

Report

on the **Feasibility Study**

of the **Bangkok-Thonburi Ring Road Project Part II**

Royal Kingdom of Thailand

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March 1973

Overseas Technical Cooperation Agency

Government of Japan

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Preface

The Government of Japan, in response to the request of the Government of the Royal Kingdom of Thailand, carried out a survey on the Bangkok-Thonburi Ring Road Project, the study being assigned to the Overseas Technical Cooperation Agency.

The Agency, fully aware of the importance of the effect that the completion of the Bangkok-Thonburi Ring Road Project will have on the social and economic development of the Kingdom, despatched a survey team composed of experts on urban transportation problems to Bangkok for a period of about two months, starting from 20 July, 1972. While in Bangkok, the full cooperation of various Government departments made it possible for the survey to be carried out very smoothly. Subsequent works in Japan are now duly completed and this report is the product of the survey.

This report covers the route selection, traffic estimation, preliminary structural design and other technical considerations as well as the rough estimation of the construction cost of the Bangkok-Thonburi Ring Road which has been planned to provide relief for the deterioration in traffic situation and impediment in urban function caused by the recent rapid increase in vehicle traffic volume of the Greater Bangkok Metropolitan area.

It is our fervent wish that the results of the survey will contribute towards the development of the Greater Bangkok Metropolitan area and towards the promotion of international friendship between Japan and the Royal Kingdom of Thailand.

March, 1973

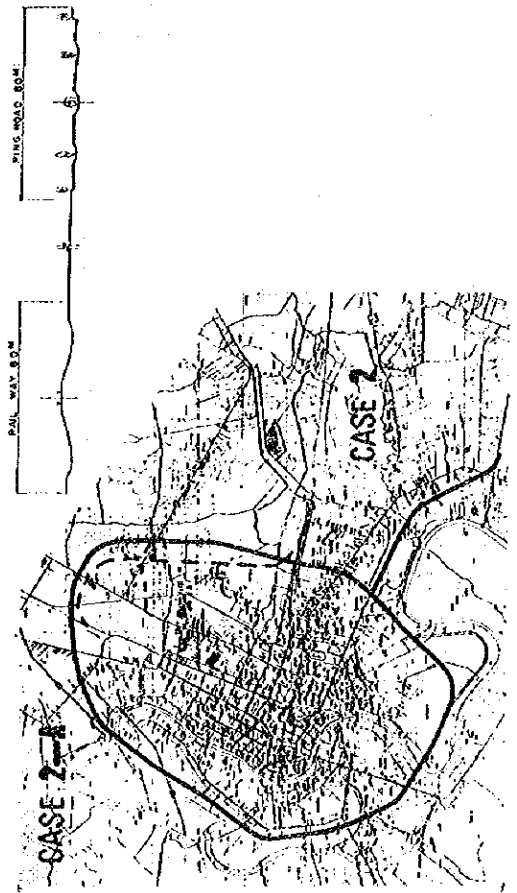
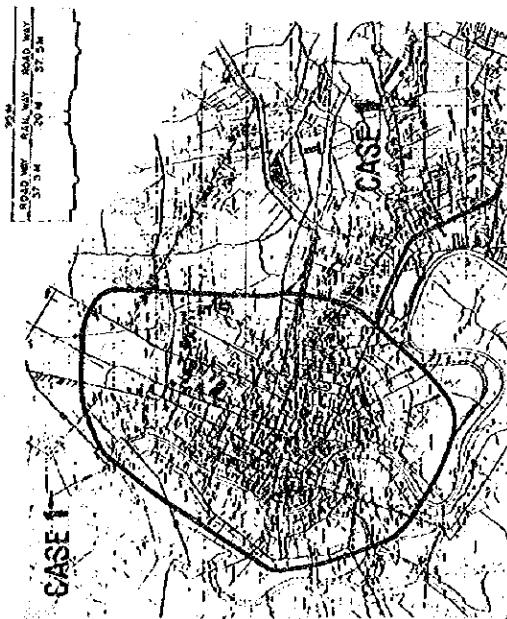


Keiichi Tatsuke

Director General

Overseas Technical Cooperation Agency

KEY PLAN



SUMMARY and CONCLUSIONS

1. The road network system in Bangkok is far from adequate to meet the traffic demand, and it is anticipated that the situation will further deteriorate in future. Early steps to improve the traffic situation are therefore absolutely necessary. However, minor steps, such as improvement of existing intersections, expansion of existing roads, or intensification of traffic regulations, will not be sufficient. Construction of new highways is necessary.

2. In the planning of a new urban roadway, the future development plan of the urban area and the trend of future traffic demand in relation to the existing road network are some of the points to be taken into consideration.

The road network system of Bangkok in the suburban area consists basically of linear radial roads, and building-up is developing along the roads. This results in many inconveniences in traffic flow. For example, the district including Din Daeng and Ratchaprarop road suffers from constant traffic congestion due to the concentration of traffic from the suburban area.

To cope with such traffic situation, we are convinced that the proposed Ring Road project is most effective.

3. The proposed Ring Road Part II as well as the Section 5, is an integral portion of the Rachadapisek Ring Road which will serve as a collector and distributor road for traffic to and from the densely built-up area of the city, and also as a bypass for through traffic. The Ring Road Part I is in the implementation stage. Unless the Part II as well as the Section 5 is also implemented, the roles of the Rachadapisek Ring Road will not be fulfilled.

From the results of our traffic and economic analysis, a conclusion is reached that the traffic demand for the proposed Ring Road is great and from the economical point of view the project is feasible. It is recommended therefore that the project be implemented as soon as possible.

4. A brief outline of the project is listed up as follows:

Ring Road Part II:

| | |
|---------------|----------|
| Total Length: | 27.59 km |
| Total Width: | 80 m |

7. Estimated benefits

On the completion of the Ring Road Part II and Section 5, the annual benefits for the key years are estimated as follows:

| | | | |
|-----------|---|-----|--------------------|
| year 1975 | : | 420 | million bahts/year |
| 1980 | : | 630 | " " |
| 1990 | : | 900 | " " |

8. Benefit-cost analysis

Assuming the investment of the whole cost in the initial year and a redemption period of 20 years, the results of benefit-cost analysis, as compared to the case of 'without' the Ring Road, are as follows:

| | | |
|--------------------------------------------|----|-------------|
| Benefit-cost ratio at discount rate of 10% | is | 1.51 |
| " | " | 12% is 1.31 |
| " | " | 15% is 1.09 |
| Internal rate of return | | is 16.6% |

9. The following schedule of stage construction is suggested for the efficient implementation of the project.

Order of priority of construction

1. Section 5 and Section 6 (Phetchaburi extension - Superhighway)
2. Section 7(a)(1) (Superhighway - Pracharat Road)
3. Section 7(a)(3) (Phetkasem Road - Tachang extension)
4. Section 7(a)(2) & 7(b) (Tachang Extension - Pracharat Road)

10. On the stage construction of the cross section, it is suggested that except for Section 7(a)(1), construction work will start with the high speed main roadway and the construction of the service roads deferred to later date. For Section 7(a)(1) (Superhighway-Pracharat Road), work will start with the service roads first and the main roadway to be continued on completion of the service roads.

11. It is suggested that work on Section 6 and Section 5 be started in 1975 and subsequently, work on a new section be started every year. By this schedule, the first sections will be open to traffic by 1977, the ring, with part of the cross section, completed by 1980, and the project wholly completed by 1984. However, the final schedule should be decided with consideration on the sources of finance, materials and manpower, as well as on popular demand.

