Apr.11-Apr.16
	18	16.00	151.20	65 - 56	1	"	Apr.16-Apr.23
	19	28.00	155.57	65 - 56	5	K.Shirayama	Apr.19-Apr.26
	20	13.00	150.93	65 - 56	4	T.Onoue	Apr.24-Apr.25
		Total	289.76			75	

Drilling machine: UD - 5

Inclination of hole: Vertical

GEOLOGICAL RECORDS OF TEST DRILLING HOLES

HOLE NO 1

LOCATION Left Bank (Laos)
ELEVATION OF SURFACE 168 M 33

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N-VALUE		
									20	40	60
	1	167.13	Surface soil		1.20	1.20		Light yellow, fine silt and clay			
	2		Silt and clay					Silty clay, light brown N=39/30 cm			
	3							N=35/30 cm			
	4							N=34/30 cm			
	5							Silty clay, N=30/30 cm			
	6							Light grey N=32/30 cm			
	7						Clayey silt N=25/30 cm				
	8						N=15/30 cm				
	9						Light brown silt N=17/30 cm				
	10	158.33			8.80	10.00		Containing gravel g 4cm			
	11		Sand and gravel					N=20/30 cm Light brown, N=21/30 cm			
	12							Earthy sand with pebble N=31/30 cm			
	13							Sand with N=24/30 cm pebble			
	14							N=23/30 cm			
	15						N=19/30 cm				
	16	151.93			6.40	16.40		N=16/30 cm			
	17		Weathered siltstone					N=16/30 cm Reddish brown fragments of N=33/30 cm siltstone			
	18							Reddish siltstone N=50/23 cm			
	19	148.93			3.00	19.40					
	20		Fresh siltstone					Fresh reddish brown			
	21										
	22										
	23										
	24	144.33			4.60	24.00					
	25										
	26										
	27										
	28										
	29										
	30										

HOLE NO 2

LOCATION Riverbed
ELEVATION OF SURFACE 155 M 73

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N-VALUE		
									20	40	60
	1							Light brownish grey, Fine sand N=27/30 cm			
	2		Fine sand and silty sand					N=27/30 cm Ditto N=47/30 cm			
	3							N=8/30 cm			
	4							Grey silty sand N=47/30 cm			
	5						Ditto N=67/30 cm				
	6						Ditto N=57/30 cm				
	7										
	8	148.03			7.70	7.70		N=17/30 cm			
	9		Sand with pebble					Grey sand N=19/30 cm with pebble N=20/30 cm			
	10							N=67/30 cm			
	11	144.03			4.00	11.70					
	12	143.53	Siltstone		4.50	12.20		Reddish brown siltstone			
	13		Shale					Reddish brown shale			
	14							cracked			
	15										
	16	139.13			4.40	15.60					
	17										
	18										
	19										
	20										

HOLE NO 3

LOCATION Riverbed
ELEVATION OF SURFACE 155 M 73

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N-VALUE		
									20	40	60
	1							Grey, fine sand N=27/30 cm			
	2		Sand					N=57/30 cm Medium grained sand			
	3							Silty sand N=87/30 cm			
	4							Fine sand			
	5										
	6	149.73			6.00	6.00		N=57/30 cm			
	7		Silt					Silt			
	8	147.73			7.00	8.00		N=167/30 cm			
	9		Fine sand					Fine sand N=167/28 cm			
	10							Ditto N=50/115 cm			
	11	144.33			3.40	11.40					
	12	143.68	Shale		0.65	12.05		Reddish brown shale			
	13		Siltstone					Massive siltstone			
	14	141.48			2.20	14.25					
	15		Shale					Silty shale cracked			
	16										
	17	138.56			2.90	17.15					
	18										
	19										
	20										

N=100

GEOLOGICAL RECORDS OF TEST DRILLING HOLES

HOLE NO 7

HOLE NO 8

HOLE NO 9

LOCATION: Riverbed
ELEVATION OF SURFACE: 149.84 M

LOCATION: Riverbed
ELEVATION OF SURFACE: 150.73 M

LOCATION: Riverbed
ELEVATION OF SURFACE: 151.61 M

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1	148.94	Gravel with rock fragments		0.90	0.90	*	Chert gravel and fragments of siltstone N=18/30cm			
	2		Gravel, sand and silt					Gravel, sand and silt N=22/30cm			
	3	146.84			2.10	3.00		NE 50/11cm Redd'sh brown siltstone			N=136
	4							Partly shaly			
	5							Vertical joint at 8.5m			
	6		Siltstone								
	7										
	8										
	9										
	10										
	11										
	12										
	13										
	14										
	15										
	16	133.63			7.21	16.21					
	17										
	18										
	19										
	20										

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1	149.23	Mud and sand		1.50	1.50		Mud and grey sand N=27/30cm			
	2		Sand and gravel					Mud, sand and gravel N=25/30cm			
	3	146.73			2.50	4.00		Sand and gravel N=37/30cm			
	4	145.73	Gravel and rock fragments		1.00	5.00		Gravel and siltstone fragments			
	5				0.80	5.80		Shaly siltstone			
	6										
	7										
	8		Siltstone					Siltstone vertical joint between 7m and 8m			
	9										
	10										
	11										
	12										
	13	137.73			7.20	13.00					
	14										
	15										
	16										
	17										
	18										
	19										
	20										

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1		Sand with pebble					N=4/30cm Fine sand with pebbles			
	2										
	3										
	4	147.31			4.30	4.30		N=19/30cm Gravel			
	5	146.96	Gravel		0.35	4.65		N=70/30cm Gravel			N=150
	6	146.56	Siltstone		0.30	4.95		Siltstone			
	7	146.06	Shale		0.60	5.55		Silty shale cracked			
	8										
	9		Siltstone					Siltstone			
	10										
	11										
	12										
	13	138.81			7.25	12.80					
	14										
	15										
	16										
	17										
	18										
	19										
	20										

GEOLOGICAL RECORDS OF TEST DRILLING HOLES

HOLE NO 10

HOLE NO 11

HOLE NO 13

LOCATION: Riverbed
ELEVATION OF SURFACE: 151.13

LOCATION: Riverbed
ELEVATION OF SURFACE: 152.16

LOCATION: Riverbed
ELEVATION OF SURFACE: 149.82

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
		150.13	Muddy sand		1.00	1.00		Muddy sand N = 3/30 cm			
	2	148.13	Sand and pebble		2.00	3.00		Fine sand with pebble N = 16/30 cm			
	4	146.03	Gravel and sand		2.10	5.10		Gravel and sand N = 26/30 cm			
	6	145.03	Weathered siltstone		1.00	6.10		Fragmental siltstone			
	7		Siltstone		2.00	8.10		Reddish brown siltstone			
	8							Ditto, brittle			
	9							Siltstone			
	10	141.03			1.00	10.10		Siltstone			
	11	140.03	Sandstone		1.00	11.10		Fine grained sandstone			
	12		Siltstone		1.70	12.80		Shaly siltstone			
	13							Ditto, cracked			
	14										
	15				0.30	13.10					
	16							Massive siltstone			
	17	134.03			4.00	17.10					

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1	150.26	Sand and gravel		1.90	1.90		Brown gravel Contents 40-70%			
	2		Clay		1.60	3.50		Hard clay			
	3	148.66									
	4		Siltstone		0.60	4.10		Reddish brown siltstone N = 50/77 cm			N = 88
	5							Reddish brown, clayey siltstone			
	6										
	7										
	8										
	12	139.66			8.40	12.50					

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1	148.57	Weathered shale		1.25	1.25		Weathered shale N = 50/25 cm			
	2	148.02	Shale		0.65	1.80		Shale, cracked			
	3		Siltstone		5.40	7.20		Siltstone			
	4										
	5										
	7	142.62									

HOLE NO 14

LOCATION: Riverbed
ELEVATION OF SURFACE: 151.35

HOLE NO 12

LOCATION: Riverbed
ELEVATION OF SURFACE: 150.95

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1	150.55	Sand		0.40	0.40		Grey med sand			
	2	149.15	Sand and gravel		1.40	1.60		Sand, pebble and gravel with siltstone fragment N = 30/30 cm			
	3		Siltstone and shale (alternated)					Fine-grained siltstone and shale (alternated)			
	4										
	5										
	6										
	7										
	8										
	9										
	10										
	11										
	12	138.95								10.20	12.00

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE						
									20	40	60				
	1		Sand and gravel					Grey sand and gravel N = 24/30 cm							
	2														
	3							Containing siltstone fragments N = 21/30 cm							
	4	147.35			4.00	4.00									
	5	146.85	Weathered siltstone		0.50	4.50		Weathered siltstone N = 23/30 cm							
	6		Siltstone					Shaly							
	7														
	8														
	9														
	10														
	11														
	12														
	13														
	14	137.85								9.00	13.50				

GEOLOGICAL RECORDS OF TEST DRILLING HOLES

HOLE NO 15

LOCATION Riverbed
ELEVATION OF SURFACE 156 M 11

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1		Fine sand		N=27/32cm			Grey, fine sand			
	2										
	3				N=57/30cm						
	4										
	5				N=67/30cm						
	6				N=127/30cm						
	8	147.61			8.50	8.50					
	9		Sand and pebble					Sand and pebble			
	10										
	11	144.81			2.80	11.30		N=58/30cm			
	12		Siltstone					Siltstone			
	13										
	14										
	15										
	16	139.81							5.00	16.30	
	17										
	18										
	19										
	20										

HOLE NO 16

LOCATION Riverbed
ELEVATION OF SURFACE 155 M 28

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1		Sand					Grey, fine sand			
	2										
	3	151.98							3.30	3.30	N=47/30cm
	4		Sand with pebble					Sand with subangular pebble			
	5										
	6	149.28			2.70	6.00		N=77/30cm			
	7		Sand					Fine sand			
	8										
	9	146.28			3.00	9.00		N=187/30cm			
	10	145.48	Weathered siltstone		0.80	9.80		Weathered siltstone			
	11		Siltstone					Clayey siltstone			
	12										
	13	143.48			2.00	11.80		Shale			
	14	143.18	Shale		0.30	12.10		Clayey siltstone			
	15	142.78	Siltstone		0.40	12.50		Shale			
	16	142.08	Shale		0.70	13.20		Siltstone			
	17	141.48	Siltstone		0.60	13.80		Shale			
	18		Shale					Sound siltstone			
	19										
	20	139.48			2.00	15.80		Shale			
	21		Shale					Shale			
	22										
	23										
	24	137.38							2.10	17.90	
	25										
	26										
	27										
	28										
	29										
	30				5.90	23.80					

HOLE NO 17

LOCATION Riverbed
ELEVATION OF SURFACE 152 M 36

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1		Sand					Grey sand			
	2										
	3								N=157/30cm		
	4								N=157/30cm		
	5								N=187/30cm		
	6								N=227/30cm		
	6	146.16	Weathered siltstone					Weathered	6.20	6.20	
	7	145.36							0.80	7.00	N=157/30cm
	8		Siltstone					Siltstone partly shale			
	9										
	10		Siltstone					Siltstone			
	11										
	12										
	13										
	14										
	15	137.36							8.00	15.00	
	16										
	17										
	18										
	19										
	20										

GEOLOGICAL RECORDS OF TEST DRILLING HOLES

HOLE NO 21

HOLE NO 22

LOCATION : Right Bank (Thoi)
ELEVATION OF SURFACE, 166 M 59

LOCATION That Luong
ELEVATION OF SURFACE, Unobserved

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1		Loam					Light brown loam N=57/30cm			
	2							N=77/30cm			
	3	162.59			4.00	4.00		N=77/30cm			
	5		Silt					Light brown clayey N=87/30cm			
	6							570 TWS 630			
	7							N=117/30cm			
	8							740 TWS 780			
	9							N=87/30cm			
	10		Sand					Gray brown 1030 TWS 1085			
	11	155.59			7.00	11.00		1125			
	12							Fine-grained N=177/30cm			
	14							Medium-grained N=237/30cm			
	15							N=237/30cm			
	18		Sand and pebbles					Coarse-grained N=367/30cm			
	19	147.59			8.00	19.00		N=377/30cm			
	20		Siltstone					Sand and pebbles N=297/30cm			
	21	145.04			1.50	21.55		N=457/15cm			
	22	144.59		0.45	22.00		Siltstone			N=90	

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	1		Soil		0.40	0.40		Surface soil			
	2		Clay		1.20	1.60		Dark grey 070 TWS N=67/30cm 135			
	3		Sand		1.30	2.90		Grey, brown, coarse N=77/30cm			
	4		Clay					Grey, silty 300 TWS N=67/30cm 340			
	5							Grey, sandy N=97/30cm			
	6							Gray brown, sandy N=147/30cm			
	7							Light yellow N=157/30cm			
	8							Yellow brown N=127/30cm			
	9		Sand					Reddish brown N=97/30cm			
	10				6.80	9.70		N=107/30cm			
	11							Clayey N=127/30cm			
	12				3.10	12.80		Grey brown N=157/30cm			
	13							N=177/30cm			
	14		Clay					Sandy, containing N=237/30cm			
	15							N=267/30cm pebbles			
	16		Sand		3.10	15.90		N=177/30cm Clayey			
	17		Clay		0.80	16.70		Sandy N=137/30cm			
	18		Sand					N=177/30cm			
	19							Containing N=197/30cm gravel			
	20							N=197/30cm			
	21							Light grey, N=187/30cm clayey			
	22							N=287/30cm			
	23		Clay					Sometimes N=317/30cm with gravels			
	24							N=347/30cm			
	25							N=257/30cm			
	26				8.00	25.80		Yellow brown N=327/30cm			
	27							Sandy, sometimes N=397/30cm with gravel			
	28		Sand with gravel		2.30	28.10		N=317/30cm			
	29							Fine grained N=347/30cm			

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
	31		Sand with gravel					N=357/30cm Containing gravel φ10-50mm			
	32							N=407/30cm			
	33							N=507/25cm			
	34							N=507/22cm			
	35							N=507/13cm			N=115
	36							Grey, N=507/21cm			N=71
	37							Containing N=507/27cm gravel φ10-50mm			
	38							N=507/26cm			
	39							N=507/21cm			N=71
	40							N=507/25cm			
	41							With N=507/23cm gravel			N=71
	42							N=507/21cm			N=71
	43							N=507/16cm			N=83
	44					16.20	44.30		N=507/17cm Finish		

GEOLOGICAL RECORDS OF TEST DRILLING HOLES

HOLE NO 26

LOCATION: Thai side
ELEVATION OF SURFACE: 165.41 M

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
28	1	158.91	Sandy silt		6.50	6.50	A	Yellow-brown N:19/30cm			
	2							N:16/30cm			
	3							Sandy N:15/30cm			
	4							silt N:15/30cm			
	5							N:12/30cm			
	6										
29	7	154.41	Silty sand		4.50	11.00	A	N:11/30cm Silty sand 80 TWS 875 90 TWS 975 N:14/30cm			
	8										
	9										
	10										
	11							Sand N:18/30cm			
	12							N:39/30cm			
	13							Sand and gravel			
	14							N:39/30cm g = 5-10cm			
	15							N:39/30cm			
	16										
31	17	141.41	Sand and gravel		13.00	24.00	A	N:18/30cm			
	18							N:39/30cm			
	19							N:40/30cm			
	20							N:41/30cm			
	21							Yellow-brown N:40/30cm			
	22							N:41/30cm			
	23							Sand and gravel N:42/30cm			
	24							N:41/30cm			
	25							Pebble N:40/30cm			
	26							Pebble and gravel N:43/30cm			
	27							Gravel			
	28							N:42/30cm			
	29							g = 10cm			
	30										

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
31	31	130.41	Gravel		11.00	35.00	A	Gravel N:41/30cm g = 5-15cm			
	32										
	33							Ditto N:43/30cm			
	34							g = 1-10cm			
	35										

HOLE NO 27

LOCATION: Lotion side
ELEVATION OF SURFACE: 168.17 M

DATE	DEPTH	ELEV. TOP OF STRATUM	CLASSIFICATION OF ROCKS	COLUMNAR SECTION	THICKNESS OF STRATUM	ACCUMULATIVE THICKNESS OF STRATA	CORE RECOVERY	DESCRIPTION	N - VALUE		
									20	40	60
31	1	145.17	Soil		6.20	23.00	A	Soil N:17/30cm			
	2							Dark grey 10-17 Clayey loam N:1/20cm			
	3							Containing humus N:12/34cm			
	4							Containing light grey N:17/30cm clay			
	5							N:13/30cm			
	6							Containing mica N:12/30cm			
	7							N:10/30cm			
	8							N:10/30cm			
	9							N:22/30cm			
	10							N:23/30cm			
	11							Accompanying brown clay N:31/30cm			
	12							N:25/30cm			
	13							N:38/30cm			
	14							Gravel g 10-30m/m N:32/30cm Sometimes g = 50m/m N:30/30cm			
	15							N:50/22cm			
	16							Weathered siltstone N:50/22cm Reddish brown decomposed			
	17										
	18										
	19										
	20										
	21										
	22										
	23										
24											
25											
26											
27											
28											
29											
30											

2.2 Soil Test

Summary of soil test

Location: Nong Khai

Items	Unit	Characteristics											
		1	2	3	4	5	6	7	8	9	10	11	12
Sample No.													
Bore Hole No.		21	21	21	22	22	24	24	25	25	26	26	27
Sampling Depth	m	5.70-6.30	7.40-7.80	10.30-11.25	0.70-1.35	3.00-3.40	6.20-6.93	9.60-10.35	6.50-7.25	7.30-8.20	8.00-8.75	9.00-7.75	1.00-1.70
I. Observation		Reddish brown	Reddish brown	Reddish brown	Grey brown	Reddish brown	Reddish brown	Reddish brown	Reddish brown	Reddish brown	Reddish brown	Reddish brown	Yellow brown
II. Properties													
(1) Natural water content, w	%	20.11	21.23	24.04	26.25	36.70	22.04	28.42	25.41	25.75	25.72	25.40	16.45
(2) Specific gravity of soil, G		2.68	2.65	2.70	2.68	2.70	2.75	2.67	2.73	2.69	2.70	2.65	2.76
(3) Wet density, r_t	g/cm ³	1.875	1.940	1.893	2.044	1.789	2.009	1.792	1.899	1.891	1.948	1.992	2.067
(4) Dry density, r_d	g/cm ³	1.561	1.600	1.526	1.619	1.308	1.646	1.395	1.514	1.503	1.549	1.588	1.775
(5) Void ratio, e		0.717	0.656	0.769	0.655	1.064	0.671	0.914	0.803	0.790	0.743	0.669	0.555
(6) Degree of saturation, S	%	75.17	85.76	84.41	100	93.13	90.33	83.02	86.39	87.68	93.46	100	81.81
III. Grain Size													
(1) Constitution													
i) Gravel part	%	-	-	-	1.0	-	-	-	-	-	-	-	-
ii) Sand part	%	3.5	3.0	5.0	16.5	13.5	2.0	1.5	19.5	51.0	33.0	31.0	25.5
iii) Silt part	%	75.0	74.0	78.5	34.5	45.0	62.0	68.5	63.5	38.0	54.0	54.0	51.0
iv) Clay part	%	21.5	23.0	16.5	48.0	41.5	36.0	30.0	17.0	11.0	13.0	15.0	23.5
(2) Max. diameter	mm	0.105	0.105	0.105	4.8	2.0	0.105	0.105	0.42	0.84	0.42	0.42	2.0
(3) 60 % diameter, D_{60}	mm	0.035	0.033	0.0403	0.016	0.013	0.017	0.018	0.06	0.13	0.07	0.063	0.06
(4) 10 % diameter, D_{10}	mm	-	-	0.0018	-	0.0017	-	-	0.0018	0.004	0.0028	0.002	-
(5) Uniformity coefficient		-	-	22.4	-	7.65	-	-	33.3	32.5	25.0	31.5	-
(6) Grain size classification		Silty clay loam	Silty clay loam	Silty loam	Clay	Clay	Silty clay	Silty clay	Silty loam	Silty loam	Silty loam	Silty loam	Silty clay loam
(7) Unified classification		CL	CL	CL	CL or CH	CL or CH	CL	CL or CH	ML or CL	SC	ML or OL	CL	CL
IV. Consistency													
(1) Liquid limit, L.L.	%	33.25	39.80	35.20	49.80	52.00	37.10	53.10	28.20	24.10	26.40	26.85	36.50
(2) Plastic Limit, P.L.	%	20.45	21.70	22.05	17.37	20.47	20.64	24.33	22.15	18.66	22.49	18.43	11.68
(3) Plasticity index, P.I.		12.80	18.10	13.15	32.43	31.53	16.46	28.77	6.05	5.46	3.91	8.42	24.82
(4) Flow index, F.I.		6.30	8.48	8.25	10.10	10.10	10.00	12.80	5.10	5.95	5.10	5.05	15.70
V. Shearing Strength													
(1) Unconfined compression													
i) Compression strength	kg/cm ²	1.195	0.883	1.051	0.505	0.471	0.861	3.920	0.426	0.290	0.498	0.556	0.664
ii) Sensitivity ratio		2.36	1.56	4.08	1.18	1.64	1.38	5.16	N.G. $\frac{1}{1}$	4.08	N.G. $\frac{1}{1}$	N.G. $\frac{1}{1}$	1.06
(2) Direct compression													
i) Cohesion, c	kg/cm ²	-	-	-	-	-	0.60	-	0.30	-	0.70	0.28	0.60
ii) Internal friction angle, β		-	-	-	-	-	40°02'	-	37°36'	-	15°39'	22°47'	30°58'
(3) Triaxial compression													
i) Cohesion, c	kg/cm ²	0.50	0.80	0.45	0.925	0.20	0.82	1.15	0.21	0.10	0.35	0.24	0.50
ii) Internal friction angle, β		12°25'	19°18'	10°46'	5°43'	8°32'	11°52'	13°30'	15°39'	16°42'	6°17'	8°32'	15°07'
VI. Consolidation													
(1) Initial void ratio, e_0		0.610	0.672	0.670	0.642	1.360	0.689	0.876	0.769	0.657	0.616	0.680	0.682
(2) Preconsolidation load, p_0	kg/cm ²	3.50	4.50	3.20	1.22	1.17	3.00	4.90	3.00	2.63	3.90	2.48	0.56
(3) Compression index, C_c		0.198	0.186	0.147	0.161	0.361	0.235	0.308	0.251	0.201	0.137	0.146	0.158
(4) Coef. of consolidation, C_v	cm ² /sec	2.8x10 ⁻²	1.66x10 ⁻²	2.1x10 ⁻³	8.2x10 ⁻³	8.1x10 ⁻³	1.22x10 ⁻²	2.0x10 ⁻²	2.22x10 ⁻²	3.1x10 ⁻²	1.29x10 ⁻²	1.15x10 ⁻²	1.7x10 ⁻²
(5) Coef. of volume compressibility, M_v	cm ² /g	1.3x10 ⁻⁵	7.0x10 ⁻⁶	8.1x10 ⁻⁶	1.95x10 ⁻⁵	4.7x10 ⁻⁵	1.21x10 ⁻⁵	8.6x10 ⁻⁶	1.38x10 ⁻⁵	1.28x10 ⁻⁵	6.3x10 ⁻⁶	1.03x10 ⁻⁵	5.4x10 ⁻⁵
(6) Coef. of permeability, K	cm ² /sec	3.6x10 ⁻⁷	1.18x10 ⁻⁷	1.74x10 ⁻⁸	1.6x10 ⁻⁷	3.8x10 ⁻⁷	1.5x10 ⁻⁷	1.75x10 ⁻⁷	3.04x10 ⁻⁷	4.0x10 ⁻⁷	8.1x10 ⁻⁸	1.2x10 ⁻⁷	9.2x10 ⁻⁷

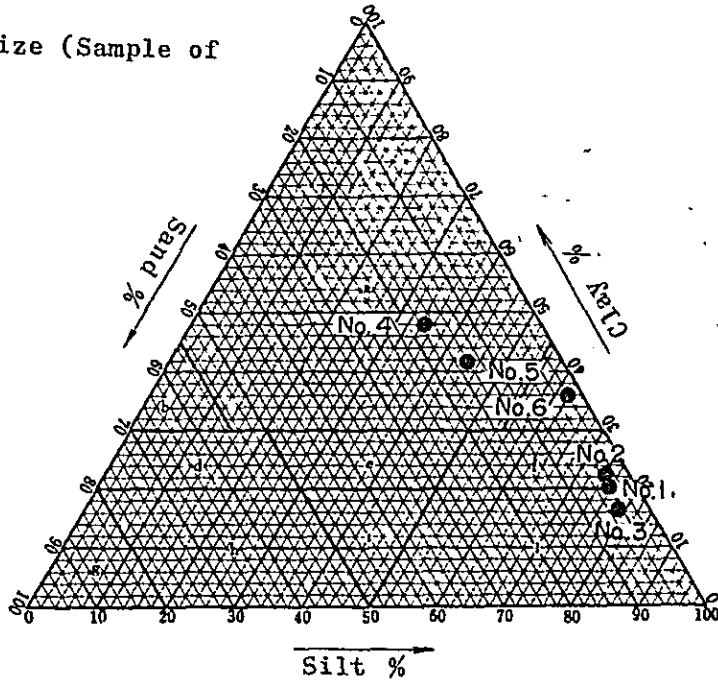
Remarks: 1 The remoulding was impossible for testing.

MECHANICAL ANALYSIS

Location Nong Khai

Soil classification of grain size (Sample of passed 2000 μ sieve)

- a CLAY
- b SANDY CLAY
- c SILTY CLAY
- d SANDY CLAY LOAM
- e CLAYEY LOAM
- f SILTY CLAY LOAM
- g SAND
- h SANDY LOAM
- i LOAM
- j SILTY LOAM

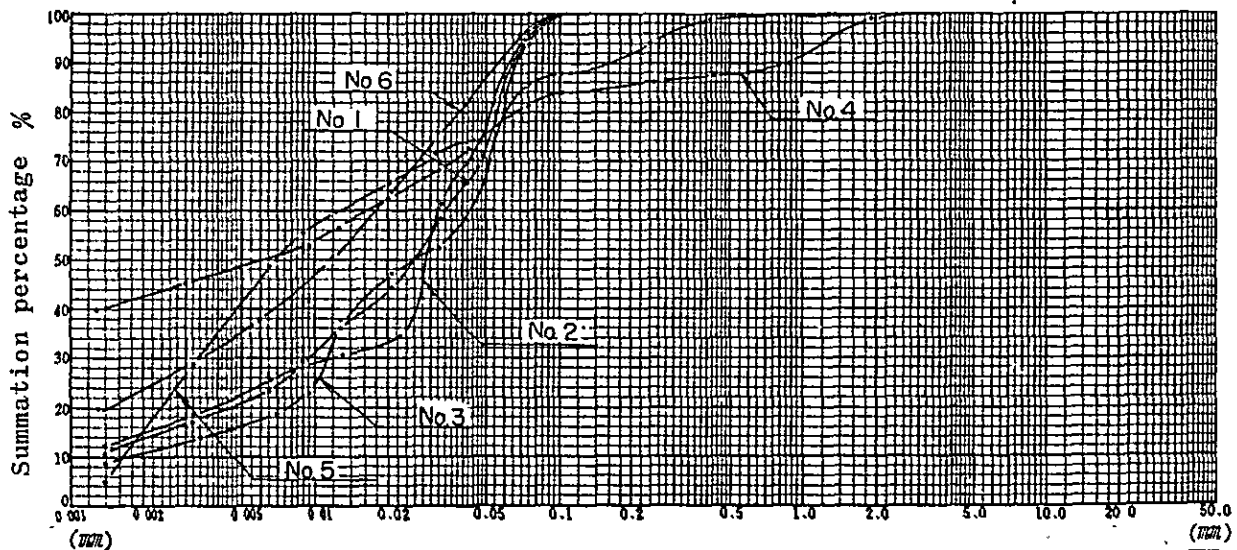


Sample No	Gravel %	Sand %	Silt %	Clay %	Max. size μ	D60 μ	D10 μ	Uniformity Coeff	2000 μ sieve	420 μ sieve	74 μ sieve	Sign of Placed part on triangular diagram	Classification	Remarks
									Passed	Percentage	Percentage			
No. 1	—	3.5	75.0	21.5	0.105	0.035	—	—	100	100	96.0	f	SILTY CLAY LOAM	
No. 2	—	3.0	74.0	23.0	0.105	0.033	—	—	100	100	97.0	f	SILTY CLAY LOAM	
No. 3	—	5.0	78.5	16.5	0.105	0.0403	0.0018	224	100	100	95.5	j	SILTY LOAM	
No. 4	1.0	16.5	34.5	48.0	48	0.016	—	—	99.0	87.0	82.0	a	CLAY	
No. 5	—	13.5	45.0	41.5	2.0	0.013	0.0017	7.65	100	98.5	86.5	a	CLAY	
No. 6	—	2.0	62.0	36.0	0.105	0.0107	—	—	100	100	98.5	c	SILTY CLAY	

Grain size accumulation curve

(No.) Sieve μ

No. 200: 75, 105
No. 40: 250, 420
No. 10: 840, 2000, 4760



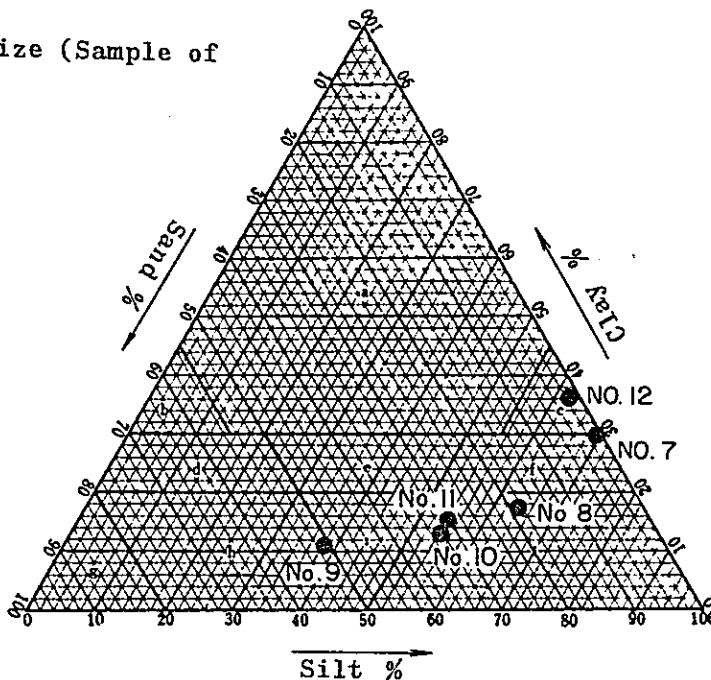
Colloid	Clay	Silt	Sand	Gravel
0.001	0.005	0.074	2.0	4.8 9.52 19.1 25.4 38.1 50.8

MECHANICAL ANALYSIS

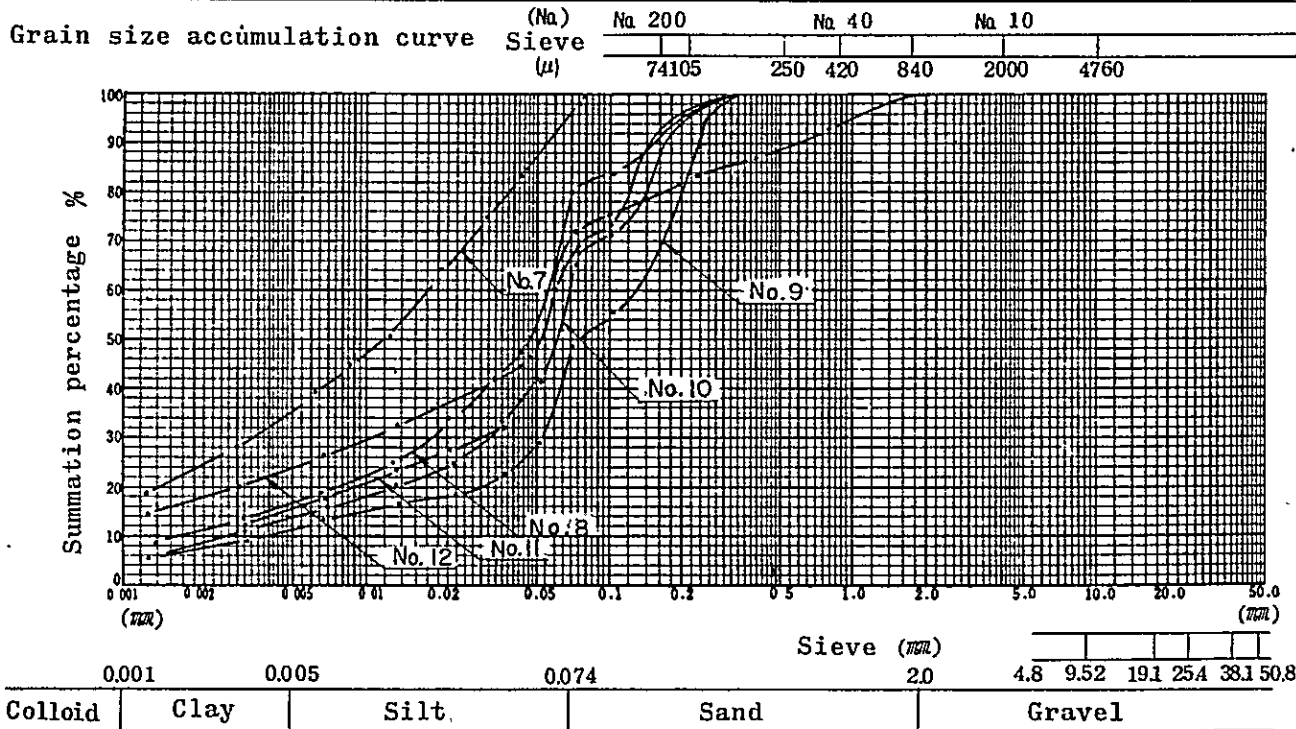
Location Nong Khai

Soil classification of grain size (Sample of passed 2000 μ sieve)

- a CLAY
- b SANDY CLAY
- c SILTY CLAY
- d SANDY CLAY LOAM
- e CLAYEY LOAM
- f SILTY CLAY LOAM
- g SAND
- h SANDY LOAM
- i LOAM
- j SILTY LOAM



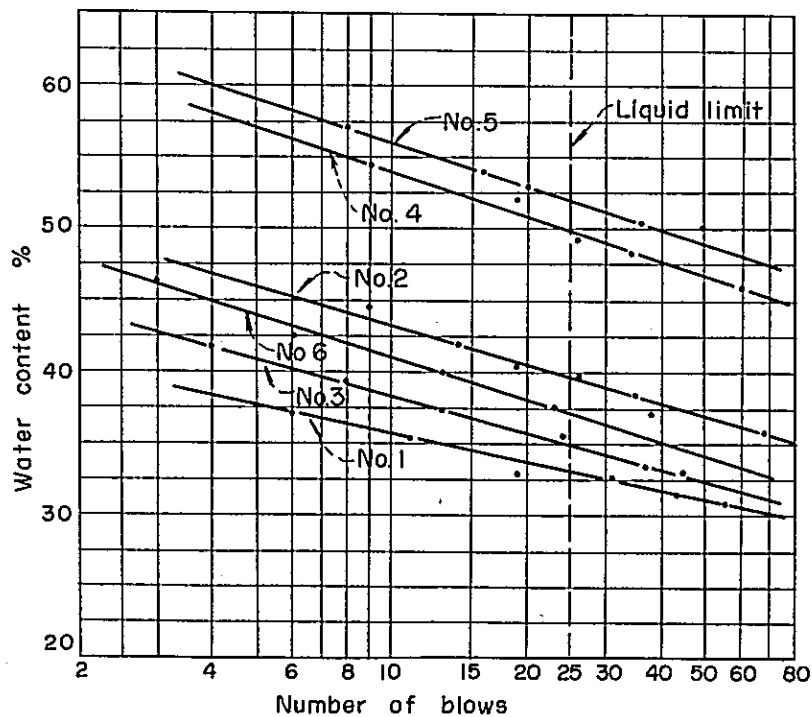
Sample No	Gravel %	Sand %	Silt %	Clay %	Max. size μ	D60 μ	D10 μ	Uniformity Coeff	2000 μ sieve	420 μ sieve	75 μ sieve	Sign of Flaked part in triangular diagram	Classification	Remarks
									Passed	Percentage	Percentage			
No. 7		1.5	68.5	30.0	0.105	0.018	—	—	100	100	98.5	c	SILTY CLAY	
No. 8		19.5	63.5	17.0	0.420	0.006	0.0018	33.3	100	100	80.5	j	SILTY LOAM	
No. 9		51.0	38.0	11.0	0.84	0.13	0.004	32.5	100	99.5	49.0	h	SANDY LOAM	
No. 10		33.0	54.0	13.0	0.42	0.007	0.0028	25.0	100	100	67.0	j	SILTY LOAM	
No. 11		31.0	54.0	15.0	0.42	0.063	0.002	31.5	100	100	69.0	j	SILTY LOAM	
No. 12		25.5	51.0	23.5	2.0	0.06	—	—	100	90.5	74.5	c	SILTY CLAY LOAM	



Liquid Limit and Plastic Limit Tests - 1

Result of Test

Sample No.	Liquid limit	Plastic limit			Plasticity index	Flow index
		(1)	(2)	Mean		
1	33.25	20.76	20.14	20.45	12.80	6.30
2	39.80	21.67	21.72	21.70	18.10	8.48
3	35.20	22.17	21.92	22.05	13.15	8.25
4	49.80	17.49	17.24	17.37	32.43	10.10
5	52.00	20.62	20.32	20.47	31.53	10.10
6	37.10	20.66	20.62	20.64	16.46	10.00

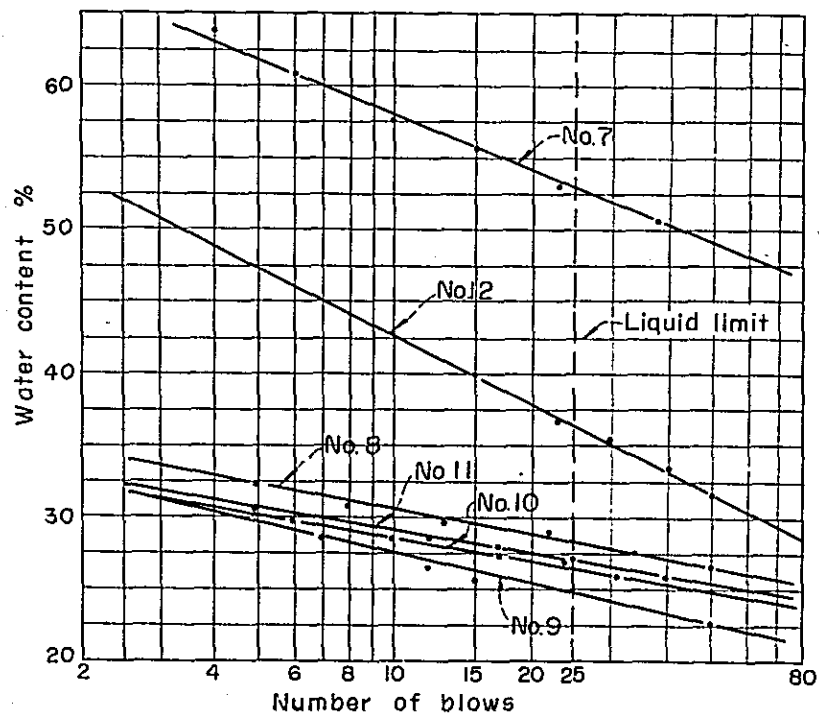


Remarks : The soil passing 0.4 mm sieve was used for the test to decide the liquid and plastic limits.

Liquid Limit and Plastic Limit Test - 2

Result of Test

Sample No.	Liquid limit	Plastic limit			Plasticity index	Flow index
		(1)	(2)	Mean		
7	53.10	24.75	23.91	24.33	28.77	12.80
8	28.20	22.27	22.02	22.15	6.05	5.10
9	24.10	18.46	18.85	18.66	5.46	5.95
10	26.40	22.54	22.43	22.49	3.91	5.10
11	26.85	18.50	18.36	18.43	8.42	5.05
12	36.50	11.85	11.50	11.68	24.82	15.70



Remarks: The soil passing 0.4 mm sieve was used for the test to decide the liquid and plastic limits.