12. The traffic analyses are carried out basing on theoretical base year OD matrices, actually surveyed data being unavailable. It is recommended that a review of the results be made when surveyed results are available.
13. Upon implementation of the project, it is recommended that trial embankments be carried out on sample locations before a decision is made on the construction method of earth works.
14. This report is the product of the utmost efforts made within the confine of time and resources, and we are convinced that it will serve as a good basis for the preliminary decision on the project. However, the Ring Road is a large scale project, and a study within confined time at this stage invariably results in many uncertainties to be confirmed in future studies made when more detailed data are available. For example, structures make up a large portion of the estimated construction cost. In this study, the types, methods of construction and cost estimations of structures were decided basing on past data. These assumptions should be verified after ample additional informations are available through boring, soil tests and other field surveys. It is recommended, therefore, that in the implementation stage, sufficient time and effort be appropriated for the re-assessment and re-evaluation of the major items basing on future up-to-date detailed data.

CONTENTS

INTRODUCTION

PART 1. ECONOMIC AND TRAFFIC STUDY

CHAPTER 1. GENERAL

| | | |
|-----|---------------------------|-----|
| 1-1 | General Outline | 1-1 |
| 1-2 | Geography | 1-6 |
| 1-3 | Climate | 1-8 |
| 1-4 | Population | 1-9 |

CHAPTER 2. ECONOMIC ACTIVITIES

| | | |
|-----|-----------------------------------|------|
| 2-1 | Primary Industry | 2-1 |
| 2-2 | Other Industries | 2-7 |
| 2-3 | Tourism | 2-13 |
| 2-4 | Future Development Plan | 2-17 |

CHAPTER 3. TRAFFIC ANALYSIS

| | | |
|-----|-----------------------------------------------------------------------|------|
| 3-1 | General Description of Transportation System in Thailand | 3-1 |
| 3-2 | Existing Road Network System in Bangkok | 3-24 |
| 3-3 | Analysis of Present Traffic Situation in Bangkok | 3-3 |
| 3-4 | Estimation of Future O-D Matrices | 3-76 |
| 3-5 | Estimation of Traffic Volume on the Ring Road | 3-99 |

CHAPTER 4. ECONOMIC ANALYSIS

| | | |
|-----|---------------------------------|------|
| 4-1 | Introduction | 4-1 |
| 4-2 | Cost Analysis | 4-5 |
| 4-3 | Benefit Analysis | 4-11 |
| 4-4 | Cost-Benefit Analysis | 4-23 |

PART 2 ENGINEERING STUDY

CHAPTER 5 SOIL MECHANICAL INVESTIGATIONS

| | | |
|-----|-----------------------------------------------------|------|
| 5-1 | Introduction | 5-1 |
| 5-2 | Geology | 5-2 |
| 5-3 | Subsurface Conditions | 5-6 |
| 5-4 | Engineering Properties of Subsurface Soil | 5-21 |
| 5-5 | Engineering Problems | |
| 5-6 | Pavement | 5-38 |
| 5-7 | Summary and Conclusions | 5-43 |

CHAPTER 6. HYDROLOGICAL INVESTIGATIONS

| | | |
|-----|----------------------------------------------|------|
| 6-1 | General Description | 6-1 |
| 6-2 | Floods in the Project Area | 6-5 |
| 6-3 | Considerations and Recommendations | 6-12 |

CHAPTER 7. HIGHWAY ENGINEERING

| | | |
|-----|---------------------------|-----|
| 7-1 | Introduction | 7-1 |
| 7-2 | Basic Data | 7-3 |
| 7-3 | Design Standard | 7-4 |

| | | |
|------|-------------------------------------------------------------|------|
| 7-4 | Required Number of Traffic Lanes for Ring Road | 7-9 |
| 7-5 | Route Selection for Ring Road Part II | 7-12 |
| 7-6 | Route Selection for Section 5 | 7-26 |
| 7-7 | Interchange | 7-34 |
| 7-8 | Structures | 7-41 |
| 7-9 | Estimation of Construction Cost | 7-50 |
| 7-10 | Maintenance | 7-51 |
| 7-11 | Considerations on Stage Construction | 7-52 |

Appendix

INTRODUCTION

I-1 General Background

The volume of vehicle traffic in the Bangkok-Thonburi Metropolitan area has been increasing at a tremendous rate for the past few years. On the other hand, the improvement and new construction of roads has not been able to maintain the same high pace. As a result, congestion has become conspicuous in most of the road sections for a large portion of the day.

Many new roads have been included in various future development plans of the Metropolitan area. The Rachadapisek Ring Road, a new loop roadway surrounding the built up portion of the Metropolitan area, has been one of the main new roadways included in the various plans. In late 1971, a contract was awarded by the Highway Department of the Royal Thai Government to a foreign consultant for the preliminary design of the southern portion of the Ring Road. This portion starts from the Tha Phra intersection in Thonburi, curving via the vicinity of the Krung Thep Bridge through the Yannawa orchard district to end at the vicinity of the New Petchaburi-Soi Asok junction. The decision on the northern portion of the Ring Road was deferred pending the outcome of a feasibility study.

I-2 Appointment of Consultants

In early 1972, an application for assistance in the feasibility study of the northern portion of the Ring Road was made by the Royal Thai Government to the Japanese Government. This application was entertained and the onus was placed with the Overseas Technical Cooperation Agency of Japan (OTCA) which is the Japanese Government sponsored organization in the implementation of overseas technical assistance. The Pacific Consultants

International, a general consultants organization in all fields of civil engineering works, was appointed by the OTCA to executive the feasibility study under a contract signed between OTCA and the consultant on 19 July, 1972. Work on the study commenced in Bangkok, Royal Thai Kingdom on the 20th July, 1972. The study period was for a 7-month period from 20 July, 1972 to 28 February, 1973.

I-3 Scope of Work

The Scope of Work as prepared by the Highway Department of the Royal Thai Government and presented to the Japanese Government are quoted as follows:

(6) Scope of Work of Experts

Engineering and Economic Evaluation

- 1) The Engineer shall examine the site of the proposed route and shall peruse the studies, surveys, plans and sections, reports, soil and subsurface exploration data, traffic studies and all other available information relevant to the project and which has already been prepared or carried out by others.
- 2) The Engineer shall carry out the necessary traffic and economic studies, additional to those which may be available from others, to determine the appropriate standards for construction and for stage construction not only of the carriageways but of the interchanges along the route.
- 3) After such examination and studies, the Engineer shall submit twenty copies of an evaluation report (hereinafter called 'the evaluation report') which will present an Engineering and Economic evaluation to propose and

justify the recommendations for alignment and stands taking into account the work already done and will propose the methods and principles to be followed by the Engineer in the design of the project, giving details of staff, equipment and timing for the various parts of the work. The report will also highlight difficulties which the Engineer foresees in completing the design or construction, and will suggest Government action that may be required to overcome them. Unless informed in writing to the contrary within four weeks of submission, the Engineer may assume that the proposals contained in his Evaluation Report are accepted by Government.

- 4) In drawing up his Evaluation Report, the Engineer shall observe the following points:
 - a) The standards of design shall be justified in terms of costs and benefits and shall be fully in accordance with generally accepted international practice, taking account the present and potential physical and economic state of development of area and of the country. Where for any reason it appears desirable to depart significantly from such practice, the Engineer shall in his Evaluation Report (or later, as may be appropriate) present sound arguments in support of his decision. These arguments may be based on economic analysis of possible costs and benefits, as well as on engineering or other reasons. The objective at all times shall be to reach an optimum solution to problems within the confines of time and resources available.

- b) Part at least of this route will be financed locally, and it is expected that construction may be by contractors already operating in Thailand.
The Engineer will use designs and methods which will minimise the use of foreign exchange and which can be readily undertaken by local labour.
 - c) The Engineer will be expected to take full account of the presence of utilities within or near the Right-of-Way. He will investigate their location, so far as is practicable, and show them on the plans. The design shall allow for the presence of utilities wherever necessary and shall be done in such a way as to minimise their removal or re-location.
- 5) As a part of the Evaluation Report, the Engineer will:
- a) Propose suitable construction lots for approval by the Government.
 - b) Show the final alignment of the road.
 - c) Show the general outline of junction designs, with reasons for the type adopted.
 - d) Give the outline design of the major structures proposed.
 - e) Give a general report on the major engineering and other decisions taken during the project to date, with supporting arguments, where appropriate, for the decisions taken.

I-4 Conduct of the Study

The execution of the feasibility was carried out by experts from the Pacific Consultants International under the supervision of a Supervisory Committee composing of experts from various departments of the Japanese Government. The study team despatched by the consultants spent variously from one to four months in Bangkok, starting from 20 July, 1972, for fact finding, data collection and discussion with officials of various relevant Departments of the Royal Thai Government. Subsequent works were carried out in the Consultants' Head Office in Tokyo whereby the full support of the whole engineering staff of the consultants as well as the consultants' electronic computer facilities was thrown in. The representatives of the supervisory committee made two separate two-week visits to Bangkok during the study team's working period in Bangkok for confirmation of essential points of decision with the Department of Highways. A team of two members each from both the Supervisory Committee and the consultants was despatch again to Bangkok in December, 1972 for report and discussion on the intermediate findings of the study. The Supervisory Committee and the study team despatch to Bangkok are of the following composition:

(A) Supervisory Committee

| | |
|------------------------------------|---|
| Chairman | 1 |
| Highway & Transportation Engineers | 3 |
| Economist | 1 |
| Structure Engineer | 1 |
| Hydrological Engineer | 1 |
| Coordinator | 1 |

(B) Study Team of the Pacific Consultants International

| | |
|----------------------------------------|---|
| Project Manager | 1 |
| Deputy Project Manager & Urban Planner | 1 |
| Highway Engineer | 1 |
| Hydrological Engineer | 1 |
| Structural Engineer | 1 |
| Transportation Economist | 1 |
| Transportation Engineer | 1 |
| Soil & Material Engineer | 1 |
| Office Manager | 1 |

I-5 Organization of the Study Team

The full list of the study team and Supervisory Committee is as follows:

(A) Supervisory Committee

Chairman:

Mr. Tsutomu Takahashi
Director, Toll Road Div., Road Bureau,
Ministry of Construction, Japan

Highway & Transportation Engineers:

Mr. Shigeru Motoyama
Special Grade Engineer, Toll Road Div., Road Bureau,
Ministry of Construction, Japan

Mr. Hiroyuki Wada
Special Grade Engineer,
Supervisor's Office for Urban Expressway
Public Corporation, City Bureau,
Ministry of Construction, Japan

Mr. Hiroshi Mitani
Deputy Director, Planning Div., Road Bureau,
Ministry of Construction, Japan

Economist:

Mr. Terufusa Hiromatsu
Deputy Manager, Economic Research Department,
Japan Highway Public Corporation

Hydrological Engineer:

Mr. Hiroaki Tamamitsu
Senior Engineer, International Cooperation Office,
Planning Bureau,
Ministry of Construction, Japan

Structural Engineer:

Mr. Osamu Tamaki
Chief, Engineering Div., Metropolitan Expressway
Public Corporation, Tokyo

Coordinator:

Mr. Fumio Higai
Overseas Technical Cooperation Agency

(B) Study Team of Pacific Consultants International

| | |
|--------------------------------------------|--------------------|
| Project Manager: | Yasuo Yanai |
| Deputy Project Manager & Urban Planner: | Fan Kai-Chang |
| Highway Engineer: | Takashi Miyakoshi |
| Hydrological Engineer: | Minoru Shibuya |
| Structural Engineer: | Yutaka Ishizuka |
| Transportation Economist: | Teruo Endo |
| Transportation Engineer: | Tomoyuki Matsumura |
| Soil & Material Engineer: | Toshinobu Akagi |
| Office Manager: | Jun Tsuboi |

I-6 Assistance from Government Department & Other Bodies in Thailand

In the course of the study, much valuable assistance, advice, opinions, and conveniences were received by the team from various governments departments and public and private

bodies in Thailand. We would like to express our appreciation to all, without whom, the study cannot be carried out so smoothly and successfully. A full list will be too long to be included here. So only the major bodies will be mentioned hereunder:

National Economic Development Board
Department of Highways
Department of Town and Country Planning
Department of Police
Department of Land Transport
Department of Harbours
Bangkok Municipality
Office of Metropolitan Traffic Planning
Express Transport Organization (ETO)
ECAFE
Asia Institute of Technology
Chulalongkorn University
Tammasett University
T. P. O' Sullivan & Partners
Thai Engineering Consultants
Japan Trade Centre, Bangkok
Japan Chamber of Trade & Industry, Bangkok Office
Tachang Bridge Construction Office
Nihon Tsuun, Bangkok Office
Embassy of Japan, Bangkok
Japan Overseas Technical Cooperation Agency, Bangkok Office

I-7 Reference Materials

In the study, extensive reference was made of many data, statistical records, reports and essays. A list of these is presented in the appendices.

I-8 The Study Approach

The study period was for only seven months. Over study approach was natural limited to the methods which are physically possible during the seven-month period.

The focus of the study was centred on the Ring Road only. The economic feasibility was considered from the point of whether the project by itself, would be feasible against the benefits that would be derivable through the implementation of the project. The comparison was between 'with' and 'without' the Ring Road. The study on priority was made among various alternative stages of construction of the Ring Road. No attempt was made of comparative feasibility between the Ring Road and any other competitive transportation projects in a comprehensive transportation plan, as such a study will involved enormous work and is completely out of the scope of this study.

It was not possible to carry out any comprehensive traffic survey, due to shortage of time, and study was limited to analyses of existing data. Data of past Origin-Destination traffic survey was not available. As a result, purely theoretically O - D matrices had to be synthesized and the results checked against existing data of cross section traffic counts.

In cost analysis, the results of the "Road Users Costs in Thailand" (T. P. O' Sullivan & Pastners) were adopted unqualified, with only minor modifications to suit the propose of the Study. No attempt was made to verify the results.

In the case of the engineering part of the study, extensive field reconnaissance was made of the routes of the various proposed alternatives, far beyond what is usually called for in a feasibility study. This is due to the fact that a great part of the routes were proposed through highly built-up area and the cost of demolition was expected to make up a substantial percentage of the construction cost of the project.

Suggestions and advices of the Department of Highways of the Royal Kingdom were sought for in the decision on matters of major importance. Also, for the works carried out in Tokyo, the intermediate findings of the consultant were presented for criticism and advices of the Supervisory Committee during the regular sessions of the Supervisory Committee Meetings.

PART 1

ECONOMIC AND TRAFFIC

STUDY

CHAPTER 1. GENERAL

1-1 General Outline

Economically speaking Thailand is relatively developed, compared to its Asian neighbors. Basically an agricultural state, it has been striving on the one hand to diversify and to increase the agricultural products, and on the other to introduce foreign capital investments and aids for the development and advancement of the manufacturing industry. Among other endeavors, an economic development five-year plan was put in effect in 1967. Preceded by a six-year plan enforced in 1961, this is the second of its kind carried in pursuit of the nation's economical modernization. However, in spite of all the efforts, a downtrend in the growth rate of economy began since the end of the 1960's, after a period of steady growth seen in the earlier part of that decade. This became more pronounced after the year 1970. The cause of this trend is attributed to the fact that in contrast to the rapid growth of imports which reflects industrial development and growth of consumption, exports stagnated, the Vietnam special order decreased, private foreign investments rapidly increased, thus causing the development of unfavorable international balance of payments, and bringing forth an ever growing trade deficit. This is an urgent problem to be solved in order to establish a sound economy.