Direct Shear Test

Sample No.	Dry density (g/cm ³)	Normal stress (kg/cm ²)	Maximum shear stress (kg/cm ²)	Cohesion c (kg/cm ²)	Internal friction angle ϕ
6	1.650	0.6	1.118	0.60	40°02'
	1.618	1.1	1.511		
	1.649	1.6	1.739		
	1.630	2.1	2.400		
8	1.491	0.6	0.758	0.30	37°36'
	1.496	1.1	1.190		
	1.498	1.6	1.373		
	1.501	2.1	1.914		
10	1.480	0.6	0.874	0.70	15°39'
	1.507	1.1	0.963		
	1.499	1.6	1.137		
	1.501	2.1	1.309		
11	1.542	0.6	0.531	0.28	22°47'
	1.520	1.1	0.766		
	1.536	1.6	0.937		
	1.510	2.1	1.163		
12	1.754	0.6	0.973	0.60	30°58'
	1.770	1.1	1.237		
	1.781	1.6	1.560		
	1.783	2.1	1.654		

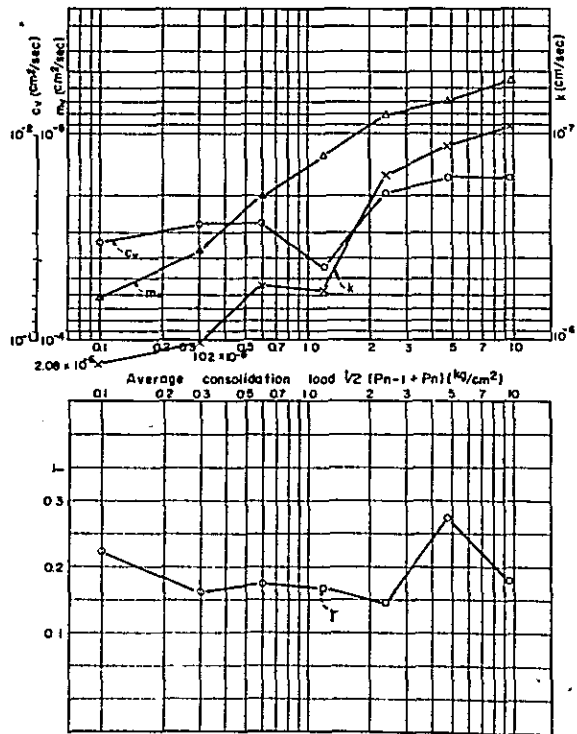
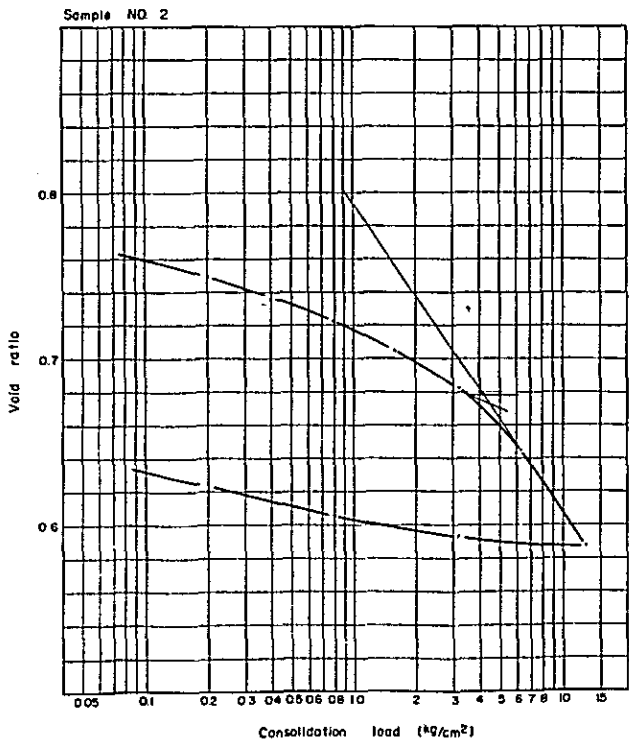
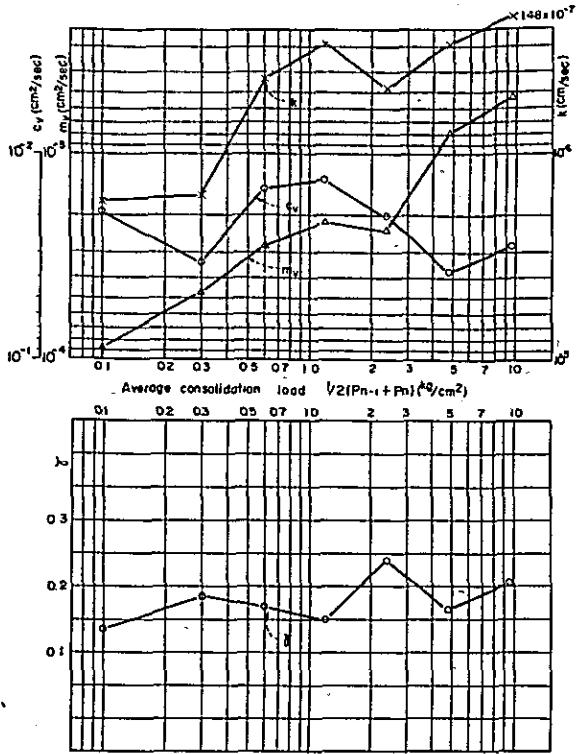
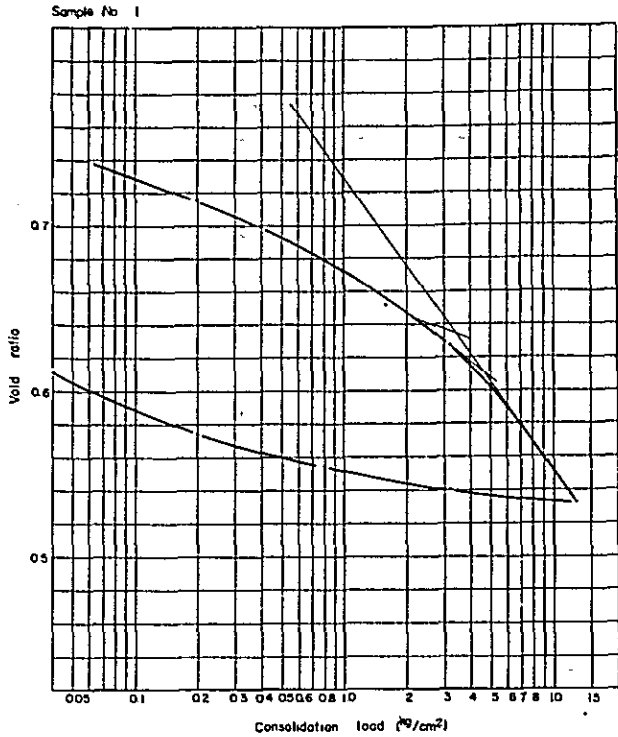
Triaxial Compression Test

Sample No.	Dry density γ_d (g/cm ³)	Lateral pressure σ_3 (kg/cm ²)	Max. compression stress σ_1 (kg/cm ²)	Cohesion c (kg/cm ²)	Internal friction angle ϕ
1	1.600	1	1.803	0.50	12°25'
	1.561	2	2.431		
	1.562	3	2.776		
2	1.600	1	3.238	0.80	19°18'
	1.615	2	4.250		
3	1.543	1	1.569	0.45	10°46'
	1.526	2	2.038		
	1.509	3	2.747		
4	1.619	1	0.725	0.925	5°43'
	1.605	2	0.921		
	1.619	3	1.198		
5	1.273	1	0.828	0.20	8°32'
	1.308	2	1.161		
	1.329	3	1.804		
6	1.622	1	2.547	0.82	11°52'
	1.619	2	3.100		
	1.646	3	3.651		
7	1.406	1	3.602	1.15	13°30'
	1.400	2	4.182		
	1.395	3	4.691		
8	1.461	1	1.160	0.21	15°39'
	1.514	2	1.985		
	1.551	3	2.583		
9	1.503	1	1.031	0.10	16°42'
	1.481	2	1.896		
10	1.611	1	1.046	0.35	6°17'
	1.549	2	1.358		
	1.549	3	2.100		
11	1.595	1	0.961	0.24	8°32'
	1.588	2	1.231		
	1.580	3	1.709		
12	1.668	1	2.10	0.50	15°07'
	1.742	2	2.78		
	1.775	3	3.31		

Unconfined Compression Test

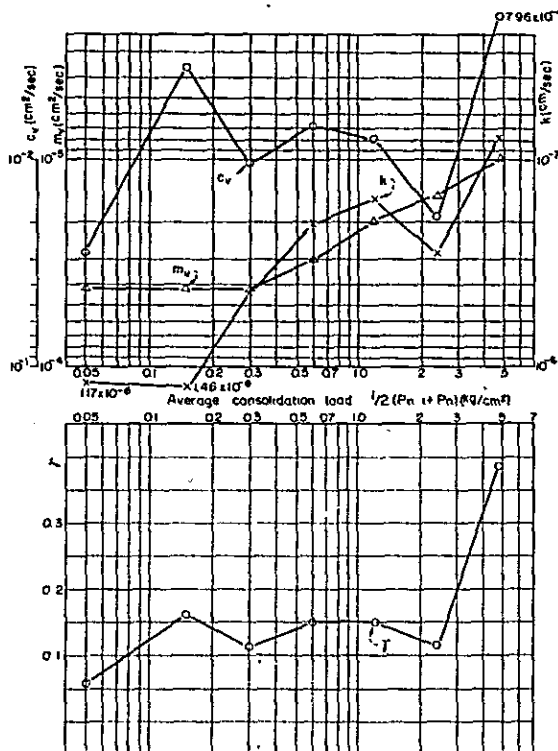
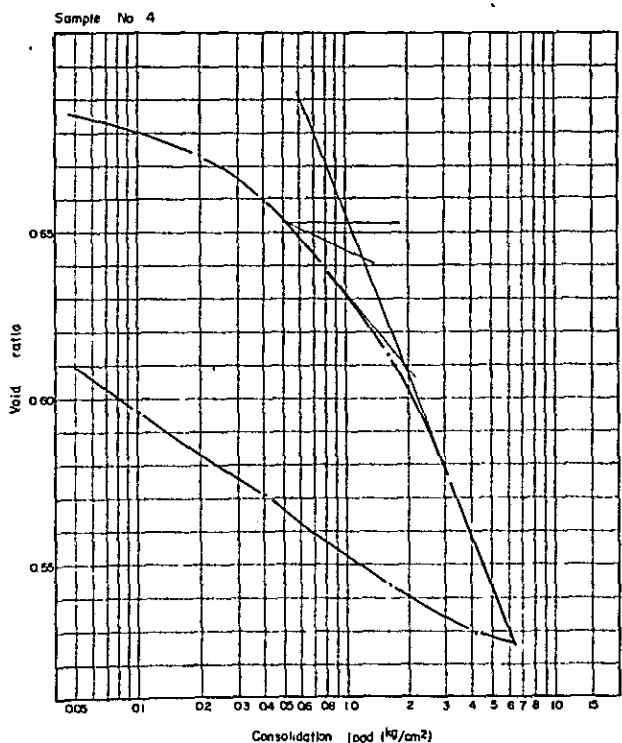
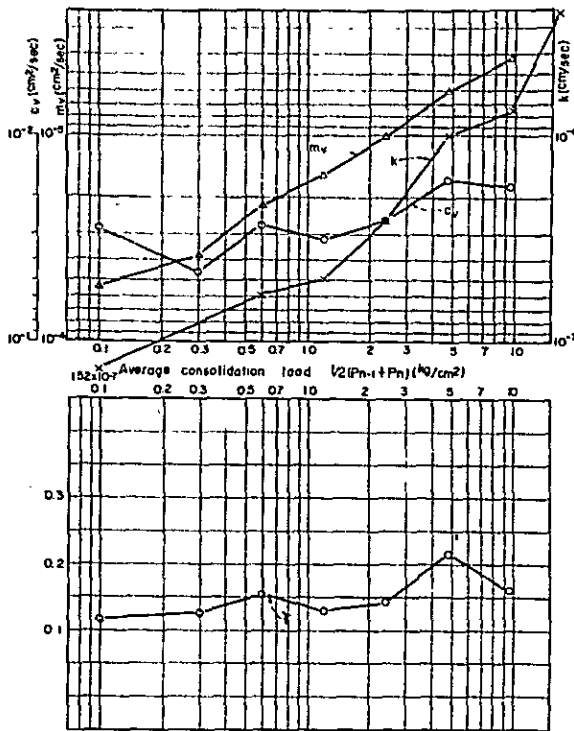
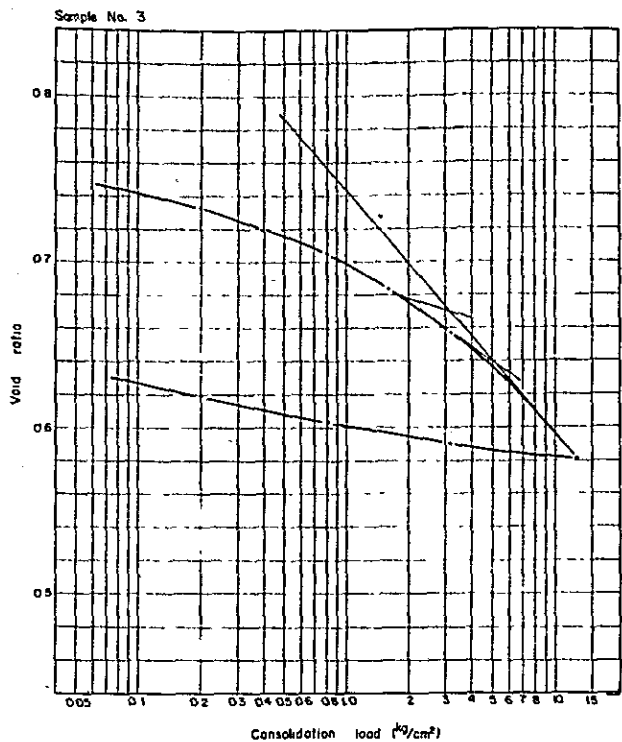
Sample No.	Mean water content (%)	Unit weight (g/cm ³)	Unconfined compression strength (kg/cm ²)		Sensitivity ratio
			undisturbed sample	disturbed sample	
1	19.06	1.878	1.195	0.508	2.36
2	27.41	1.878	0.883	0.564	1.56
3	22.89	1.858	1.051	0.258	4.08
4	26.77	2.030	0.505	0.427	1.18
5	38.27	1.803	0.471	0.287	1.64
6	24.86	1.965	0.861	0.626	1.38
7	28.70	1.797	3.920	0.760	5.16
8	27.18	1.848	0.426	-	-
9	21.17	1.838	0.290	0.071	4.08
10	25.76	1.934	0.498	-	-
11	25.85	1.950	0.556	-	-
12	16.51	1.992	0.664	0.627	1.06

CONSOLIDATION TEST - I



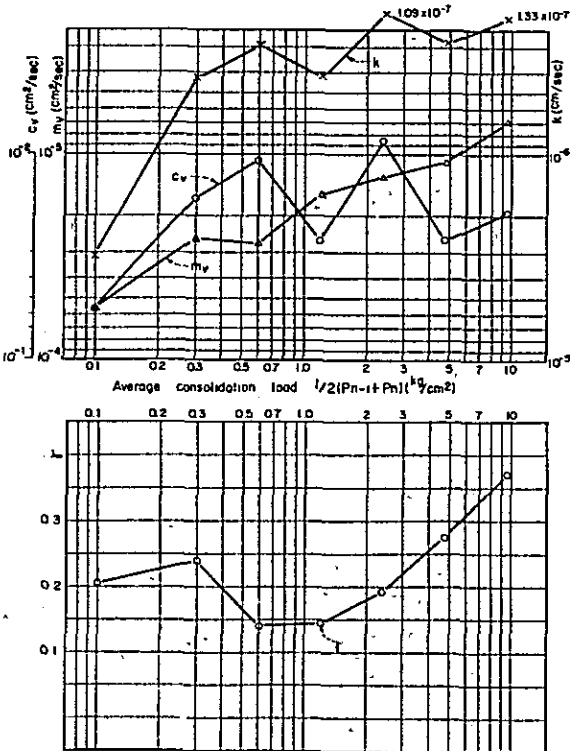
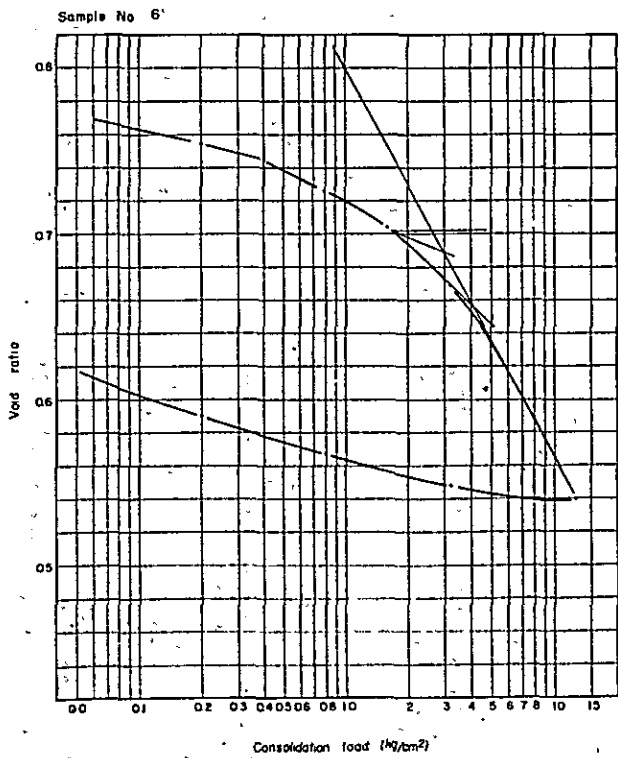
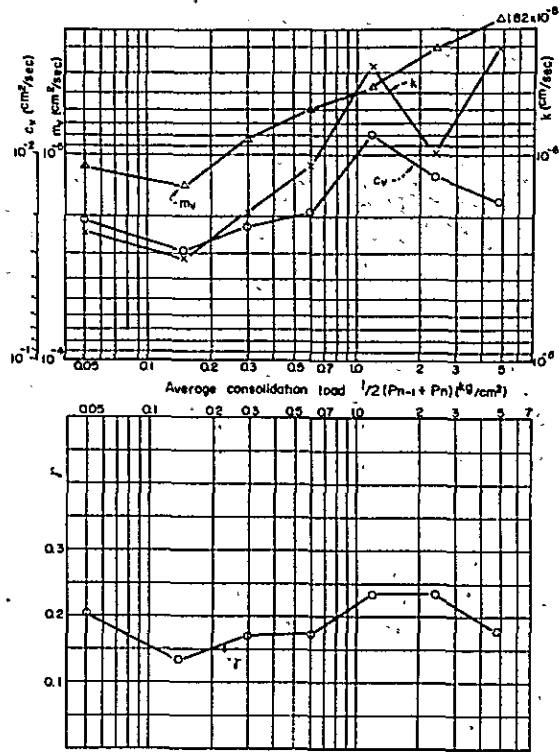
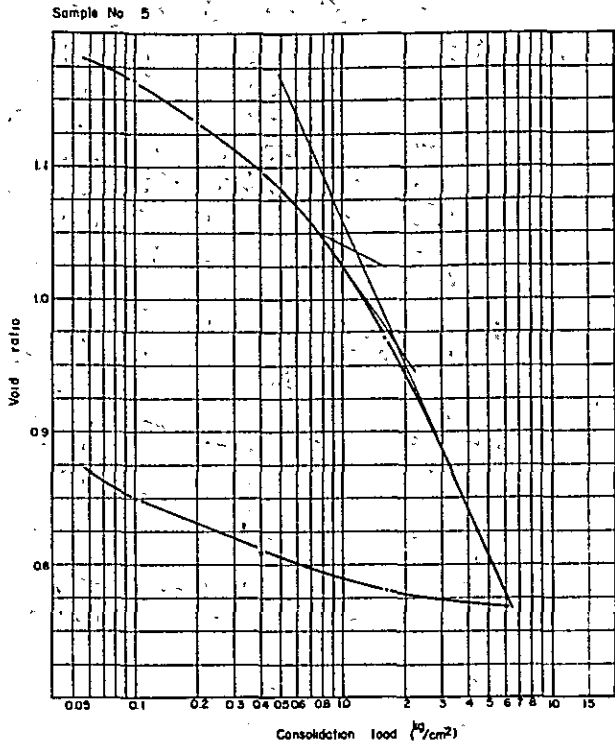
Remarks: c_v Coefficient of consolidation
 m_v Coefficient of volume compressibility
 k Coefficient of permeability
 γ Primary compression ratio

CONSOLIDATION TEST — 2



Remarks c_v - Coefficient of consolidation
 m_v - Coefficient of volume compressibility
 k - Coefficient of permeability
 Y - Primary compression ratio

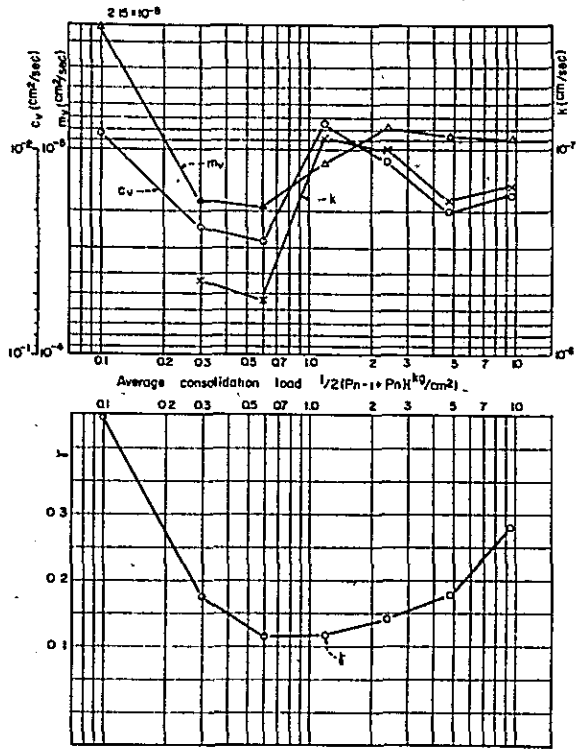
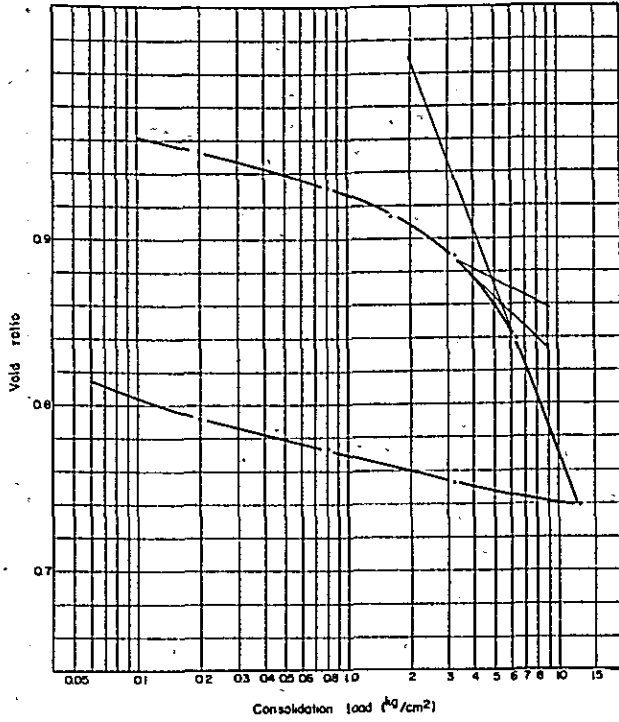
CONSOLIDATION TEST — 3



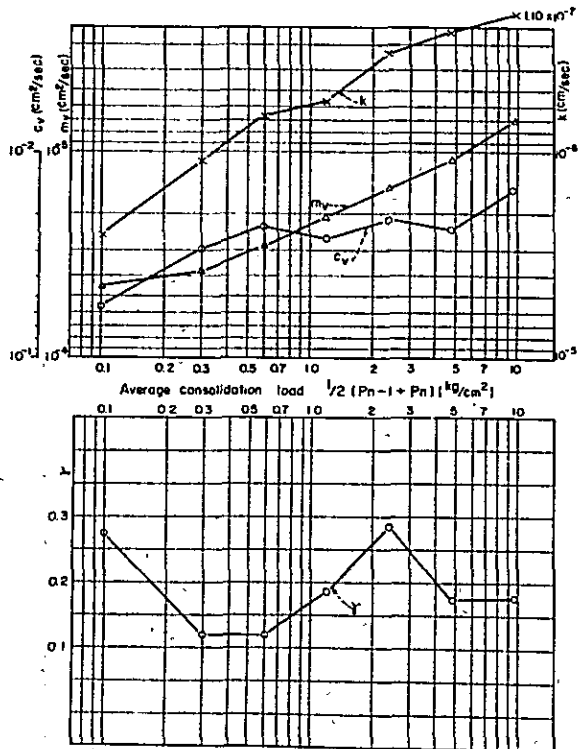
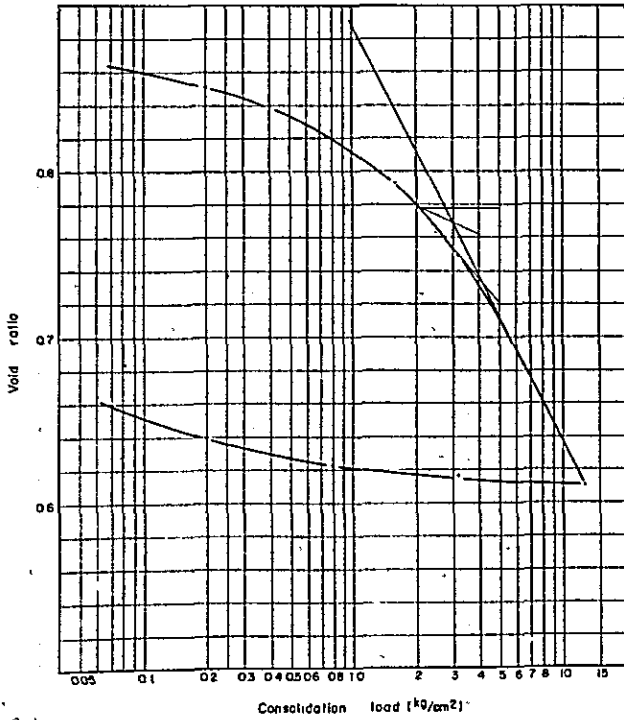
Remarks c_v - Coefficient of consolidation
 m_v - Coefficient of volume compressibility
 k - Coefficient of permeability
 r - Primary compression ratio

CONSOLIDATION TEST - 4

Sample No 7

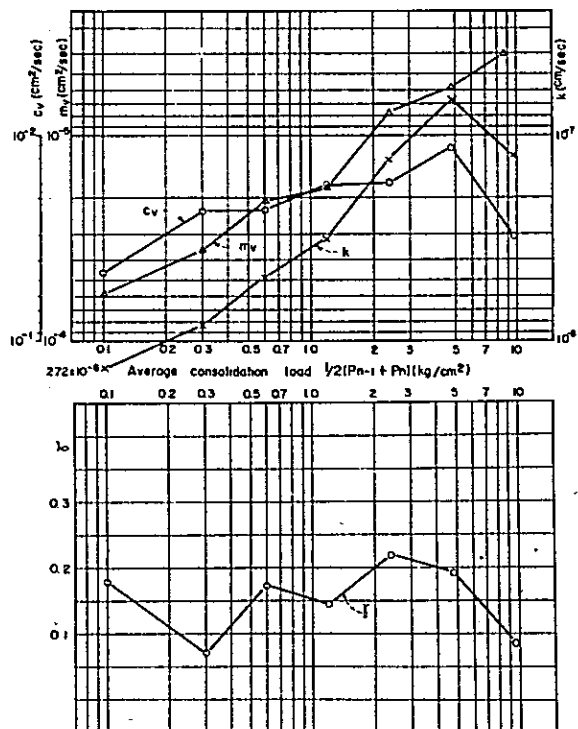
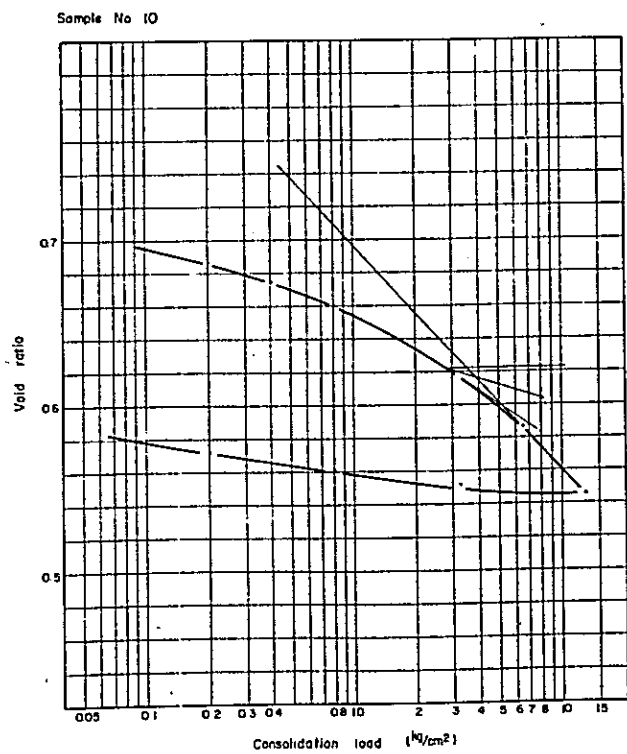
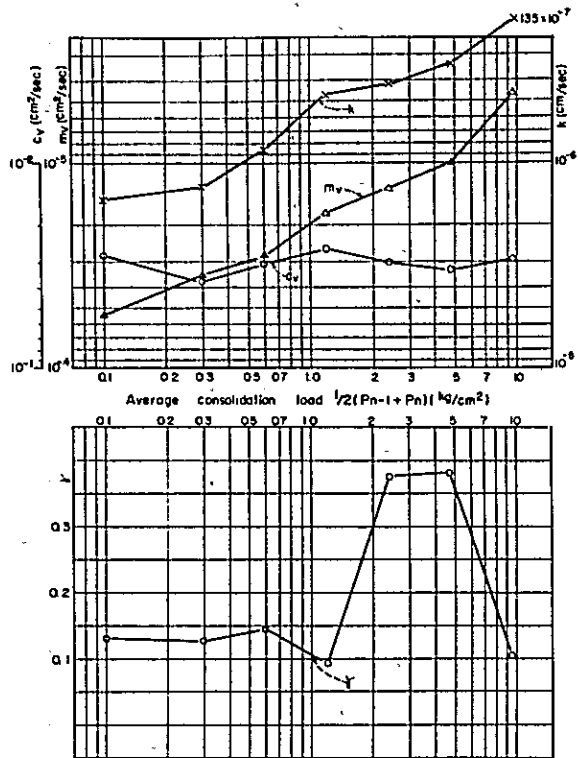
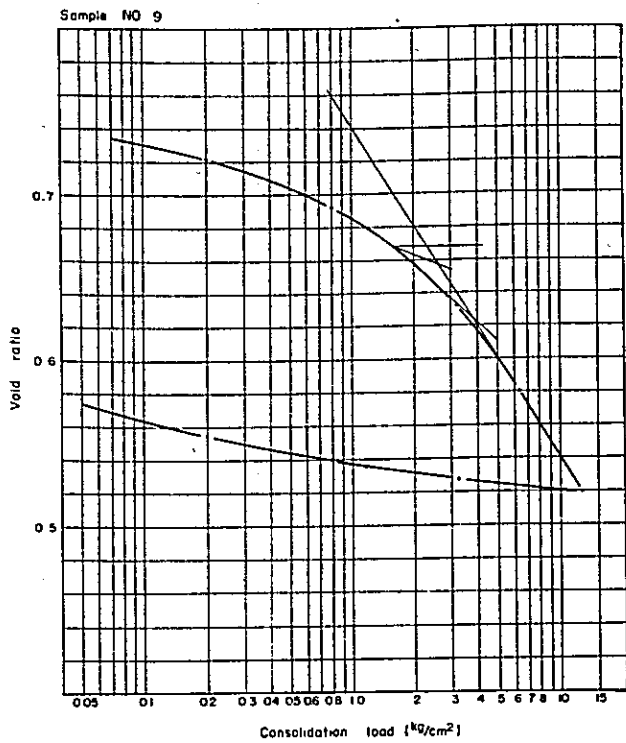


Sample NO. 8



Remarks. c_v Coefficient of consolidation
 m_v Coefficient at volume compressibility
 k Coefficient of permeability
 γ Primary compression ratio

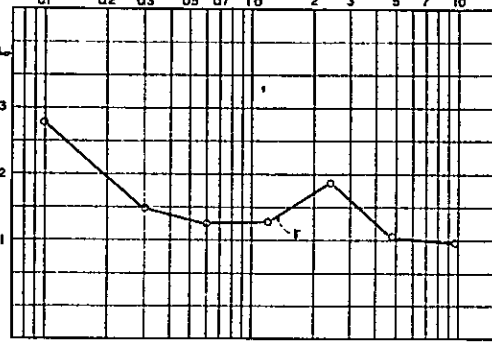
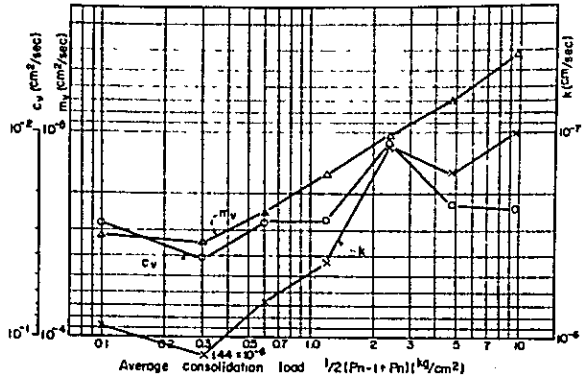
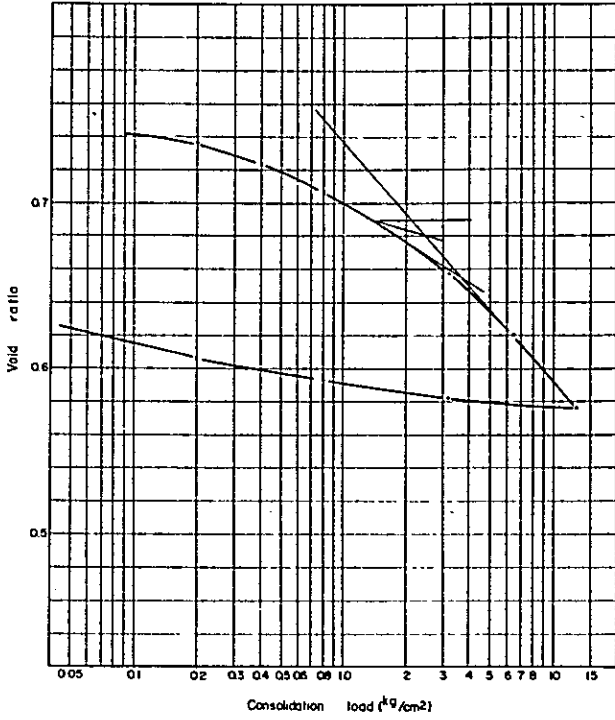
CONSOLIDATION TEST — 5



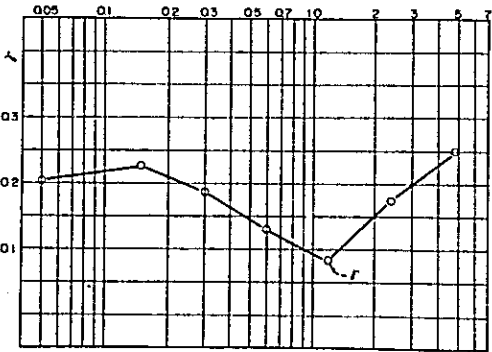
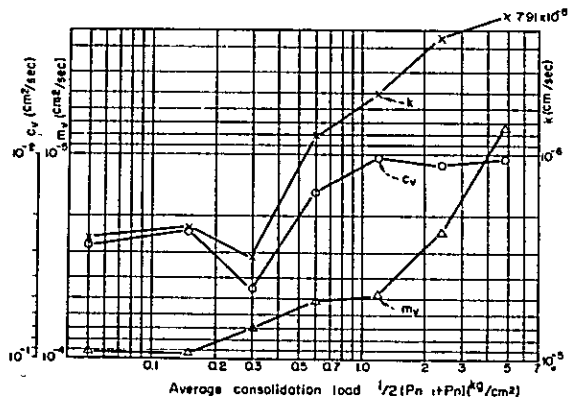
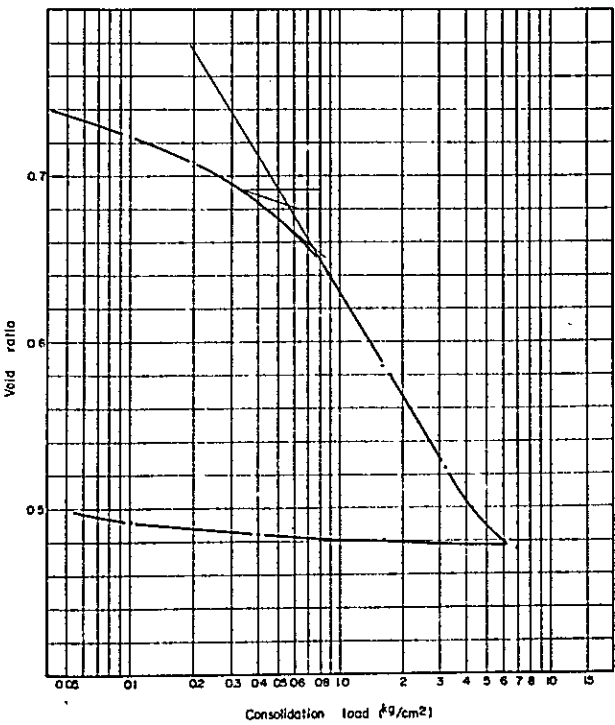
Remarks. c_v Coefficient of consolidation
 m_v Coefficient of volume compressibility
 k Coefficient of permeability
 γ Primary compression ratio

CONSOLIDATION TEST — 6

Sample No 11



Sample No 12



Remarks:
 cv - Coefficient of consolidation
 mv - Coefficient of volume compressibility
 k - Coefficient of permeability
 γ - Primary compression ratio

Compressive Strength Test
of Core Samples (Siltstone)

Sample No.	Bore hole No.	Sampling depth (m)	Compressive strength (kg/cm ²)
1	5	7.5 - 7.7	173
2	6	10.45 - 10.60	170
3	9	7.3 - 7.4	165
Mean			169

Remarks

- (1) The test was made with the dry condition at the laboratory of Chuo Univ. in Tokyo on Aug. 17, 1968.
- (2) Furthermore, two more samples were tested in the laboratory of N.E.A. in Bangkok on Apr. 1, 1968. The results were as follows:

Bore hole No.	Sampling depth (m)	Compressive strength (kg/cm ²)
1	20	115.3
3	12	126.9

APPENDIX III

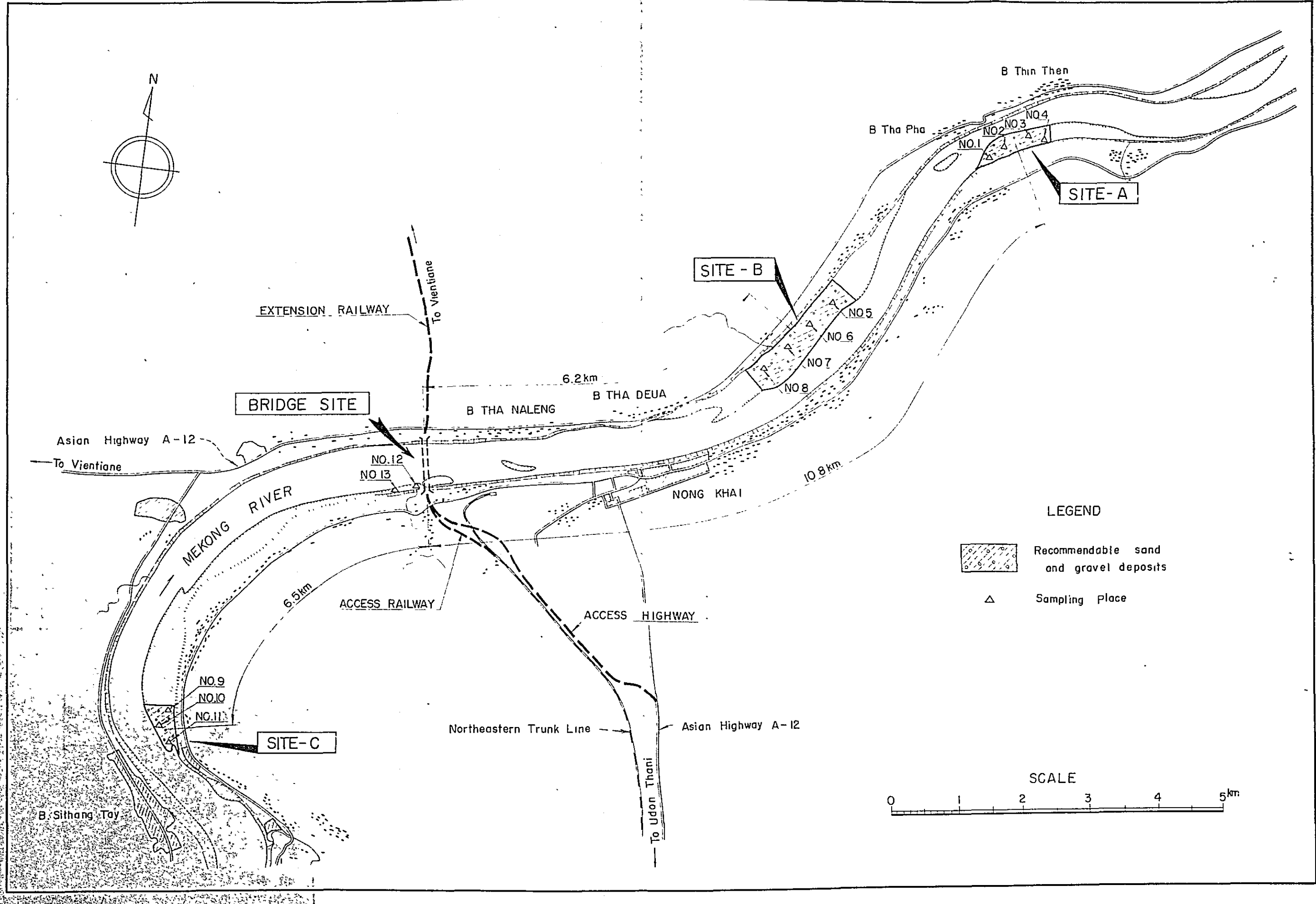
MATERIAL SURVEY

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3.1 Concrete Aggregate

RECOMMENDABLE SAND AND GRAVEL DEPOSITS



SAND TEST

1) SIEVE ANALYSIS

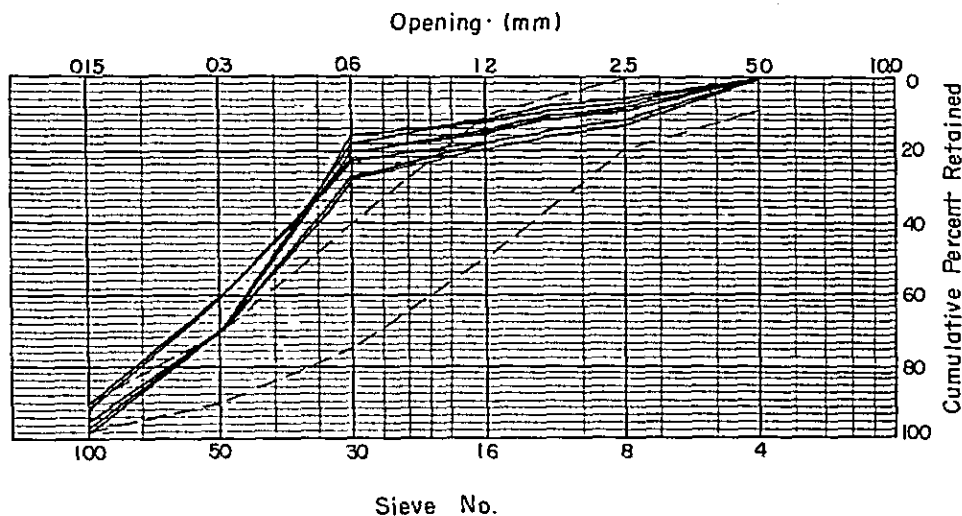
SITE: A

W: Cumulative weight retained (gram)
%: Cumulative percent retained

F.M.: Fineness modulus

Sample No.	1				2			
	516 grms		541.7 grms		516 grms		500 grms	
Sieve No.	W	%	W	%	W	%	W	%
4	0	0	0	0	0	0	0	0
8	71	13.4	63.2	11.7	37	7.2	27	5.4
12	84	15.9	76.4	14.1	43	8.3	34	6.8
16		(19.0)		(18.0)		(11.0)		(10.0)
30	146	27.6	165.2	26.8	79	15.3	86	17.2
50	371	70.1	384.2	71.0	360	69.8	402	80.4
100	499	94.4	513.2	94.9	487	94.5	490	98.0
Passing	529	100.0	541.7	100.0	516	100.0	500	100.0
Max. size	2.7 mm		3.0 mm		1.3 mm		1.3 mm	
F.M.	2.25		2.22		1.98		2.11	

Sample No.	3				4			
	504 grms		505 grms		503 grms		495 grms	
Sieve No.	W	%	W	%	W	%	W	%
4	1	0.2	0	0	0	0	0	0
8	42	8.3	44	8.7	66	13.1	67	13.5
12	50	9.9	53	10.5	76	15.1	76	15.4
16		(13.0)		(13.0)		(18.5)		(18.5)
30	98	19.4	96	19.0	110	21.9	113	22.8
50	355	70.5	355	70.4	302	60.0	300	60.6
100	487	96.6	489	96.8	450	89.3	447	90.4
Passing	504	100.0	505	100.0	503	100.0	495	100.0
Max. size	1.8 mm		1.8 mm		2.9 mm		2.9 mm	
F.M.	2.08		2.08		2.03		2.06	



SAND TESTSITE: A

2) UNIT WEIGHT

Sample No.		2		3	
Weight of sample (gm)		3,212	3,226.5	3,319	3,331
Volume of sample (cm ³)		2,000	2,000	2,000	2,000
Unit weight (kg/m ³)		1,610	1,610	1,660	1,670

3) SPECIFIC GRAVITY

Sample No.		2			
Weight of sample (gm)	A =	500	500		
Capacity of flask (cm ³)	B =	500	500		
Water added to flask (cm ³)	C =	311	309		
Specific gravity	A/(B - C)	2.64	2.62		