Fig. 1-1 Per capita national income of Asian nations. (in U. S. dollars) 1970

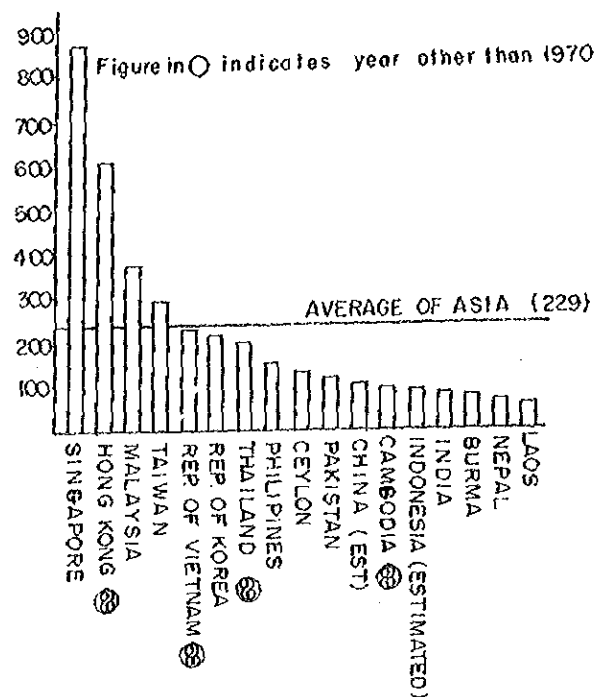


Fig. 1-2 Trend in per capita national income (in bahts)

| Year | Central Area | | Whole Country | |
|------|---------------|-------|---------------|-------|
| | Annual income | Index | Annual income | Index |
| 1960 | 3440 | 100 | 2070 | 100 |
| 1961 | 3530 | 103 | 2110 | 102 |
| 1962 | 3680 | 107 | 2210 | 107 |
| 1963 | 3850 | 112 | 2310 | 112 |
| 1964 | 4030 | 117 | 2350 | 114 |
| 1965 | 4290 | 125 | 2480 | 120 |
| 1966 | 4560 | 133 | 2690 | 130 |
| 1967 | 4850 | 141 | 2760 | 133 |
| 1968 | 5100 | 148 | 2900 | 140 |
| 1969 | 5400 | 156 | 3080 | 149 |
| 1970 | 5550 | 161 | 3160 | 153 |

Fig. Trend in per capita national income

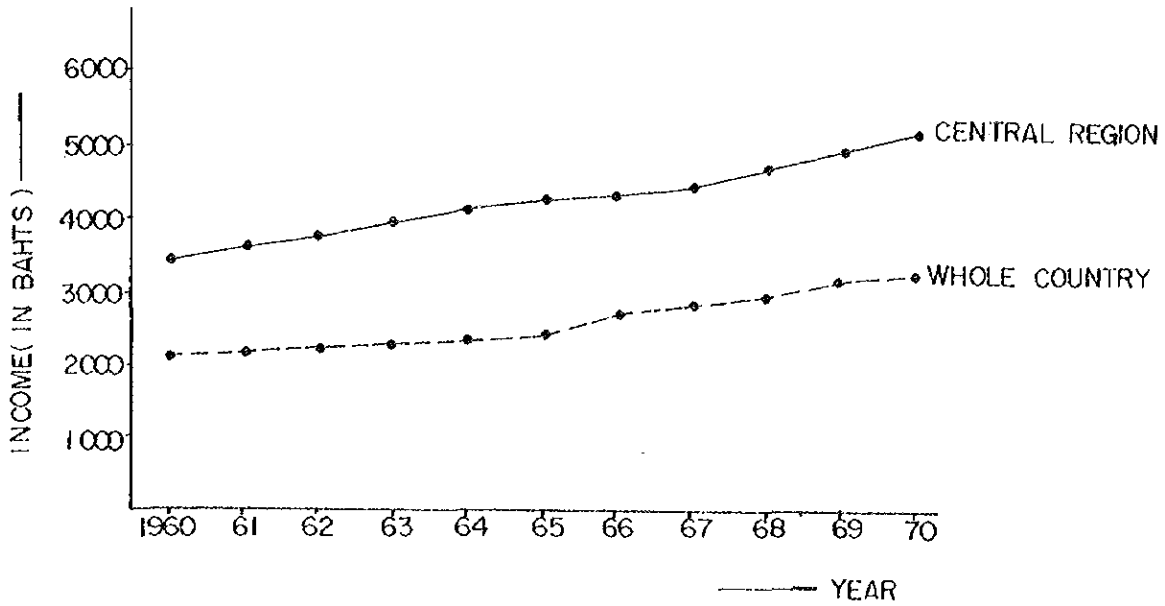


Fig. 1-4 Composition of G.N.P. (1962 value)

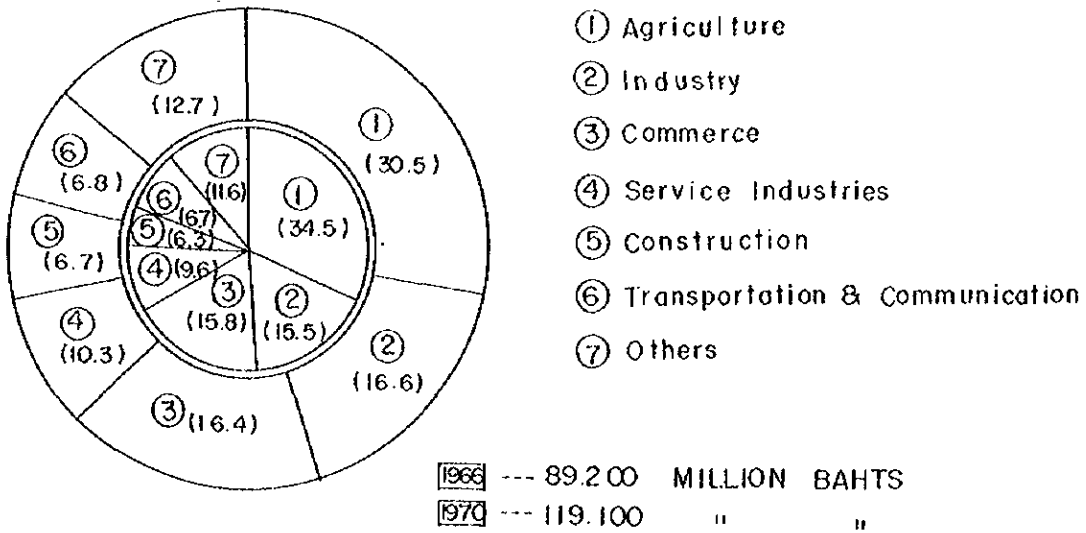


Fig. 1-5 Growth rate of G. N. P. and per capita national income

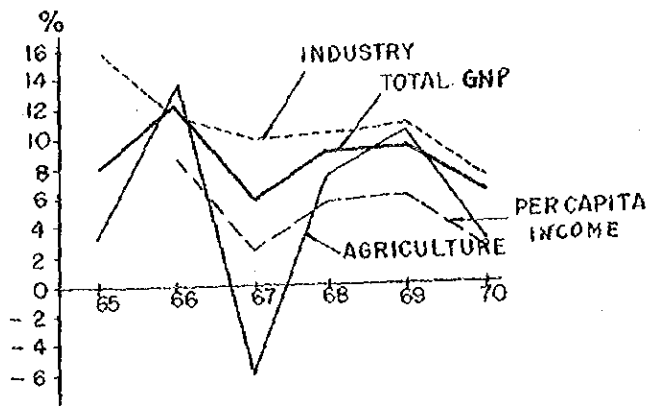


Fig. 1-6 Trend in gross national production of Thailand by industry
(based on 1962 price)