4) ABSORPTION

Sample No.		2			
Weight, surface dry condition (gm)	A =	500.0	500.0		
Weight, oven dry condition (gm)	B =	494.2	493.8		
Absorption (A - B)/B x 100 (%)		1.17	1.25		

5) MATERIAL PASSING NO.200 SIEVE

Sample No.		1		4	
Weight of sample before washing (gm)		500.0	500.0	500.0	500.0
Weight of sample after washing (gm)		495.2	496.1	494.3	496.1
Decreased amount (gm)		4.8	3.9	5.7	3.9
Percentage		0.96	0.78	1.14	0.78

SAND TEST

1) SIEVE ANALYSIS

SITE: B

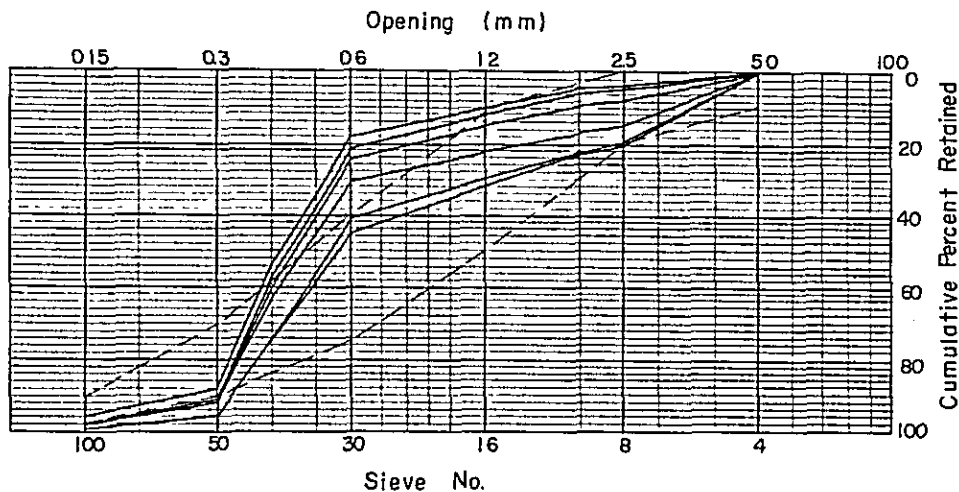
W: Cumulative weight retained (gram)

F.M.: Fineness modulus

%: Cumulative percent retained

Sample No.	5				6			
	500 grms		500 grms		500 grms		500 grms	
Sieve No.	W	%	W	%	W	%	W	%
4	3	0.6	4	0.8	4	0.8	0.5	0.1
8	98	19.6	101	20.2	76	15.2	39.5	7.9
10	112	22.4	113	22.6	84	16.8	46.5	9.3
16		(32.5)		(30.0)		(23.0)		(15.0)
30	226	45.2	203	40.6	152	30.4	117.5	23.5
40	371	74.2	375	75.0	307	61.5	293.5	58.7
50	483	96.6	477	95.5	461	92.2	457.5	91.5
100	499	99.8	495	99.0	491	98.2	487.5	97.5
Passing	500	100.0	500	100.0	500	100.0	500.0	100.0
Max. size	3.5 mm		3.5 mm		1.9 mm		3.2 mm	
F.M.	2.94		2.86		2.60		2.36	

Sample No.	7				8			
	500 grms		500 grms		500 grms		500 grms	
Sieve No.	W	%	W	%	W	%	W	%
4	0	0	1	0.2	0.8	0.2	0	0
8	19	3.8	25	5.0	14.3	2.9	18	3.6
10	22	4.4	30	6.0	109.6	21.9	22	4.4
16		(11.0)		(12.5)		(23.5)		(13.5)
30	91	18.2	105	21.0	127.8	25.6	138	27.6
40	267	53.4	283	56.6	420.8	84.2	437	87.5
50	441	88.2	455	91.0	484.8	97.0	491	98.2
100	479	95.8	492	98.4	488.8	97.8	495	99.0
Passing	500	100.0	500	100.0	500.0	100.0	500	100.0
Max. size	1.2 mm		1.5 mm		2.2 mm		1.5 mm	
F.M.	2.17		2.28		2.47		2.42	



SAND TEST

SITE: B

2) UNIT WEIGHT

Sample No.		6		7	
Weight of sample	(gm)	3,317.2	3,322.2	3,327.2	3,322.2
Volume of sample	(cm ³)	2,000	2,000	2,000	2,000
Unit weight	(kg/m ³)	1,560	1,560	1,660	1,660

3) SPECIFIC GRAVITY

Sample No.			6			
Weight of sample	(gm)	A =	500	500		
Capacity of flask	(cm ³)	B =	500	500		
Water added to flask	(cm ³)	C =	306	307		
Specific gravity	A/(B - C)		2.58	2.59		

4) ABSORPTION

Sample No.			6			
Weight, surface dry condition	(gm)	A =	500.0	500.0		
Weight, oven dry condition	(gm)	B =	495.2	494.7		
Absorption	(A - B)/B x 100 (%)		0.97	1.07		

5) MATERIAL PASSING NO.200 SIEVE

Sample No.		5		8	
Weight of sample before washing	(gm)	641.8	570.0	522.6	500.0
Weight of sample after washing	(gm)	634.9	563.0	517.2	497.7
Decreased amount	(gm)	6.9	7.0	5.4	2.3
Percentage		1.1	1.2	1.0	0.5

6) ORGANIC IMPURITIES

Sample No. 5
 Result Trace

SAND TEST

1) SIEVE ANALYSIS

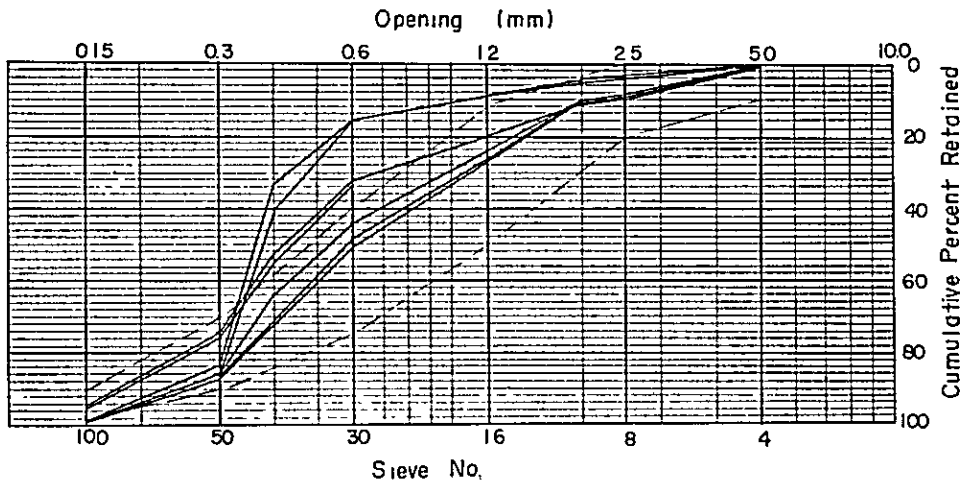
SITE : C

W: Cumulative weight retained (gram)
%: Cumulative percent retained

F.M.: Fineness modulus

Sample No.	9				10			
	500 grms		500 grms		500 grms		500 grms	
Sieve No.	W	%	W	%	W	%	W	%
4	0	0	0	0	0	0	0	0
8	19	3.8	15	3.0	47	9.4	49	9.8
10	23	4.6	20	4.0	55	11.0	58	11.6
16		(9.0)		(9.0)		(20.0)		(20.0)
30	79	15.8	79	15.8	165	33.0	163	32.6
40	164	32.8	200	40.0	272	54.4	263	52.5
50	418.9	83.8	434	86.8	380	76.0	371	74.2
100	498.0	99.6	498	99.6	475	95.0	472	94.4
Passing	500	100.0	500	100.0	500	100.0	500	100.0
Max. size	1.1 mm		1.1 mm		2.3 mm		2.3 mm	
F.M.	2.12		2.14		2.33		2.31	

Sample No.	11 - 1				11 - 2			
	500 grms		500 grms		500 grms		500 grms	
Sieve No.	W	%	W	%	W	%	W	%
4	0	0	0	0	0	0	0	0
8	43	8.6	42.4	8.5	43	8.6	47	9.4
10	51	10.2	50.0	10.0	50	10.0	53	10.6
16		(24.0)		(24.0)		(26.0)		(27.0)
30	217	43.5	217.0	43.4	242	48.4	251	50.2
40	323	64.6	319.0	63.8	354	70.8	356	71.2
50	430	86.0	431.0	86.2	433	86.6	437	87.4
100	490	98.0	489.0	97.8	492	98.4	492	98.4
Passing	500	100.0	500	100.0	500	100.0	500	100.0
Max. size	2.0 mm		2.0 mm		2.0 mm		2.2 mm	
F.M.	2.60		2.60		2.68		2.72	



SAND TEST

SITE: C

2) UNIT WEIGHT

Sample No.	10		11-1		11-2	
Weight of sample (gm)	3,408.2	3,427.3	3,438.2	3,446.2	3,404.2	3,410.2
Volume of sample (cm ³)	2,000	2,000	2,000	2,000	2,000	2,000
Unit weight (kg/m ³)	1,700	1,710	1,720	1,720	1,700	1,710

3) SPECIFIC GRAVITY

Sample No.			11-1		11-2	
Weight of sample (gm)	A =		500.0	500.0	500.0	500.0
Capacity of flask (cm ³)	B =		500.0	500.0	500.0	500.0
Water added to flask (cm ³)	C =		307.5	307.5	308.3	308.3
Specific gravity	A/(B - C)		2.60	2.60	2.60	2.60

4) ABSORPTION

Sample No.			11-1		11-2	
Weight, surface dry condition (gm)	A =		500.0	500.0	500.0	500.0
Weight, oven dry condition (gm)	B =		496.3	495.7	494.9	494.5
Absorption (A - B)/B x 100 (%)			0.75	0.87	1.03	1.11

5) MATERIAL PASSING NO.200 SIEVE

Sample No.	9 and 10			
Weight of sample before washing (gm)	502.0	500.7		
Weight of sample after washing (gm)	499.1	497.8		
Decreased amount (gm)	2.9	2.9		
Percentage	0.58	0.58		

6) ORGANIC IMPURITIES

Sample No. 9
Result Trace

SAND TEST

1) SIEVE ANALYSIS

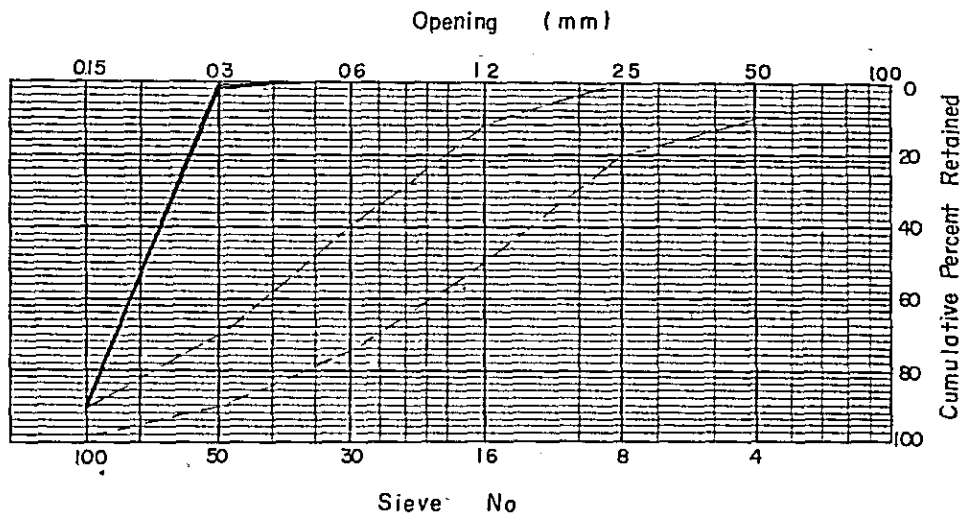
SITE: Bridge Site

W: Cumulative weight retained (gm)

F.M.: Fineness modulus

%: Cumulative percent retained

Sample No.	12				13			
Weight of sample	494.2 grms		490.5 grms		500.4 grms		500.9 grms	
Sieve No.	W	%	W	%	W	%	W	%
4								
8								
12								
16		(0)		(0)		(0)		
30	0	0	1.7	0.3	0.2	0	0	0
50	2.0	0.4	5.2	1.1	5.7	1.1	5.7	1.1
100	444.4	88.9	439.2	89.6	447.7	89.5	453.7	90.6
Passing	494.2	100.0	490.5	100.0	500.4	100.0	500.9	100.0
Max. size	0.3 mm		0.3 mm		0.3 mm		0.3 mm	
F.M.	0.89		0.91		0.91		0.92	



SAND TESTSITE: Bridge site

2) UNIT WEIGHT

Sample No.		12		13	
Weight of sample	(gm)	2,903	2,903	2,925	2,927
Volume of sample	(cm ³)	2,000	2,000	2,000	2,000
Unit weight	(kg/m ³)	1,450	1,450	1,460	1,460

3) SPECIFIC GRAVITY

Sample No.			13			
Weight of sample	(gm)	A =	500.0	500.0		
Capacity of flask	(cm ³)	B =	500.0	500.0		
Water added to flask	(cm ³)	C =	303.2	302.5		
Specific gravity	A/(B - C)		2.54	2.53		

4) ABSORPTION

Sample No.			13			
Weight, surface dry condition	(gm)	A =	500.0	500.0		
Weight, oven dry condition	(gm)	B =	491.9	492.1		
Absorption	(A - B)/B (%)		1.65	1.61		

5) MATERIAL PASSING NO.200 SIEVE

Sample No.			13			
Weight of sample before washing	(gm)		500.0	500.0		
Weight of sample after washing	(gm)		491.3	487.7		
Decreased amount	(gm)		8.7	12.3		
Percentage			1.74	2.46		

GRAVEL TEST

1) SIEVE ANALYSIS

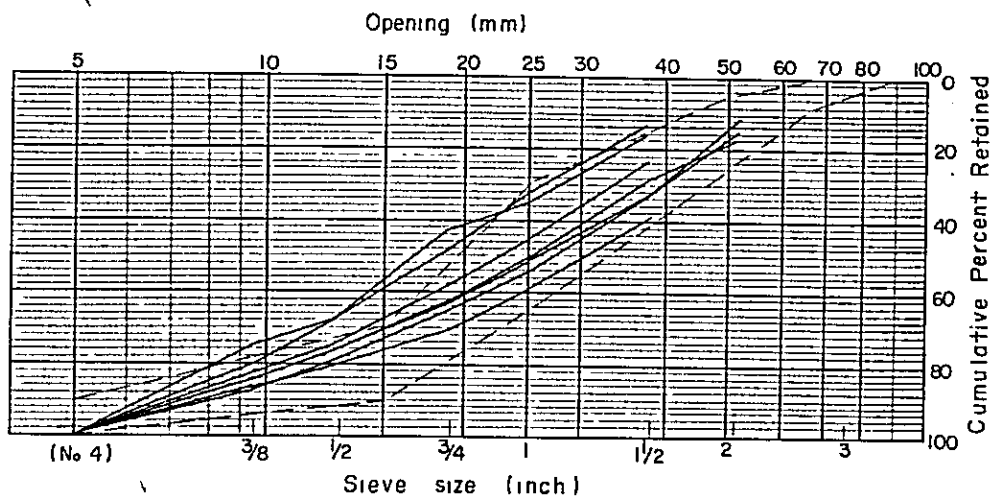
SITE: A

W: Cumulative weight retained (grm)
%: Cumulative percent retained

F.M.: Fineness modulus

Sample No.	1				2			
	21,515 grms		21,464 grms		4,855 grms		6,302 grms	
Sieve size (inch)	W	%	W	%	W	%	W	%
2	3,680	17.1	3,675	17.1				
1½	6,008	27.9	5,999	27.9	1,114	23.0	2,433	38.6
1	11,150	51.8	11,090	51.6	2,238	46.1	3,762	59.7
¾	13,437	62.5	13,337	62.1	2,808	57.9	4,449	70.6
½			16,067	74.8	3,553	73.2	5,076	80.5
⅜	17,887	83.0	17,874	83.3	3,981	82.1	5,457	86.6
No.4	21,515	100.0	21,464	100.0	4,855	100.0	6,302	100.0
Max. size	(60 mm)		(60 mm)		(55 mm)		(70 mm)	
F.M.	7.73		7.73		7.63		7.96	

Sample No.	3				4			
	4,125 grms		5,131 grms		11,831 grms		13,884 grms	
Sieve size (inch)	W	%	W	%	W	%	W	%
2					1,747	14.8	1,566	11.3
1½	542	13.1	820	16.0	3,835	32.4	4,521	32.5
1	1,298	31.5	1,800	35.1	5,940	50.2	7,630	55.0
¾	1,994	48.4	2,194	42.8	7,370	62.3	9,027	65.0
½	2,804	68.0	3,498	68.2	9,080	76.8	11,015	79.3
⅜	3,082	74.8	4,088	79.7	10,052	85.0	12,053	86.7
No.4	4,125	100.0	5,131	100.0	11,831	100.0	13,884	100.0
Max. size	(45 mm)		(50 mm)		(55 mm)		(50 mm)	
F.M.	7.36		7.38		7.80		7.84	



GRAVEL TESTSITE: A

2) UNIT WEIGHT

Sample No.		2		3	
Weight of sample	(gm)	18,596	18,585	19,944	19,956
Volume of sample	(cm ³)	10,776	10,776	10,776	10,776
Unit weight	(kg/m ³)	1,730	1,720	1,850	1,850

3) SPECIFIC GRAVITY and ABSORPTION

Sample No.		2		3	
(Surface dry condition)	Weight in air (gm) A =	2,000.0	2,000.0	2,000.0	2,000.0
	Weight in water (gm) B =	1,237.5	1,235.0	1,233.0	1,242.5
	Weight, oven dry condition (gm) C =	1,991.0	1,990.0	1,984.0	1,983.2
	Specific gravity $A/(A - B)$	2.62	2.61	2.61	2.64
	Absorption $(A - C)/C \times 100$ (%)	0.45	0.50	0.86	0.85

GRAVEL TEST

1) SIEVE ANALYSIS

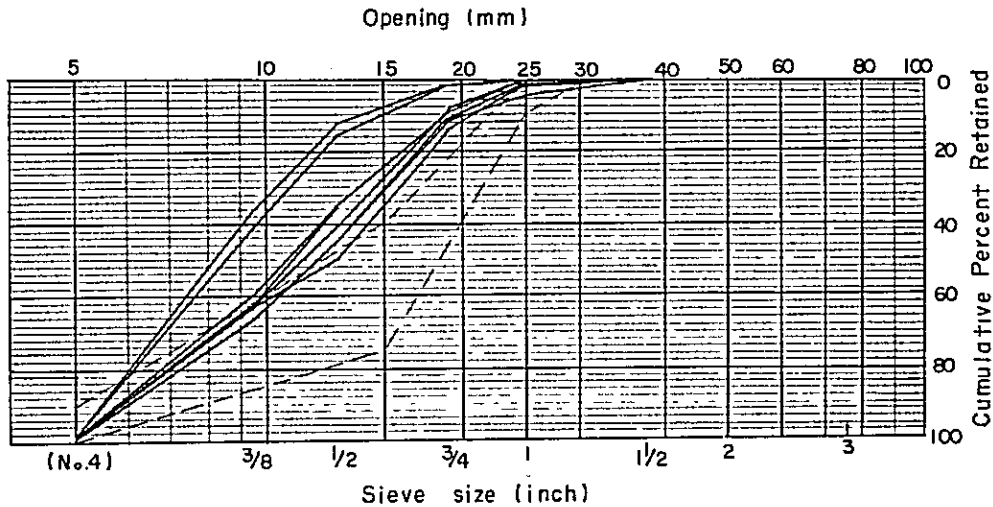
SITE: B

W: Cumulative weight retained (grm)
%: Cumulative percent retained

F.M.: Fineness modulus

Sample No.	5				6			
	5,000 grms		5,000 grms		5,000 grms		5,000 grms	
Sieve size (inch)	W	%	W	%	W	%	W	%
2								
1½	0	0	0	0	0	0	0	0
1	43	0.9	107	2.1	90	1.8	55	1.1
¾	372	7.4	676	13.5	439	8.8	567	11.4
½	2,043	40.8	1,989	39.8	2,259	45.2	2,222	44.5
⅜	3,145	62.9	3,151	63.0	3,152	63.0	3,348	67.0
No.4	5,000	100.0	5,000	100.0	5,000	100.0	5,000	100.0
Max. size	20 mm		22 mm		20 mm		20 mm	
F.M.	6.70		6.77		6.72		6.78	

Sample No.	7				8			
	4,957 grms		4,912 grms		1,420 grms		1,370 grms	
Sieve size (inch)	W	%	W	%	W	%	W	%
2								
1½	0	0	0	0	0	0	0	0
1	222	4.5	180	3.7	13	0.9	12	0.9
¾	472	9.5	459	9.3	20	1.4	20	1.5
½	1,732	35.0	1,690	34.4	178	12.5	204	14.8
⅜	2,940	59.3	3,041	62.0	527	37.1	559	40.6
No.4	4,957	100.0	4,912	100.0	1,420	100.0	1,370	100.0
Max. size	20 mm		20 mm		15 mm		15 mm	
F.M.	6.69		6.71		6.38		6.42	



GRAVEL TESTSITE: B

2) UNIT WEIGHT

Sample No.		6 and 7			
Weight of sample	(gm)	18,726	18,626		
Volume of sample	(cm ³)	10,776	10,776		
Unit weight	(kg/m ³)	1,740	1,730		

3) SPECIFIC GRAVITY and ABSORPTION

Sample No.		6			
[Surface dry condition]	Weight in air (gm) A =	2,106.3	2,052.1		
	Weight in water (gm) B =	1,288.0	1,256.9		
	Weight, oven dry condition (gm) C =	2,080.6	2,025.9		
	Specific gravity $A/(A - B)$	2.58	2.58		
	Absorption $(A - C)/C \times 100$ (%)	1.23	1.28		

GRAVEL TEST

1) SIEVE ANALYSIS

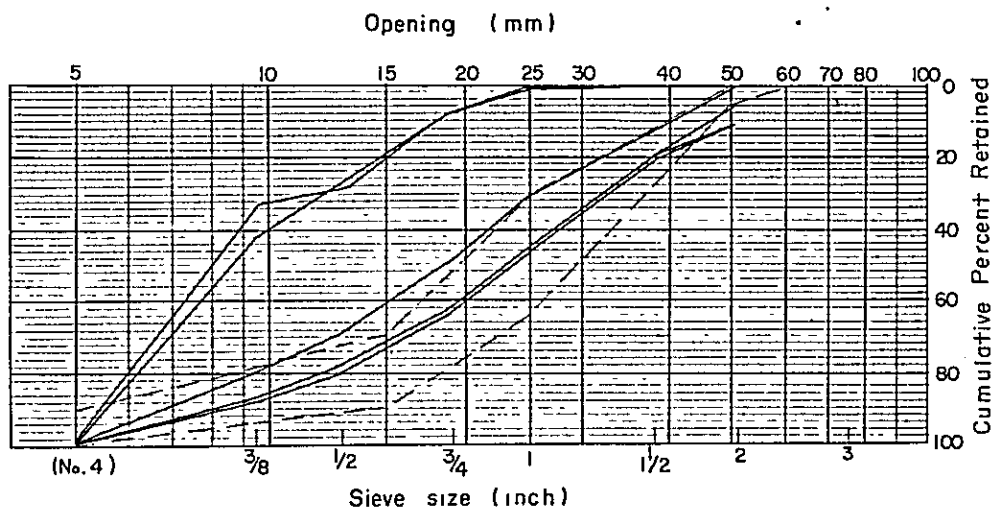
SITE: C

W: Cumulative weight retained (grm)
%: Cumulative percent retained

F.M.: Fineness modulus

Sample No.	9				10			
	2,120 grms		2,120 grms		15,000 grms		15,000 grms	
Sieve size (inch)	W	%	W	%	W	%	W	%
2						(0)		(0)
1½	0	0	0	0	1,645	11.0	1,645	11.0
1	32	1.5	18	0.8	4,570	30.4	4,652	31.0
¾	166	7.8	173	8.2	7,275	48.5	7,278	48.5
½	601	28.3	582	27.5	10,344	69.0	10,338	69.0
⅜	720	34.0	914	43.1	12,088	80.0	12,016	80.1
No.4	2,120	100.0	2,120	100.0	15,000	100.0	15,000	100.0
Msx. size	18 mm		18 mm		40 mm		40 mm	
F.M.	6.42		6.51		7.39		7.40	

Sample No.	11 - 1				11 - 2			
	20,833 grms		20,544 grms		18,841 grms		18,728 grms	
Sieve size (inch)	W	%	W	%	W	%	W	%
2	1,192	5.7	1,192	5.8	2,077	11.0	2,077	11.1
1½	4,250	20.4	4,199	20.4	3,802	20.2	3,632	19.4
1	9,910	47.6	9,647	47.0	8,456	44.9	8,399	44.8
¾	13,218	63.5	12,996	63.2	11,861	62.9	11,690	62.4
½	16,680	80.1	16,458	80.0	14,925	78.2	14,755	78.8
⅜	18,291	87.8	18,160	88.4	16,400	87.0	16,230	86.6
No.4	20,833	100.0	20,544	100.0	18,841	100.0	18,728	100.0
Max. size	45 mm		45 mm		50 mm		50 mm	
F.M.	7.72		7.72		7.70		7.68	



GRAVEL TESTSITE: C

2) UNIT WEIGHT

Sample No.		10		11-1 and 11-2	
Weight of sample	(gm)	19,656	19,656	20,226	20,026
Volume of sample	(gm)	10,776	10,776	10,776	10,776
Unit weight	(kg/m ³)	1,820	1,820	1,880	1,860

3) SPECIFIC GRAVITY AND ABSORPTION

Sample No.		11 - 1		11 - 2	
(Surface dry condition)	Weight in air (gm) A =	5,053.5	5,135.0	5,023.3	5,083.9
	Weight in water (gm) B =	3,120.4	3,192.5	3,099.1	3,134.5
	Weight, oven dry condition (gm) C =	5,022.6	5,102.7	4,989.6	5,050.6
	Specific gravity $A/(A - B)$	2.62	2.64	2.61	2.61
	Absorption $(A - C)/C \times 100$ (%)	0.62	0.63	0.68	0.66

COMPRESSIVE STRENGTH TEST OF CONCRETE

1) DESIGN MIX

Cement:	250 kg/m ³	Gravel:	1,380 kg/m ³
Water:	150 kg/m ³	Sand:	640 kg/m ³
w/c:	60 %		

2) RESULTS OF TESTS

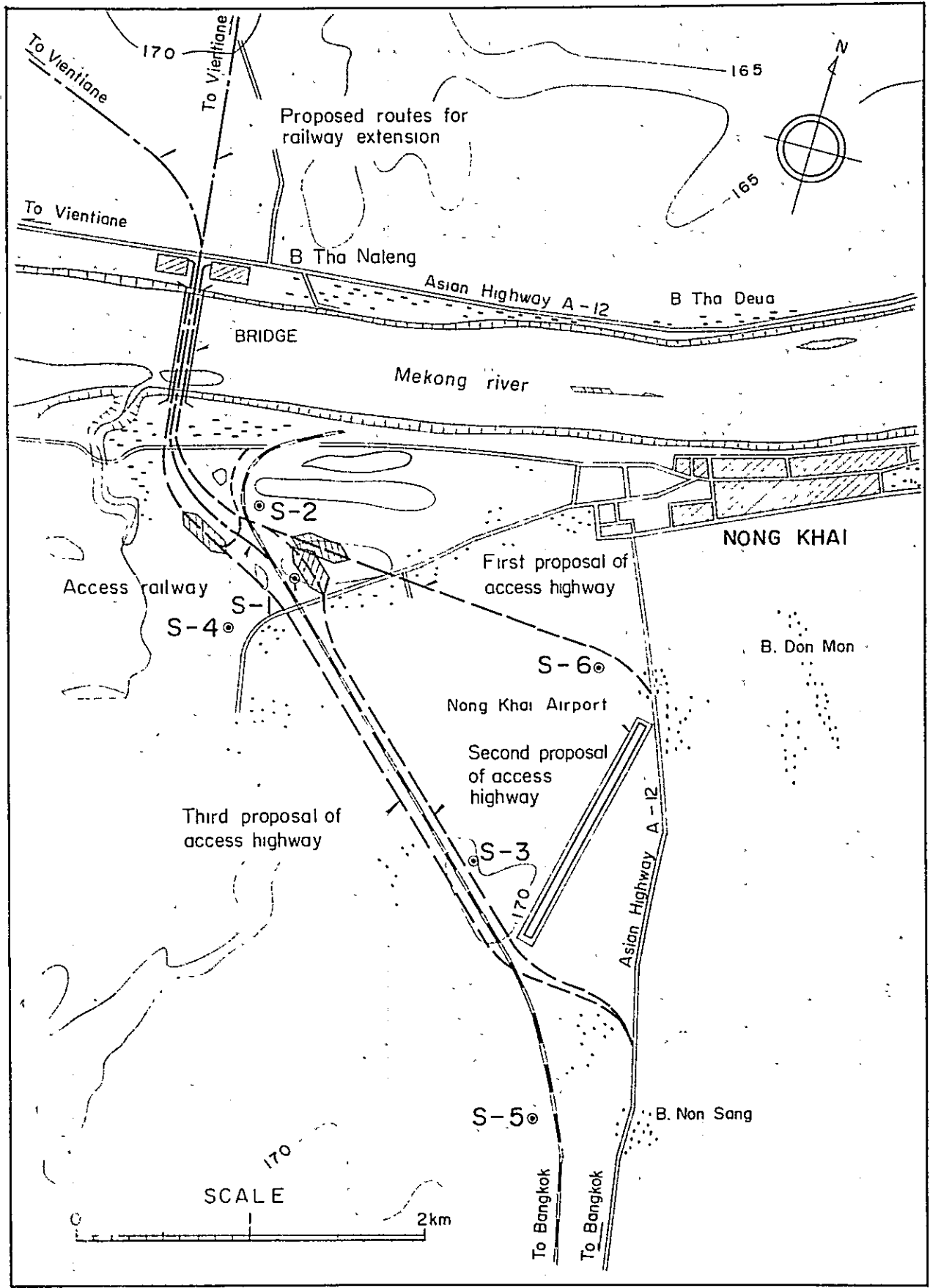
Sampling site		Weight (kg)	Apparent density (kg/m ³)	Slump (cm)	Compressive strength (kg/cm ²)		
Sand	Gravel						
A	A	13.48	2,550	6.5, 6.5	σ_7	109	
		13.43	2,540			104	
		13.52	2,550			95	
	Mean	13.48	2,550	6.5		103	
A	A,B,C	13.08	2,470	7.2, 7.7	σ_{28}	108	
		12.90	2,440			114	
		13.05	2,460			133	
	Mean	12.98	2,450	7.5		113	
B	B,C	13.10	2,470	7.0, 7.5	σ_{28}	172	
		13.14	2,480			169	
		12.92	2,440			172	
	Mean	13.03	2,460	7.3		175	
C	C	13.06	2,460	10.1, 10.9	σ_7	105	
		13.45	2,540			105	
		13.16	2,480			115	
	Mean					108	
	C	C	13.37	2,530	10.1, 10.9	σ_{28}	178
			13.34	2,520			177
			13.40	2,530			176
Mean	13.46	2,540			172		
Mean	13.32	2,510	10.5	Mean	176		

Remarks:

- 1) Specimen size: 15 cm dia. x 30 cm high ($V = 5,300 \text{ cm}^3$)
- 2) Cement used: Ordinary Portland cement made in Thailand (Tiger brand)
- 3) σ_7 : Compressive strength at 7-day age
- 4) σ_{28} : Compressive strength at 28-day age

3.2 Embankment Material

LOCATION OF SAMPLING PLACES FOR HIGHWAY EMBANKMENT MATERIAL



Summary of soil test

Locations: Nong Khai

Items	Unit					
	1	2	3	4	5	6
Sample No.						
Observation	Grey Brown	Grey Brown	Yellow Brown	Yellow Brown	Yellow Brown	Red Brown

Properties	1	2	3	4	5	6
(1) Natural water contents, %	12.20	30.71	9.56	12.41	14.27	14.21
(2) Specific gravity of soil, G	2.73	2.75	2.71	2.70	2.71	2.72

III. Grain size

(1) Proportion

Gravel part	%
i) Gravel part	0
ii) Sand part	0.1
iii) Silt part	26.9
iv) Clay part	73.0

(2) Maximum diameter

(3) 60% diameter, D₆₀

(4) Grain size classification

(5) Unified classification

(6) AASHTO's classification

Properties	1	2	3	4	5	6
(1) Proportion	0	0	6.0	0	8.0	0
i) Gravel part			24.0	5.0	12.0	8.0
ii) Sand part			38.0	68.0	41.0	58.0
iii) Silt part			32.0	27.0	39.0	34.0
iv) Clay part			4.8	0.42	4.8	0.25
(2) Maximum diameter			0.037	0.047	0.04	0.04
(3) 60% diameter, D ₆₀			Clay	Silty clay	Clay	Silty clay
(4) Grain size classification			CH	ML or CL	CH	ML or CR
(5) Unified classification			A-7	A-6	A-7	A-7
(6) AASHTO's classification			A-7	A-6	A-7	A-7

IV. Consistency

(1) Liquid limit, L.L.

(2) Plastic limit, P.L.

(3) Plasticity index, P.I.

(4) Flow index, F.I.

Properties	1	2	3	4	5	6
(1) Liquid limit, L.L.	63.52	56.55	45.20	34.50	50.80	54.70
(2) Plastic limit, P.L.	28.32	25.07	16.81	16.04	16.61	16.00
(3) Plasticity index, P.I.	35.20	31.48	28.39	18.46	34.19	38.70
(4) Flow index, F.I.	9.73	13.60	13.12	9.76	13.60	6.80

V. Compaction

(1) Optimum water contents

(2) Max. density, ρ_{max}

Properties	1	2	3	4	5	6
(1) Optimum water contents	17.8	17.7	12.5	13.2	12.0	14.0
(2) Max. density, ρ _{max}	1.638	1.750	1.970	1.918	1.896	1.881

VI. Shearing strength

(1) Triaxial compression

i) Cohesion, c

ii) Internal friction angle, β

Properties	1	2	3	4	5	6
(1) Triaxial compression	2.05	1.75	1.10	1.10	1.55	1.75
i) Cohesion, c	33°00'	16°42'	33°01'	19°18'	21°48'	30°58'
ii) Internal friction angle, β						

VII. Consolidation

(1) Initial void ratio e₀

(2) Preconsolidation load, P₀

(3) Compression index, C_c

(4) Coef. of consolidation, C_v

(5) Coef. of volume compressibility, M_v

(6) Coef. of permeability, k

Properties	1	2	3	4	5	6
(1) Initial void ratio e ₀	0.539	0.573	0.383	0.381	0.407	0.358
(2) Preconsolidation load, P ₀	0.58	0.61	0.35	0.47	0.91	0.60
(3) Compression index, C _c	0.539	0.573	0.383	0.381	0.407	0.358
(4) Coef. of consolidation, C _v	0.58	0.61	0.35	0.47	0.91	0.60
(5) Coef. of volume compressibility, M _v	0.195	0.196	0.150	0.148	0.086	0.266
(6) Coef. of permeability, k	4.8x10 ⁻³	1.1x10 ⁻²	7.0x10 ⁻³	9.8x10 ⁻³	1.9x10 ⁻²	1.7x10 ⁻³

VIII. Modified C.B.R.

1.31

1.11

6.20

0.68

IX. Swelling test

(1) Case 1

Curing period

0 day,

Swelling ratio

Direct compression, C

1 day,

Swelling ratio

Direct compression, C

7 days,

Swelling ratio

Direct compression, C

14 days,

Swelling ratio

Direct compression, C

Surcharge load

0.15 kg/cm²

Swelling ratio

Direct compression, C

0.30 kg/cm²

Swelling ratio

Direct compression, C

0.45 kg/cm²

Swelling ratio

Direct compression, C

Mixing ratio

30 %

Swelling ratio

Direct compression, C

60 %

Swelling ratio

Direct compression, C

Curing period

1 day,

Swelling ratio

Direct compression, C

7 days,

Swelling ratio

Direct compression, C

Properties	1	2	3	4	5	6
0 day, Swelling ratio Direct compression, C	21.84	0.112	2.077	0.099	9.21	58.28
1 day, Swelling ratio Direct compression, C	22.84	0.112	2.077	0.099	0.17	0.028
7 days, Swelling ratio Direct compression, C	0.955	0.128	0.099	0.60	23.016	0.024
14 days, Swelling ratio Direct compression, C	0.128	0.67	0.76	0.82	0.60	0.035
Surcharge load 0.15 kg/cm ² Swelling ratio Direct compression, C	1.90	0.62	15.39	0.76	-0.79	-
0.30 kg/cm ² Swelling ratio Direct compression, C	0.67	0.67	0.67	0.67	31.48	-
0.45 kg/cm ² Swelling ratio Direct compression, C	0.67	0.67	0.67	0.67	0.82	-
Mixing ratio 30 % Swelling ratio Direct compression, C	-0.75	0.48	-0.05	0.70	-1.39	-
60 % Swelling ratio Direct compression, C	5.431	0.56	2.52	0.75	0.57	-
Curing period 1 day, Swelling ratio Direct compression, C	0.76	0.76	0.76	0.76	18.16	-
7 days, Swelling ratio Direct compression, C	0.92	0.92	0.92	0.92	0.90	-
Mixing ratio 30 % Swelling ratio Direct compression, C	0.76	0.76	0.76	0.76	2.31	-
60 % Swelling ratio Direct compression, C	0.76	0.76	0.76	0.76	0.33	-
Curing period 1 day, Swelling ratio Direct compression, C	7.059	0.92	11.019	0.72	25.011	-
7 days, Swelling ratio Direct compression, C	0.92	0.92	0.92	0.92	0.80	-
Mixing ratio 30 % Swelling ratio Direct compression, C	-	-	30.55	0.12	16.13	-
60 % Swelling ratio Direct compression, C	-	-	21.89	0.28	0.12	-
Curing period 1 day, Swelling ratio Direct compression, C	-	-	0.38	0.60	3.038	-
7 days, Swelling ratio Direct compression, C	-	-	27.52	0.17	0.76	-
Mixing ratio 30 % Swelling ratio Direct compression, C	-	-	23.017	0.60	12.34	-
60 % Swelling ratio Direct compression, C	-	-	0.60	0.77	0.15	-
Curing period 1 day, Swelling ratio Direct compression, C	-	-	5.21	1.12	31.048	-
7 days, Swelling ratio Direct compression, C	-	-	1.12	1.77	0.12	-
Mixing ratio 30 % Swelling ratio Direct compression, C	-	-	33.002	1.77	3.23	-
60 % Swelling ratio Direct compression, C	-	-	1.77	1.77	0.76	-
Curing period 1 day, Swelling ratio Direct compression, C	-	-	2.55	1.60	12.34	-
7 days, Swelling ratio Direct compression, C	-	-	1.60	1.81	0.15	-
Mixing ratio 30 % Swelling ratio Direct compression, C	-	-	11.052	1.81	31.048	-
60 % Swelling ratio Direct compression, C	-	-	1.81	1.81	0.77	-

Remarks-

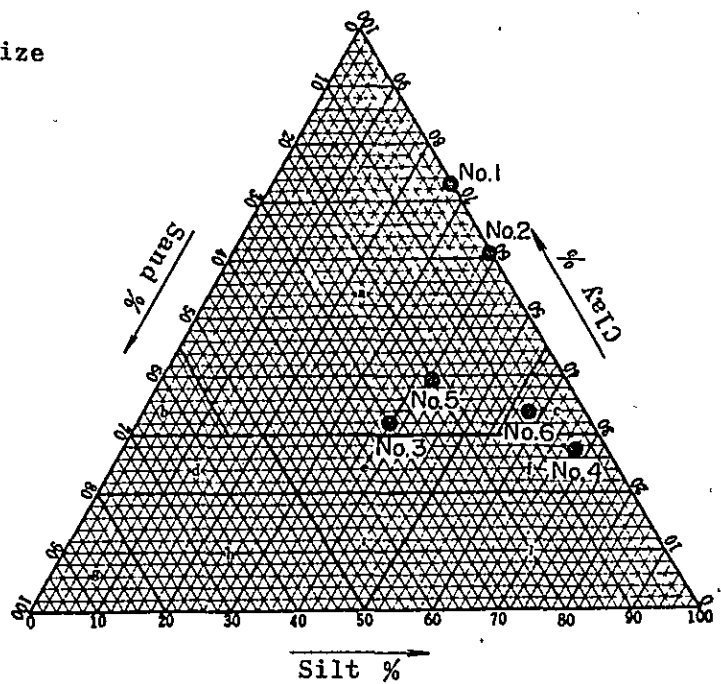
- (1) The details of the cases in the swelling test are described in this paragraph of the report.
- (2) In making CUR and swelling test, the sample No.1 was mixed with the sample No.2, and the sample No.3 with the sample No.4, because of the similar characteristic each other.
- (3) The CUR test was made after the samples were saturated with water for four days.
- (4) The specimen that was used for the swelling test was 6 cm in diameter and 2 cm in height.
- (5) The negative swelling ratio means the compression ratio.

GRAIN SIZE ANALYSIS

Location Nong Khai

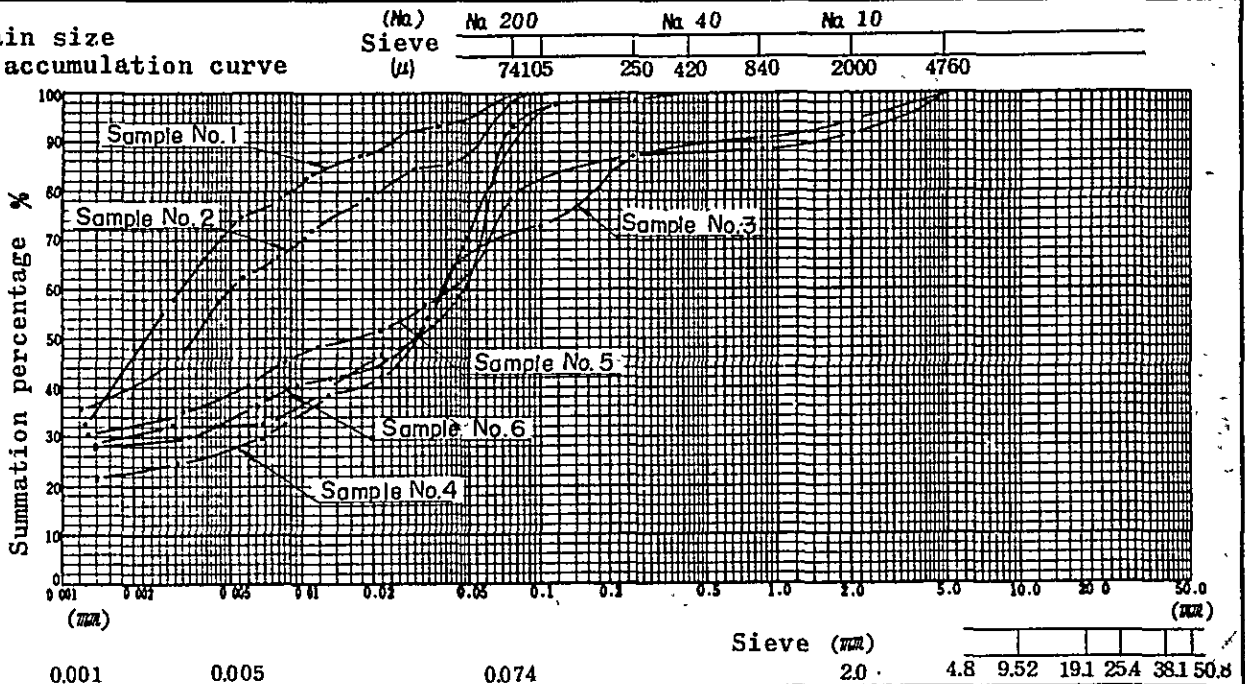
Soil classification of grain size

- a CLAY
- b SANDY CLAY
- c SILTY CLAY
- d SANDY CLAY LOAM
- e CLAYEY LOAM
- f SILTY CLAY LOAM
- g SAND
- h SANDY LOAM
- i LOAM
- j SILTY LOAM



Sample No.	Gravel %	Sand %	Silt %	Clay %	Max. size (mm)	D60 (mm)	D10 (mm)	Uniformity Coeff	2000µ sieve			Sign of Placed part in triangle diagram	Classification	Remarks
									420µ sieve	76µ sieve	Passed Percentage			
1	0	0.1	26.9	73	0.105	0.0032	—	—	100	100	99.9	a	CLAY	
2	0	1	38	61	0.105	0.0049	—	—	100	100	98.9	a	CLAY	
3	6	24	38	32	4.8	0.037	—	—	94.1	900	70.2	a	CLAY	
4	0	5	68	27	0.42	0.047	—	—	100	100	94.8	f	SILTY CLAY LOAM	
5	8	12	41	39	4.8	0.04	—	—	92.2	87.5	79.7	d	CLAY	
6	0	8	58	34	0.25	0.04	—	—	100	100	92.1	c	SILTY CLAY	

Grain size accumulation curve

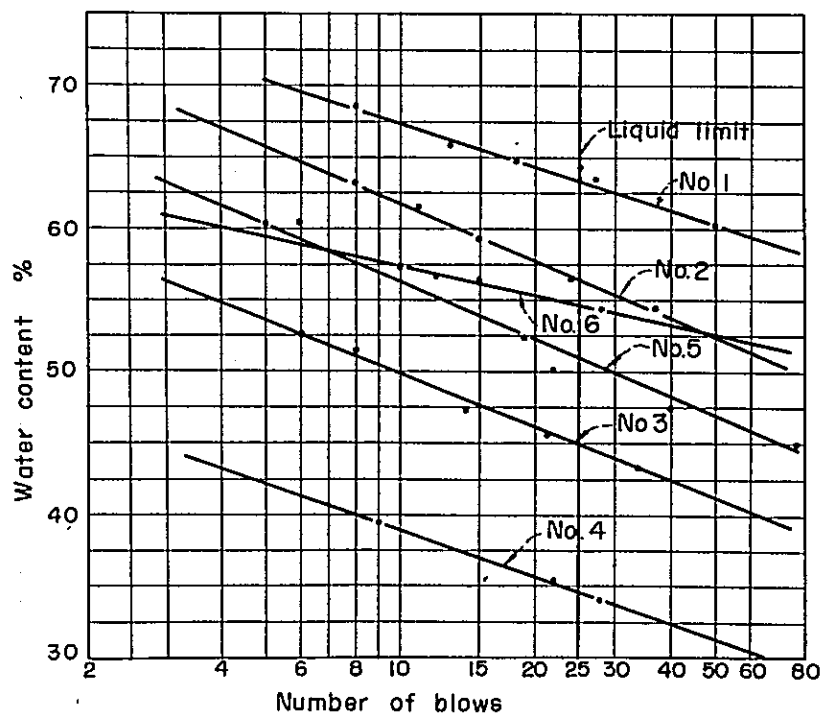


Colloid	Clay	Silt	Sand	Gravel
---------	------	------	------	--------

Liquid Limit and Plastic Limit Tests

Result of Test

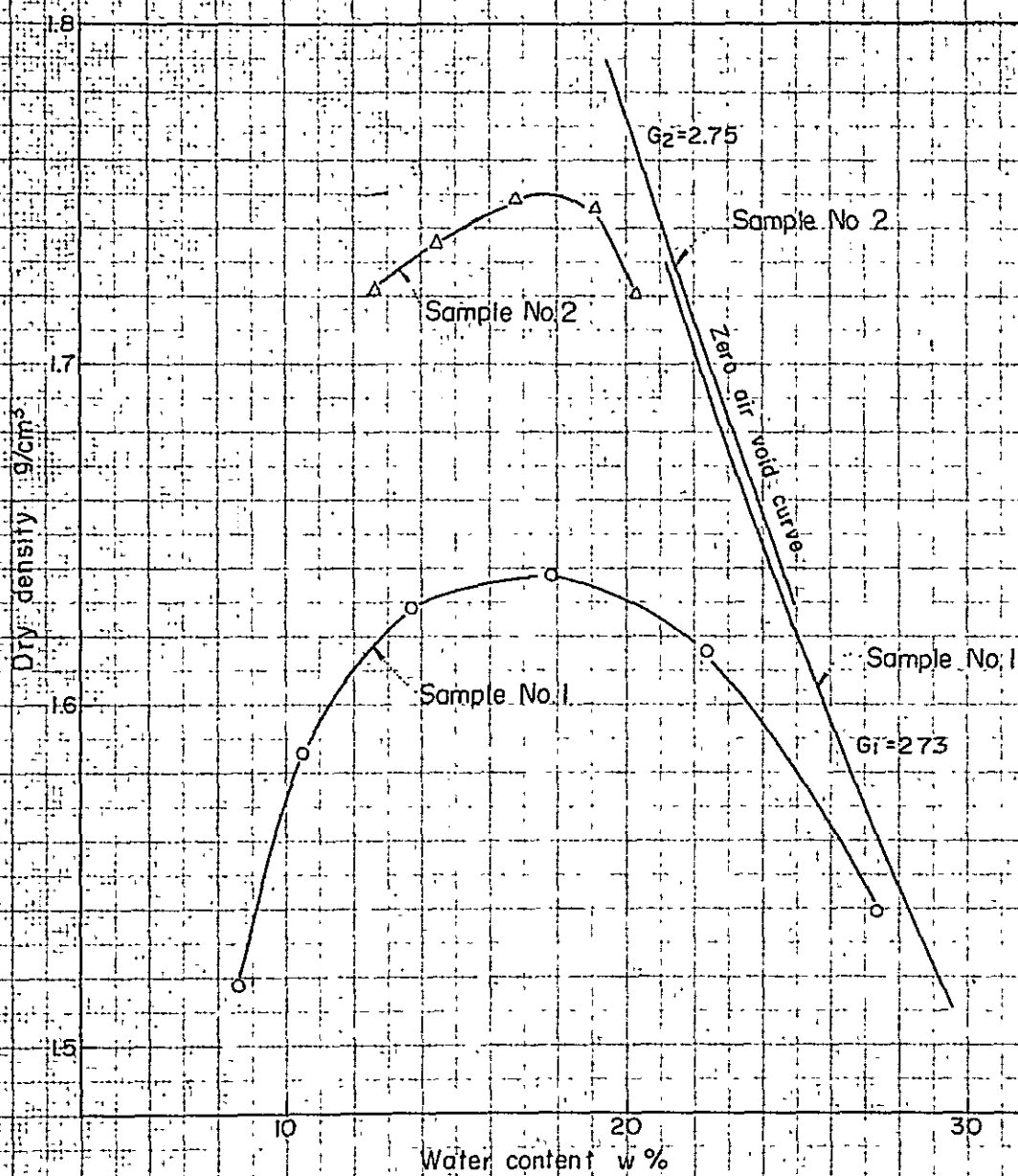
Sample No.	Liquid limit	Plastic limit			Plasticity index	Flow index
		(1)	(2)	Mean		
1	63.52	28.82	27.82	28.32	35.20	9.73
2	56.55	25.08	25.05	25.07	31.48	13.60
3	45.20	16.72	16.90	16.81	28.39	13.12
4	34.50	16.01	16.06	16.04	18.46	9.76
5	50.80	16.71	16.50	16.61	34.19	13.60
6	54.70	16.07	15.93	16.00	38.70	6.80



Remarks : The soil passing 0.4 mm sieve was used for the test to decide the liquid and plastic limits.

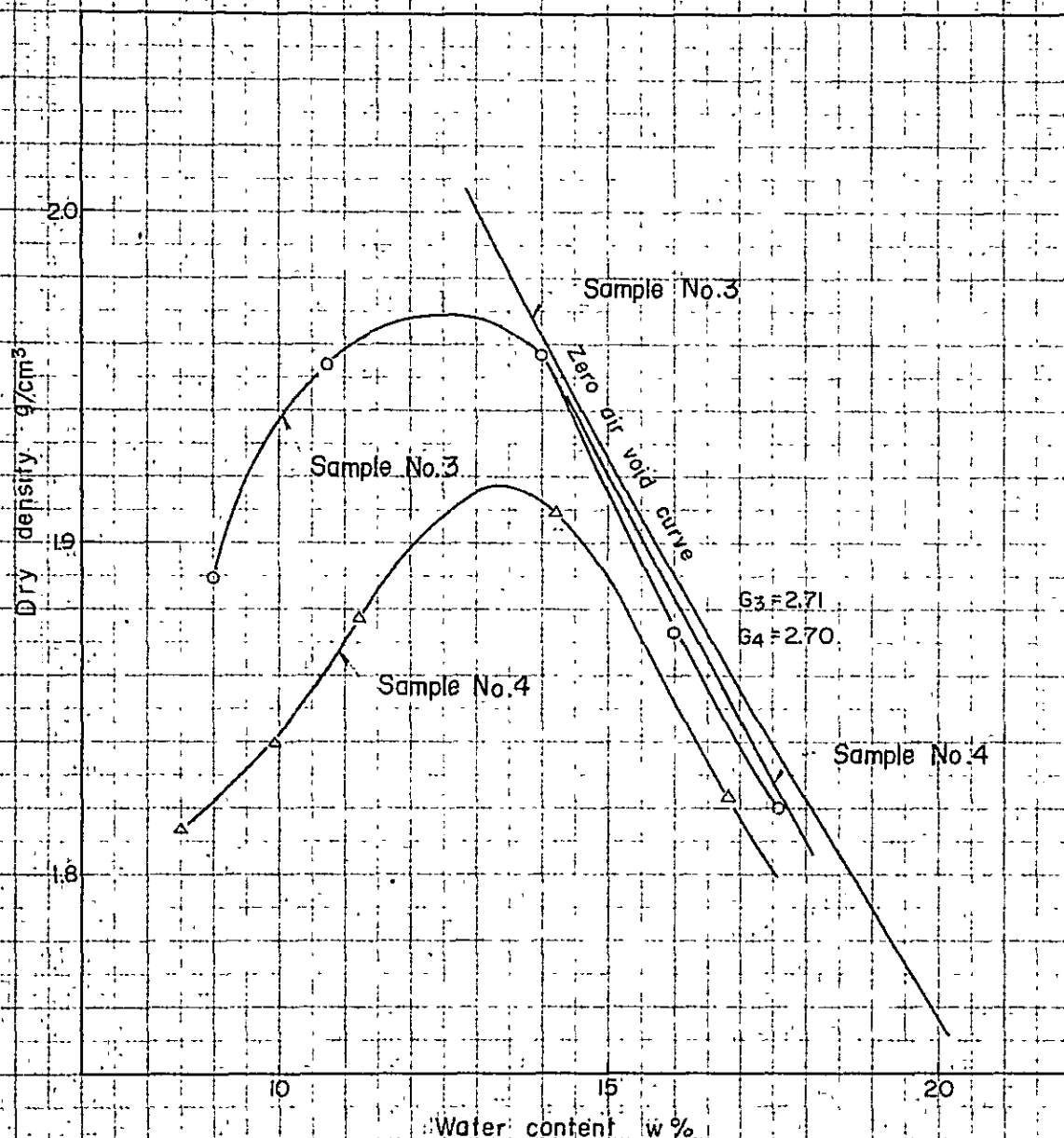
COMPACTION TEST

Sample No.	Optimum water content	Max. dry density
1	17.8 %	1.638 g/cm ³
2	17.7	1.750



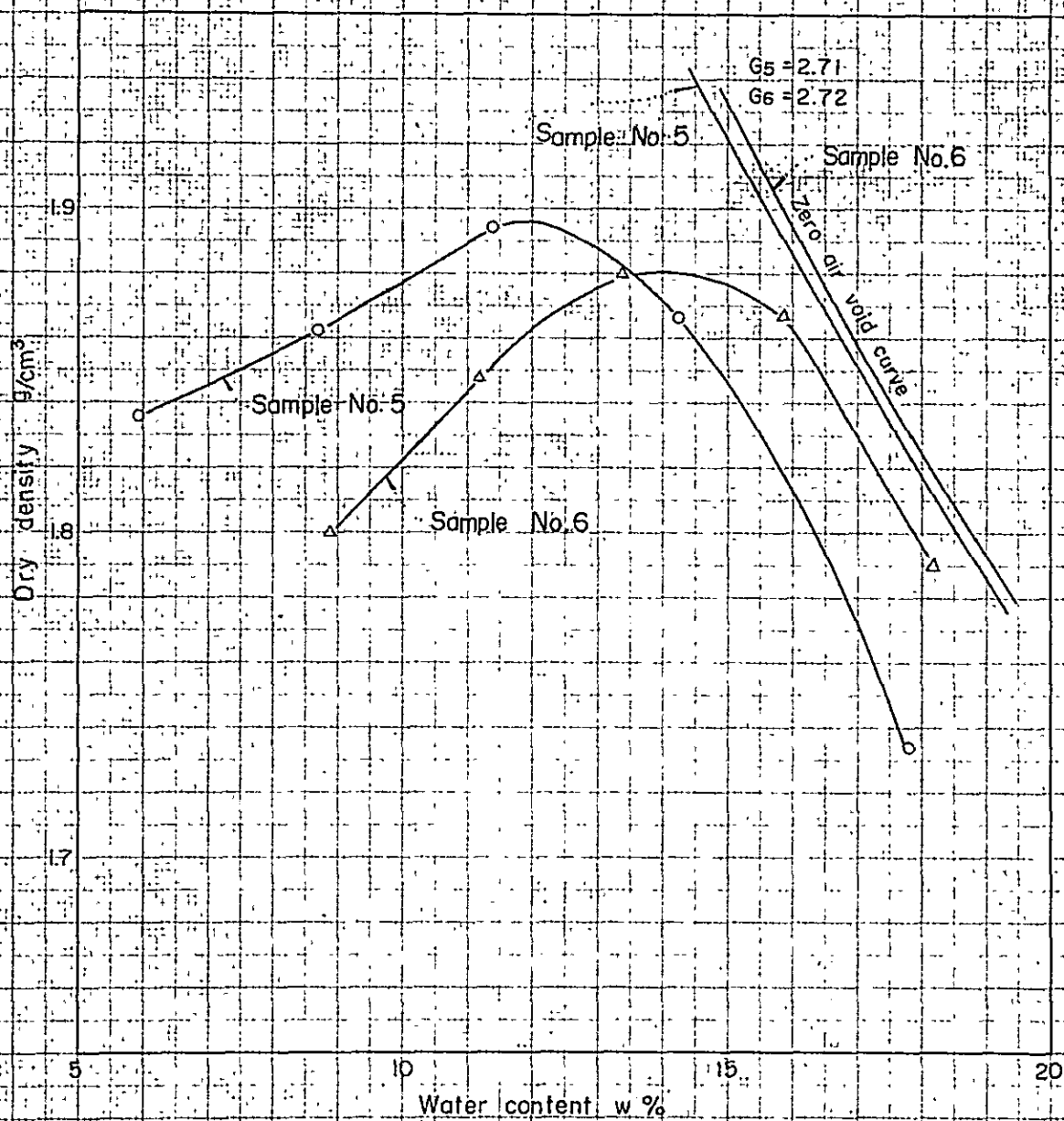
COMPACTION TEST - 2

Sample No.	Optimum water content	Max. dry density
3	12.5 %	1.970 g/cm ³
4	13.2	1.918



COMPACTION TEST - 3

Sample No.	Optimum water content	Max. dry density
5	2.0	1.896
6	4.0	1.881



Direct Shear Test - 1

Case No.	Test condition	Sample No.	Dry density (g/cm ³)	Normal stress (kg/cm ²)	Maximum shear stress (kg/cm ²)	Cohesion c (kg/cm ²)	Internal friction angle ϕ	Shearing strength τ (kg/cm ²)
		1 & 2	1.445 1.453 1.428	0.6 1.1 1.1	0.121 0.208 0.129	0.112	0°55'	0.128
	Curing period, 0 day	3 & 4	1.559 1.542 1.620	0.6 1.6 2.1	0.0831 0.112 0.140	0.062	2°07'	0.099
		5	1.830 1.725 1.876 1.764	0.6 1.1 1.6 2.1	0.396 0.741 0.869 1.329	0.17	23°16'	0.60
		6	1.222 1.309 1.197	0.1 0.6 1.1	0.0265 0.0542 0.0358	0.028	0°24'	0.035
(1)	Curing period, 1 day	3 & 4	1.734 1.690 1.693	0.6 1.1 1.6	0.269 0.240 0.270	0.26	0°04'	0.27
		5	1.753 1.738 1.837	0.6 1.1 1.6	0.491 0.770 1.039	0.14	28°49'	0.69
	Curing period, 7 days	3 & 4	1.766 1.766 1.648	0.6 1.1 1.6	0.271 0.369 0.332	0.24	3°27'	0.30
		5	1.758 1.776 1.784	0.6 1.1 1.6	0.548 0.782 1.025	0.25	25°39'	0.73
	Curing period, 14 days	3 & 4	1.616 1.587 1.544 1.598	0.6 1.1 1.6 2.1	0.153 0.308 - 0.278	0.1	6°17'	0.21
		5	1.658 1.709 1.691 1.699	0.6 1.1 1.6 2.1	0.494 0.675 1.036 1.217	0.24	20°19'	0.61

Direct Shear Test - 2

Case No.	Test condition	Sample No.	Dry density (g/cm^3)	Normal stress (kg/cm^2)	Maximum shear stress (kg/cm^2)	Cohesion c (kg/cm^2)	Internal friction angle β	Shearing strength τ (kg/cm^2)
		1 & 2	1.688 1.703 1.722	0.6 1.1 1.6	0.655 0.677 0.960	0.02	2°52'	0.67
	Surcharge load, 0.15 kg/cm^2	3 & 4	1.958 1.986 1.978	0.6 1.1 1.6	0.634 0.793 1.036	0.48	15°39'	0.76
		5	1.903 1.827 1.922	0.6 1.1 1.6	0.573 0.909 1.216	0.20	31°48'	0.82
		1 & 2	1.734 1.729 1.713	0.6 1.1 1.6	0.521 0.663 0.833	0.48	5°43'	0.58
(2)	Surcharge load, 0.30 kg/cm^2	3 & 4	1.992 1.985 1.946	0.6 1.1 1.6	0.729 0.885 0.774	0.70	2°52'	0.75
		5	1.934 1.932 1.928	0.6 1.1 1.6	0.770 0.940 1.104	0.57	18°16'	0.90
		1 & 2	1.706 1.707 1.741	0.6 1.1 1.6	0.879 0.990 1.020	0.78	7°59'	0.92
	Surcharge load, 0.45 kg/cm^2	3 & 4	1.965 1.965 1.981	0.6 1.1 1.6	0.659 0.713 0.856	0.52	11°19'	0.72
		5	1.930 1.945 1.971	0.6 1.1 1.6	0.602 0.845 1.121	0.33	25°11'	0.80

Direct Shear Test - 3

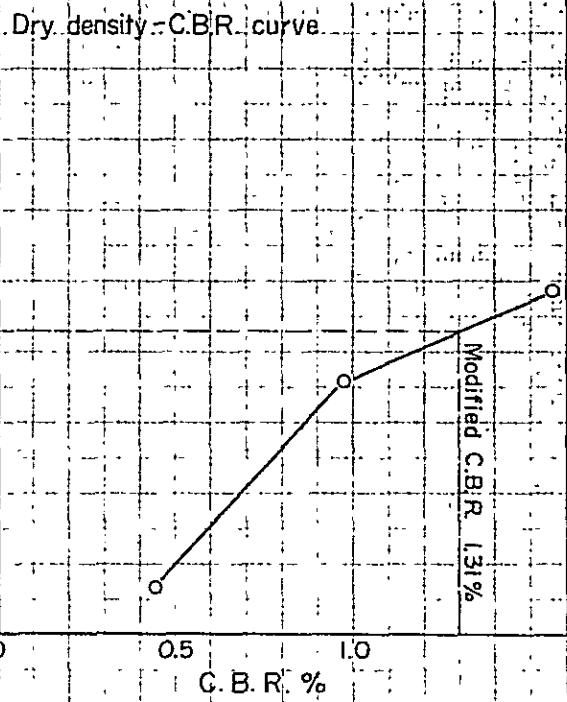
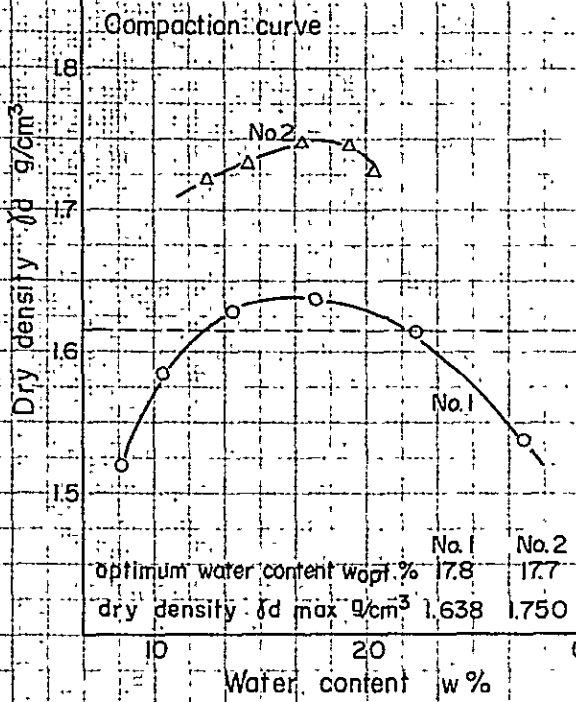
Case No.	Test condition	Sample No.	Dry density (g/cm ³)	Normal stress (kg/cm ²)	Maximum shear stress (kg/cm ²)	Cohesion c (kg/cm ²)	Internal friction angle ϕ	Shearing strength, τ (kg/cm ²)
(3)	Mixing ratio, 30 %	3 & 4	1.394	0.6	0.480	0.28	21°49'	0.68
			1.616	1.1	0.600			
			1.600	1.6	1.023			
		1.622	2.1	1.140				
		1.759	0.6	0.516	0.12	32°38'	0.76	
		1.800	1.1	0.856				
	1.784	1.6	1.157					
	1.668	2.1	1.486					
	Mixing ratio, 60 %	3 & 4	1.569	0.6	0.403	0.17	23°17'	0.60
			1.581	1.1	0.629			
			1.599	1.6	0.891			
		1.557	2.1	1.311				
1.687		0.6	0.515	0.15	31°48'	0.77		
1.771		1.1	0.847					
1.738	1.6	1.152						
1.710	2.1	1.489						
(4)	Curing period, 1 day	3 & 4	1.900	0.6	1.526	1.12	33°02'	1.77
			1.790	1.1	2.359			
			1.838	2.1	2.444			
		1.905	0.6	3.243	3.20	1°44'	3.23	
		1.903	1.6	3.880				
		1.900	2.1	3.268				
	1.831	0.6	1.786	1.60	11°52'	1.81		
	1.838	1.1	1.829					
	1.977	1.6	2.753					
	1.858	0.6	1.556	0.77	51°49'	2.04		
	1.838	1.1	2.210					
	1.880	2.1	2.352					

Triaxial Compression Test

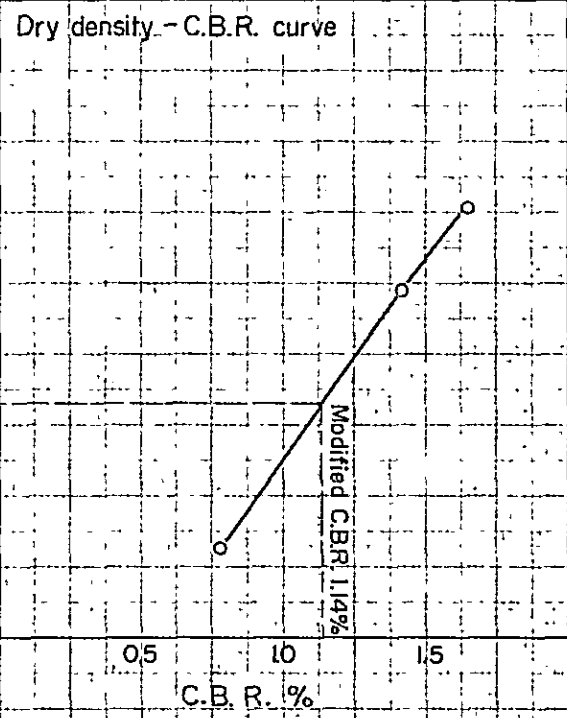
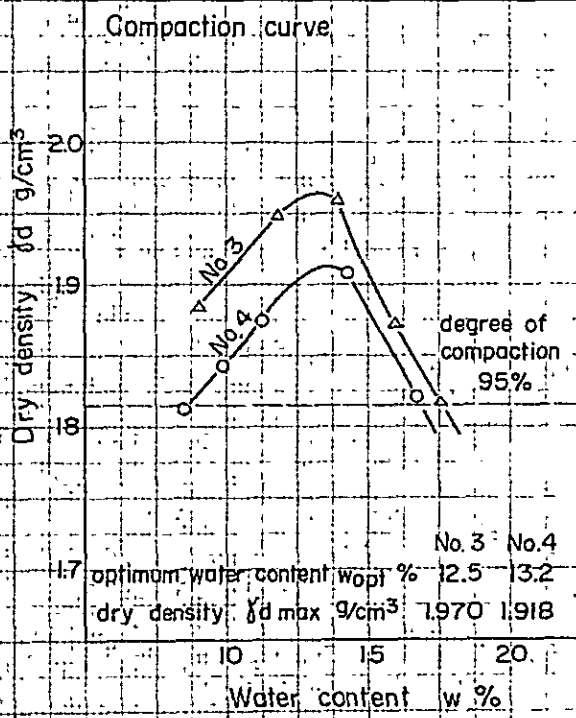
Sample No.	Dry density γ_d (g/cm ³)	Lateral pressure σ_3 (kg/cm ²)	Max. compression stress σ_1 (kg/cm ²)	Cohesion c (kg/cm ²)	Internal friction angle β
1 & 2	1.720	0.5	10.48	2.05	35°00'
	1.720	1.0	12.72		
	1.725	1.5	10.88		
	1.715	2.0	14.92		
	1.402	1.0	3.961	1.75	16°42'
	1.398	2.0	6.490		
	1.396	3.0	7.502		
	1.382	4.0	7.734		
3 & 4	1.920	0.5	5.41	1.10	33°01'
	1.927	1.0	10.18		
	1.922	1.5	10.19		
	1.926	2.0	9.17		
	1.600	1.0	4.029	1.10	19°18'
	1.605	2.0	5.226		
	1.603	3.0	6.153		
5	1.903	0.5	9.90	2.10	33°01'
	1.900	1.0	12.19		
	1.901	1.5	11.64		
	1.904	2.0	12.37		
	1.618	1.0	4.703	1.55	21°48'
	1.615	2.0	7.118		
	1.611	3.0	8.263		
6	1.889	0.5	7.64	1.75	30°58'
	1.884	1.0	8.44		
	1.889	1.5	9.83		
	1.881	2.0	14.26		
	1.599	1.0	4.286	1.75	16°42'
	1.606	2.0	6.520		
	1.596	3.0	7.575		
	1.589	4.0	8.444		

C. B. R. TEST

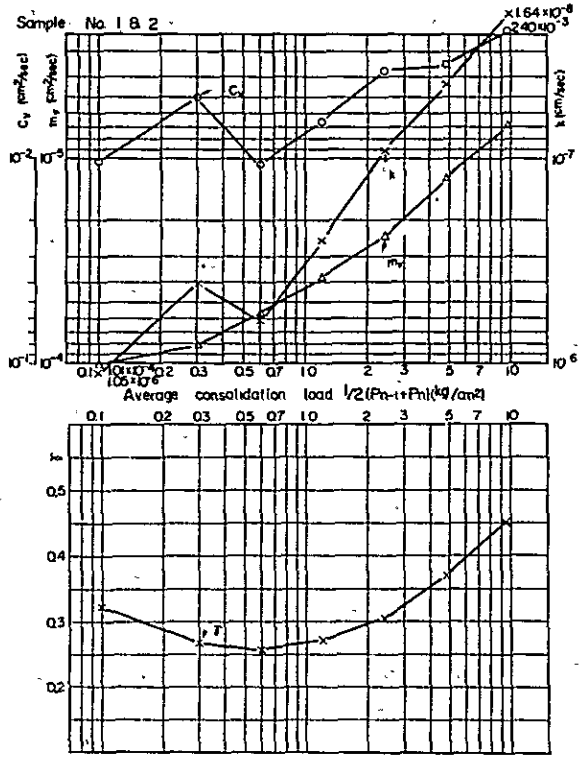
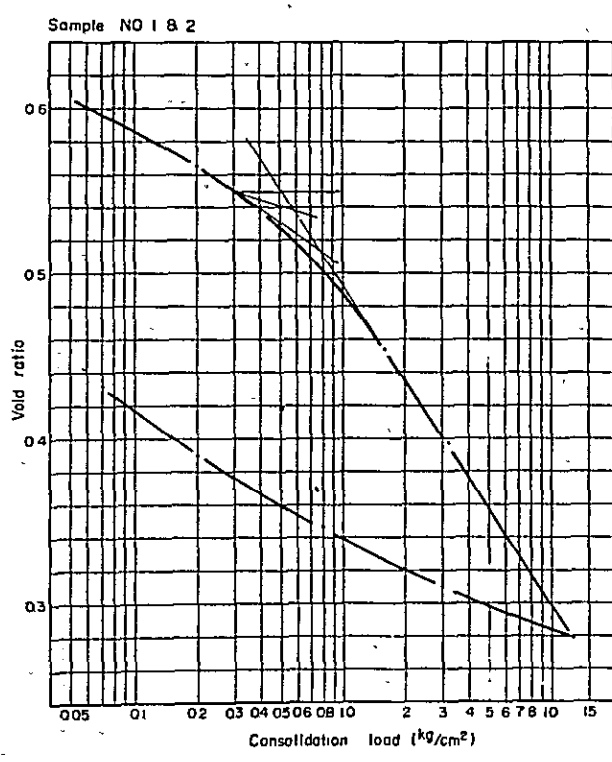
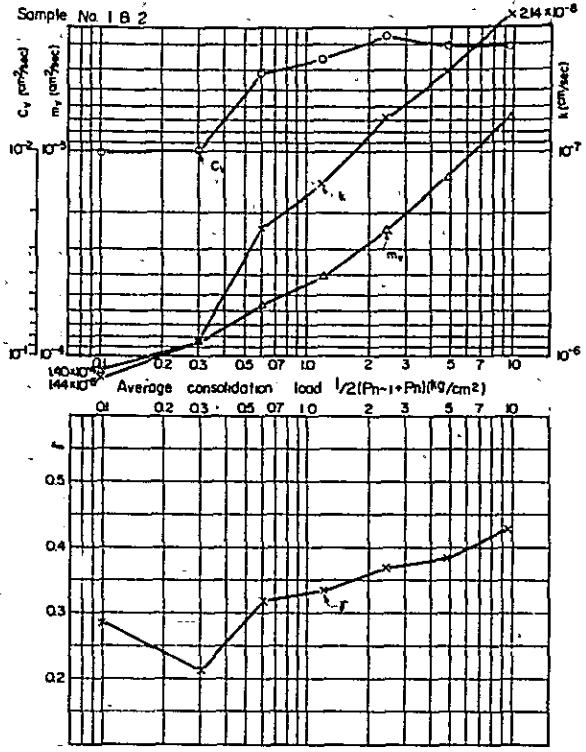
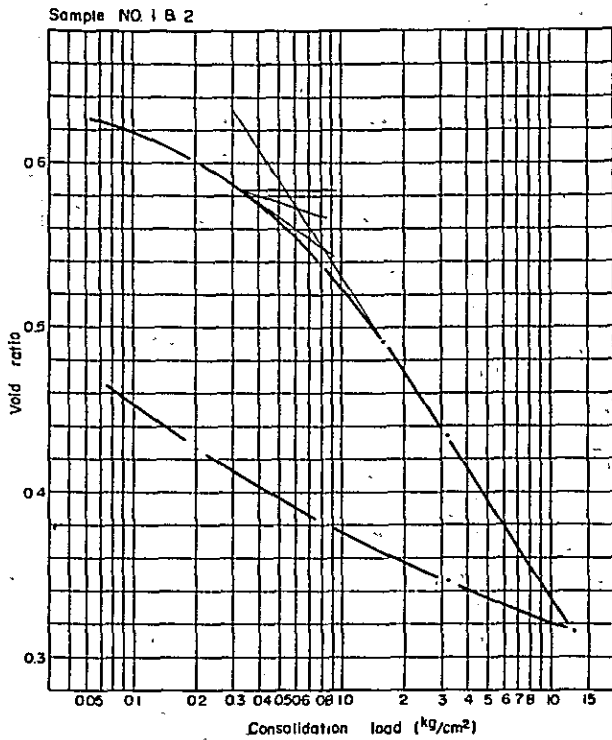
Sample No. 1 & 2



Sample No. 3 & 4

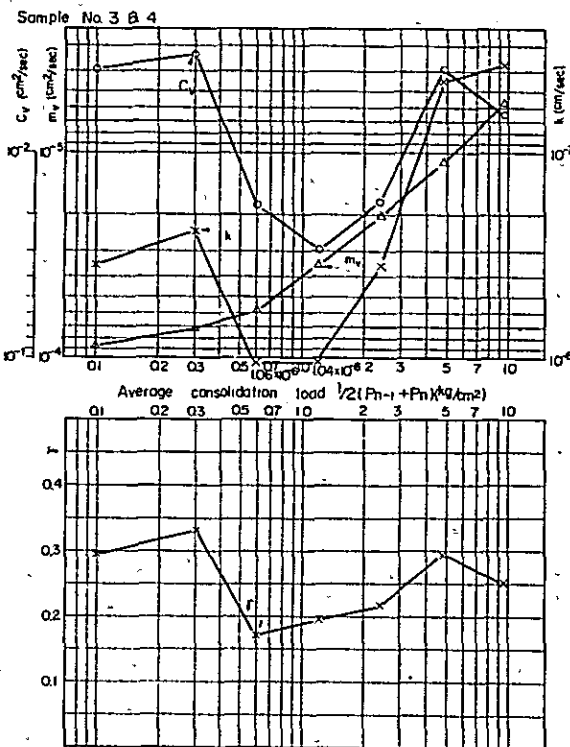
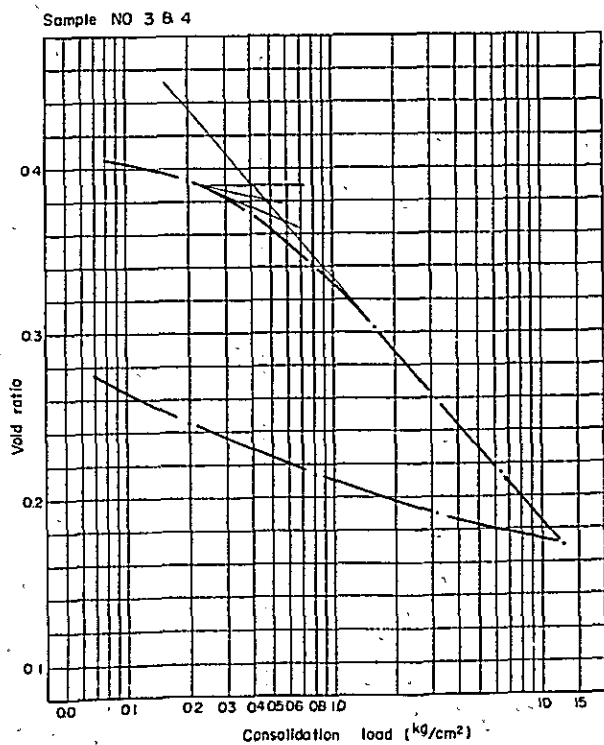
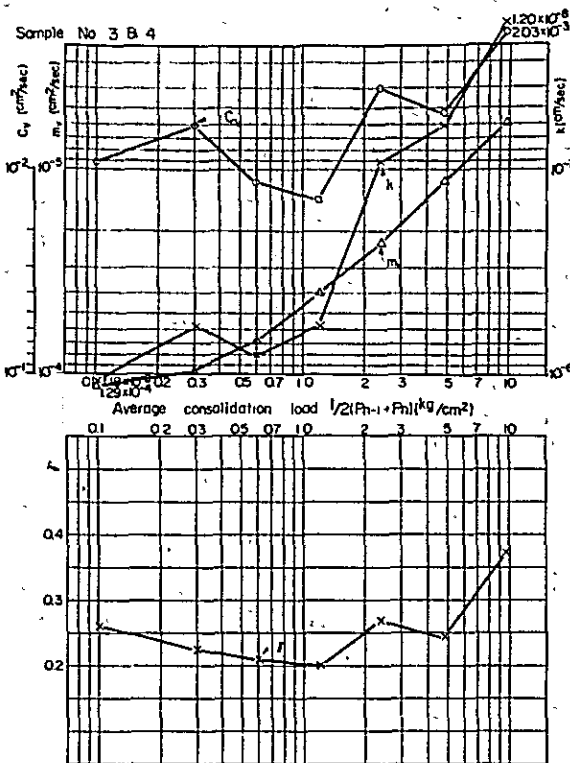
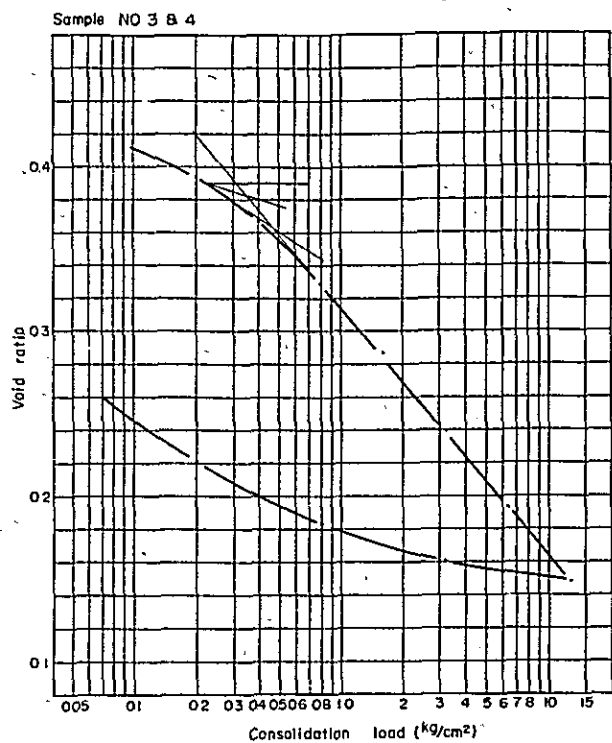


CONSOLIDATION TEST - I



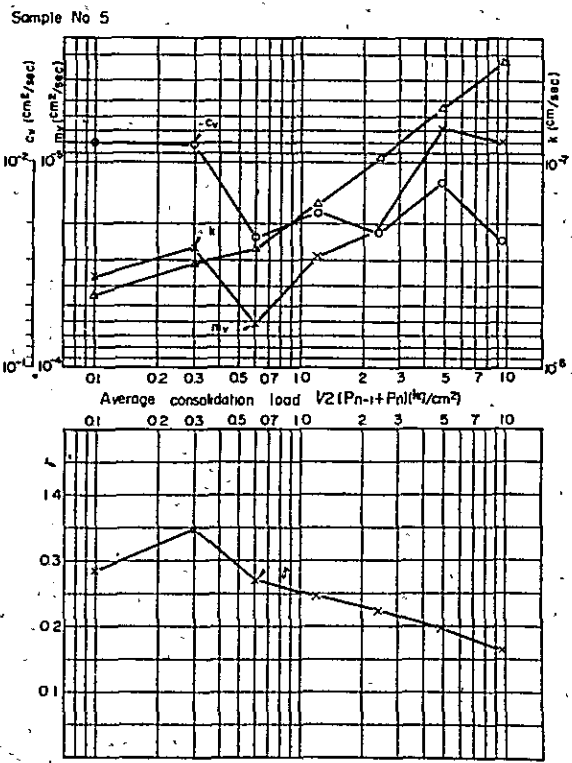
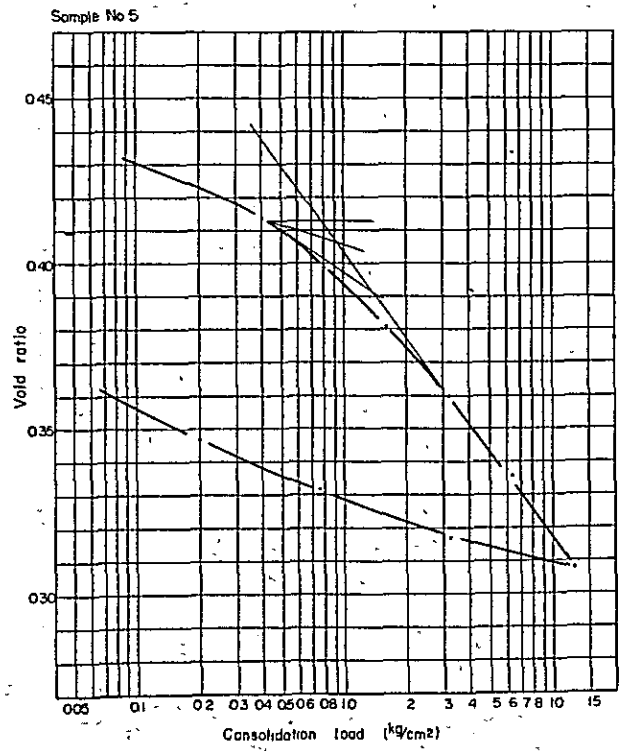
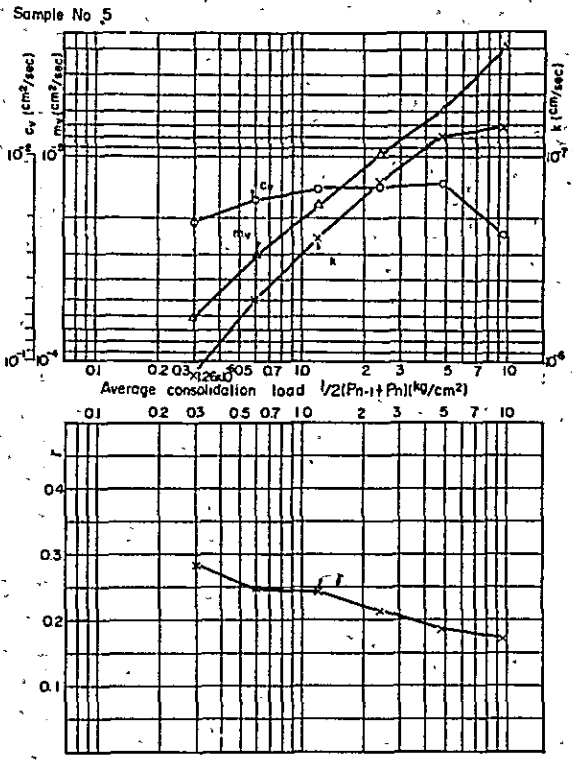
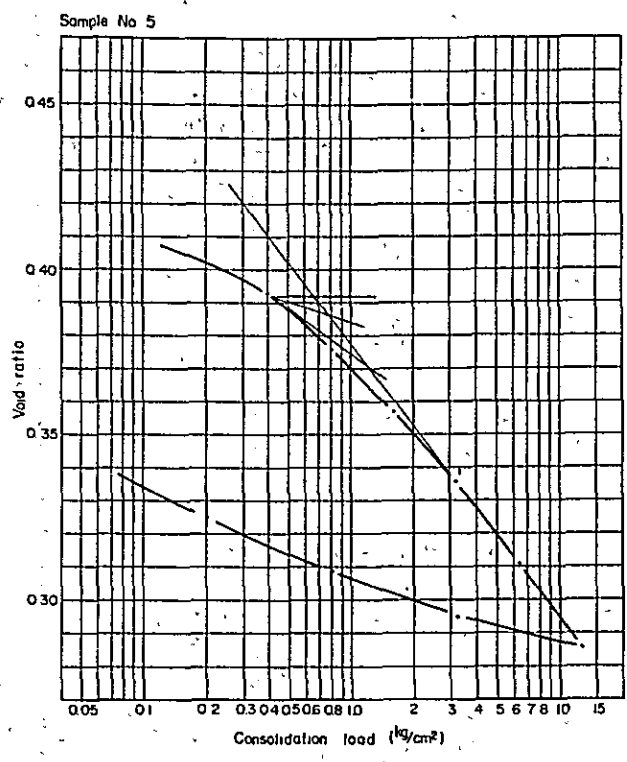
Remarks. C_c Coefficient of consolidation
 m_v Coefficient of volume compressibility
 k Coefficient of permeability
 γ Primary compression ratio