(in million baths)

| Year Industry | 1966 | | 1967 | | 1968 | | 1969 | | 1970 | | 1971 (estimated) | | 1966 - 1970 Average rate of growth |
|------------------------------------------|--------|-------|--------|-------|---------|-------|---------|-------|---------|-------|---------------------|-------|------------------------------------------|
| | Amount | % | Amount | % | Amount | % | Amount | % | Amount | % | Amount | % | |
| Agriculture | 30,785 | 34.5 | 28,971 | 30.8 | 31,091 | 30.3 | 34,234 | 30.5 | 35,224 | 30.5 | 37,300 | 29.5 | 5.6 |
| Mining | 1,418 | 1.6 | 1,630 | 1.7 | 1,780 | 1.7 | 1,912 | 1.7 | 1,984 | 1.7 | 2,100 | 1.7 | 10.8 |
| Manufacturing | 13,795 | 15.5 | 15,157 | 16.1 | 16,680 | 16.2 | 18,456 | 16.4 | 19,820 | 16.6 | 21,400 | 16.9 | 9.9 |
| Construction | 5,604 | 6.3 | 6,669 | 7.1 | 7,266 | 7.1 | 7,599 | 6.8 | 8,014 | 6.7 | 8,400 | 6.6 | 4.3 |
| Power & water services | 809 | 0.9 | 982 | 1.0 | 1,189 | 1.2 | 1,428 | 1.3 | 1,681 | 1.4 | 1,900 | 1.5 | 21.9 |
| Transportation & communication | 6,013 | 6.7 | 6,524 | 6.9 | 686.3 | 6.7 | 7,638 | 6.8 | 8,131 | 6.8 | 8,600 | 6.8 | 7.8 |
| Commerce | 14,133 | 15.8 | 15,877 | 16.9 | 17,249 | 16.8 | 18,819 | 16.7 | 19,514 | 16.4 | 20,500 | 16.2 | 8.8 |
| Banking, insur- ance & real estate | 2,620 | 2.9 | 3,068 | 3.3 | 3,565 | 3.5 | 4,124 | 3.7 | 4,749 | 4.0 | 5,100 | 4.1 | 17.3 |
| Housing | 1,931 | 2.2 | 2,009 | 2.1 | 2,091 | 2.0 | 2,187 | 1.9 | 2,304 | 1.9 | 2,400 | 1.9 | 4.4 |
| Administrative & defence services | 3,542 | 4.0 | 3,827 | 4.1 | 4,363 | 4.3 | 4,765 | 4.2 | 5,366 | 4.5 | 5,700 | 4.5 | 9.3 |
| Services | 8,539 | 9.6 | 9,397 | 10.0 | 10,441 | 10.2 | 11,216 | 10.0 | 12,305 | 10.3 | 13,000 | 10.3 | 9.7 |
| Total | 89,190 | 100.0 | 94,109 | 100.0 | 102,578 | 100.0 | 112,378 | 100.0 | 119,101 | 100.0 | 126,400 | 100.0 | 9.5 |
| Index | 100 | | 106 | | 115 | | 126 | | 134 | | 142 | | |

Sources: Bank of Thailand monthly bulletin; The Thai National Economic Development Board.

1-2 Geography

1-2-1 The Kingdom

Thailand lies in the central part of Indo-China peninsula. Its neighbours are Burma on the west and northwest, Malaysia on the south, Laos on the east with the Mekong river forming much of the boarder, and Cambodia on the southwest.

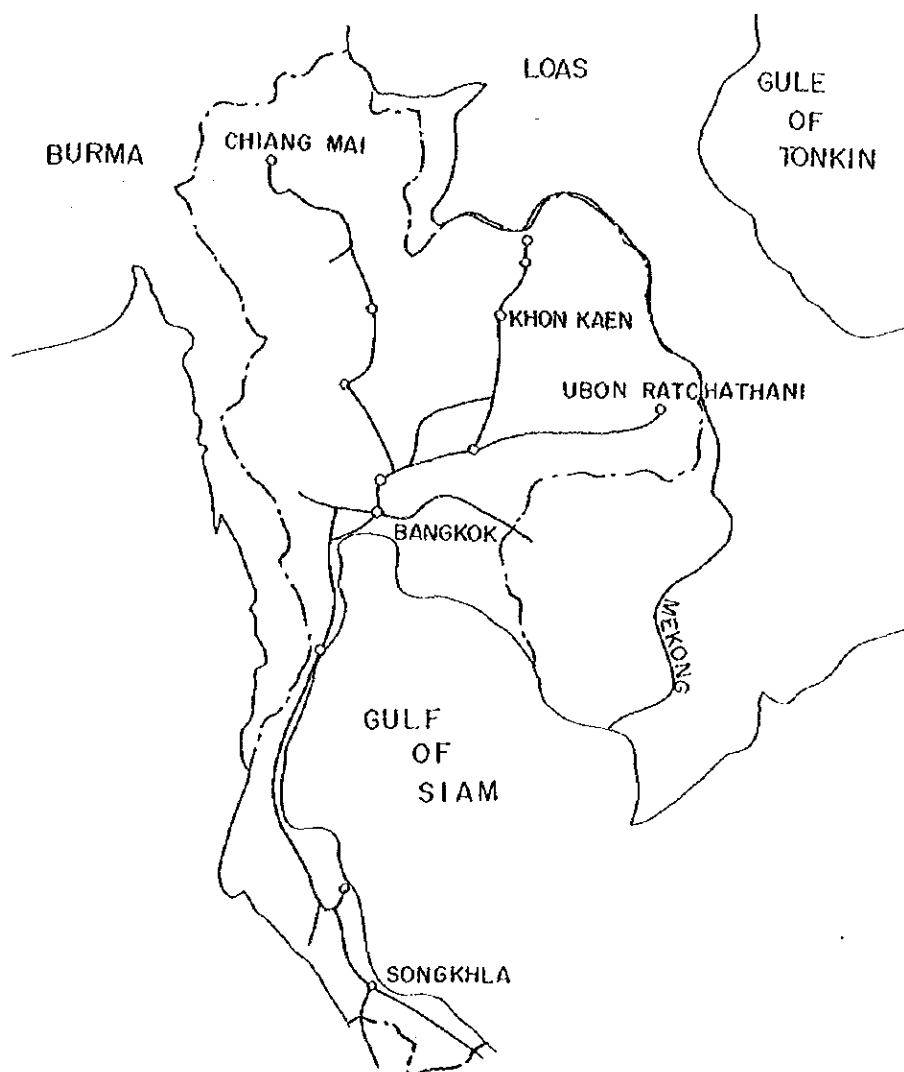
Western Thailand contains the Indo-China range which is the extension of the Himalayans. This extends southward, forming the Thanon-Toi-Chai, the Tenasserim, and the Bhukett ranges, and reaches further into the Malay peninsula. Through the central region penetrates the 1,200 km Chao Phraya River which begins in the northern mountains, flows southward across this alluvial delta plain, the rice basket of Thailand, and empties into the Gulf of Siam. In northeastern Thailand stretches the Korat Plateau which is 100 - 200 m above sea level.

1-2-2 The Metropolitan region

Bangkok lies in the vast central alluvial plain formed by the Chao Phraya River, and is situated on the bank of this river near the Gulf of Siam. Its growth as the capital of Thailand dated from 1782, and is also the commercial and cultural center of this nation today. Thonburi is situated across the Chao Phraya River from Bangkok and was formerly the capital before Bangkok took over its place, and now forms part of the Greater Bangkok Metropolitan. All transportation systems of Thailand center at these two cities. Railways and highways networks radiate from Bangkok to all parts of the country. Don Muang International Airport in Bangkok, which is an air center of South East Asia, is the aviation

center of Thailand for both domestic and international airways. The seaport of Bangkok, located on the mouth of Chao Phraya River, serves as the main access to Thailand from the sea and the center of seaways linking Bangkok with northern and central parts of Thailand.

Fig. 1-7 Location of Thailand



1-3 Climate

The entire territory of Thailand lies in the tropical monsoon region, with two seasons in a year, the rainy (May - Nov.) & the dry season (Nov. - Apr.)

However, even during rainy seasons, it is seldom to have rain throughout the day, but, rainfall is received in the form of heavy squalls which only continue for 1 - 2 hours.

Fig. 1-8 Features of local climates

| Place | Yearly average temperature | Average highest temperature | Average lowest temperature | Average yearly rainfall | Dry season | Rainy season | Hot season |
|------------------------------|----------------------------|-----------------------------|----------------------------|-------------------------|-------------|--------------|------------|
| Chiang Mai (Northern Region) | 21.4 - 28.7°C | 35.7°C | 13.4°C | 1245.6 mm | Nov. - Apr. | May - Oct. | Feb. - May |
| Bangkok (Central Region) | 25.6 - 30.1 | 34.8 | 20.2 | 1469.9 | Nov. - Apr. | May - Oct. | Mar. - May |
| Songkhla (Southern Region) | 26.5 - 28.9 | 33.2 | 23.8 | 2231.3 | Dec. - Mar. | Apr. - Nov. | Apr. - May |

Source: "Economical outline of Thailand, 1972"
 Edited by The Bangkok Japanese Chamber of Commerce and Industry.

The above table gives the features of local climates of 3 cities each representing a geographical region. Among these, Bangkok has the highest yearly average temperature and Chiang Mai the lowest.

1-4 Population

With a territory of 514,000 km², Thailand had a population of 34,152,000, according to the census of 1970. Since the population was 26,392,000 at the 1960 census, it had a population growth of 7,760,000 during the last decade.

A considerable population concentration is seen in the region of Bangkok and Thonburi cities which contained 8.9% of the population at the 1970 census.

Classified by amphoe, Dusit had the highest rate of population growth in the Metropolitan area between 1964 and 1970 of 67.7%, followed by Bangkhen, Bangna, Phayathai, Samsen. The recent trend in population change is that the highly populated urban center of Bangkok is slowing down in the rate of population growth, whereas such suburban areas as the eastern area (Bangkhen) around Phetchaburi Road, Sukhumwit Road and Rama 4 Road, and the northern area (Dusit) including Pahol Yothin Road, Mittraphap Road and Rama 5 Road increase in a striking pace.

Fig. 1-9 Estimated population of Thailand (By sex)

1960 - 1970

(in persons)

| Year | Male | Female | Total | Index |
|-------------|------------|------------|------------|-------|
| 2503 (1960) | 13,221,000 | 13,171,000 | 26,392,000 | 1.00 |
| 2504 (1961) | 13,599,000 | 13,569,000 | 27,168,000 | 1.03 |
| 2505 (1962) | 13,977,000 | 13,967,000 | 27,944,000 | 1.06 |
| 2506 (1963) | 14,355,000 | 14,365,000 | 28,720,000 | 1.09 |
| 2507 (1964) | 14,733,000 | 14,763,000 | 29,496,000 | 1.12 |
| 2508 (1965) | 15,111,000 | 15,161,000 | 30,272,000 | 1.15 |
| 2509 (1966) | 15,489,000 | 15,559,000 | 31,048,000 | 1.18 |
| 2510 (1967) | 15,867,000 | 15,957,000 | 31,824,000 | 1.21 |
| 2511 (1968) | 16,245,000 | 16,355,000 | 32,600,000 | 1.24 |
| 2512 (1969) | 16,623,000 | 16,753,000 | 33,376,000 | 1.27 |
| 2513 (1970) | 17,002,000 | 17,150,000 | 34,152,000 | 1.30 |

Note: Table prepared with an assumed constant growth rate of 3.1.

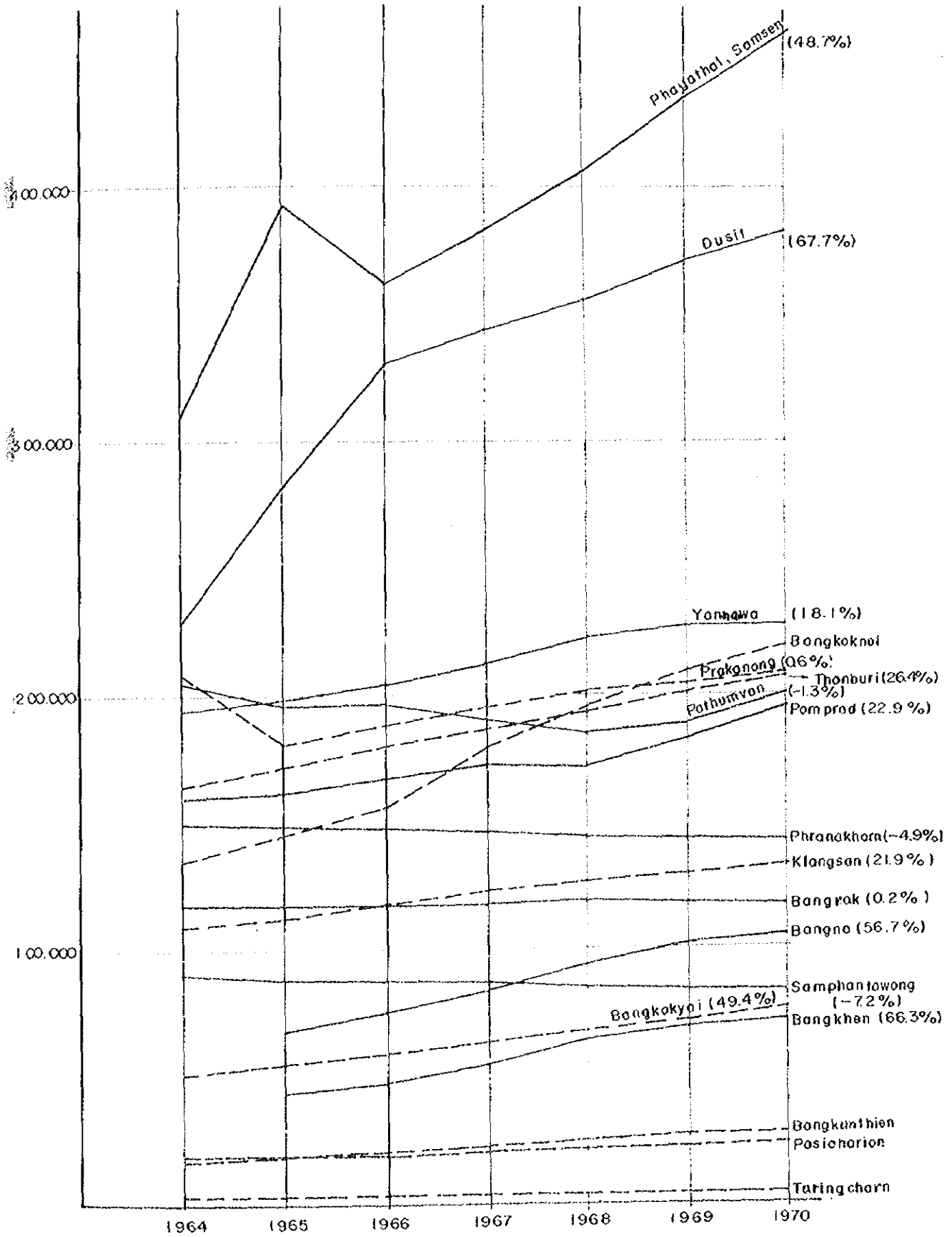
Source: Statistical year Book Thailand.