CONSOLIDATION TEST — 2



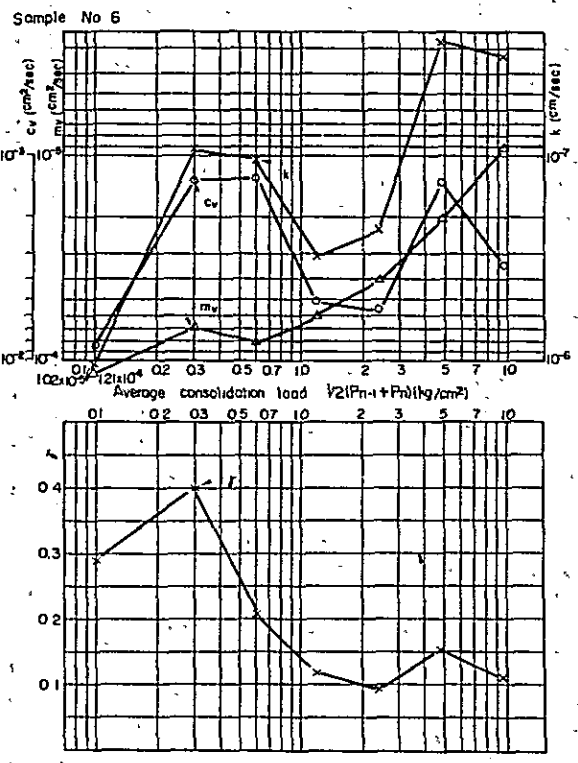
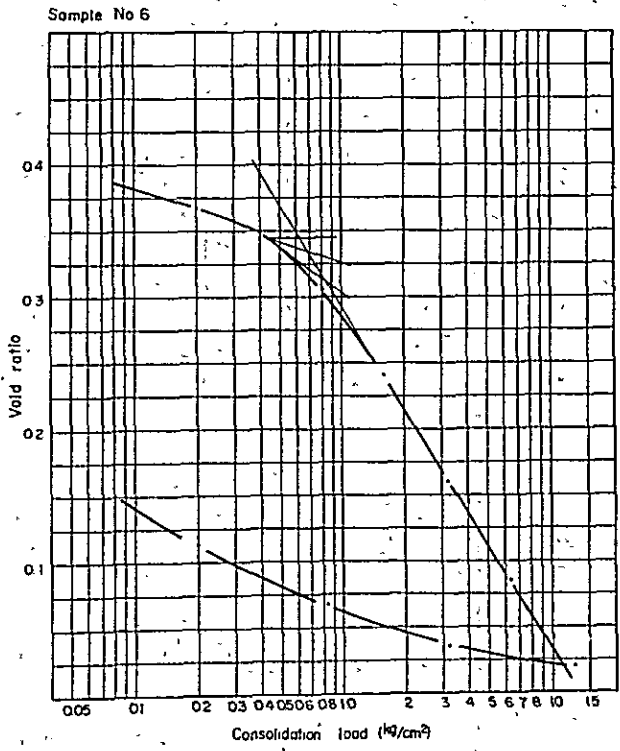
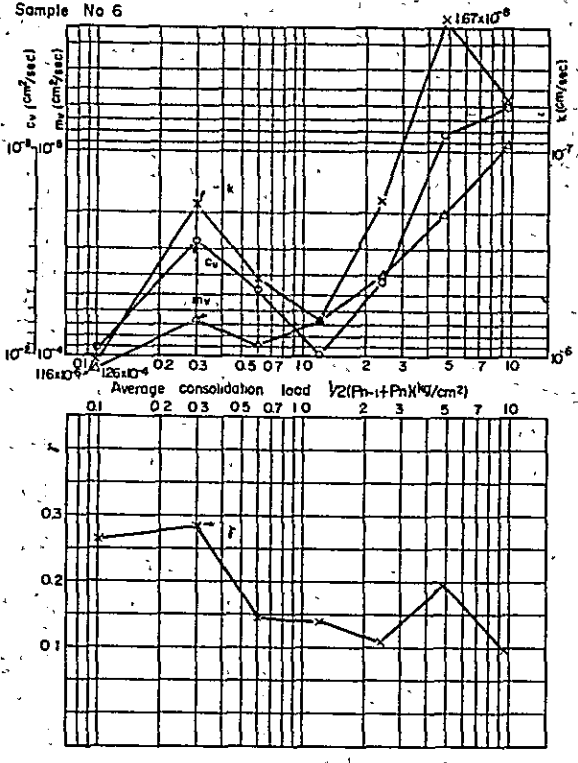
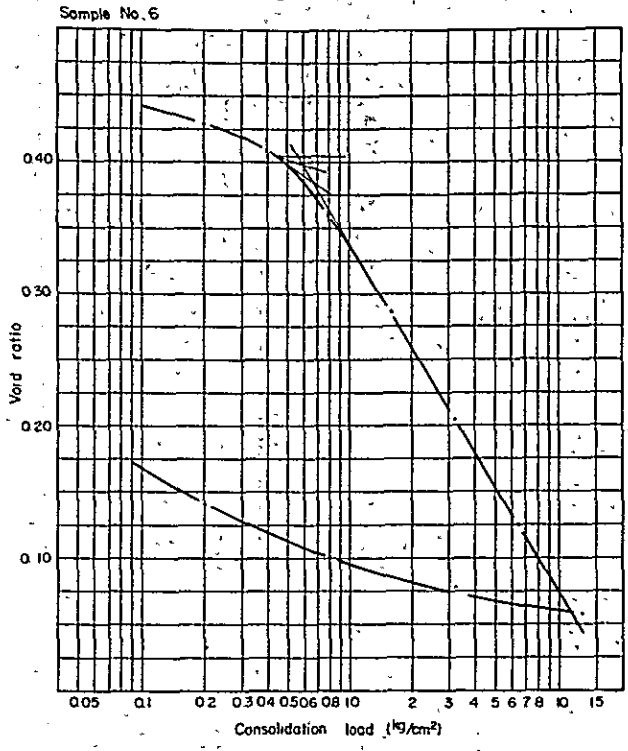
Remarks C_c : Coefficient of consolidation
 m_v : Coefficient of volume compressibility
 k : Coefficient of permeability
 Y : Primary compression ratio

CONSOLIDATION TEST — 3



Remarks c_v : Coefficient of consolidation
 m_v : Coefficient of volume compressibility
 k : Coefficient of permeability
 C_p : Primary compression ratio

CONSOLIDATION TEST — 4

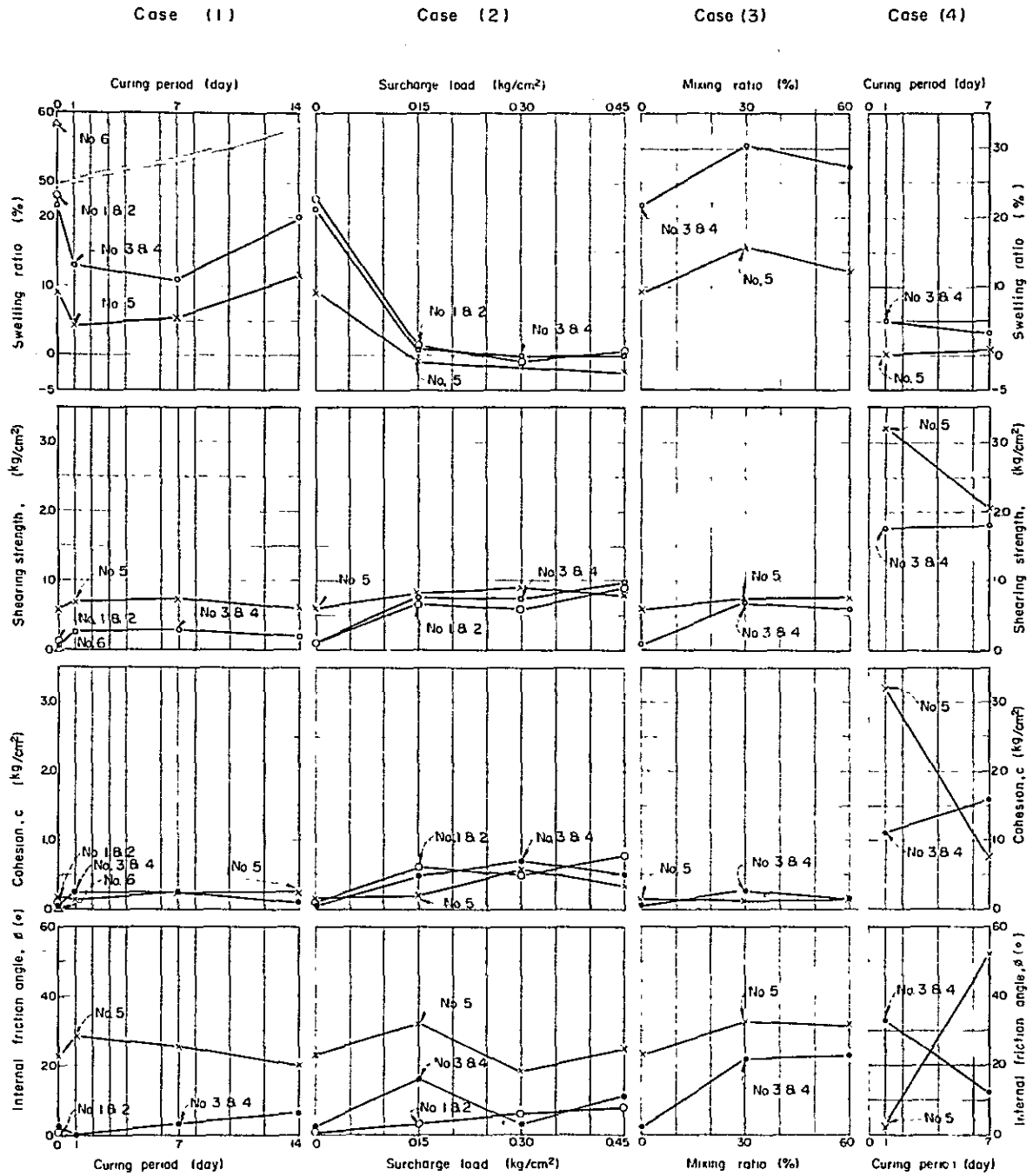


Remarks c_v Coefficient of consolidation
 m_v Coefficient of volume compressibility
 k Coefficient of permeability
 f Primary compression rate

Swelling Test

	1 & 2		3 & 4				5				6		Unit: Percent						
	(1)	(2)	Mean	(1)	(2)	(3)	(4)	Mean	(1)	(2)	(3)	(4)		Mean					
1. Case (1)																			
Curing period, 0 day	24.26	21.21	23.04	22.84	26.37	24.99	21.43	12.55	21.34	5.05	12.15	6.29	13.45	9.24	47.41	60.27	65.85	59.60	58.28
1 day	-	-	-	-	11.59	13.29	14.05	-	12.98	4.84	6.33	1.76	-	4.31	-	-	-	-	-
7 days	-	-	-	-	8.39	9.95	14.97	-	11.10	6.05	5.74	4.77	-	5.52	-	-	-	-	-
14 days	-	-	-	-	15.56	22.66	21.63	-	19.95	13.09	10.11	11.47	-	11.56	-	-	-	-	-
2. Case (2)																			
Surcharge load, 0.15 kg/cm ²	3.10	1.18	0.16	1.50	2.61	0.13	-0.04	-	0.90	-0.79	-0.77	-0.81	-	-0.79	-	-	-	-	-
0.30 kg/cm ²	-0.98	-0.91	-0.36	-0.75	-0.03	-0.04	-0.08	-	-0.05	-1.85	-1.38	-0.95	-	-1.39	-	-	-	-	-
0.45 kg/cm ²	1.31	0.98	-0.02	0.76	-0.07	-0.05	-0.18	-	-0.10	-2.04	-2.36	-2.52	-	-2.31	-	-	-	-	-
3. Case (3)																			
Mixing ratio, 30 %	-	-	-	-	33.45	32.90	28.12	27.74	30.55	14.90	16.23	15.89	17.49	16.13	-	-	-	-	-
60 %	-	-	-	-	27.31	27.05	29.61	26.11	27.52	11.12	9.76	13.26	15.22	12.34	-	-	-	-	-
4. Case (4)																			
Curing period, 1 day	-	-	-	-	4.19	10.45	0.14	6.05	5.21	0.11	0.21	0.21	-	0.18	-	-	-	-	-
7 days	-	-	-	-	4.05	4.19	2.40	-	3.55	0.64	1.19	0.33	1.1	1.09	-	-	-	-	-

SWELLING TEST



- Remarks
- Case (1) The samples were molded in the state of optimum water content and saturated with water after curing for several days at the unloaded condition
 - Case (2) The samples were molded in the state of optimum water content and saturated with water at the loaded condition after the compressive deformation due to loading was almost completed (generally after 24 hours)
 - Case (3) The samples were mixed with the sand of 0.3 millimeter in maximum size by 30 percent of the soil sample in weight in the state of optimum water content or by 60 percent of it and saturated with water at the unloaded condition immediately after molding
 - Case (4) The samples were mixed with cement by five percent in weight in the state of optimum water content and saturated with water after curing for several days at the unloaded condition

APPENDIX IV

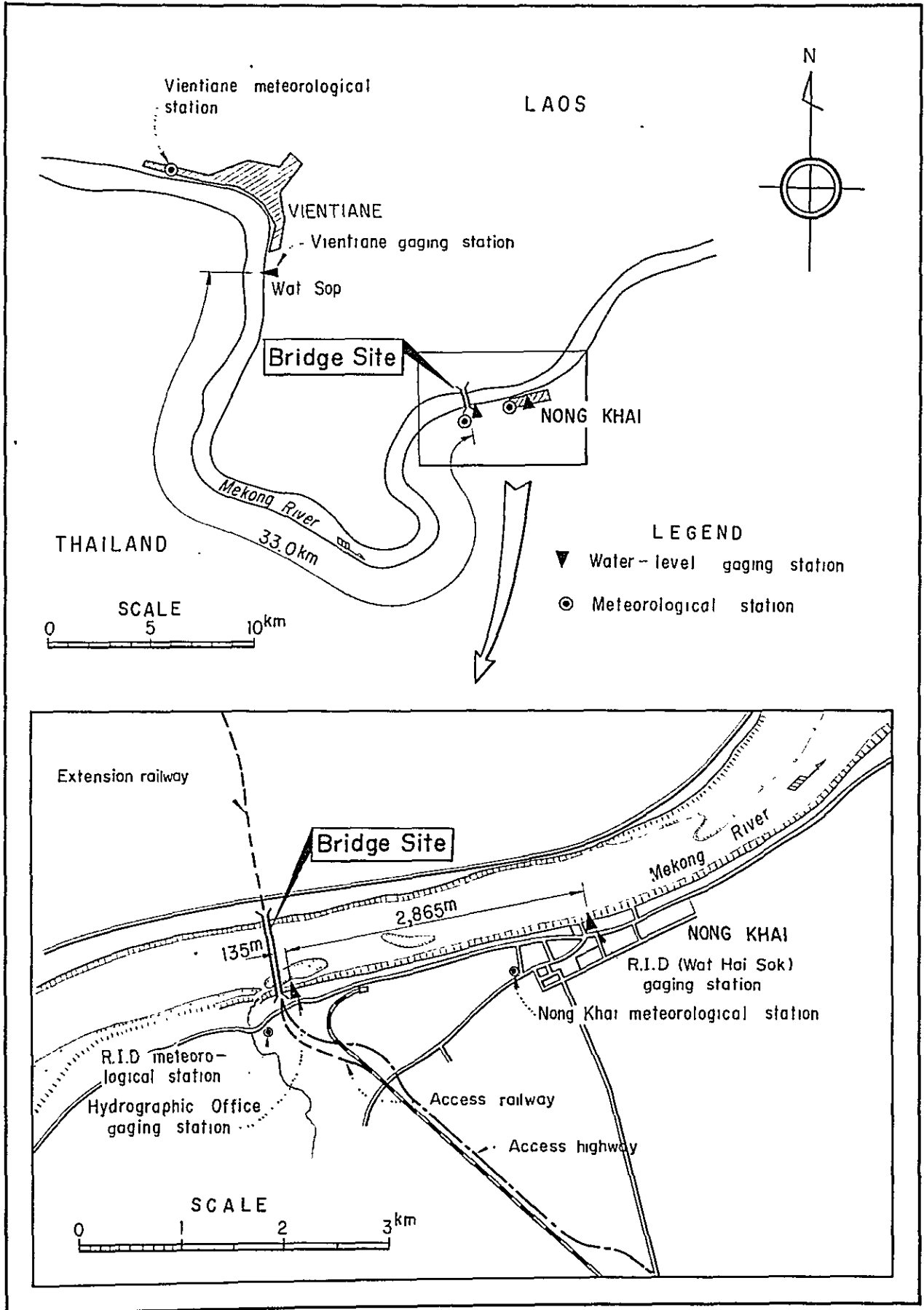
HYDROLOGY

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4.1. Hydrologic Data

LOCATION OF THE WATER-LEVEL GAGING STATIONS AND THE METEOROLOGICAL STATIONS



Hydrologic data collected in the first and second phase investigations

Data	Vientiane	Hydro. Office	Available Period			
			W.H.S. /1	B.D.K. /2	W.S.K. /3	A.T.B. /4
Water-level	Jan. '66-Apr. '68	Jun. '64-Apr. '68	Jun. '55-Mar. '68	Jan. '63-Dec. '67*	Jan. '63-Dec. '67*	Jan. '63-Dec. '67*
Water-level and discharge	Jan. '66-Mar. '67	-	-	-	-	-
Water temperature	Jan. '60-Dec. '61	-	-	-	-	-
Flow velocity	-	1966-1968	-	-	-	-
Flow velocity at the flood time	-	Sept. '66	-	-	-	-
Stage hydrograph	- *	-	-	-	-	-
Flood hydrograph	1923-1967	-	Jun. '37-Mar. '66	-	-	-

Remarks

- /1 W.H.S.: Wat Hai Sok gaging station
- /2 B.D.K.: Ban Dok Kham gaging station
- /3 W.S.K.: Wat Sri Mong Kol gaging station
- /4 A.T.B.: Ampho Tha Bo gaging station

* The data asterisked are not compiled in Appendices because they were not available for the hydrologic analysis on the Second Phase Report.

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system, Mekong

Name of stream: _____

Drainage area (Km²): _____

Year 1964

	Jan		Feb		Mar		Apr		May		June		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1													1
2													2
3													3
4													4
5													5
6													6
7													7
8													8
9													9
10													10
11											15752		11
12											15754		12
13											15771		13
14											15784		14
15											15778		15
16											15765		16
17											15750		17
18											15737		18
19											15721		19
20											15711		20
21											15718		21
22											15733		22
23											15755		23
24											15773		24
25											15793		25
26											15841		26
27											15861		27
28											15871		28
29											15886		29
30											15921		30
31													31
MAX													MAX
MIN													MIN
TOTAL													TOTAL
DAYS													DAYS
MEAN													MEAN

H : W. S. EL. in m , Q : Discharge in m³/sec.
 Zero point of water gauge : El. 154.211 m

WATER LEVEL AND DISCHARGE

STATION *Hydrographic Office*

River system, *Mekong* Name of stream: _____ Drainage area (Km²), _____ Year *1964*

	July		Aug		Sept		Oct		Nov		Dec		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	159.45		161.37		162.01		161.91		159.51		158.32		1
2	159.32		161.91		163.69		161.71		159.42		158.02		2
3	159.08		162.37		163.34		161.60		159.36		157.83		3
4	158.83		162.47		163.18		161.85		159.30		157.72		4
5	158.72		162.29		163.01		162.15		159.21		157.76		5
6	158.93		161.93		163.09		162.20		159.14		157.80		6
7	161.45		161.58		163.36		161.85		159.10		157.73		7
8	164.38		161.55		164.09		161.32		158.95		157.59		8
9	164.56		161.77		164.85		161.33		158.85		157.47		9
10	165.32		162.13		165.07		161.33		158.72		157.40		10
11	165.41		162.31		164.91		162.22		158.60		157.38		11
12	165.12		162.36		164.73		162.73		158.56		157.37		12
13	165.84		162.31		164.57		162.74		158.43		157.31		13
14	164.29		162.19		164.38		162.57		158.35		157.23		14
15	165.00		162.10		164.02		162.33		158.20		157.15		15
16	163.62		162.20		163.99		162.05		158.16		157.08		16
17	163.16		162.72		163.89		161.82		158.08		157.01		17
18	162.72		163.20		163.82		161.62		158.01		156.96		18
19	162.38		163.37		163.54		161.37		157.93		156.90		19
20	162.11		163.31		164.08		161.17		157.88		156.87		20
21	161.75		163.16		163.93		161.01		157.82		156.82		21
22	161.63		163.13		163.64		160.84		157.74		156.79		22
23	161.75		163.68		163.41		160.66		157.72		156.74		23
24	161.95		164.26		163.39		160.53		157.71		156.71		24
25	161.80		164.79		163.28		160.48		157.65		156.69		25
26	161.67		165.18		163.21		160.44		157.63		156.72		26
27	161.65		165.81		163.06		160.34		157.74		156.72		27
28	161.61		165.63		163.88		160.10		158.38		156.70		28
29	161.44		165.18		162.60		159.91		158.64		156.65		29
30	161.43		164.84		162.28		159.76		158.55		156.63		30
31	161.16		164.38				159.61				156.60		31
MAX	165.84		165.81		165.07		162.74		163.64		158.32		MAX
MIN	158.72		161.37		162.28		159.61		157.63		156.60		MIN
TOTAL	5027.53		5255.47		4912.33		5001.75		4733.36		4872.67		TOTAL
DAYS	31		31		30		31		30		31		DAYS
MEAN	162.18		163.08		163.74		161.35		158.44		157.18		MEAN

H : W. S. EL in m , Q : Discharge in m³/sec

N.K. Form No 1202

Zero point of water gauge: El. 154.211^m

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system: Mekong Name of stream: _____ Drainage area (Km²): _____ Year: 1965

	Jan		Feb		Mar		Apr		May		June		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	156.57		155.86		155.34		154.87		154.94		156.10		1
2	156.55		155.84		155.32		154.86		154.90		156.30		2
3	156.53		155.82		155.30		154.84		154.88		156.52		3
4	156.50		155.78		155.30		154.84		154.86		156.61		4
5	156.48		155.79		155.29		154.85		154.87		156.66		5
6	156.46		155.79		155.29		154.87		154.89		156.72		6
7	156.43		155.79		155.25		154.84		154.91		156.82		7
8	156.39		155.80		155.22		154.84		155.01		156.90		8
9	156.36		155.83		155.10		154.84		155.09		156.90		9
10	156.34		155.82		155.16		154.84		155.07		156.92		10
11	156.31		155.81		155.14		154.86		155.07		156.96		11
12	156.27		155.81		155.12		154.86		155.16		157.13		12
13	156.23		155.80		155.09		154.90		155.24		157.39		13
14	156.20		155.79		155.07		154.91		155.34		157.63		14
15	156.17		155.78		155.06		154.91		155.40		157.89		15
16	156.16		155.76		155.04		154.89		155.42		158.62		16
17	156.14		155.75		155.04		154.90		155.39		158.33		17
18	156.13		155.77		155.06		154.89		155.34		158.70		18
19	156.13		155.79		155.06		154.92		155.20		159.13		19
20	156.13		155.78		155.05		154.91		155.16		159.32		20
21	156.12		155.71		155.04		154.98		155.61		159.53		21
22	156.09		155.65		155.02		155.03		155.05		159.47		22
23	156.10		155.59		155.01		155.07		155.07		159.47		23
24	156.13		155.84		155.01		155.10		155.19		159.60		24
25	156.16		155.50		155.01		155.12		155.45		159.89		25
26	156.13		155.45		155.02		155.12		155.52		160.21		26
27	156.08		155.10		155.00		155.08		155.54		160.52		27
28	156.07		155.37		154.98		155.04		155.59		160.66		28
29	155.96				154.95		155.01		155.62		160.56		29
30	155.93				154.92		154.97		155.70		160.71		30
31	155.90				154.89				155.84				31
MAX	156.57		155.86		155.34		155.12		155.84		160.71		MAX
MIN	155.90		155.37		154.89		154.84		154.86		156.10		MIN
TOTAL	4843.15		4360.17		4808.15		4647.96		4812.32		4748.17		TOTAL
DAYS	31		28		31		30		31		30		DAYS
MEAN	156.23		155.72		155.10		154.93		155.24		158.27		MEAN

H : W.S. EL. in m, Q : Discharge in m³/sec.
 Zero point of water gauge: El. 154.211^m

WATER LEVEL AND DISCHARGE

STATION *Hydrographic Office*

River system, *Mekong* Name of stream: _____ Drainage area (Km²), _____ Year *1965*

	July		Aug		Sept		Oct		Nov		Dec		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	160.90		162.26		162.90		161.51		163.64		158.40		1
2	160.89		162.29		162.67		161.28		163.29		158.31		2
3	162.77		162.56		162.41		161.01		162.72		158.21		3
4	160.67		162.61		162.35		160.78		162.25		158.15		4
5	160.54		162.72		162.46		160.63		162.15		158.06		5
6	160.49		162.49		162.25		160.53		162.83		158.00		6
7	160.70		162.12		162.01		160.45		163.36		157.95		7
8	160.86		161.75		161.91		160.43		163.65		157.89		8
9	160.78		161.50		162.18		160.44		163.15		157.83		9
10	160.72		161.10		162.56		160.39		162.56		157.79		10
11	161.02		161.07		162.87		160.39		162.00		157.73		11
12	161.70		161.41		162.98		160.32		161.56		157.66		12
13	161.99		161.98		162.91		160.21		161.20		157.62		13
14	161.83		162.61		162.85		160.13		160.88		157.66		14
15	161.60		162.91		162.93		160.01		160.58		157.53		15
16	161.33		162.82		162.84		159.98		160.21		157.54		16
17	161.02		162.82		162.91		159.96		160.02		157.58		17
18	160.84		163.11		162.79		159.95		159.83		157.59		18
19	160.98		163.33		162.76		159.88		159.65		157.53		19
20	161.70		163.53		162.69		159.69		159.50		157.56		20
21	161.65		163.44		162.69		159.58		159.37		157.42		21
22	161.40		163.34		162.63		159.54		159.24		157.72		22
23	161.63		163.07		162.35		159.37		159.13		158.68		23
24	161.78		162.78		162.09		159.20		159.03		158.35		24
25	161.73		162.64		161.89		159.02		158.96		158.41		25
26	161.61		162.73		161.86		158.85		158.86		158.06		26
27	161.63		162.79		161.92		158.70		158.77		158.64		27
28	162.56		162.70		161.88		158.68		158.70		158.29		28
29	162.60		162.78		161.82		159.76		158.60		158.02		29
30	162.40		162.95		161.75		162.53		158.49		157.78		30
31	162.33		163.04				163.93				157.63		31
MAX	162.60		163.53		162.98		163.93		163.65		158.41		MAX
MIN	160.49		161.07		161.75		160.01		158.49		157.42		MIN
TOTAL	5002.73		5039.25		4873.21		4967.15		4821.39		4898.39		TOTAL
DAYS	31		31		30		31		30		31		DAYS
MEAN	161.38		162.56		162.44		160.23		160.81		158.02		MEAN

H : W S.E.L. in m , Q : Discharge in m³/sec

N.K. Form 161202

Zero point of water gauge : El. 154.211^m

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system, Mekong Name of stream: _____ Drainage area (Km²): _____ Year 1966

	Jan		Feb		Mar		Apr		May		June		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	157.46		156.35		155.73		155.20		155.33		156.77		1
2	157.38		156.37		155.71		155.20		155.30		156.89		2
3	157.29		156.43		155.69		155.19		155.37		157.05		3
4	157.21		156.46		155.67		155.18		155.23		157.13		4
5	157.06		156.45		155.65		155.15		155.20		157.13		5
6	157.11		156.38		155.63		155.12		155.18		157.03		6
7	157.06		156.33		155.61		155.11		155.22		157.13		7
8	157.02		156.26		155.58		155.11		155.19		157.08		8
9	156.98		156.21		155.54		155.17		155.19		157.14		9
10	156.94		156.17		155.51		155.26		155.21		157.24		10
11	156.91		156.13		155.48		155.30		155.23		157.42		11
12	156.86		156.00		155.46		155.33		155.28		157.67		12
13	156.84		156.06		155.46		155.34		155.35		157.81		13
14	156.82		156.04		155.47		155.35		155.43		157.91		14
15	156.79		156.01		155.46		155.34		155.56		158.02		15
16	156.75		155.99		155.44		155.34		155.67		158.15		16
17	156.72		155.96		155.45		155.29		155.76		158.31		17
18	156.67		155.93		155.43		155.27		155.77		158.48		18
19	156.63		155.89		155.42		155.28		155.79		158.74		19
20	156.60		155.86		155.41		155.37		155.81		158.02		20
21	156.60		155.84		155.43		155.41		155.86		159.55		21
22	156.52		155.82		155.43		155.46		156.03		159.93		22
23	156.49		155.81		155.41		155.35		156.47		160.12		23
24	156.46		155.79		155.34		155.32		157.03		160.18		24
25	156.44		155.77		155.30		155.30		157.10		160.36		25
26	156.42		155.76		155.27		155.38		156.98		160.77		26
27	156.38		155.75		155.20		155.33		157.00		161.02		27
28	156.35		155.75		155.19		155.39		157.04		161.31		28
29	156.34				155.20		155.39		157.13		161.64		29
30	156.35				155.19		155.36		157.06		161.63		30
31	156.35				155.17				156.87				31
MAX	157.46		156.46		155.73		155.46		157.13		161.64		MAX
MIN	156.34		155.75		155.17		155.11		155.18		156.77		MIN
TOTAL	4839.02		4369.57		4818.97		4658.59		4832.64		4758.63		TOTAL
DAYS	31		28		31		30		31		30		DAYS
MEAN	156.77		156.06		155.45		155.28		155.89		158.62		MEAN

H : W.S. EL. in m, Q : Discharge in m³/sec.
 Zero point of water gauge : El. 154.211^m

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system Mekong Name of stream: _____ Drainage area (Km²): _____ Year 1966

	July		Aug		Sept		Oct		Nov		Dec		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	161.40		162.75		167.43		162.29		160.42		157.85		1
2	160.99		163.37		167.52		162.03		160.25		157.79		2
3	160.60		164.01		167.72		161.79		160.45		157.71		3
4	160.27		164.28		167.97		161.88		160.49		157.65		4
5	160.00		164.42		168.11		161.76		160.33		157.60		5
6	160.26		165.03		168.23		161.82		160.06		157.54		6
7	161.80		165.50		168.29		162.08		159.80		157.48		7
8	162.52		165.46		168.36		162.62		159.61		157.44		8
9	162.60		165.30		168.38		163.20		159.43		157.41		9
10	162.58		165.15		168.39		163.27		159.29		157.42		10
11	162.29		165.08		168.35		162.96		159.15		157.43		11
12	161.97		164.97		168.23		162.54		159.03		157.43		12
13	161.76		164.93		168.07		162.12		158.91		157.37		13
14	161.49		164.97		167.82		161.76		158.81		157.29		14
15	161.20		164.99		167.51		161.46		158.73		157.22		15
16	161.16		165.05		167.13		161.20		158.64		157.13		16
17	161.37		165.26		166.75		161.01		158.57		157.07		17
18	161.86		165.44		166.51		160.91		158.51		157.02		18
19	162.58		165.48		166.26		160.97		158.44		156.97		19
20	163.07		165.34		165.92		161.22		158.38		156.93		20
21	163.13		165.12		165.57		161.51		158.32		156.92		21
22	163.20		165.02		165.19		161.71		158.23		156.91		22
23	163.16		165.24		164.74		161.61		158.16		156.92		23
24	163.01		165.86		164.27		161.34		158.09		156.93		24
25	162.78		166.04		163.90		161.07		158.05		156.89		25
26	162.60		166.16		163.67		160.89		158.01		156.83		26
27	162.31		166.40		163.51		160.65		157.98		156.76		27
28	162.07		166.65		163.23		160.47		157.94		156.69		28
29	162.74		166.89		162.90		160.67		157.94		156.64		29
30	161.45		167.13		162.55		160.53		157.91		156.59		30
31	161.83		167.33				160.53				156.55		31
MAX	163.16		167.33		168.39		163.27		160.49		157.85		MAX
MIN	161.40		162.75		162.55		160.47		157.91		156.55		MIN
TOTAL	5020.85		5124.62		4992.51		5009.69		4767.84		4872.38		TOTAL
DAYS	31		31		30		31		30		31		DAYS
MEAN	161.94		165.31		166.42		161.60		158.92		157.17		MEAN

H : W. S. EL. in m, Q: Discharge in m³/sec

N.K. Form No 1202

Zero point of water gauge: El. 154.211 m

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system: Mekong Name of stream: _____ Drainage area (Km²): _____ Year 1967

	Jan		Feb		Mar		Apr		May		June		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	156.51		155.87		155.31		155.10		155.76		156.01		1
2	156.48		155.83		155.28		155.06		155.76		155.98		2
3	156.44		155.80		155.25		155.02		155.82		155.90		3
4	156.41		155.76		155.23		154.98		155.82		155.84		4
5	156.36		155.74		155.24		154.95		155.78		155.84		5
6	156.32		155.72		155.29		154.94		155.71		155.91		6
7	156.29		155.69		155.33		154.93		155.61		156.00		7
8	156.26		155.66		155.32		154.95		155.54		156.07		8
9	156.24		155.66		155.29		155.00		155.49		156.30		9
10	156.20		155.66		155.24		155.03		155.49		156.67		10
11	156.18		155.65		155.20		154.98		155.61		157.08		11
12	156.18		155.67		155.18		154.95		155.68		157.75		12
13	156.18		155.71		155.17		154.92		155.71		158.38		13
14	156.20		155.70		155.16		154.93		155.77		158.25		14
15	156.23		155.68		155.19		154.96		155.81		158.16		15
16	156.21		155.64		155.25		155.01		155.85		157.94		16
17	156.17		155.60		155.30		155.08		155.87		157.72		17
18	156.18		155.56		155.34		155.12		155.83		157.52		18
19	156.21		155.53		155.38		155.20		155.82		157.35		19
20	156.27		155.40		155.40		155.30		155.86		157.27		20
21	156.29		155.48		155.39		155.39		155.83		157.24		21
22	156.27		155.46		155.32		155.46		155.80		157.26		22
23	156.26		155.43		155.26		155.51		155.70		157.31		23
24	156.23		155.41		155.19		155.57		155.56		157.45		24
25	156.16		155.40		155.16		155.64		155.48		157.28		25
26	156.09		155.38		155.13		155.67		155.45		157.14		26
27	156.05		155.36		155.08		155.70		155.46		157.00		27
28	156.00		155.34		155.06		155.74		155.52		156.97		28
29	155.96				155.07		155.77		155.65		156.99		29
30	155.94				155.08		155.79		155.81		157.06		30
31	155.91				155.10				155.94				31
MAX	156.51		155.87		155.40		155.79		155.94		158.38		MAX
MIN	155.91		155.34		155.06		154.92		155.45		155.84		MIN
TOTAL	4842.68		4356.79		4812.19		4656.65		4826.99		4709.74		TOTAL
DAYS	31		28		31		30		31		30		DAYS
MEAN	156.22		155.60		155.23		155.27		155.71		156.99		MEAN

H : W. S. EL. in m, Q: Discharge in m³/sec.
 Zero point of water gauge: El. 154 211^m

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system: Mekong Name of stream: _____ Drainage area (Km²): _____ Year 1967

	July		Aug		Sept		Oct		Nov		Dec		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	157.23		162.74		161.08		163.03		158.64		158.32		1
2	157.41		162.53		160.93		162.91		158.68		158.12		2
3	157.52		162.25		160.77		163.03		158.65		157.92		3
4	157.70		161.86		160.62		163.01		158.57		157.78		4
5	157.85		161.45		160.51		162.81		158.45		157.65		5
6	157.78		161.02		160.41		162.57		158.35		157.58		6
7	157.67		160.62		160.40		162.31		158.23		157.52		7
8	157.69		160.25		160.43		162.08		158.18		157.46		8
9	157.69		159.90		160.49		161.84		158.13		157.38		9
10	157.71		159.71		160.72		161.61		158.14		157.39		10
11	157.79		159.74		161.03		161.37		158.10		157.22		11
12	157.79		160.14		161.04		161.12		158.16		157.15		12
13	157.70		160.83		161.33		160.84		157.96		157.08		13
14	157.66		161.50		161.63		160.81		157.84		157.01		14
15	157.66		162.00		161.98		160.30		157.78		156.96		15
16	157.73		161.85		162.72		160.07		157.74		156.92		16
17	158.19		161.60		162.48		159.90		157.69		156.90		17
18	158.73		161.53		163.90		159.78		157.65		156.88		18
19	158.96		161.87		163.88		159.73		157.76		156.88		19
20	158.96		162.33		163.45		159.78		158.34		156.93		20
21	158.35		162.85		163.00		160.01		158.84		157.00		21
22	160.37		163.73		162.75		160.28		159.01		157.00		22
23	160.79		164.05		162.65		160.21		159.89		156.92		23
24	160.63		164.09		162.63		159.85		158.77		156.84		24
25	160.31		165.00		162.99		159.51		158.79		156.77		25
26	159.97		164.65		163.48		159.22		158.73		156.71		26
27	158.74		163.10		163.74		159.60		158.56		156.65		27
28	159.70		162.57		163.68		158.81		158.34		156.61		28
29	161.66		162.13		163.69		158.69		158.23		156.57		29
30	162.86		161.70		163.38		158.63		158.43		156.52		30
31	162.98		161.31				158.62				156.48		31
MAX	162.98		165.00		163.90		163.03		159.01		158.32		MAX
MIN	157.23		159.71		160.40		158.62		157.65		156.48		MIN
TOTAL	4925.78		5020.90		4962.78		4982.39		4750.63		4871.11		TOTAL
DAYS	31		31		30		31		30		31		DAYS
MEAN	158.90		161.96		162.09		160.72		158.35		157.13		MEAN

H : W.S.EL. in m , Q : Discharge in m³/sec

N.K. Form 161202

Zero point of water gauge : El. 154.211 m

WATER LEVEL AND DISCHARGE

STATION Hydrographic Office

River system: Mekong

Name of stream: _____

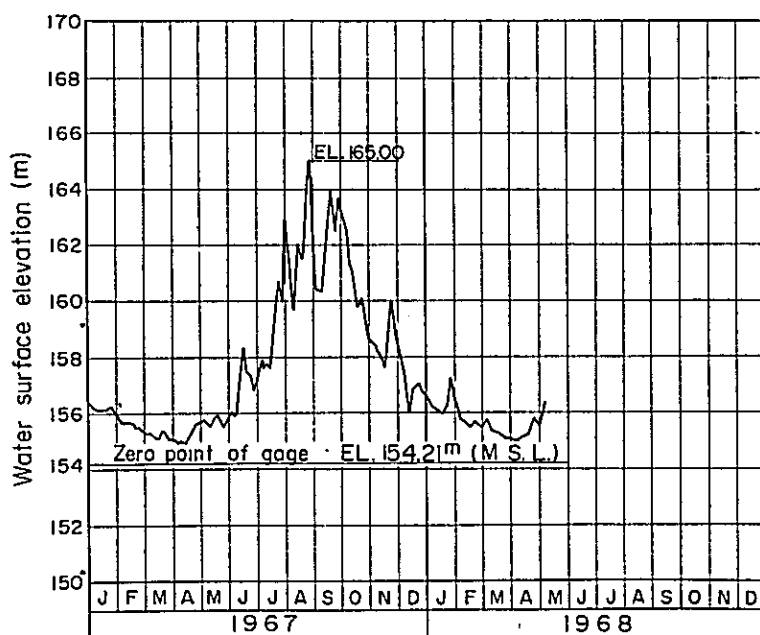
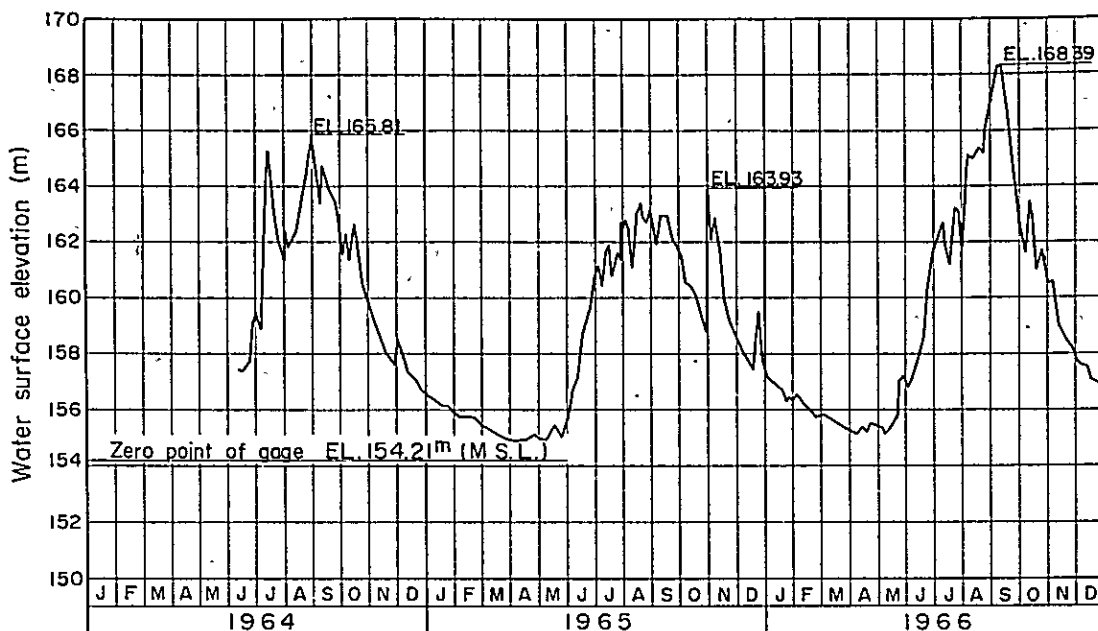
Drainage area (Km²): _____