Fig. 1-10 Area and Population

| | Area (Km ²) | Population (1000 persons) | Density (Persons/Km ²) |
|---------------|-------------------------|---------------------------|------------------------------------|
| Bangkok City | 124.7 | 2,132 | 17,097 |
| Thonburi City | 51.0 | 919 | 18,020 |
| (1) Total | 175.7 | 3,051 | 17,365 |
| (2) Thailand | 514,000 | 34,152 | 66 |
| Share (1)/(2) | 0.034% | 8.9% | |

Source: Statistical year Book Thailand 1970

Fig. 1-11 Trend of population increase in Bangkok by amphoes



CHAPTER 2. ECONOMIC ACTIVITIES

2-1 Primary Industry

2-1-1 Agriculture

During the last decade the relative importance of agriculture among all industries had decreased due to the progress of industrialization and development of the tertiary industry. However a comparison of the total agricultural product and the gross domestic product which were 34,234 million Bahts (based on 1962 price) and 112,378 million Bahts respectively, for the year 1969 shows that agriculture remains still a dominating industry. Rice crop is the staple agricultural product. Though the central region including the Chao Phraya River delta plain remains the biggest rice producing center, total area of rice crop land in other parts of the country surpasses that of this region in recent years. Productivity of the central plain however is greater, so much so that more than half of the rice produced in the country is yielded in this region.

Natural conditions are not very favorable for rice production. The amount of precipitation needed for good yield of tropical rice crop, 1600 - 1800 mm is received only in the peninsular portion and the northeastern area along the Mekong river. Rice culturing in other areas rely much on artificial irrigation and flooding of rivers. In view of this, production of other crops, especially of farm product such as maize, has been encouraged in recent years, and agricultural industry as a whole is thus heading towards diversification.

Table 2-1 Present ratio of land use

(in square kilometers)

| | Forest | Lakes and marsh | Agricultural land | Others | Total |
|---------|---------|-----------------|-------------------|---------|---------|
| Area | 264,709 | 2,072 | 119,308 | 127,911 | 514,000 |
| Percent | 51.5 | 0.4 | 23.2 | 24.9 | 100.0% |

Breakdowns of agricultural land (in km²)

| | Farm-land | Rice field | Afforestation | Others | Total |
|---------|-----------|------------|---------------|--------|---------|
| Area | 105,611 | 65,424 | 6,638 | 7,059 | 119,308 |
| Percent | 88.5 | 54.8 | 5.6 | 5.9 | 100.0% |

Table 2-2 Harvest of agricultural products

(in 1000 metric tons)

| Year Product | 1965 | 1966 | 1967 | 1968 | 1969 | 1969/ 1965 |
|-----------------|---------|----------|---------|----------|----------|---------------|
| Unhulled rice | 9,217.0 | 11,845.0 | 9,594.0 | 10,772.0 | 13,346.0 | 1.45 |
| Rubber | 217.4 | 218.0 | 220.0 | 258.8 | 281.8 | 1.30 |
| Maize | 1,021.3 | 1,122.0 | 1,250.0 | 1,500.0 | 1,700.0 | 1.67 |
| Cassava | 1,475.0 | 1,891.7 | 2,062.5 | 2,611.5 | 2,700.0 | 1.83 |
| Sugar cane | 4,480.0 | 3,827.0 | 4,526.0 | 5,846.1 | 6,740.5 | 1.50 |
| Green bean | 124.8 | 131.8 | 122.5 | 183.8 | 202.1 | 1.62 |
| Peanut | 130.6 | 219.9 | 132.1 | 157.9 | 180.5 | 1.38 |
| Soy bean | 19.1 | 37.9 | 52.8 | 44.8 | 61.2 | 3.20 |
| Sesame | 18.3 | 19.9 | 22.7 | 22.1 | 22.1 | 1.21 |
| Coconut | 1,170.0 | 1,069.0 | 1,074.1 | 1,098.7 | 1,111.2 | 0.95 |
| Castor bean | 31.6 | 41.9 | 37.6 | 42.6 | 42.6 | 1.35 |
| Raw cotton | 59.8 | 88.8 | 80.7 | 131.5 | 92.2 | 1.54 |
| Jute | 8.7 | 10.9 | 7.4 | 4.4 | -- | -- |
| Tobacco | 75.5 | 88.4 | 78.0 | 85.5 | 80.0 | 1.06 |
| Kenaf | 528.6 | 661.4 | 421.4 | 316.8 | 355.0 | 0.67 |

Table 2-3 Harvested area and annual crop per hectare by product

(during 1966)

| Product | Harvested area (1000 ha.) | Annual crop (1000 ton) | Annual crop per hectare |
|------------|------------------------------|---------------------------|----------------------------|
| Rice | 6,949 | 11,845.0 | 1.70 t/ha |
| Rubber | 373 | 218.0 | 0.58 |
| Maize | 644 | 1,122.0 | 1.74 |
| Sugar cane | 160 | 3,827.0 | 23.92 |
| Coconut | 247 | 1,069.0 | 4.32 |
| Peanut | 153 | 219.9 | 1.44 |
| Raw cotton | 79 | 88.8 | 1.13 |
| Kenaf | 498 | 661.4 | 1.33 |
| Cassava | 130 | 1,891.7 | 14.55 |

2-1-2 Forestry

About 65 million acres, or 52% of the land of Thailand are covered with forest and pastureland, the majority of which being under state control. Forestry products include yang lumber (a species of lawan) resin, rattan and bamboo, etc.

Teak is traditionally an important item of Thailand's exports. It is a product of the northern region. Its production was about 250 - 330 thousand cu. m. in the middle 1950's, but had declined to 150 thousand cu. m. by 1966, after which slight increase had continued. Such slump in teak production is due to excessive lumbering in the past.

Teak had been the fourth principle item of exports until 1958, following rice, rubber and tin, but had become the seventh since 1961, being out ranked by farm products including maize. About 20,000 cu. m. are exported annually

during recent years.

Table 2-4 Trend in annual crop of forestry products

(in. 1000 m³)

| Product \ Year | 1960 | 1966 | 1967 | 1968 | 1969 |
|----------------|-------|-------|-------|-------|-------|
| Teak | 154 | 151 | 182 | 263 | 296 |
| Yang | 321 | 535 | 525 | 525 | 471 |
| Other timber | 791 | 1,356 | 1,622 | 1,788 | 1,607 |
| Firewood | 1,191 | 1,296 | 1,604 | 1,681 | 876 |
| Charcoal | 629 | 511 | 562 | 451 | 44 |

Source: 1) Economical outlook of Thailand, 1972

2) World trade guidebook.

2-1-3 Fishery

Fish is the main source of protein supply for Thai people. Coastal fishery develops over the full extent of the lengthy coastline. While fishing fleets operate mainly in the Gulf of Thailand, they also sail as far as the Gulf of Bengal to find their fishing ground during some seasons of a year.

The annual catch of salt-water fish increased strikingly, approximately 9 times between 1960 and 1969, whereas fresh water fish production which amounts to only less than 10 percent of the total fishery production, showed only a small rate of increase.

Total 2-5 Trend in the quantity of fishery haul

(thousand tons)

| Year | 1960 | 1966 | 1967 | 1968 | 1969 |
|-------------------|------|------|------|-------|-------|
| Salt-water catch | 146 | 635 | 762 | 1,004 | 1,278 |
| Fresh-water catch | 73 | 85 | 85 | 85 | 92 |
| Total | 219 | 720 | 848 | 1,089 | 1,370 |

Source: The Fishery Agency, Ministry of Agriculture and Forestry.

*The main salt-water products are mackerel, shark and shrimp.

The main fresh-water products are carp, loach, and eel.

2-1-4 Livestock industry

Domestic animals raised in Thailand include elephants, horses, cattle, buffaloes, hogs, ducks, and chicken. There were 11,000 elephants, 175,000 horses, 5,167,000 bulls, 6,878,000 buffaloes, according to a survey conducted in 1966.

Traditionally a Buddhist country, the Buddhist proscription against killing is taken very seriously in Thailand, and the consumption of animal food began only comparatively recently, while work animals had been kept by farmers since very ancient days.

In the northeastern region where there is extensive grasslands suitable for grazing, cattle and buffaloes have been popularly raised since olden days, and modern stock farms where breed improvement of beef cattle is being researched are found in Saraburi under foreign aid.

Cattle, buffaloes, hogs, chicken and duck eggs are exported to Hongkong and Singapore during recent years.