Year 1968

	Jan		Feb		Mar		Apr		May		June		
	H	Q	H	Q	H	Q	H	Q	H	Q	H	Q	
1	156.44		156.09		155.49		155.02		155.65				1
2	156.41		155.99		155.55		155.02		155.81				2
3	156.38		155.94		155.65		155.01		156.00				3
4	156.34		155.88		155.72		155.01		156.14				4
5	156.31		155.86		155.75		155.03		156.26				5
6	156.28		155.82		155.70		155.03						6
7	156.25		155.79		155.60		155.01						7
8	156.23		155.75		155.51		155.00						8
9	156.21		155.72		155.42		155.02						9
10	156.20		155.69		155.40		155.06						10
11	156.17		155.65		155.37		155.07						11
12	156.16		155.62		155.38		155.06						12
13	156.13		155.59		155.39		155.07						13
14	156.11		155.56		155.38		155.08						14
15	156.08		155.55		155.33		155.16						15
16	156.07		155.53		155.29		155.22						16
17	156.03		155.52		155.27		155.22						17
18	156.01		155.51		155.28		155.20						18
19	156.00		155.50		155.22		155.23						19
20	156.00		155.66		155.20		155.30						20
21	156.02		155.65		155.17		155.42						21
22	156.08		155.67		155.14		155.56						22
23	156.05		155.70		155.11		155.65						23
24	156.31		155.65		155.05		155.80						24
25	156.58		155.60		155.03		155.81						25
26	157.04		155.53		155.07		155.72						26
27	157.06		154.48		155.04		155.61						27
28	156.79		155.45		155.03		155.55						28
29	156.54		155.45		155.02		155.50						29
30	156.34				155.05		155.57						30
31	156.20				155.04								31
MAX	157.06		156.09		155.75		155.81						MAX
MIN	156.00		155.45		155.02		155.00						MIN
TOTAL	4844.82		4573.41		4814.45		4658.04						TOTAL
DAYS	31		29		31		30						DAYS
MEAN	156.28		155.64		155.31		155.27						MEAN

H : W. S. E. L. in m, Q : Discharge in m³/sec,
 Zero point of water gauge : El. 154.211^m

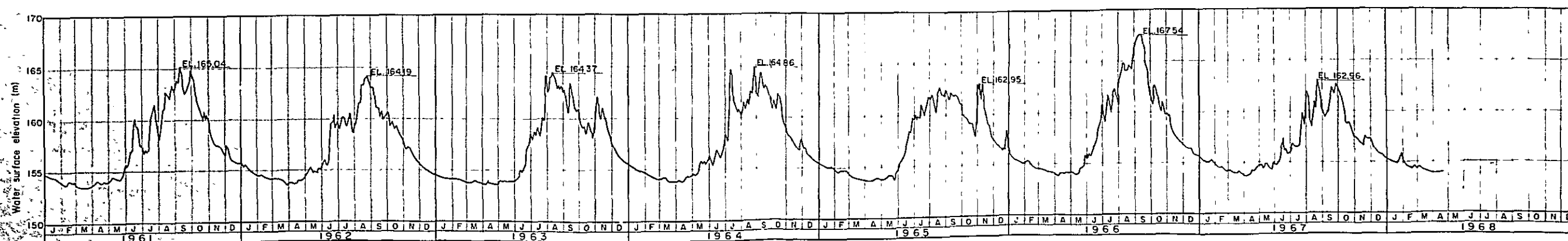
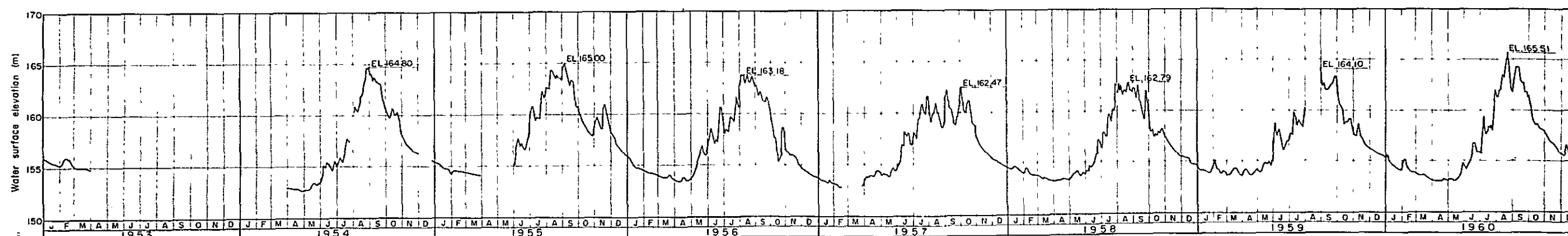
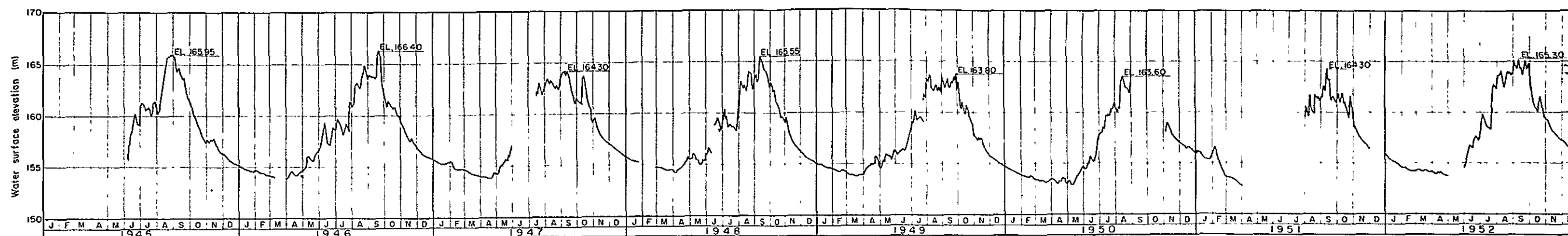
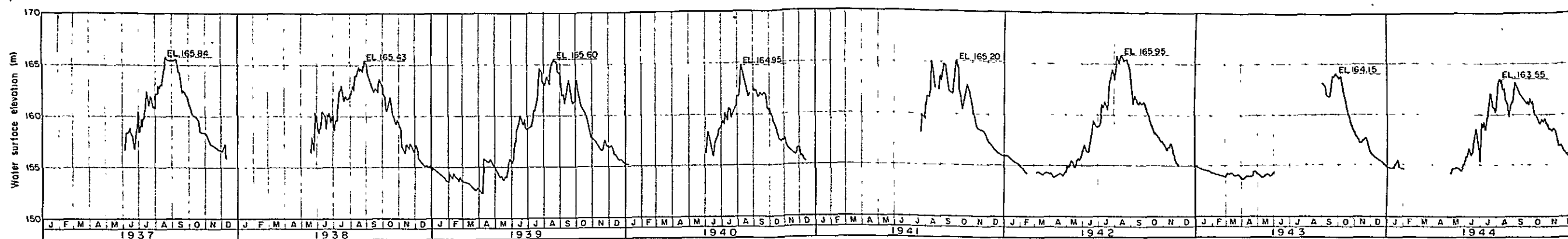
WATER-LEVEL OF THE MEKONG AT HYDROGRAPHIC OFFICE G. S.



Remarks :

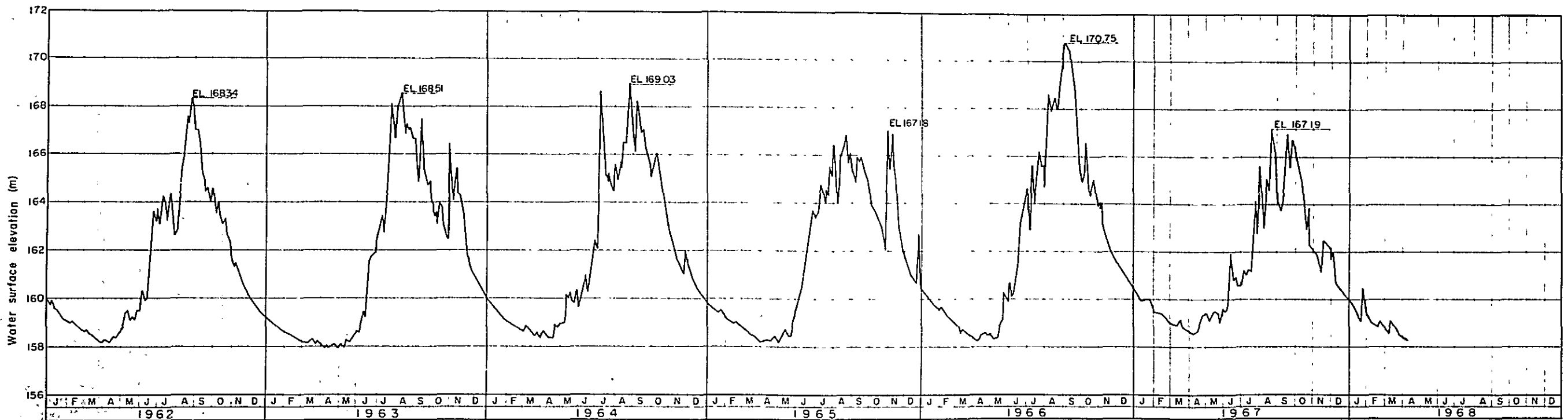
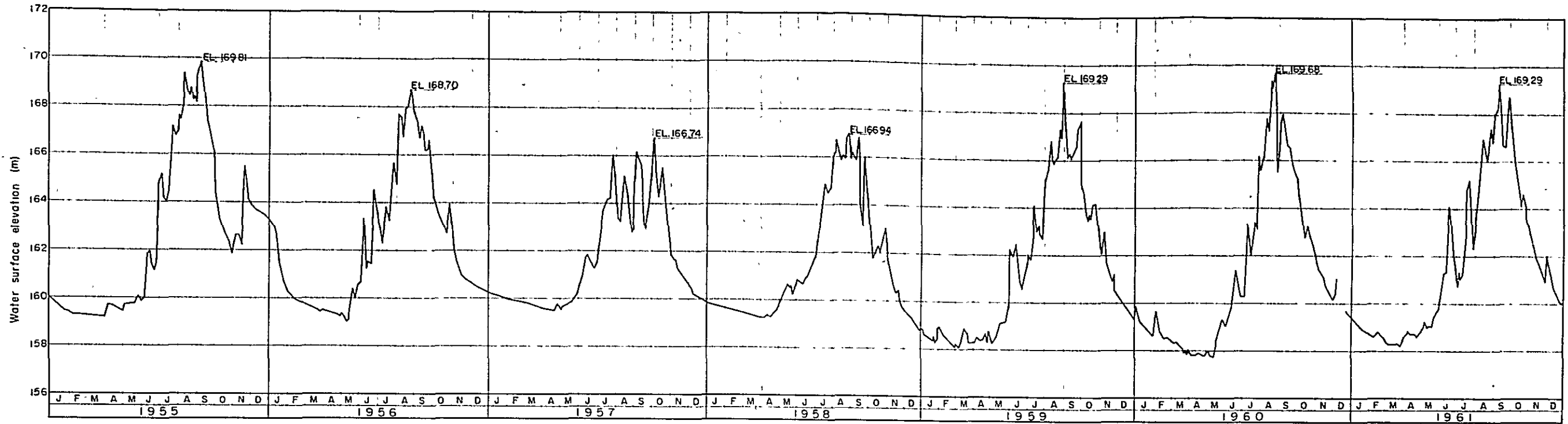
- 1) Gage was installed on June 11, '64 and data were taken ever since.
- 2) Figures given here show daily mean value of three readings a day taken at 6:00 ,12:00 and 18.00.

WATER LEVEL OF THE MEKONG AT R.I.D. (WAT HAI SOK) GAGING STATION



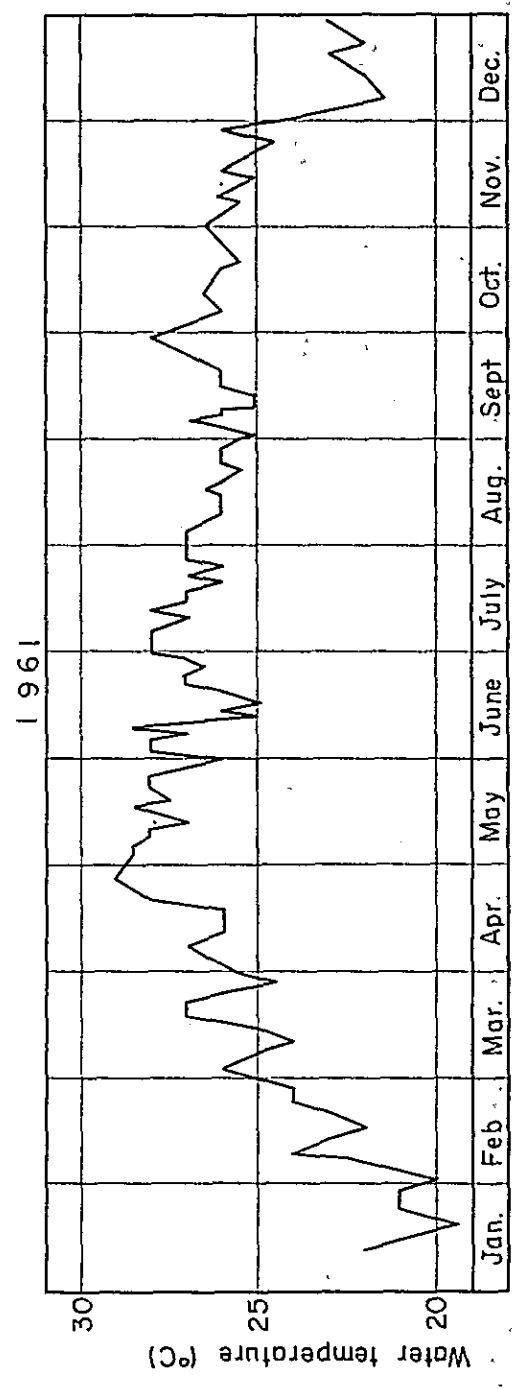
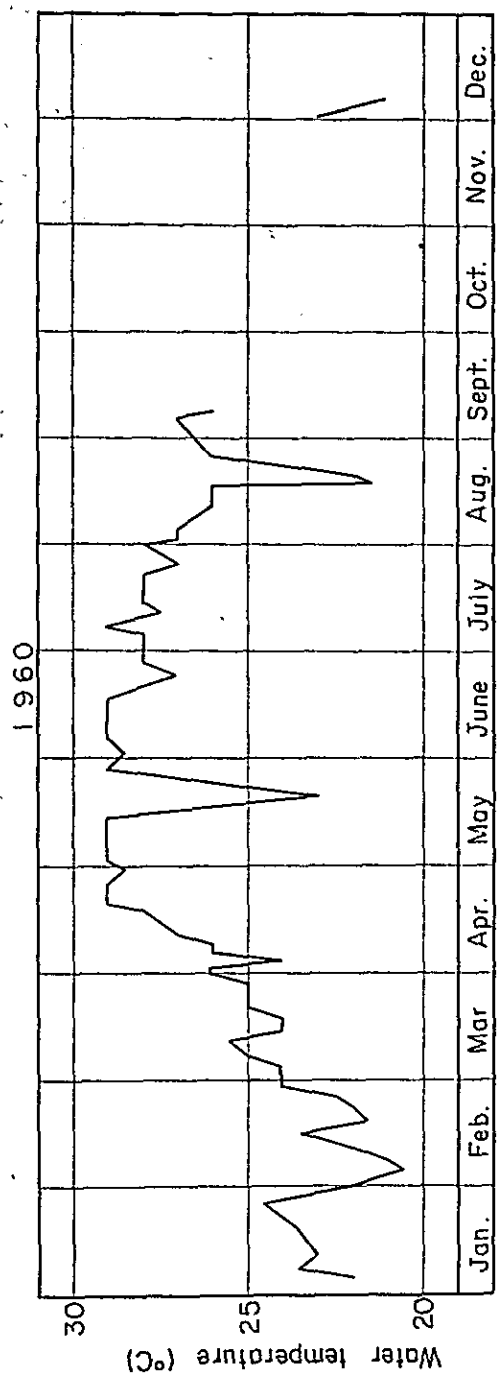
Remarks: Zero point of present staff gage, EL. 153.000

WATER-LEVEL OF THE MEKONG AT VIENTIANE (WAT SOP) GAGING STATION

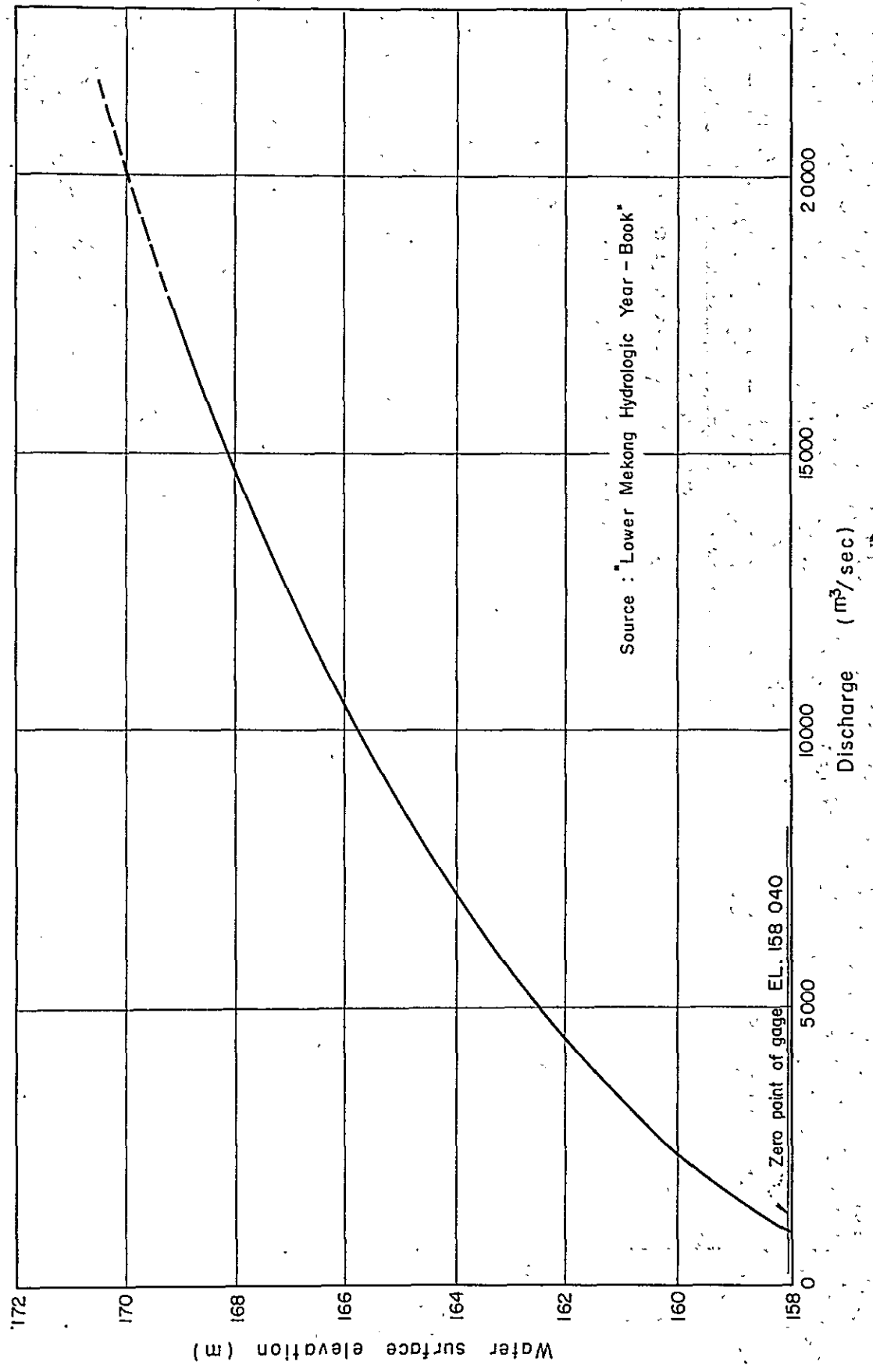


Remarks: Zero point of gage EL. 158.040 m

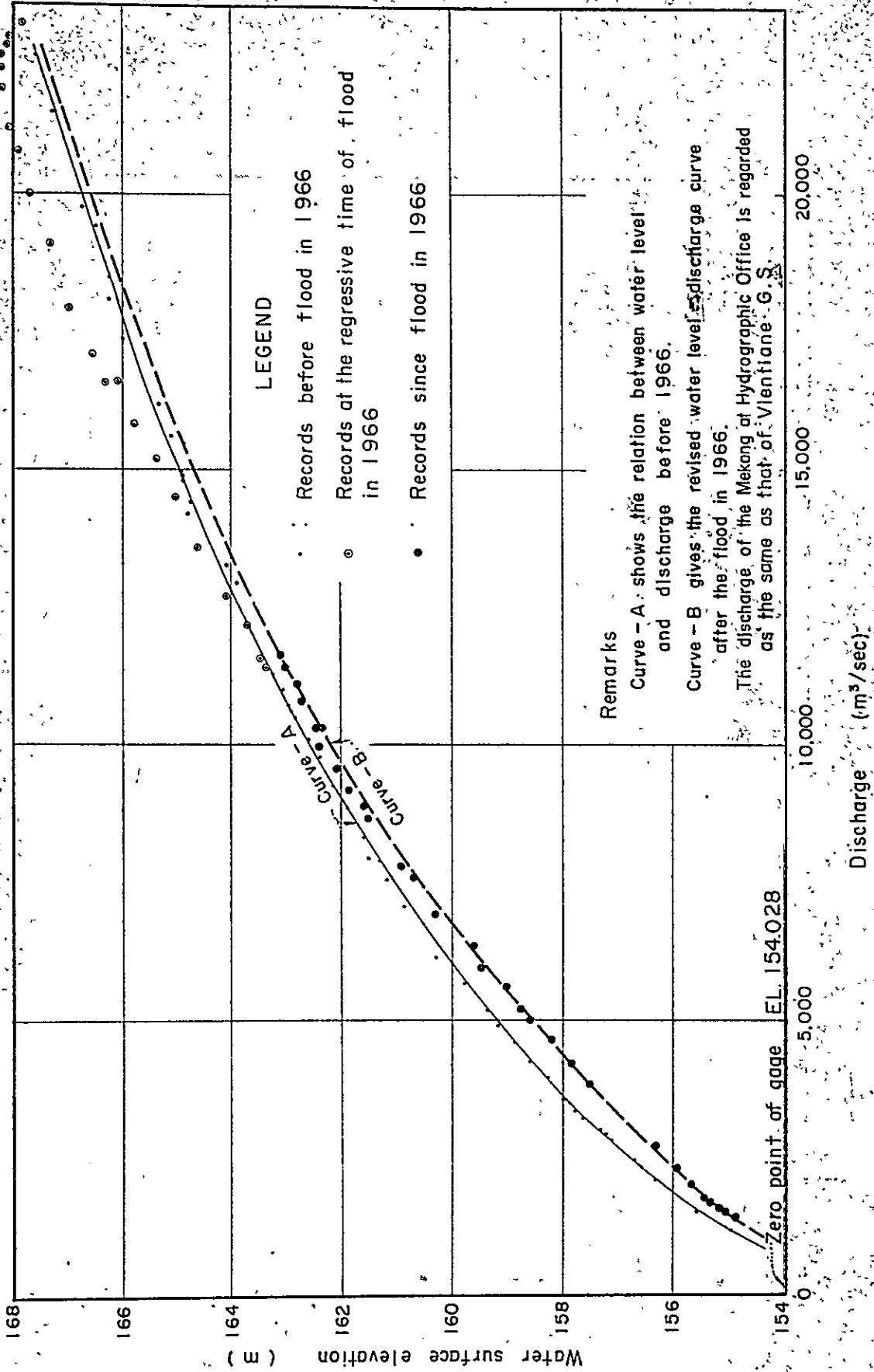
WATER TEMPERATURE OF THE MEKONG
NEAR VIENTIANE



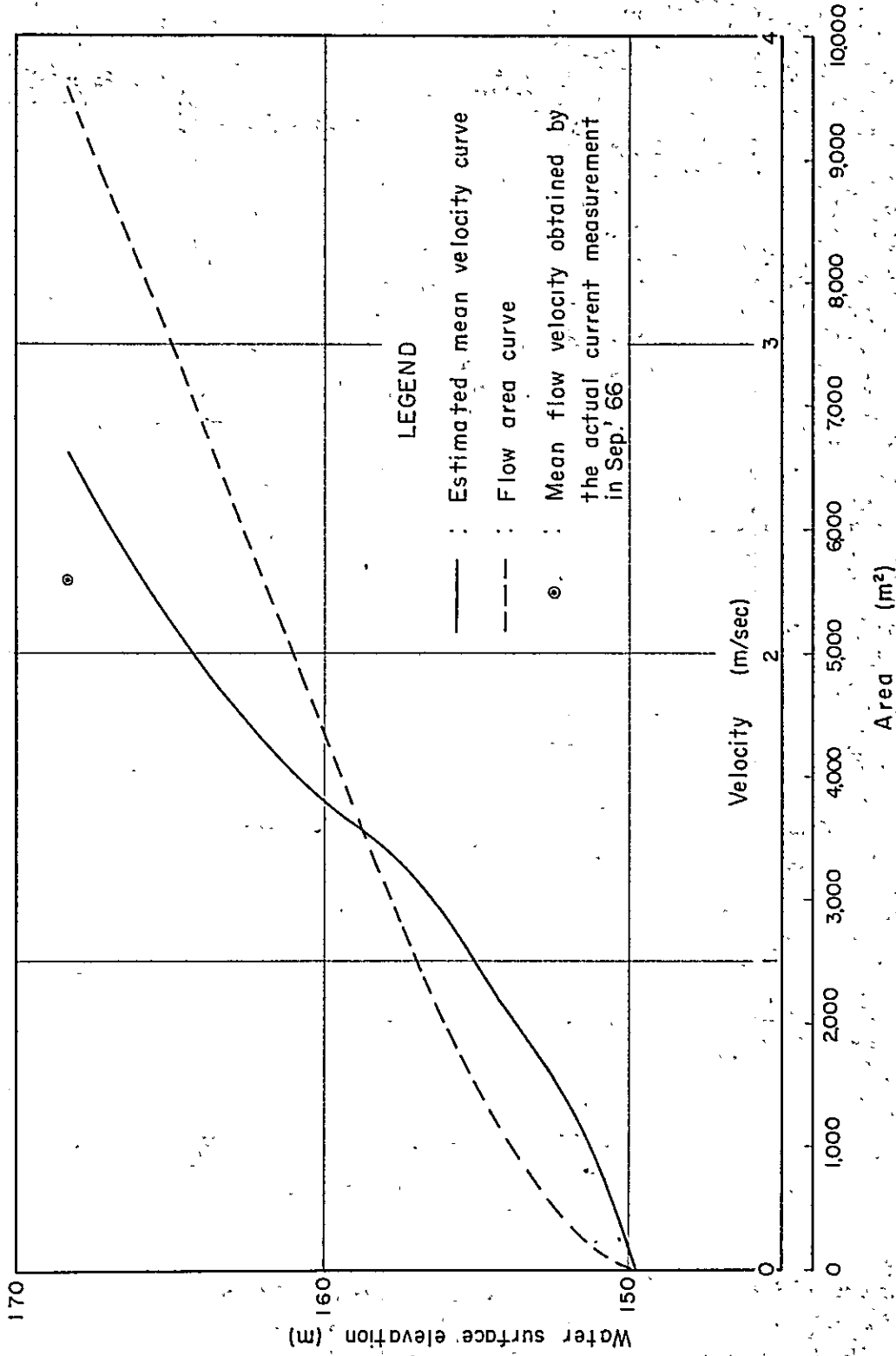
WATER LEVEL - DISCHARGE CURVE AT VIENTIANE (WAT SOP) G.S.



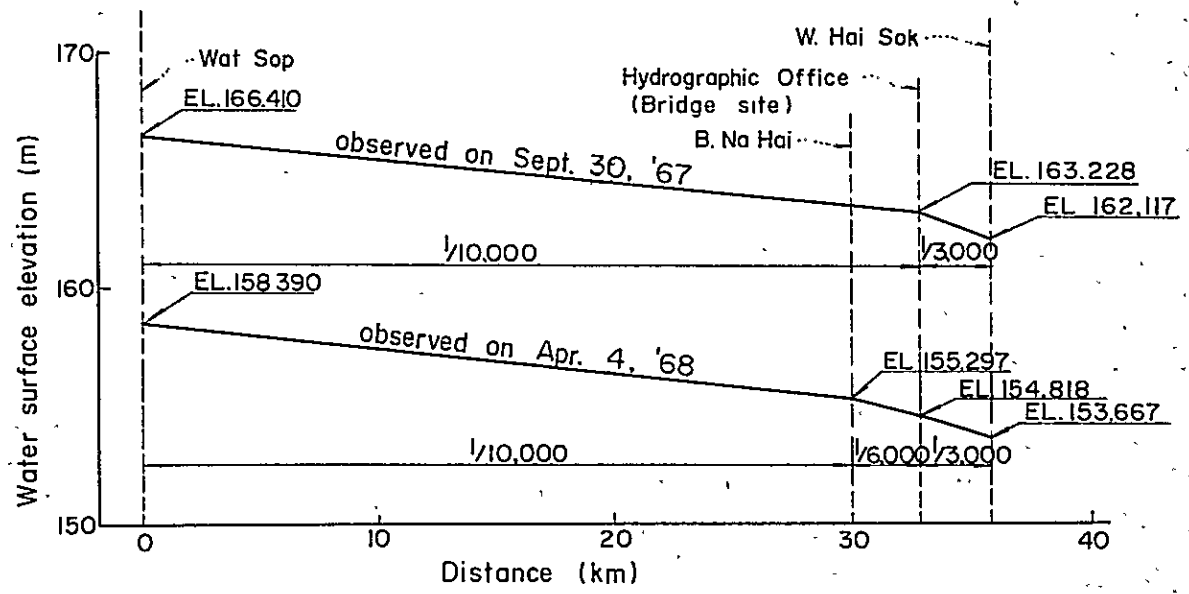
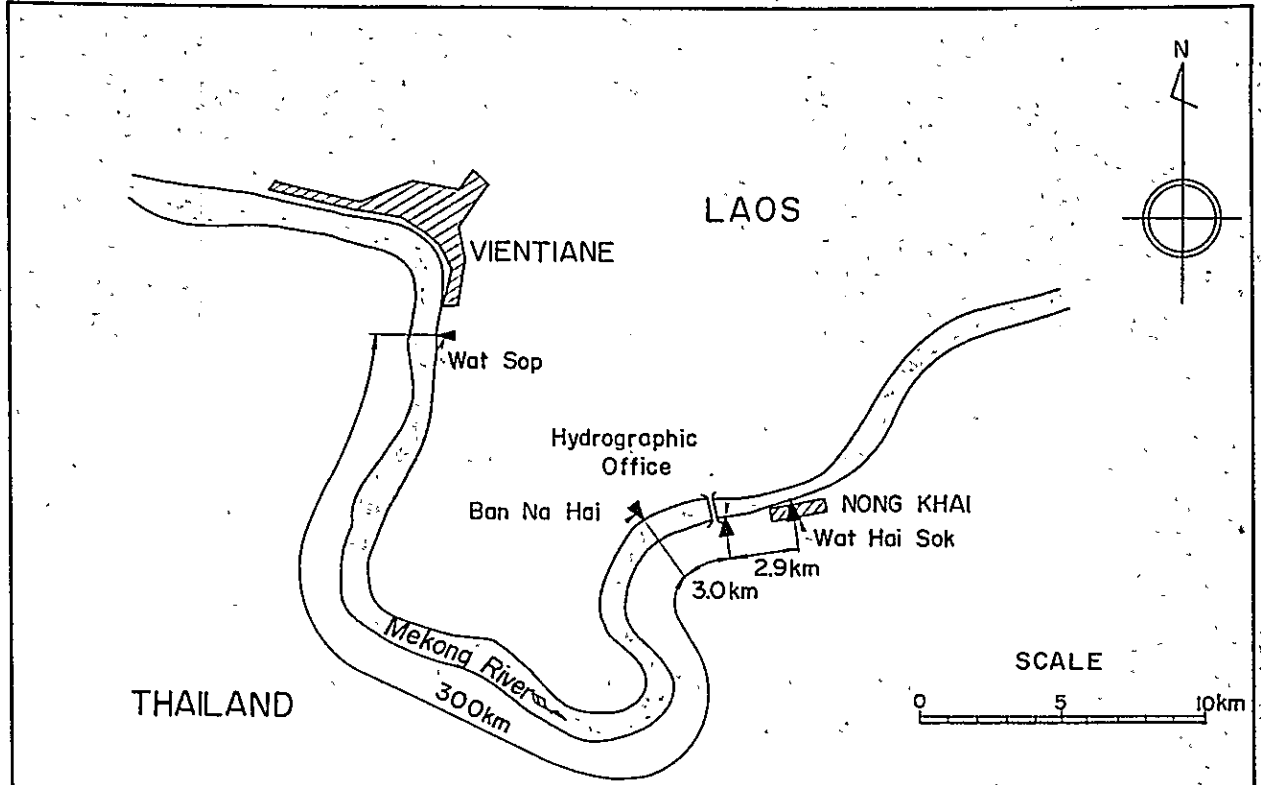
WATER LEVEL-DISCHARGE CURVES AT HYDROGRAPHIC OFFICE G. S.



FLOW AREA AND MEAN FLOW VELOCITY
OF THE MEKONG AT THE BRIDGE SITE



OBSERVATION OF WATER SURFACE GRADE



Remarks:

In both the first and second phase investigations, water stages were observed simultaneously at different places up- and downstream from the bridge site.

Based on the results of survey and the Manning's formula, the roughness coefficients "n" of the Mekong at the immediately up and downstream reaches of the bridge site were estimated at 0.032 and 0.046 in the dry season and 0.025 and 0.049 in the rainy season respectively.

4.2. Probability Calculation

This probability calculation is to estimate probable high-water levels at the bridge site of the Mekong, for the purpose of determining the Design High-water Level for the bridge planning.

Probability Calculation

1. General

The Plan of Operation provides that the Nong Khai/Vientiane Bridge is required to have a vertical clearance of 10 meters above preponderant high-water level for the sake of navigation on the Mekong. It suggests that the bottom surface of the bridge girder at its both ends, under which a course for navigation would be provided, is 10 meters above preponderant high-water level. This kind of preponderant high-water level is one of the planning criteria for bridge. In this sense, the preponderant high-water level is called design high-water level hereinafter.

The design high-water level is decided by means of the probability calculation referring to the water-level records of the Mekong in the past years.

2. Condition

There are two water-level gaging stations near the bridge site, one belongs to the Hydrographic Office and is located 135 meters downstream of the bridge site, and the other is R.I.D.'s gaging station at Wat Hai Sok located 3,000 meters downstream of the bridge site.

The gaging station of the Hydrographic Office was established in June 1964 and R.I.D.'s gaging station was established in 1937. R.I.D.'s gaging station has therefore been keeping the water-level records for many years. For this reason, the design high-water level is estimated according to the following procedure.

- (1) To calculate probable high-water levels from the past water-level records at R.I.D.'s gaging station.
- (2) To estimate the relationship between the water-levels of both the gaging stations.
- (3) To assume from this relationship the probable high-water levels at the Hydrographic Office.

- (4) Finally, to estimate the probable high-water levels at the bridge site from the probable high-water levels at the Hydrographic Office, based upon the fact that the water surface slope of the Mekong ranges from 1/3,000 to 1/4,000 both in dry and rainy seasons.

The water-level records of R.I.D.'s gaging station are available during the period from 1937 to 1967, excluding four years; 1943, 1950, 1953 and 1959. In these four years, the records are not available.

The highest high-water level of each year is taken, and the calculation of the probable high-water levels is made in accordance with the order-statistic method on the assumption that the samples would describe the log-normal distribution.

The followings are calculating formulas:-

$$\xi = \alpha (\log x - \log x_0)$$

$$\text{or } \log x = \frac{1}{\alpha} \xi + \log x_0$$

where, x : Probable high-water level
 x_0 : Geometric mean of the annual highest high-water levels
 ξ : Normal variable for an arbitrary year

The normal variable is calculated from the following formula.

$$\phi_0 (\xi) = \frac{1}{\sqrt{\pi}} \int_{-\infty}^{\xi} e^{-\xi^2} d\xi = 1 - W_0 (\xi)$$

where, ϕ_0 : Non-exceeding probability (Gaussian distribution function)
 W_0 : Exceeding probability

The value α is a parameter which indicates the degree of dispersion and is calculated from the following formula.

$$\frac{1}{\alpha} = \frac{\sigma_{\log x}}{\sigma_{\xi}}$$

$$\sigma_{\log x} = \sqrt{\frac{1}{n} \sum (\log x_i)^2 - (\log x_o)^2}$$

Where, x_i : Observation value
 n : Number of observation
 σ_{ξ} : Product rate of probability of observation values

3. Probability calculation

(1) Probable high-water levels at R.I.D.'s gaging station

The basic calculation given in Table A.4.2.2. is made from the values of observation, the highest high-water levels of each year at R.I.D.'s gaging station.

$$\sigma_{\log x} = \sqrt{\frac{1}{27} \times 12.3261 - 0.66524^2} = 0.1182$$

$\sigma_{\xi} = 0.6908$ in case of $n = 27$ according to Hazen's method

$$\frac{1}{\alpha} = \frac{0.1182}{0.6908} = 0.1711$$

Consequently,

$$\log x = 0.1711 \xi + 0.66524$$

From this equation, the probable high-water levels in eight recurrence years are given in the following table, and Fig.4.2.1.

Table A.4.2.1. Probable high-water levels in R.I.D.'s gaging station

Recurrence year	ξ	$\log x$	x	W.L.
2	0	0.66524	4.63	164.63
5	0.5951	0.76707	5.85	165.85
10	0.9062	0.82030	6.61	166.61
20	1.1630	0.86426	7.32	167.32
40	1.3859	0.90237	7.99	167.99
50	1.4520	0.91372	8.20	168.20
100	1.6450	0.94670	8.85	168.85
200	1.8215	0.97690	9.48	169.48

Table A.4.2.2 Basic calculation

i	Year	H.W.L.	x_i	$\log x_i$	$(\log x_i)^2$
1	1966	167.58	7.58	0.87967	0.77382
2	1946	166.40	6.40	0.80618	0.64993
3	1942	165.95	5.95	0.77452	0.59988
4	1945	165.95	5.95	0.77452	0.59988
5	1937	165.84	5.84	0.76641	0.58737
6	1960	165.72	5.72	0.75740	0.57365
7	1939	165.60	5.60	0.74819	0.55979
8	1948	165.55	5.55	0.74429	0.55397
9	1938	165.43	5.43	0.73480	0.53993
10	1952	165.30	5.30	0.72428	0.55128
11	1941	165.20	5.20	0.71600	0.51266
12	1949	165.08	5.08	0.70586	0.49823
13	1955	165.02	5.02	0.70070	0.49098
14	1940	164.95	4.95	0.69461	0.48248
15	1964	164.90	4.90	0.69020	0.47638
16	1954	164.80	4.80	0.68124	0.46409
17	1963	164.39	4.39	0.64246	0.41275
18	1947	164.30	4.30	0.63347	0.40128
19	1951	164.30	4.30	0.63347	0.40128
20	1962	164.21	4.21	0.62428	0.38972
21	1949	163.80	3.80	0.57978	0.33614
22	1956	163.77	3.77	0.57634	0.33217
23	1944	163.55	3.55	0.55023	0.30275
24	1958	163.04	3.04	0.48283	0.23312
25	1965	162.98	2.98	0.47422	0.22488
26	1967	162.97	2.97	0.47276	0.22350
27	1957	162.47	2.47	0.39270	0.15421
Total				17.96141	12.32613

$$\log x_0 = \frac{1}{n} \sum \log x_i = \frac{1}{27} \times 17.96141 = 0.66524$$

(2) Probable high-water levels at the Hydrographic Office

The relationship between R.I.D.'s gaging station and the Hydrographic Office concerning the water-levels recorded on same days during the period of June 1964 to April 1968, is given in Fig. 4.2.2.

The distance between these two gaging stations is only 2,865 meters, and there seems not to be the increase or decrease of discharge. Based upon the above relationship, the probable high-water levels at the Hydrographic Office can be obtained from those of R.I.D. gaging station in Table A.4.2.1. The figures obtained are shown in Table A.4.2.3.

(3) Probable high-water levels at the bridge site

The bridge site is located 135 meters upstream of the Hydrographic Office. Assuming that the water surface slope of the Mekong is 1/3,000 to 1/4,000, the water level at the bridge site will be 4 cm higher than that at the Hydrographic Office.

The probable high-water levels at the bridge site are given in Table A.4.2.3.

Table A.4.2.3 Probable high-water levels at the Hydrographic Office and the bridge site

Recurrence Years	R.I.D.	Hydrographic Office	Bridge site
2	164.63	165.49	165.53
5	165.85	166.68	166.72
10	166.61	167.41	167.45
20	167.32	168.10	168.14
40	167.99	168.74	168.78
50	168.20	168.95	168.99
100	168.85	169.60	169.64
200	169.48	170.20	170.24

Remarks: The figures are based on the elevations of zero point of the staff gage given at each gaging station.

(4) Conclusion

Based on the water-level records of R.I.D.'s gaging station, the number of days per year and the maximum duration in days in which the past water levels were above the probable high-water levels given in Table A.4.2.3, are studied for the determination of the design high-water level.

Table A.4.2.4. The number of days per year and the maximum duration in days

Recurrence Year	2	5	10	20	50	100	200
Probable H.W.L.	164.63	165.85	166.61	167.32	168.20	168.85	169.48
Calendar year							
1937	25	0	0	0	0	0	0
1938	11	0	0	0	0	0	0
1939	14	0	0	0	0	0	0
1940	2	0	0	0	0	0	0
1941	12	0	0	0	0	0	0
1942	25	3	0	0	0	0	0
1943	-	-	-	-	-	-	-
1944	0	0	0	0	0	0	0
1945	21	4	0	0	0	0	0
1946	10	5	0	0	0	0	0
1947	0	0	0	0	0	0	0
1948	9	0	0	0	0	0	0
1949	0	0	0	0	0	0	0
1950	-	-	-	-	-	-	-
1951	0	0	0	0	0	0	0
1952	11	0	0	0	0	0	0
1953	-	-	-	-	-	-	-
1954	4	0	0	0	0	0	0
1955	6	0	0	0	0	0	0
1956	0	0	0	0	0	0	0
1957	0	0	0	0	0	0	0
1958	0	0	0	0	0	0	0
1959	-	-	-	-	-	-	-
1960	6	0	0	0	0	0	0
1961	4	0	0	0	0	0	0
1962	0	0	0	0	0	0	0
1963	0	0	0	0	0	0	0
1964	3	0	0	0	0	0	0
1965	0	0	0	0	0	0	0
1966	31	20	14	7	0	0	0
1967	0	0	0	0	0	0	0
Total	194	32	14	7	0	0	0
Ratio	1/50	1/310	1/700	1/1400	0	0	0
Max. duration in days	29	20	14	7	0	0	0

According to the results given in Table A.4.2.4., the past water levels remained one day in 50 days above the probable high-water level of the two-year recurrence, one day in 310 days in the case of a five-year recurrence and one day in 700 days in the case of a ten-year recurrence.

The case of the two-year recurrence shows quite a high percentage and the other two show very low percentage.

The maximum duration in days occurred in 1966, in which the highest water-level is equivalent to the probable high-water level of 25-year recurrence. It was 29 days in the case of a probable high-water level of the two-year recurrence, 20 days in the case of a five-year recurrence and 14 days in the case of a ten-year recurrence respectively.

In view of the above facts, it seems most appropriate to decide the design high-water level on the basis of the probable high-water level of 5-year recurrence, which is EL 166.72 at the bridge site.

Consequently, the design water-level for bridge planning is decided as the elevation EL 167 taking the allowance of 28 centimeters.

Fig.4.2.1 PROBABLE HIGH-WATER LEVEL AT R.I.D. (WAT HAI SOK) G.S.

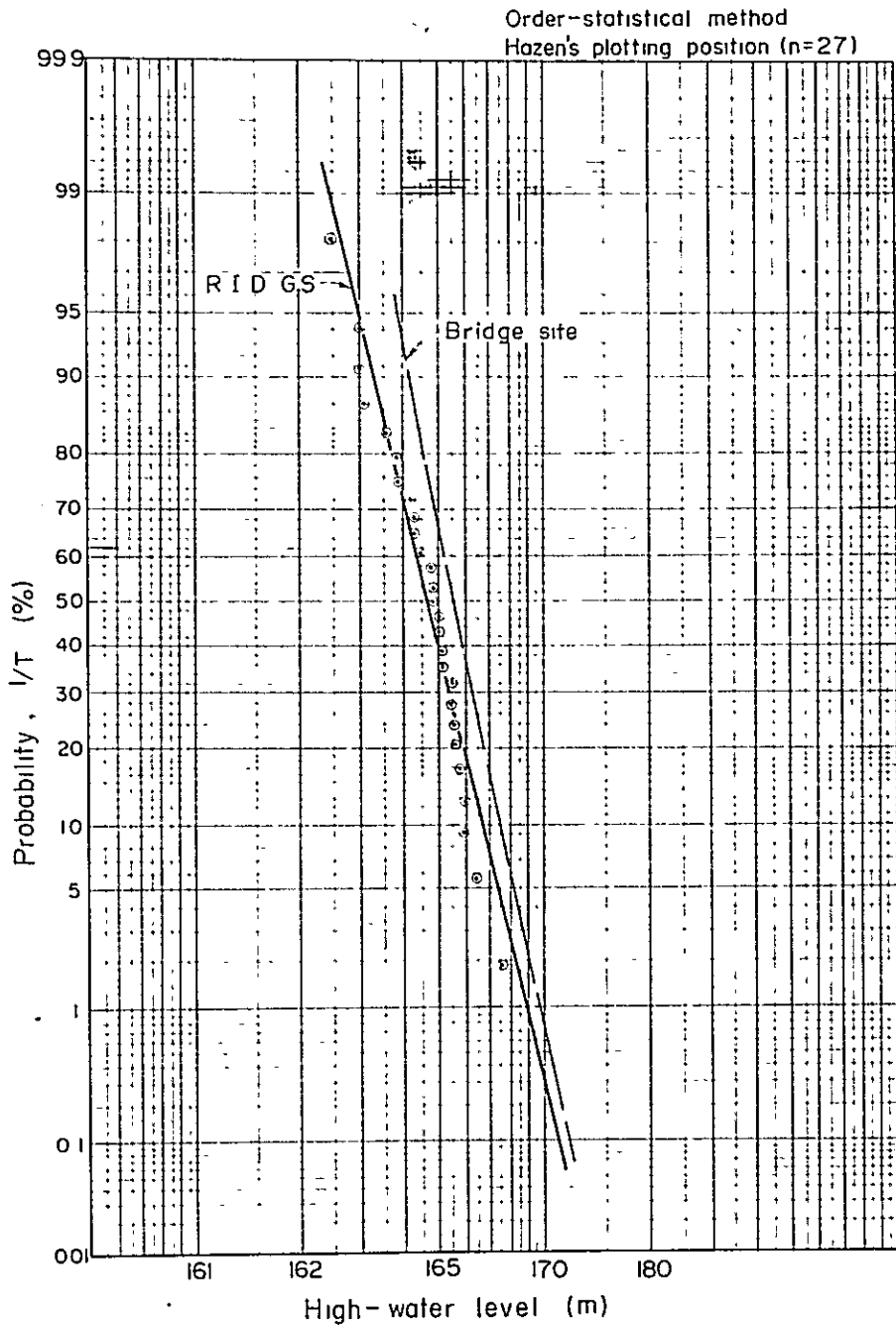
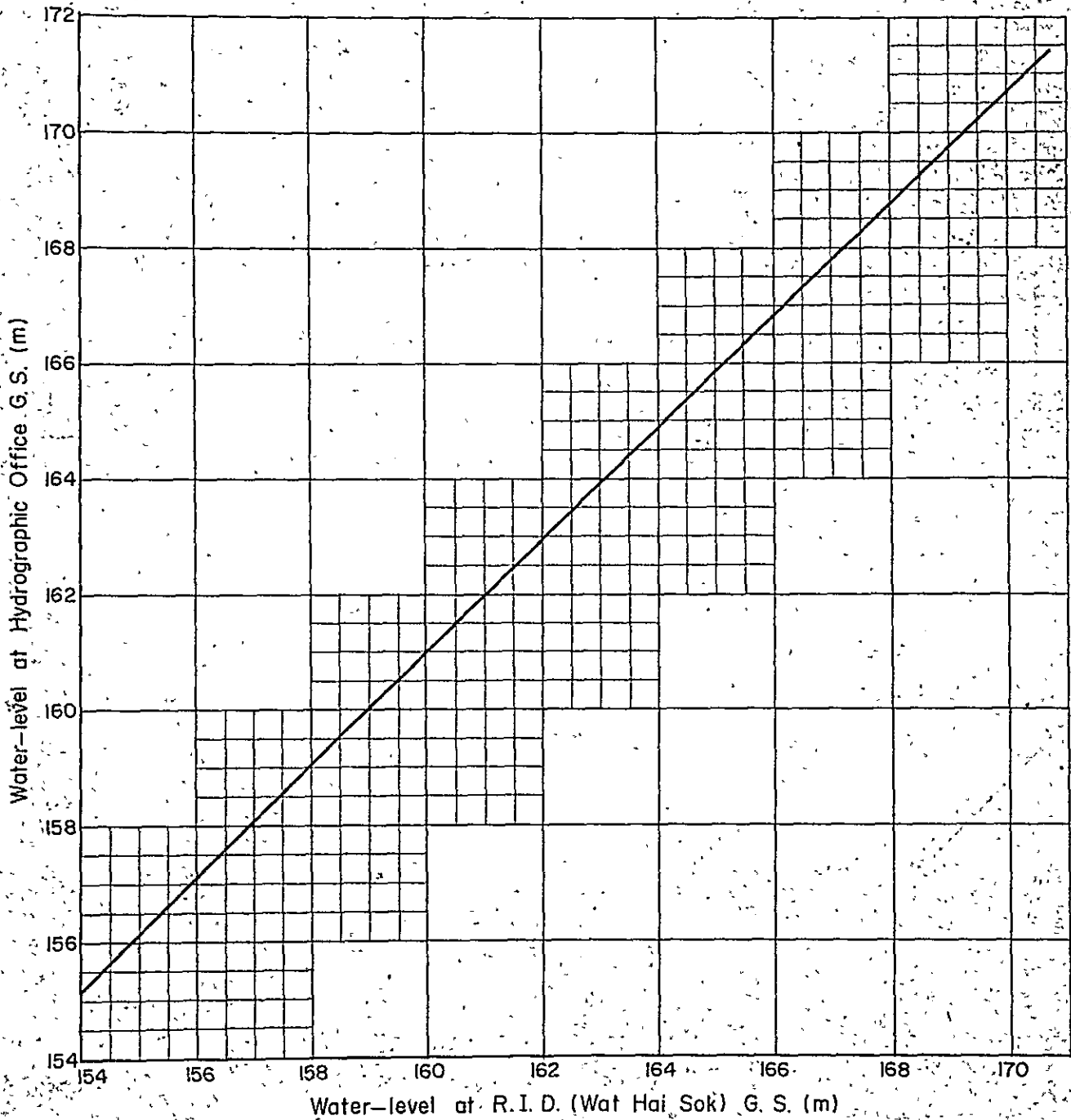


Fig.4.22. RELATION. OF WATER-LEVEL BETWEEN R.I.D.(WAT HAI SOK) G. S. AND HYDROGRAPHIC OFFICE G. S. 1971



APPENDIX V

METEOROLOGY

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Remarks:

Location of the meteorological stations are shown in the figure at page 93.

Meteorological Data Collected in the First
and Second Phase Investigations

Data	Available Period		
	Vientiane	Nong Khai	R.I.D. /1
Air temperature	Jan.'58-Feb.'68*	Mar.'64-Dec.'67	Jan.'65-Apr.'68*
Daily rainfall	Jan.'58-May.'68*	Jan.'64-Dec.'67*	Apr.'55-Apr.'68
Relative humidity	Jan.'58-Feb.'68*	Mar.'64-Dec.'67	-
Evaporation	Jan.'58-Feb.'68*	-	-
Prevailing wind direction and wind velocity	Jan.'59-Apr.'68	Feb.'66-Dec.'67	-

Remarks

/1 R.I.D. Meteorological station at Nong Khai

* The data asterisked were not compiled in the Appendices because they were not available for the meteorological analysis on the Second Phase Report.

DAILY RAINFALL RECORD

STATION: *R.T.D. (Nong Khai)*El. _____ Annual total: *1157.5* Year *1958*

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1										27.6				1	
2					3.2	57.6								2	
3														3	
4						6.9		4.4	19.0					4	
5							24.5							5	
6						7.4	31.2		16.2	2.8				6	
7							3.4	9.0		30.0				7	
8								18.0	13.6					8	
9								11.5						9	
10					10.3				8.1	45.5				10	
11						1.4	5.2		17.5	2.6				11	
12						4.8	14.6		14.1					12	
13						6.7	13.1				4.6			13	
14							7.7				14.2			14	
15									50.6		6.4			15	
16							1.8		2.2					16	
17						8.3	52.5			9.5				17	
18							8.0		6.4	8.2				18	
19							1.5			1.4				19	
20								61.2						20	
21							10.6	9.0		7.0				21	
22							1.5	6.0		2.4				22	
23								9.7						23	
24						35.3	56.7	11.7						24	
25							1.1	2.0	5.5					25	
26							40.9							26	
27							62.6		34.6					27	
28								7.8	70.0					28	
29								85.0						29	
30						1.8		12.5						30	
31									9.1					31	
Max					10.3	57.6	62.6	85.0	70.0	45.5	14.2			Max	
Days					2	10	16	13	13	10	3			Days	
Total					135	131.7	335.4	247.8	266.9	137.0	25.2			Total	

Unit: *m.m.*

DAILY RAINFALL RECORD:

STATION: *R.T.D. (Nong Khai)*

El. _____

Annual total: *1607.2*Year *1959*

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1							2.9			7.7					1
2							9.3	7.3	12.1	5.9					2
3			4.3					9.0	3.6						3
4									4.6	34.0					4
5			0.7			2.0		2.6		23.1					5
6										116.5					6
7										10.0					7
8								3.0		122.2					8
9									3.7	60.5					9
10				0.5		12.8		82.4		132.3					10
11								19.2							11
12				0.2				20.3		67.2					12
13										26.8					13
14										25.4					14
15						1.0				16.0					15
16					26.3		0.8	11.0	40.0						16
17						46.8	11.8		18.0						17
18								10.1	1.3	6.7					18
19				0.8				7.3	12.6						19
20								2.2	0.9	57.4					20
21						5.0		2.9	40.7						21
22				5.6	27.6	43.5	7.8			3.9					22
23					9.2	10.9	10.0		1.5						23
24						21.5	0.3	13.9	25.6	3.6					24
25								2.2	7.8	8.2					25
26				2.8					3.2						26
27				1.3		20.5	16.6	34.5	11.8						27
28							23.7	9.7	44.5						28
29								1.1	8.1						29
30								59.0							30
31						15.6									31
Max			4.3	5.6	27.6	46.8	23.7	82.4	44.5	132.3					Max
Days			2	6	3	9	9	18	17	18					Days
Total			5.0	11.2	63.1	179.6	83.2	297.7	240.0	727.4					Total

Unit: *mm*

DAILY RAINFALL RECORD

STATION: R.I.P. (Nong Khai)

El. _____ Annual total: 1616.5 Year 1960

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1			3.4			3.0		1.7			4.3				1
2									3.2		1.8				2
3						12.5	62.0	53.1	42.1	0.2					3
4							9.0	10.1		8.5					4
5							0.6			4.2					5
6									5.4	11.8					6
7									23.0	54.0					7
8						4.9			34.0	152.8					8
9							12.1		38.7	19.4					9
10						19.1				2.5					10
11			4.6				2.0	18.4	2.0	20.5					11
12			4.4	8.3				20.0	3.5						12
13								4.0	25.1		18.3				13
14									14.5	45.2	8.5				14
15						3.1	1.3	15.2							15
16							0.7	13.5	4.9		3.5				16
17						1.4			8.1						17
18						3.8			52.7						18
19									66.7	13.1					19
20						39.4		17.5	14.1		23.0				20
21						3.7	8.7								21
22			7.0			20.3		26.2	44.6	4.8					22
23						13.3		1.5	2.0	26.3					23
24				4.2		17.0	5.8	16.9	58.2	5.5					24
25			3.0	2.4				27.7	3.2	11.8					25
26			1.0				1.3		3.0	8.9					26
27				34.7						31.6					27
28							8.1		3.8	9.3					28
29					14.3			35.6	0.5	12.6					29
30							1.3		24.9						30
31								23.1	9.7						31
Max			4.6	34.7	14.3	39.4	62.0	53.1	66.7	152.8	23.0				Max
Days			5	4	2	12	12	15	24	19	6				Days
Total			16.4	52.4	18.5	141.5	112.9	284.5	487.9	443.0	59.4				Total

Unit: mm

DAILY RAINFALL RECORD

STATION *R. I. D. (Nong Khai)*Ei. _____ Annual total: 1686.5 Year 1961

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1				11.4				2.2		15.7	43.4				1
2									2.1	19.1	4.6				2
3						24.6									3
4				0.6					20.5	13.1					4
5							7.6		7.5	2.6					5
6				0.3				1.6	9.7	1.1					6
7				11.4			1.2	2.8		20.8					7
8							9.0	2.5		9.4					8
9					20.9		10.7	6.4	7.6	105.7	77.6				9
10							14.7	7.5		1.7					10
11					9.1	26.3	7.6		5.3	3.5					11
12							17.6		4.3						12
13							8.9		26.6	4.2					13
14							8.5		4.8	16.2					14
15							0.5		28.7	5.2					15
16							3.7	1.9		8.5					16
17						33.8	18.4	6.3	1.4	16.4					17
18						38.4		33.3		15.2					18
19					3.8	0.6		16.2	24.1	20.4					19
20						7.6		0.4	6.0	8.2					20
21						60.6		2.2	144.1	44.7					21
22						0.4									22
23							2.1		0.3						23
24						65.2	17.8			11.2					24
25						1.6	38.5			72.0					25
26					5.6		0.8		37.5	3.2					26
27						19.7	0.5	8.6	2.4						27
28						16.0		14.5	2.5						28
29							2.2	30.4	1.03	14.9	23.9				29
30						13.5	0.2		2.2		5.9				30
31								53.3	9.7						31
Max															Max
Days															Days
Total															Total

Unit: *mm*

DAILY RAINFALL RECORD

STATION: *R. I. D. (Nong Khai)*El. _____ Annual total: *1,857.7* Year: *1962*

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1				1.6		4.6	49.7	48.2	21.9	2.6				1	
2				0.2				0.8	12.6	5.3				2	
3					10.9	4.6	1.6		31.4	33.4				3	
4							12.9		11.5	5.8	13.9			4	
5					2.4	16.9	4.7			0.7	8.9			5	
6				1.0					9.2		79.1			6	
7									45.1		11.4			7	
8						56.9			13.3		29.6			8	
9				0.7			24.4			8.3				9	
10				11.1			20.0	11.5	5.5	10.6	1.2			10	
11							0.8	6.7	60.0	17.3	0.8			11	
12					19.3	14.2	29.5	4.7	35.6				1.3	12	
13								48.0	11.5	4.4				13	
14								31.8			10.7			14	
15							4.2	38.6						15	
16			5.2					1.4		0.5				16	
17				9.3			68.6	3.0	12.4	21.6				17	
18			10.0			73.9	28.7	16.2	3.1	19.2				18	
19			1.3				5.7	27.1	29.0	0.1				19	
20					0.1			1.6	13.3					20	
21						20.0			6.3					21	
22					5.5	26.5				74.5				22	
23					6.9			0.6	20.0	19.7				23	
24					10.0	4.3	1.5	16.3	2.4	21.0				24	
25					6.4	2.3		4.3	39.4	0.7				25	
26							19.7		28.6					26	
27				2.4	2.9					29.8				27	
28					1.2					40.8				28	
29					26.2					1.2				29	
30					22.5	3.2	0.2		6.8					30	
31						5.8		5.7	35.4					31	
Max			10.0	11.1	26.2	73.9	68.6	48.2	60.0	74.5	79.1		1.3	Max	
Days			3	7	12	12	15	17	22	20	8		1	Days	
Total			16.5	26.3	114.3	233.2	272.2	266.5	454.3	317.5	155.6		1.3	Total	

Unit: *mm*

DAILY RAINFALL RECORD

STATION: R.I.D. (Nong Khai)

El. _____ Annual total: 1308.0 Year 1963

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1								0.2	5.3	2.4					1
2							1.0		0.8						2
3						12.6	40.4	1.6		2.8					3
4							38.6	29.8							4
5						11.7		0.2	2.0	13.0	24.0	18.6			5
6						12.4	1.4		21.1		2.3				6
7				75.0					20.6		0.7	4.5	1.1		7
8						1.3	7.7		25.9	4.3			0.8		8
9			0.4				48.2			33.1		41.5			9
10							33.8	0.5		37.4					10
11									6.2						11
12							2.2	3.4	11.4	0.5					12
13								14.9	0.8						13
14															14
15			0.7				27.9	24.3							15
16					1.3	1.1	6.2			6.0					16
17					1.3			1.7		11.7					17
18					4.0	15.8		3.7	6.1	55.9					18
19					2.2	64.8		5.5	12.4	8.3					19
20						5.5	38.5	7.6							20
21								6.8							21
22								2.7	4.1	6.3					22
23								3.0	11.2						23
24		0.7				17.8		16.1	0.5	4.0					24
25						0.8		33.1	0.7						25
26						33.3	42.6	9.7	36.0	3.6	4.4				26
27								6.4	3.9		18.1				27
28				15.9		2.6	6.7		1.5		0.2				28
29								4.9			9.7				29
30						10.7		56.7							30
31								2.4							31
Max		0.7	0.7	75.0	64.8	48.2	56.7	36.0	55.9	24.0	41.5	1.1		Max	
Days		1	2	6	13	13	22	18	14	7	3	2		Days	
Total		0.7	1.1	99.7	190.4	295.2	235.2	170.5	189.3	59.4	64.6	1.9		Total	

Unit: mm

N. K. Form No. 1101

DAILY RAINFALL RECORD

STATION *R.I.D. (Nong Khai)*

El. _____ Annual total: *1585.5* Year *1964*

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1					3.5	29.3				15.0	20.0				1
2						0.7		5.7		1.5	3.4				2
3						59.0	6.3	8.4		25.6	25.7				3
4						25.1		8.7	25.7	13.0	3.4				4
5							8.9			11.0					5
6							23.7		2.5	21.2					6
7			1.7				7.1	34.5	3.1	8.4					7
8				6.0	14.5			2.5			24.9				8
9							3.1		7.2		6.4				9
10				7.7	1.2				3.7		15.6				10
11				15.0	4.3	1.8	65.0								11
12				14.0		4.3				4.0					12
13				21.0		1.3			50.0	27.0	6.3				13
14			3.0					8.4	9.8						14
15					2.0	46.0			2.3	28.8	13.5				15
16					0.7	3.3				5.5					16
17				22.1				3.7		18.3					17
18					21.8	7.5				41.9					18
19										11.2					19
20			8.8			4.2									20
21				4.2	69.7	3.8	37.2	12.0							21
22			6.3			0.4	15.0	0.9	0.7	8.0					22
23					49.2	5.7		16.0	8.9	20.1					23
24				9.8	9.8			1.7		11.2					24
25					23.1	7.4	0.5	42.5							25
26						33.7		4.6							26
27					17.2	0.2		3.7	26.2						27
28				14.5	18.9	2.1		1.2	0.5						28
29					27.1			38.5							29
30				2.6	4.3			0.6							30
31								16.0							31
Max			8.8	22.1	69.7	46.0	65.0	50.0	28.8	25.7				Max	
Days			4	11	18	19	11	19	18	12				Days	
Total			19.8	120.4	377.9	208.6	189.6	242.0	268.7	158.5				Total	

Unit: *mm*

DAILY RAINFALL RECORD

STATION: *R. I. D. (Nong Khai)*El. _____ Annual total: *1339.8* Year *1965*

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1						1.9			6.6						1
2		5.2								26.7					2
3										1.6					3
4							5.2	10.0	4.2	5.0		2.2			4
5					16.6		5.7			0.9	4.0				5
6					2.0		13.1	7.9		63.3					6
7						0.2	2.9		0.6	20.5					7
8					15.1		1.5		1.0	5.6	3.2				8
9							2.2			55.0					9
10							3.7				3.5				10
11					2.8		32.1	21.8		55.5	1.9				11
12							9.8	11.4							12
13						8.7	2.3		65.6	20.3					13
14							10.4			10.8					14
15						1.0	44.2								15
16						118.0	59.1		1.4						16
17									1.5						17
18					33.7		2.3		14.6						18
19							0.3		14.5						19
20						4.2	32.2		29.1	11.5					20
21							9.1	57.8	16.0	1.1					21
22					40.5		8.2		1.8						22
23						0.4	1.4	9.2	56.2						23
24					36.5			8.7		14.3	7.2				24
25					0.8			1.5			0.7				25
26					12.9			6.4	19.2						26
27						2.1	0.4	14.2	0.7		2.5				27
28						16.8	12.1		0.9						28
29						2.6	12.3	12.2	4.5						29
30						4.1	11.8								30
31						14.6									31
Max		5.2			40.5	118.0	59.1	57.8	65.6	63.3	7.2	2.2			Max
Days		1			9	12	23	11	17	14	7	1			Days
Total		5.2			160.9	174.6	282.3	161.1	238.4	292.1	23.0	2.2			Total

Unit: *mm*

DAILY RAINFALL RECORD

STATION: R. I. D. (Nong Khai)

El. _____ Annual total: 1529.7 Year 1966

D	M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M	D
1					0.2			2.3	16.6						1
2								12.9	35.6	12.2					2
3				7.6		58.5	3.4	3.4	17.5						3
4						1.7	15.0	28	10.8						4
5					2.5		65.0		7.7						5
6						30.0	5.4			10.7					6
7									2.8						7
8							0.6		0.6						8
9						14.3	10.1		2.0	20.5	15.7				9
10									6.0		2.8				10
11									71.6						11
12					1.7				6.4						12
13					14.3			22.7		5.5					13
14						15.1				25.3					14
15						10.9	36.2	30.0	22.5	0.8					15
16					5.0	4.28		3.2	158.5						16
17						7.9		3.1							17
18						21.2		1.1							18
19						21.0	33.3	17.2							19
20					0.6	5.7		10.2	5.4						20
21					81.4			3.5	37.3						21
22							25.5	2.5	30.3						22
23						1.20		18.6	11.2						23
24					12.4	11.5			5.6						24
25						30.1	5.6		4.2		35.4				25
26						15.5	1.8		15.7		14.6				26
27					13.8	21.7	2.5		21.0						27
28								16.9	5.4						28
29							1.2	0.8							29
30									71.8						30
31									2.5						31
Max				7.6	81.4	58.5	65.0	30.0	158.5	25.3	35.4				Max
Days				1	9	16	13	16	24	6	4				Days
Total				7.6	131.9	319.9	205.6	151.2	569.0	75.0	68.5				Total

Unit: mm

N. K. Form No. 1101

DAILY RAINFALL RECORD

STATION *R.T.D. (Nong Khai)*El. _____ Annual total: *1326.4* Year *1967*

D \ M	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	M \ D
1			1.2		22.3								1
2			19.0			1.9	26.3						2
3						6.0							3
4						0.2	2.6	0.1					4
5						3.2	3.5						5
6				4.0		21.8			3.4				6
7						4.3	2.0	4.5					7
8			7.2		0.2	26.2		12.9					8
9						0.5	20.9	5.2					9
10					9.5		11.0	24.0	16.3				10
11						3.2		5.8			17.3		11
12				3.0		11.0	4.8	2.3			4.5		12
13				22.0	0.5		25.0	3.5					13
14													14
15				18.1			26.2						15
16					13.5		10.0		36.8				16
17			4.5	2.5	14.7				18.8				17
18					21.3								18
19					4.1			49.1	27.0				19
20			16.1			53.5		221.2	8.4				20
21				0.4				7.0	31.8				21
22				52.5			12.3	56.9	2.8				22
23		2.6					2.9	3.4					23
24							13.5	11.7					24
25						0.5	3.8	2.2					25
26				47.5			11.9	2.5	20.8				26
27						0.3		4.7	3.6				27
28					2.8	62.0		21.0					28
29					12.2	3.0		8.4					29
30													30
31			12.2										31
Max		2.6	19.0	52.5	22.3	62.0	26.3	221.2	36.8		17.3		Max
Days		1	6	8	10	15	15	14	10		2		Days
Total		2.6	60.2	150.0	101.1	197.6	176.7	446.4	169.7		21.8		Total

Unit: *mm*

N. K. Form No. 1101

DAILY RAINFALL RECORD

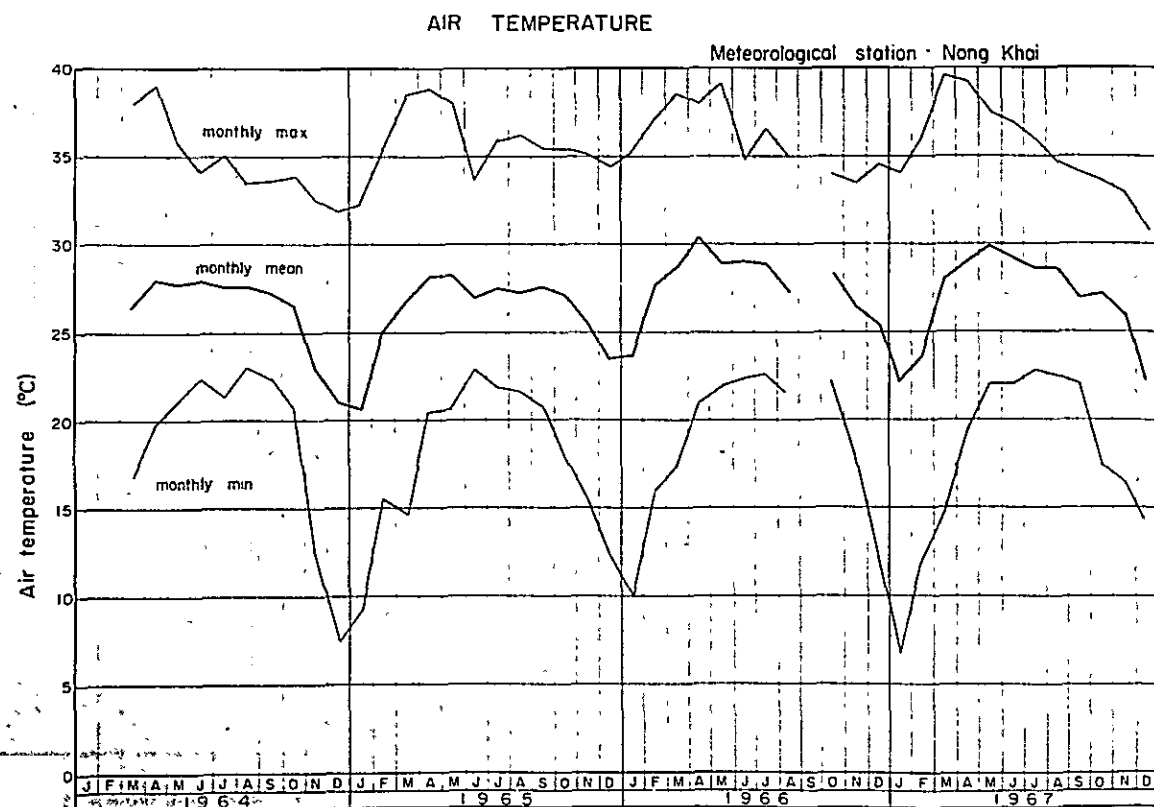
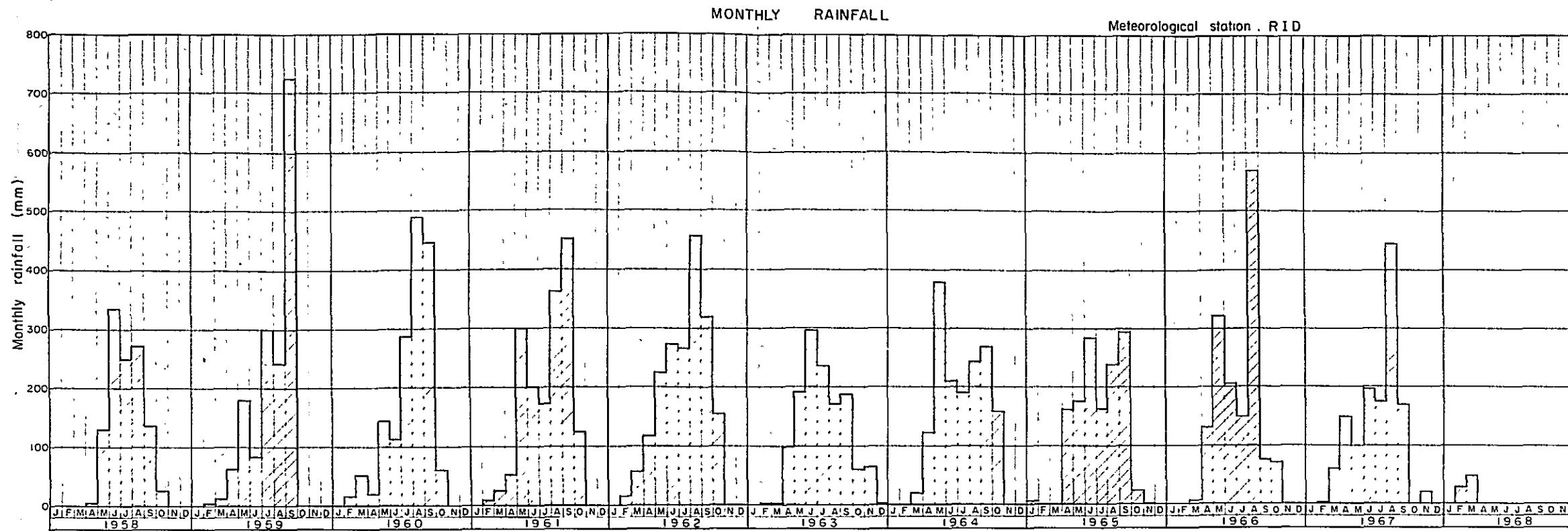
STATION R.I.D. (Nong Khai)

EI. _____ Annual total: _____ Year 1968

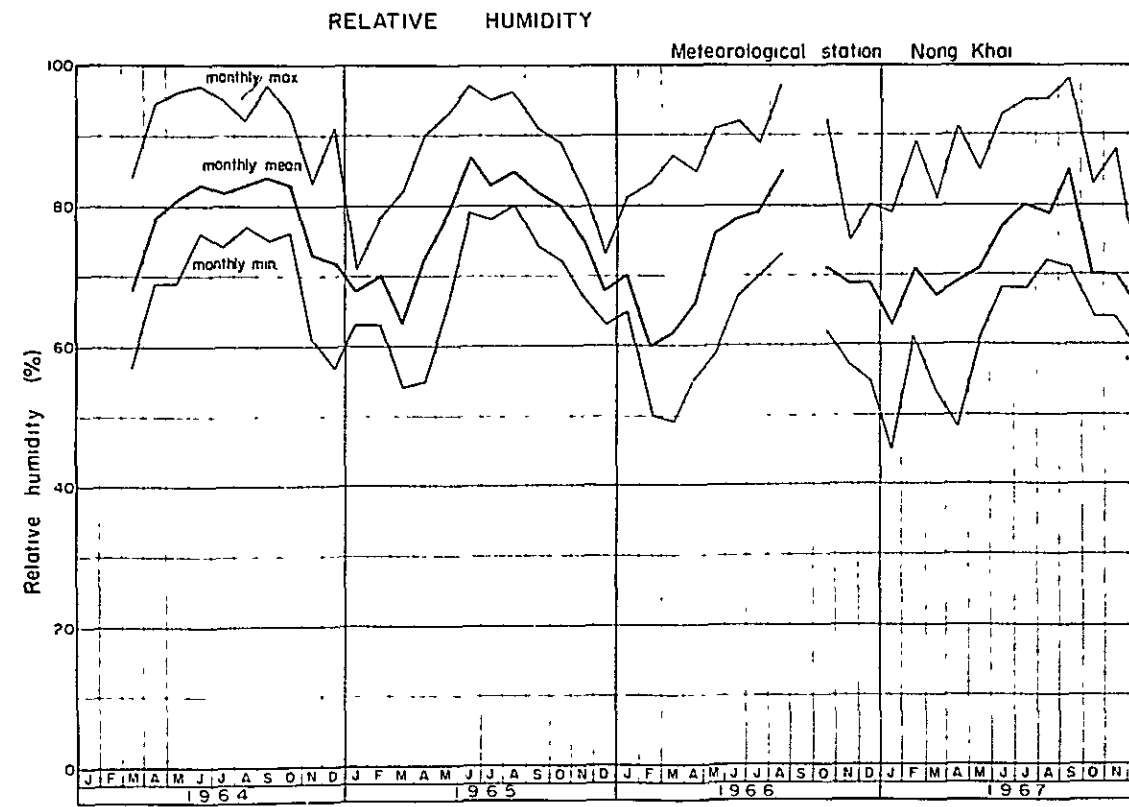
D	M												D
	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	
1													1
2													2
3													3
4				2.7									4
5				19.5									5
6													6
7			25.0	2.3									7
8			3.2										8
9													9
10			1.0										10
11													11
12			3.5										12
13													13
14													14
15				25.2									15
16													16
17													17
18													18
19													19
20													20
21													21
22													22
23													23
24													24
25													25
26													26
27													27
28													28
29													29
30													30
31													31
Max			25.0	19.5									Max
Days			4	4									Days
Total			32.7	49.7									Total

Unit: mm

RAINFALL , AIR TEMPERATURE AND RELATIVE HUMIDITY



Remarks. These lines are described based on the daily-max, daily-mean and daily-min air temperature records
 It was due to flood that records in September 1966 were not registered



Remarks. These lines are described based on the daily-mean relative humidity records
 It was due to flood that records in September 1966 were not registered

Daily Prevailing Wind Direction and Mean Wind
Velocity at Nong Khai Meteorological Station

Year: 1966

Date	Jan/1		Feb.		Mar.		Apr.		May		Jun.	
	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.
1			W	0.6	NE	1.4	SE	1.9	SE	2.3	SW	1.2
2			W	0.5	NE	2.1	SW	1.6	SE	1.4	SW	2.3
3			W	0.6	SE	0.6	SW	1.2	E	3.3	SW	1.2
4			W	0.6	SW	1.0	SE,SW	1.2	NE	1.2	SW	1.2
5			E	1.0	V	0.6	SW	1.4	SE	1.2	SW	1.2
6			NE	1.9	N	1.0	E	1.9	SW	1.2	SW	0.6
7			E	1.0	SW	0.8	SW	1.4	NE	1.2	SW	1.2
8			E	0.8	N	2.3	NE	1.6	NE	1.2	SW	1.9
9			E	1.0	NE	0.4	SW	1.2	SW	1.6	SW	1.6
10			W	1.2	S	1.2	NE	1.9	SW	2.0	SW	1.9
11			NE	0.6	NE	0.8	N	1.0	SW	1.2	SW	1.9
12			NE	2.3	NW	1.0	NE	2.1	SW	1.6	SW	2.5
13			SE	2.3	W	1.0	SW	1.4	SW	1.0	SW	2.5
14			E	1.6	NW	1.2	SW	1.4	SW	2.3	E	1.4
15			W	1.0	W	1.4	SW	1.4	E	1.2	SW	1.2
16			W	0.8	W	1.0	SW	1.6	SW	1.2	SW	1.0
17			W	0.8	E	1.2	S,W	1.9	SW	2.7	SW	1.0
18			NE	1.0	SW	1.0	NE	3.9	SW	2.5	NE	2.9
19			W	1.4	SE	2.4	SW	1.2	SW	1.6	SW	1.6
20			W	0.8	E	1.6	SW	2.7	NE	2.9	SW	1.0
21			NE	0.8	SW	1.2	E	3.5	E	3.9	SW	1.9
22			SW	1.4	SW	2.9	NE	0.8	E	1.0	SW	1.2
23			E	3.7	SW	1.0	W	1.0	SW	0.8	SW	2.5
24			NE	2.1	SW	1.0	SW	1.4	SW	2.7	SW	1.0
25			W	0.8	SW	1.0	SW	1.0	E	1.2	SW	1.4
26			NE	1.2	E	2.3	SW	2.1	NE,SW	3.5	SW	1.0
27			NE	1.6	E	2.5	SW	2.1	SW	1.4	SW	0.6
28			E	1.2	SE	1.0	SW	0.8	SW	1.6	NE,SW	1.2
29					SW	1.0	SW	1.2	SW	1.5	SW	1.2
30					E	2.0	NE	3.1	SW	1.8	SW	2.1
31					E	1.2			SW	1.2		

/1 : No available data in January

Abbreviation:

Dir. Prevailing direction

Vel. Mean wind velocity
in m/sec.

- continued

Year: 1966

Date	Jul.		Aug.		Sep./1		Oct.		Nov.		Dec.	
	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.
1	SW	0.4	E	1.4	-	-	NE	1.3	WE	2.4	NE	3.6
2	SW	1.0	SW	2.1	-	-	NE	0.4	E	3.5	NE	5.6
3	SW	1.6	NE	0.6	-	-	NE	0.6	NW	1.0	NE,SW	0.5
4	SW	1.2	SE	1.0	-	-	NE	1.6	NE	1.2	N	0.7
5	SW	1.4	SW	0.6	-	-	NE	0.9	NE	2.1	NE	0.7
6	SW	1.2	W	0.8	-	-	NE	1.8	SW	4.7	E	0.8
7	SW	1.0	SW	0.8	-	-	NE	1.3	NE	0.6	N	0.5
8	SW	1.6	SW	2.3	-	-	NE	0.9	NE	0.2	NE	0.2
9	SW	1.6	SW	0.4	-	-	NE	0.8	NE	0.1	NE	0.2
10	SW	1.2	SW	1.2	-	-	E	0.9	NE	0.4	NE	0.4
11	SW	1.0	SW	0.8	-	-	W	2.1	NE	0.3	E	0.6
12	SW	1.4	SW	1.0	-	-	NE	0.7	E	0.4	NE	1.1
13	SW	1.6	SW	1.0	-	-	SW	0.8	E	0.4	NE	0.7
14	SW	1.4	SW	1.2	-	-	SW,NE	0.6	E	0.9	NE	0.4
15	NW	1.2	SW	1.4	-	-	E,W	1.0	NE	1.1	E	0.5
16	SW	0.8	SW	1.0	-	-	E	0.8	NE	0.3	E	0.5
17	NE,SW	1.4	NE	1.2	-	-	NE	1.5	NE	1.5	ESE	0.2
18	S	1.4	SW	1.0	-	-	NE	1.8	NE	0.9	NW	0.4
19	E	1.4	W	2.5	-	-	NE	2.5	NE	0.4	W	0.2
20	SW	2.1	SW	1.6	-	-	NE	1.9	NE	0.9	E	0.4
21	SE,SW	1.4	SW	1.2	-	-	NE	2.4	NE	5.9	E,S	1.1
22	SW	1.4	SW	2.9	-	-	NE	1.4	NE	1.8	W	0.8
23	SW	1.4	SW	1.0	-	-	SW	1.2	NE	0.4	N	0.2
24	SW	1.6	C	1.0	-	-	SW	0.8	NE	0.5	ENE	0.8
25	SW	2.9	SW	1.0	-	-	SE	0.6	NE	1.1	NE	0.7
26	NW,SW	3.3	SW	2.3	-	-	E	1.8	NE	2.3	NE	0.7
27	SW	2.8	SW	2.5	-	-	E	1.0	NE	0.2	NE	1.8
28	SW	1.0	SW	0.8	-	-	E	1.8	ENE	0.2	NE	1.1
29	SW	1.9	E	1.0	-	-	NE	1.4	NW	0.6	NE	0.2
30	SW	1.9	SW	2.1	-	-	NE	0.6	NE	2.6	NE	0.4
31	SW	1.9	SE	1.2	-	-	NE	1.1			NE	0.6

/1 : No observation due to flood in September.

Abbreviation:

Dir. Prevailing direction

Vel. Mean wind velocity
in m/sec.

Daily Prevailing Wind Direction and Mean Wind
Velocity at Nong Khai Meteorological Station

Year: 1967

Date	Jan.		Feb.		Mar.		Apr.		May		Jun.	
	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.
1	NE	1.2	NW	2.1	NE	1.4	SW	1.0	S	1.1	SE	1.4
2	NE	3.1	E	1.4	SE	1.4	SW	1.2	S	1.3	SE,SW	1.2
3	NE	6.4	E	4.0	W	1.0	SW	1.9	W	1.2	SW	1.0
4	NE	3.0	NE	0.8	W	1.2	SW	1.0	N	1.4	SW	1.0
5	NE	1.5	NE	2.7	SE	1.6	SE	2.9	SW	0.8	SE	1.4
6	C	0.4	NE	3.5	SE	4.5	SE	1.2	S	1.4	E	1.2
7	C	0.7	NE	1.6	SE	3.1	SW	1.4	C	1.3	SW	1.2
8	NE	1.4	NE	1.0	SE	1.2	SW	0.8	SE	4.7	W	1.2
9	C	0.5	E	1.2	E	1.9	SW	1.4	SW	1.4	SE	3.5
10	NE	1.2	NE	1.0	E	2.7	SW	1.2	SE	1.6	SW	1.0
11	NE	3.7	SE	3.1	E	1.0	SW	1.2	SE	1.9	E	1.4
12	NE	1.2	NE	2.3	E	1.0	SW	1.9	W	0.6	E	1.4
13	NE	2.5	SE	1.0	E	1.2	SW	1.2	SW	1.6	SE	0.8
14	NE	0.7	NE	3.7	E	1.2	SW	1.4	SE	2.5	SW	0.8
15	NE	1.8	NE	1.9	E	2.1	E	2.1	SE	1.6	W	1.2
16	NE	2.8	NE	1.2	C	0.6	C	0.8	SE	4.1	SW	1.2
17	NE	2.2	NE	2.3	NE	1.2	SE	1.9	E	4.5	SW	2.1
18	NE	1.5	NE	3.1	C	0.6	SW	1.0	E	1.6	SW	2.1
19	SE	0.4	NE	1.2	SE	0.6	SW	1.9	W	2.5	SW	2.1
20	NE	0.8	NW	1.0	SW	0.8	SE,SW	1.6	SW	1.9	SW	1.9
21	E	1.0	C	0.6	S	0.8	SE	2.4	SE	0.8	SE,SW	1.0
22	E	0.8	NW	0.6	C	0.6	SW	1.8	SW	4.4	SW	1.2
23	C	2.7	W	1.0	C	0.2	SW	1.2	SW	4.1	W	1.2
24	SE	0.6	SE	1.0	E	1.9	SE	1.0	SW	2.3	SW	1.4
25	SW	0.5	E	1.4	E	1.9	SE	1.6	SW	2.4	E	1.0
26	SW	2.1	E	5.2	NW	0.8	SE,SW	2.1	SW	2.0	SW	2.5
27	SW	1.0	SE	4.7	W	0.8	SE	1.9	SW	0.8	SE	2.3
28	SW	1.0	E	2.1	SW	1.2	S	1.3	SE	3.7	SE	1.9
29	SW	1.0			SW	1.2	S	0.6	SW	1.9	SW	2.1
30	E	0.8			SW	0.8	SSW	1.2	SW	1.2	SW	1.4
31	NE	5.2			SW	1.2			SW	2.7		

Abbreviation:

Dir. Prevailing direction

Vel. Mean wind velocity in m/sec.

- continued

Year: 1967

Date	Jul.		Aug.		Sep.		Oct.		Nov.		Dec.	
	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.
1	SW	1.4	SW	1.9	SW	2.5	NE	1.4	E	2.9	NE	1.2
2	E	0.8	SW	3.3	SW	1.2	E	1.6	NE	4.3	NE	0.8
3	E	1.0	SW	4.3	SW	2.7	E	2.0	NE	4.0	NE	0.6
4	C	1.2	SW	3.1	W	2.1	NE	2.0	NE	2.4	NE	0.5
5	SW	2.5	SW	2.5	NE	1.2	E	1.2	NE	4.2	NE	0.3
6	SW	3.1	SW	2.1	SW	1.0	E	1.2	NE	2.9	NE	0.7
7	SW	1.2	SW	2.3	E	0.6	NE	1.2	NE	3.2	NE	1.0
8	SW	1.2	SW	1.0	NE	1.6	NE	0.8	NE	3.3	NE	1.2
9	SE	1.0	E	1.2	NE,SW	1.6	W	1.2	NE	2.1	NE	0.6
10	SW	1.2	E	1.2	NE	1.0	SW	0.8	NE	2.9	E	1.4
11	E	1.0	E,W	1.0	N	0.8	E	1.6	NE	5.2	E	0.6
12	SE,SW	1.6	SW	1.2	NE	0.6	SW	1.0	E	9.5	NE	1.0
13	SW	1.6	W	1.6	W	0.6	E	0.8	E	4.6	E	1.6
14	SE,SW	1.4	SW	2.0	W	0.6	E	0.8	NE	4.9	NE	0.6
15	E	1.0	W	1.0	NE	1.2	E	1.9	NE	3.2	NE	0.4
16	E	0.4	SE	1.2	E	1.2	W	1.0	NE	2.8	NE	0.2
17	SW	1.0	SW	1.6	NE	1.2	W	1.2	NE	3.2	NE	0.8
18	SW	1.4	SW	1.4	NE	1.2	E,W	0.8	NE	2.9	NE	0.6
19	S,W	1.4	SW	1.2	SW	0.8	NE	1.0	NE	2.9	NE	0.4
20	W	1.6	E	0.8	SW	2.1	E	1.0	NE	2.5	NE	0.3
21	NE	1.6	SW	1.4	SW	1.0	NE	1.2	NE	2.8	NE	0.3
22	E	1.2	SW	1.2	E	0.6	NE	1.6	NE	2.5	NE	0.3
23	W	1.4	SW	2.3	SE	1.4	NE	1.0	NE	2.5	NE	0.5
24	SW	1.4	SW	1.4	E	0.6	NE	2.1	W	2.1	NE	0.3
25	SW	1.9	E	1.0	E	0.8	NE	0.8	NE	3.1	NE	0.3
26	SW	1.2	W	0.8	E	1.0	NE	1.2	NE	2.8	NE	0.3
27	SW	1.4	E	2.3	E	1.4	NE	1.2	NE	0.8	NE	0.3
28	SW	3.1	SW	1.2	E	0.8	NE	1.2	NE	1.2	NE	0.3
29	SW	1.4	SW	3.1	C	0.2	NE	0.8	E	3.1	E	0.5
30	NE	1.2	W	1.9	W	1.0	W	1.2	E	3.1	NE	0.4
31	SW	1.2	SW	1.6			NE	0.8			NE	0.4

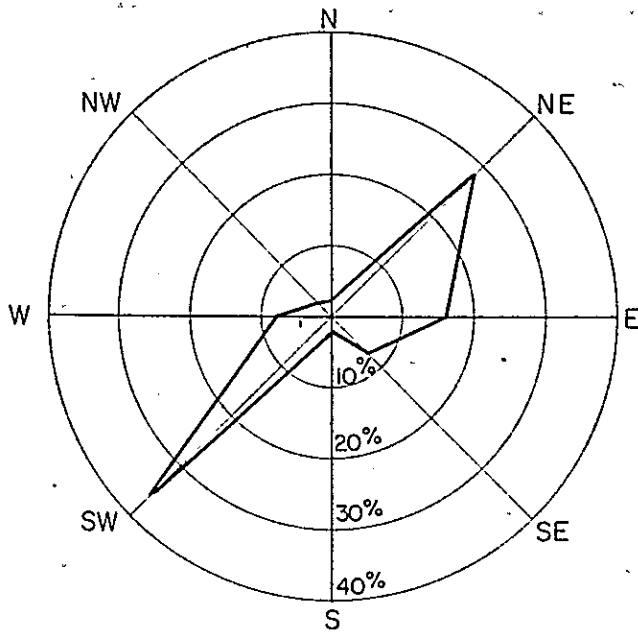
Abbreviation:

Dir. Prevailing direction

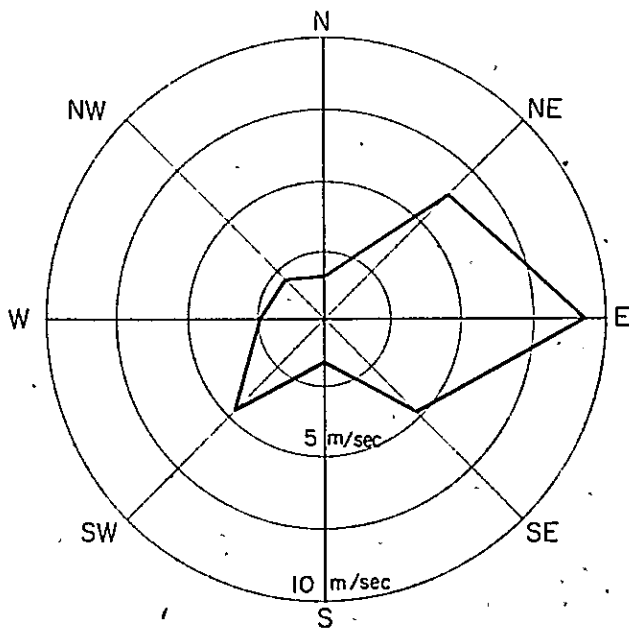
Vel. Mean wind velocity in m/sec.

WIND DIAGRAM

Meteorological station Nong Khai
Period: Feb. 1966 to Dec. 1967



DAILY PREVAILING DIRECTION



MAX. WIND VELOCITY

Monthly Max. Wind Velocity and Its Direction
at Vientiane Meteorological Station

Period : 1959 to 1968

	1959		1960		1961		1962		1963	
	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.
Jan.	ENE	5	ENE	4	SSW	4	SSE	3	SW	7
Feb.	NNW	4	E	8	E	3	ENE	3	E	4
Mar.	Var.	-	NW	4	ESE	4	E	3	Var.	-
Apr.	Var.	-	S	10	S	10	SSW	8	Var.	-
May	Var.	-	ENE	5	NNE	8	NW	3	SSE	3
Jun.	Var.	-	E	4	SW	4	N	3	N	3
Jul.	WNW	13	NE	3	ENE	6	W	8	SW	3
Aug.	WNW	5	ESE	4	SSW	3	ENE	8	N	3
Sep.	WSW	6	NE	4	WNW	3	W	4	WSW	4
Oct.	NE	5	SE	5	WNW	5	HE	1	SE	2
Nov.	E	5	ESE	3	NNW	4	N	6	N	2
Dec.	ESE	8	ENE	5	N	2	NNE	5	N	2

	1964		1965		1966		1967		1968	
	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.	Dir.	Vel.
Jan.	ESE	3	E	3	NNW	4	E	8	SE	3
Feb.	WSW	6	S	3	NE	4	E	4	E	8
Mar.	ESE	3	E	4	W	4	S	9	ESE	27
Apr.	NNW	4	W	4	N	10	SSE	8	SSE	8
May	S	4	W	10	NE	4	N	9		
Jun.	NNW	4	N	3	NNW	4	NNW	7		
Jul.	SSE	3	W	8	SE	5	W	8		
Aug.	S	3	NW	20	N	3	SW	6		
Sep.	S	3	W	4	S	4	SW	7		
Oct.	SW	2	NE	7	ESE	4	ESE	6		
Nov.	W	3	E	4	N	5	E	6		
Dec.	SE	3	-	-	-	-	E	3		

Remarks : Dir. = Wind direction

Vel. = Monthly max. wind velocity in m/sec

APPENDIX VI

ECONOMIC SURVEY

The data given here are concerned with the general economy, from which the future traffic and the benefits are estimated.

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Population of Vientiane City in 1966
(by Census in 1966)

	Male	Female	Total
Laotian	50,348	45,768	96,116
Foreigner	18,518	17,619	36,137
Total	68,866	63,387	132,253

Population of Nong Khai Prefecture in 1966
(by Bureau of Statistics of Nong Khai Prefecture)

Country	Population
Nong Khai	69,390
Phong Visay	69,789
Muang Kan	55,697
Saika	29,140
Sri Chiang Mai	28,967
Tha Bo	44,850
Total	297,833

Gross National Product of Laos

Year	Estimated gross national product (10 ⁶ US\$)		Population (10 ³ persons)		Estimated gross national product per head (US\$)				
	Laos	Self-supporting economy	Market economy	Vientiane	Laos	Self-supporting economy	Market economy		
1962	159.6	115.4	44.2	-	2,450	2,082.5	367.5	55.43	120.20
1963	166.9 (1.046)	119.4 (1.035)	47.5 (1.075)	-	2,509 (1.024)	2,133 (1.024)	376 (1.024)	55.98 (1.01)	126.21 (1.05)
1964	174.5 (1.046)	123.5 (1.034)	51.0 (1.074)	-	2,569 (1.024)	2,184 (1.024)	385 (1.024)	56.54 (1.01)	132.52 (1.05)
1965	182.9 (1.048)	127.9 (1.036)	55.0 (1.078)	-	2,635 (1.026)	2,240 (1.026)	395 (1.026)	57.11 (1.01)	139.15 (1.05)
1966	187.4 (1.025)	131.0 (1.024)	56.4 (1.024)	18.4	2,698 (1.024)	2,293 (1.024)	405 (1.024)	57.11 (1.00)	139.15 (1.00)
1967	196.1 (1.046)	135.5 (1.034)	60.6 (1.074)	19.7 (1.071)	2,765 (1.025)	2,350 (1.025)	415 (1.025)	57.68 (1.01)	146.11 (1.05)
1973	261.3 (1.051)	166.9 (1.036)	94.4 (1.078)	30.7 (1.077)	3,207 (1.025)	2,725 (1.025)	482 (1.025)	61.23 (1.01)	195.80 (1.05)
1990	630.0 (1.056)	300.6 (1.035)	329.4 (1.076)	108.2 (1.086)	4,880 (1.025)	4,146 (1.025)	734 (1.025)	72.51 (1.01)	448.76 (1.05)

Remarks: Figures in the brackets show index to value of the foregoing year.

Major Industries in Laos

Item	Number of factory	Unit	Annual production					Remarks	
			1962	1963	1964	1965	1966		1967
Match manufactory	1	case	-	-	-	2,500	3,000	3,500	1 case = 7,200 boxes, 1 box = 50 matches
Cigarette manufactory	3	case	-	-	-	40,570	46,000	50,000	1 case = 50 cartons, 1 carton = 10 packages, 1 package = 20 cigarettes
Hubber sandal manufactory	4	dz.	-	-	30,000	56,000	72,000	78,000	
Pizzy drink manufactory	6	btl.	5,400,000	5,700,000	6,300,000	6,800,000	7,200,000	8,000,000	1 at Saravane, 1 at Savannakhet, 3 in Vientiane, 1 at Luang-Prabang
Plastic bag manufactory	2	ton	-	-	-	72	72	75	Polyethylene bags
Mechanical rice-mill	208	ton	-	-	88,200	100,000	110,000	110,000	8 of 1st class (2,400 tons/year and per unit) 10 of 2nd class (1,500 tons/year and per unit) 190 of 3rd class (200 tons/year and per unit)
Textile printing	1	m	-	-	-	-	600,000	600,000	
Alcohol distillery	14	litre	-	-	1,200,000	1,200,000	1,800,000	1,800,000	Small distilleries capable of production from 150 to 200 litres/day
Power saw-mills	76	m ³	-	-	150,000	156,000	160,000	200,000	Sawn wood
Ice manufactory	8	ton	-	-	20,000	26,000	30,000	35,000	4 in Vientiane, 1 at Luang-Prabang, 1 at Savannakhet, 1 at Pakse and 1 at Khammune
Candle manufactory	3	case	-	36,000	36,000	40,000	40,000	45,000	1 case = 100 packages

Export of Laos in 1966

Articles	Weight (kg)	Exported to	Amount (Kips)
Musical instruments "Kha drum"	35	England	50,000
Bensoin in bulk and dust	300	U.S.A.	1,700,000
- do -	1,000	France	5,712,400
Tin ore	12,040		3,563,640
Cardamon	114,608	Hongkong	10,008,800
Tin ore	480,700	- Penang	218,680,800
Dried cuttle fish	6,680	Thailand	1,649,280
Vegetables	2,000	"	832,000
Dried fruits	580	"	340,000
Green coffee-bean	1,500	"	150,000
Tree-barks "penak boug"	29,000	"	292,500
Benzoin in bulk and dust	1,800	"	9,529,500
Fruits preserved in cans	9,950	"	1,760,000
Other preserved fruits	11,465	"	1,453,200
Crushed stones	1,050,000	"	1,290,000
Raw tin ore	2,000	"	60,000
Monosodium glutamate	6,220	"	2,100,000
Woods only barked and sawn of all kinds	7,087,097	"	37,825,074
Wooden furnitures	500	"	20,000
Waste irons	78,126	"	277,000
Waste coppers	22,800	"	1,150,000
Green coffee-beans	493,314	Singapore	53,558,080
Cardamon	57,780	"	5,342,400
Paddy bran	18,880	"	37,760
Soya seed	21,440	South Vietnam	343,000
	9,509,815		357,725,434

Import of Laos in 1966

Articles	Weight (kg)	Amount (Kips)
Rice and other cereals	42,150,225	2,492,026,152
Foodstuff, sugar and other food preparations	2,077,025	266,133,592
All oil products (aircraft and motor-car gasolines, oils, greases)	81,131,828	1,381,481,563
Structural metals, cement asbestos cement bolt and nut works, spanners, line	19,744,472	587,266,751
Other electric machinery and apparatus	2,039,014	833,092,444
Motor cars, tractors and cycles	2,465,060	1,271,250,396
Total	170,089,248	10,017,158,506

Level of Monthly Consumption in Nong Khai and Vientiane

	Gasoline	Cement	Rice	Steel bar	Beer	Hog	Refrigeration	Water melon	(Unit: tons)
Nong Khai prefecture	600	250	2,500	150	16	-	2	3	
Vientiane city	750	1,650	1,500	200	40	74	10	135	

	Gasoline	Cement	Rice	Steel bar	Beer	Hog	Refrigeration	Water melon	(Unit: tons/1,000 persons)
Nong Khai prefecture	8.65	3.60	36.02	2.16	0.23	-	0.03	0.04	
Vientiane city	5.67	12.47	11.34	1.51	0.30	0.56	0.08	1.02	

Remarks: (1) In the estimation of consumption per 1,000 persons, population (69,400) as of December 1966 for Nong Khai prefecture and population (132,300) as of July 1966, respectively, were used.

(2) The values of cement and water melon in Nong Khai prefecture are doubtful if they are of Nong Khai prefecture.

Prices in Nong Khai and Vientiane - 1

Items	Unit	Nong Khai		Item	Unit	Vientiane	
		(Baht)	(Kip)			(Baht)	(Kip)
<u>1. Miscellaneous sereals</u>							
Rice (Laos)	kg	2.35	58.75	Banana	kg	4	100
Rice (ordinary)	"	2.65	66.25	Water melon	no.	5	125
Bread	"		25	Shaddock	kg	3.5	87.5
Cassava	"	2.5	62.5	Orange	no.	4	100
Black bean	"	5	125	Pineapple	kg	4	100
				Coconut	no.	1.5	37.5
				Grape	kg	12	300
<u>2. Vegetables</u>							
Convalvulus	kg	2	50				
Tomato	"	0.50	12.5	4. <u>Meat and eggs</u>			
Chilipepper	"	6	150	Beef	kg	14	350
War gourd	"	1	25	Pork (with bone)	"	12	300
Chinese cabbage	"	3	75	Pork (fat of meat)	"	12	300
Lettuce	"	2	50	Pork (high quality)	"	17	425
Beefsteak plant	"	5	125	Port (with hide)	"	10	250
Cabbage	"	1	25	Chicken	no.	10	250
Green piece	"	6	150	Duck's egg	"	0.6	15
Japanese onion	"	2	50	Fish	kg	14-15	350-375
Garlics	"	6	150	Fish (salted)	"	18-20	450-500
Bean sprouts	"	2.5	62.5			20-25	550-625
Cucumber	"	1	25	5. <u>Dry food and condiments</u>			
Dry onion	"	6	150	Dried onion	kg	2.5	62.5
Potato	"	6	150	Dry cattle fish	"	17-28	425-700
Manpao	"	0.75	18.75	Thin threads of beach-jolly	"	16	400
Wild tomato	"	3	75	Salt	"	0.5	6.2
Long bean	"	4	100				
<u>3. Fruits</u>							
Apple	kg	5	125	6. <u>Other foodstuff</u>			
				Condensed milk	can	2.5-3.0	62.5-75
							11.0

Prices in Nong Khai and Vientiane - 2

Items	Unit	Nong Khai		Vientiane		Item	Unit	Nong Khai		Vientiane	
		(Baht)	(Kip)	(Baht)	(Kip)			(Baht)	(Kip)		
Lard	kg	12	300	300		City bus	km	0.50	12.5	30	
Soup (Chinese style)	bottle	5-6	125-150	100		Movie (2nd class)	person	3-5	75-125	100	
Black coffee	cup	0.5	12.5	15		Play (2nd class)	"	2-3	50-75	100	
Milk coffee	"	1	25	25		Drama and sports	"	"	"	100	
Ice		0.4	10	15						100	
<u>7. Electricity and fuel</u>											
Electricity	kwh	1.30	32.5	40		<u>11. Magazines and smoking</u>					
Petroleum	lit.	50	2	36		Daily paper	no.	0.5-1.0	12.5-25	50	
Electric bulbs	no.	4-5	100-125	140		Weekly magazine	"	3.0-3.5	75-87.5	140	
						Tabacco	box	2.5-3.5	62.5-87.5	20	
<u>8. Daily commodities</u>											
Saucer	no.	3-5	75-125	170		<u>12. Electric instruments</u>					
Nail	kg	5	125	150		Refrigerator	no.	3,675	91,875	60,000	
Aluminum streaming basket	no.	18	450	450-550		Fan	"	604	15,100	16,000	
Washing soap	box	9	225	200		<u>13. Vehicles</u>					
Toilet soap	no.	3	75	70		Bicycle (Thailand)	no.	550	13,750	14,500	
Vacuum bottle	"	70	1,750	1,350		Bicycle (Japan)	"	850	21,250	18,000	
Soap	"	40	1,000	570		Motorcycle	"	6,200	155,000	135,000	
Powder soap	box	8	200	85		<u>14. Construction material</u>					
Toilet paper	no.	3	75	60		Cement	ton	-	14,000	19,000	
Match	10 boxes	2	50	50		Steel bar (9 mm dia.)	"	3,080	77,000	78,000	
<u>9. Medicals</u>											
Aspirin	tab.	0.10	2.5	7.5		Steel bar (6 mm dia.)	"	3,250	81,250	100,000	
Quinine	"	0.05	1.25	7.5		Steel plate		20.5		380	
<u>10. Charges</u>											
Hair dressing (man)	person	5-7	125-175	150		Veneer		50	1,250	1,400	
Hair dressing (woman)	"	10-15	250-375	300		Timber		900	22,500	12,800	

Ferry Freight at Nong Khai

(Unit: tons)

Year	Freight from Laos	Freight to Laos
1962	2,511	* 30,045
1963	5,328	30,822
1964	32,524	30,766
1965	2,328	38,540
1966	10,463	42,459
1967	34,858	83,095

Ferry Freight at Sri Chiang Mai in 1967

(Unit: tons)

Month	Freight from Laos	Freight to Laos
Jan.	6.7	6.7
Feb.	8.7	21.7
Mar.	3.1	177.9
Apr.	9.7	7.6
May	0.1	90.0
June	1.8	212.0
July	0.2	34.4
Aug.	11.0	89.4
Sep.	1.6	48.4
Oct.	7.5	195.7
Nov.	3.6	82.2
Dec.	4.4	18.5
Total	58.8	985.2

APPENDIX VII

OTHER PLANNING DATA

CONTENTS

	Page
(1) Other planning data collected in the first and the second phase investigations.....	153
(2) Highway construction programme in Laos	154
(3) Average daily traffic on highways in Thailand (1964)	155
(4) 7-year plan for highway construction and improvement in Thailand (1965-71)	155
(5) Railway construction programme in Thailand	156
(6) City plan of Vientiane	157

Other Planning Data

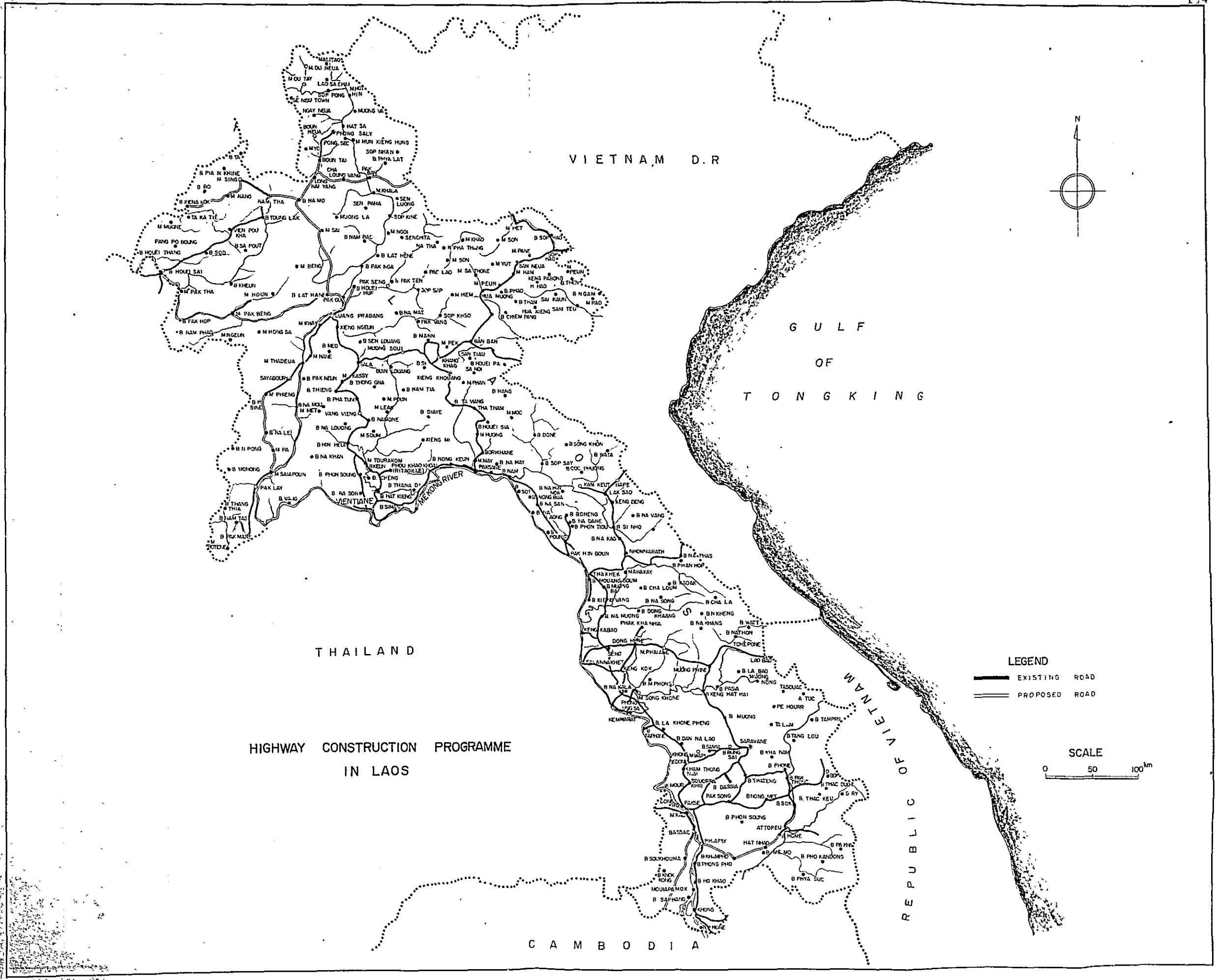
Many kinds of planning data listed below were collected during the First and the Second Phase Investigations. These data are useful for the feasibility study on Nong Khai/Vientiane Bridge Project, but are voluminous. Therefore, the data compiled in Appendices are limited to only the important ones for use as reference.

Collected data

- 1) Design standards of highway and railway in Thailand.
- 2) Over-all road network plans in Laos and Thailand.
- 3) Map of city plan of Vientiane.
- 4) Regulations for aviation in Thailand.
- * 5) Data relative to the expenses of compensation for various kinds of land, houses and valuable trees, current prices of local materials and daily or monthly salaries of technicians and laborers in and around Vientiane and Nong Khai.
- * 6) Data concerning the present car ferry services crossing the Mekong between Tha Naleng and Nong Khai.
- 7) Sources of electricity available for construction work near the bridge site.
- 8) Data in regard to the river-bed erosion due to the structure built in the river.
- 9) Seismic data
- 10) Maps covering a part of the project area or the whole

Remarks

- * These data were used for the estimation of the construction cost and for the study of the additional ferry construction as the alternative plan of the bridge, but were not compiled in this "Appendices" as well as in the Second Phase Report.



VIETNAM D.R.

GULF OF TONGKING

THAILAND

HIGHWAY CONSTRUCTION PROGRAMME IN LAOS

CAMBODIA

VIETNAM REPUBLIC

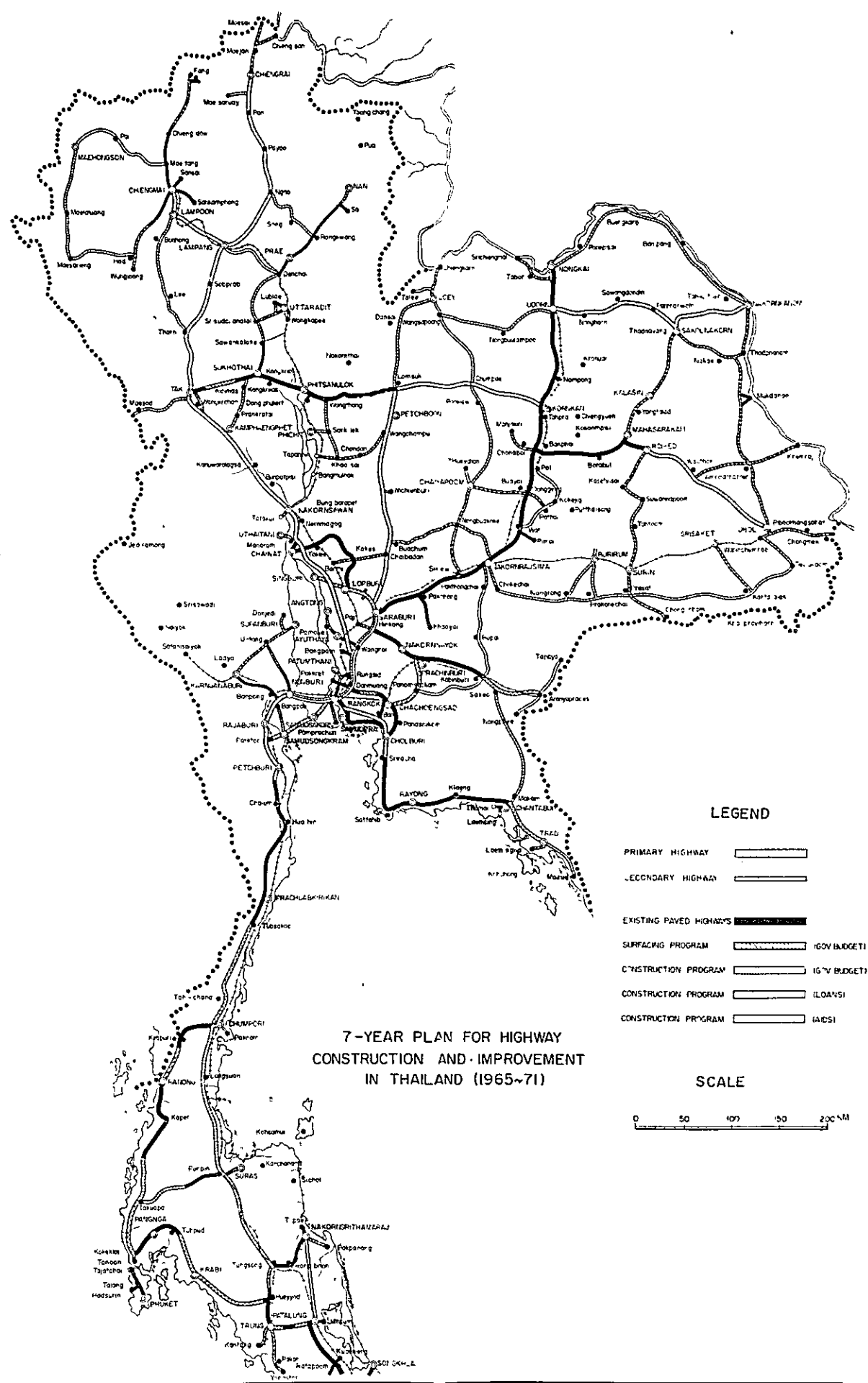
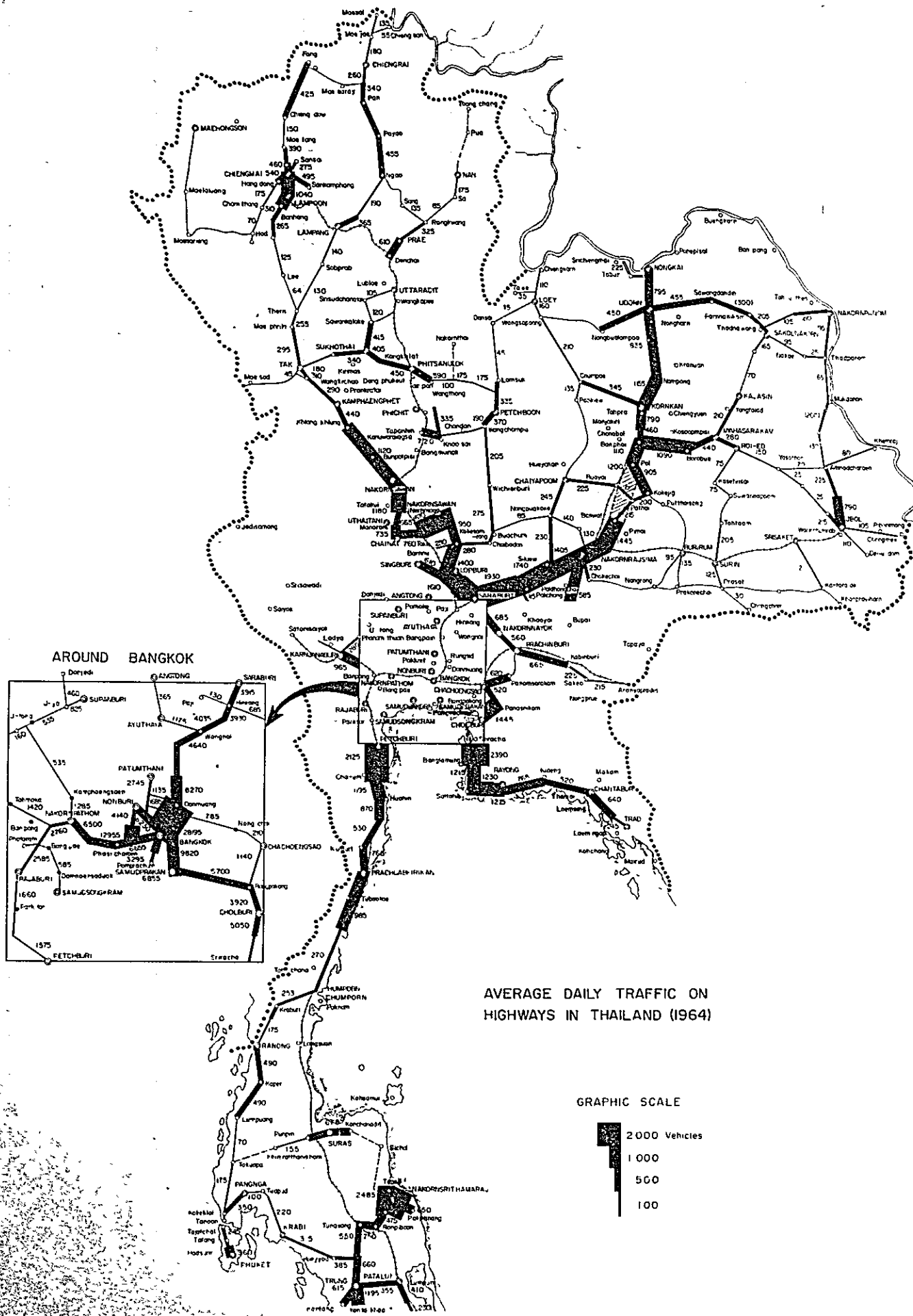
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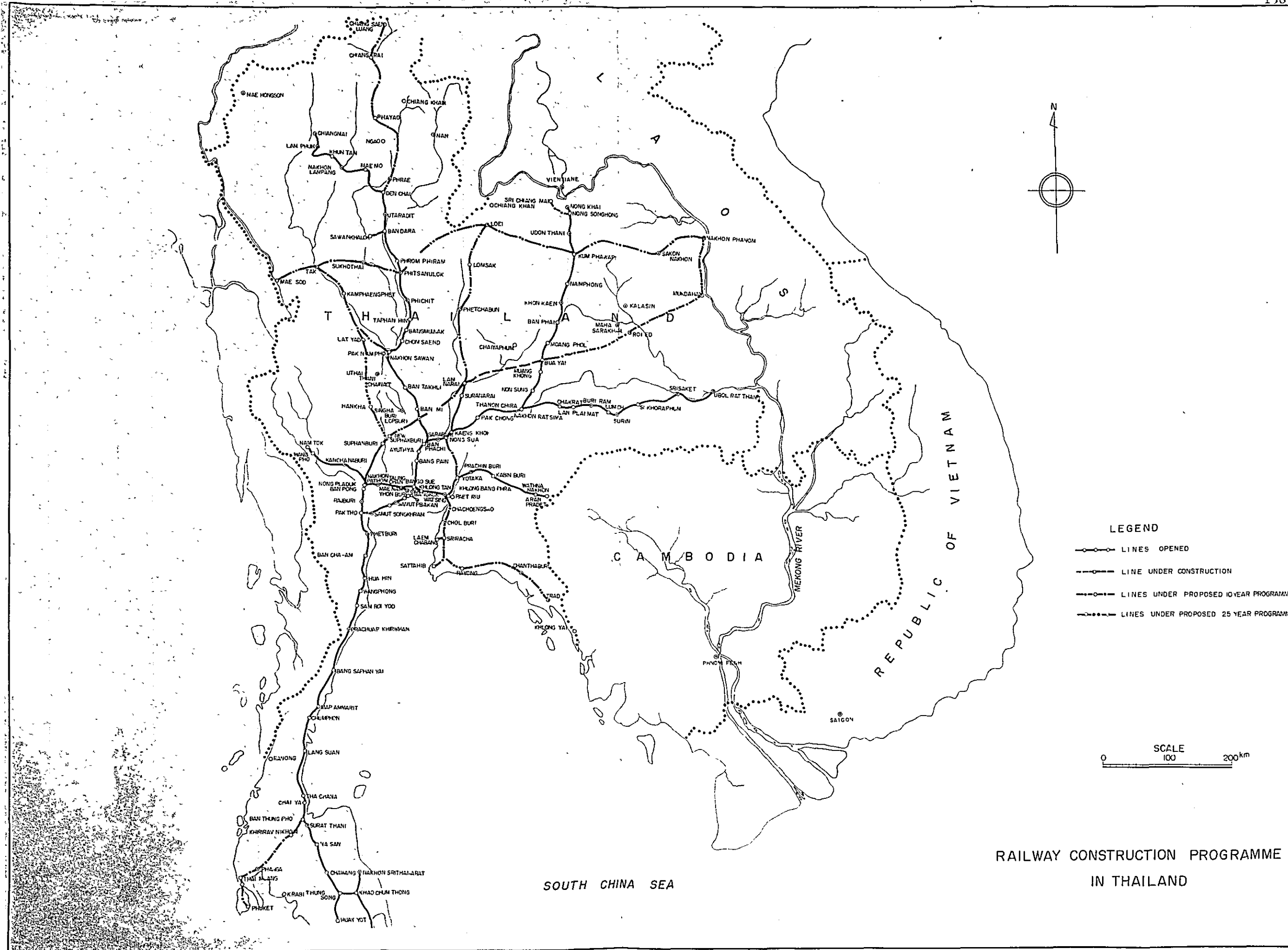
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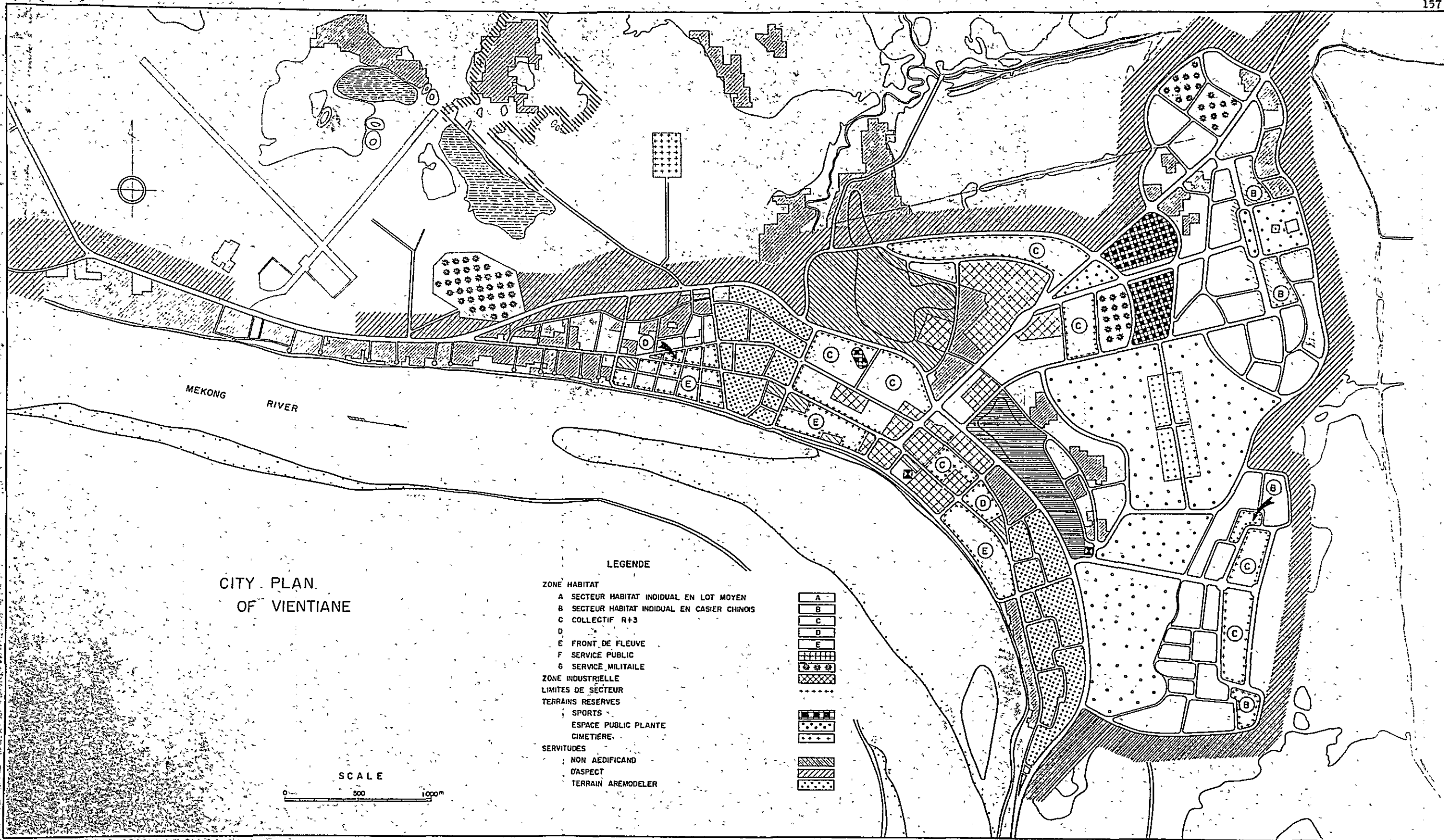
- - - PROPOSED ROAD

SCALE

0 50 100 km







CITY PLAN
OF VIENTIANE

LEGENDE

- ZONE HABITAT
 - A SECTEUR HABITAT INDIVIDUAL EN LOT MOYEN
 - B SECTEUR HABITAT INDIVIDUAL EN CASIER CHINOIS
 - C COLLECTIF R+3
 - D
 - E FRONT DE FLEUVE
 - F SERVICE PUBLIC
 - G SERVICE MILITAIRE
- ZONE INDUSTRIELLE
- LIMITES DE SECTEUR
- TERRAINS RESERVES
 - SPORTS
 - ESPACE PUBLIC PLANTE
 - CIMETIERE
- SERVITUDES
 - NON AEDIFICAND
 - D'ASPECT
 - TERRAIN AREMODELER

A
B
C
D
E
F
G
SPORTS
ESPACE PUBLIC PLANTE
CIMETIERE
NON AEDIFICAND
D'ASPECT
TERRAIN AREMODELER