2-2 Other Industries

2-2-1 Foreign trade

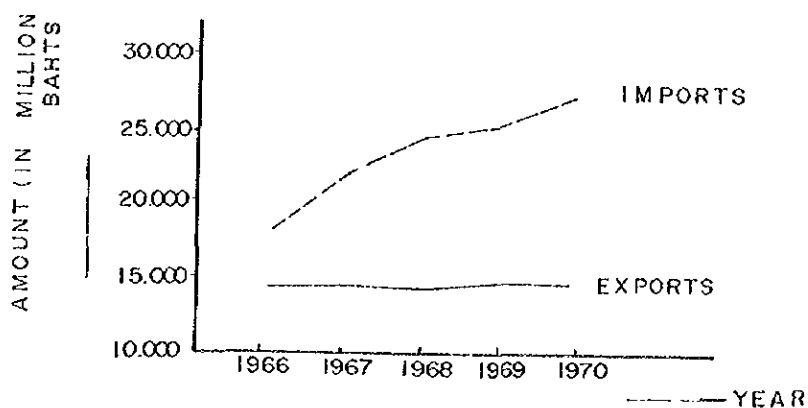
1. Trade balance

Table 2-6 Trade balance of Thailand

(in million Bahts)

| Year | | 1960 | 1966 | 1967 | 1968 | 1969 | 1970 | 1970 Jan - July |
|---------|-----------------|--------|--------|--------|---------|---------|---------|--------------------|
| Exports | Amount | 8,614 | 14,099 | 14,166 | 13,679 | 14,722 | 14,786 | 9,544 |
| | Growth rate (%) | -- | 63.7 | 0.5 | -3.4 | 7.6 | 0.4 | -- |
| Imports | Amount | 9,622 | 18,054 | 22,188 | 24,103 | 25,966 | 27,009 | 15,085 |
| | Growth rate (%) | -- | 87.6 | 22.9 | 8.6 | 7.7 | 4.0 | -- |
| Balance | | -1,008 | -4,405 | -8,022 | -10,424 | -11,244 | -12,237 | -5,541 |

Source: Bank of Thailand: Monthly Bulletin



As shown in the above table, foreign trade has been steadily growing. The growth of imports reflecting the industrial development and increase of consumer demand exceeds that of exports, so that trade balance remains constantly in the deficit.

2. Pattern of trade

(i) Exports

Agricultural and forestry products such as rice, crude rubber, maize, and jute are the principle exports items. Japan is the biggest market, which accounted for over 20% for the past several years (1966 - 1970) and, in 1970, 25.5% of Thailand's exports.

This is followed by the United States (14.4%), the Netherlands (8.6%) and Hongkong.

(ii) Imports

Imports include varieties of industrial products such as industrial materials, chemical products, capital goods etc. Japan's share of Thailand imports, even bigger than that of exports, was 37.4% in 1970.

The U.S. was the second major partner with a share of 14.8%, and West Germany the third, sharing 8.5% of the same.

Fig. 2-1 Breakdown of imports classified by trading partner and by category of product

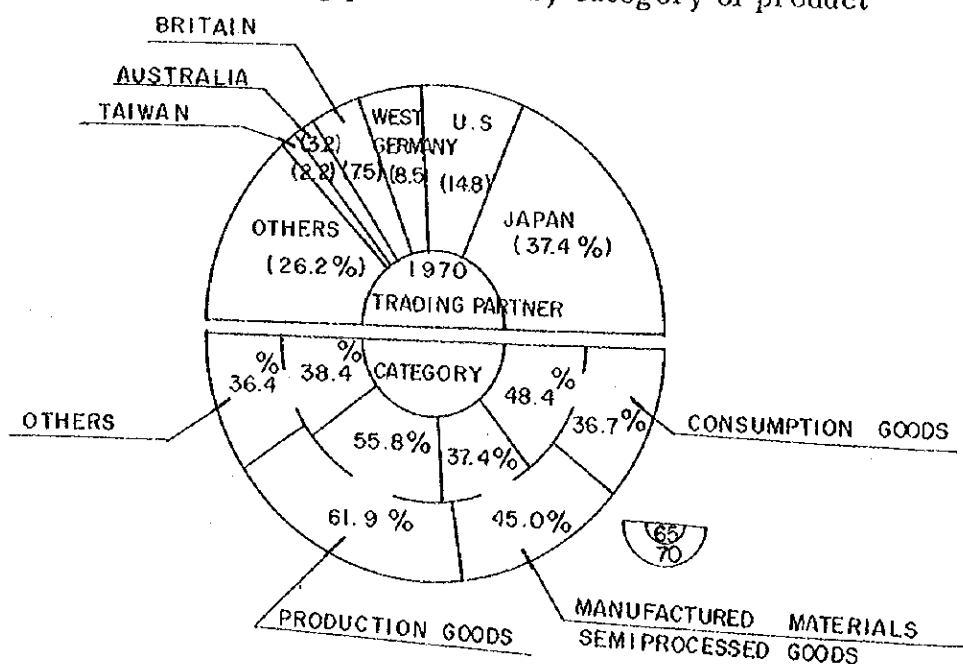


Table 2-7 Trend in annual amount of main imports by commodity

(in million bahts)

| Year Item | 1960 | 1966 | 1967 | 1968 | 1969 | 1970 |
|----------------------------|-------|--------|--------|--------|--------|--------|
| Food stuffs | 784 | 975 | 1,035 | 1,109 | 1,345 | 1,072 |
| Beverages & tobacco | 108 | 296 | 313 | 479 | 495 | 303 |
| Materials | 143 | 521 | 624 | 623 | 828 | 1,379 |
| Mineral oil | 1,025 | 1,873 | 1,588 | 1,995 | 1,829 | 2,363 |
| Animal & vegetable oil | 20 | 26 | 37 | 38 | 59 | 38 |
| Chemical products | 974 | 2,141 | 2,629 | 2,862 | 3,319 | 3,479 |
| Manufactured goods | 3,289 | 5,481 | 6,393 | 6,248 | 6,313 | 6,356 |
| Machines | 2,390 | 5,800 | 7,728 | 8,821 | 9,426 | 9,530 |
| Miscellaneous manufactures | - | 955 | 1,217 | 1,338 | 1,684 | 1,298 |
| Others | - | 311 | 479 | 411 | 526 | 1,083 |
| Gold | - | 125 | 145 | 179 | 142 | 108 |
| Total | 9,622 | 18,504 | 22,188 | 24,103 | 25,966 | 27,009 |

(Source) Bank of Thailand: Monthly Bulletin

Table 2-8 Trend in annual imports by trading partner

(in million bahts)

| Year Market | 1960 | 1966 | 1967 | 1968 | 1969 | 1970 |
|---------------------|-------|----------------|----------------|----------------|----------------|----------------|
| Japan | 2,463 | 6,572 (35.5) | 8,046 (36.3) | 8,274 (34.3) | 9,515 (36.6) | 10,106 (37.4) |
| United States | 1,605 | 3,033 (16.4) | 3,646 (16.4) | 4,512 (18.7) | 3,922 (15.1) | 4,011 (14.8) |
| West Germany | 811 | 1,442 (7.8) | 1,946 (8.8) | 2,021 (8.4) | 2,354 (9.1) | 2,286 (8.5) |
| Britain | - | 1,516 (8.2) | 1,610 (7.3) | 1,673 (6.9) | 2,034 (7.8) | 2,014 (7.5) |
| Italy | 127 | 349 (1.9) | 454 (2.1) | 592 (2.5) | 597 (2.3) | 487 (1.8) |
| Taiwan | 78 | 393 (2.1) | 576 (2.6) | 579 (2.4) | 617 (2.4) | 602 (2.2) |
| Australia | - | 495 (2.7) | 598 (2.7) | 578 (2.4) | 749 (2.9) | 851 (3.2) |
| The Nether- land | 462 | 481 (2.6) | 470 (2.1) | 456 (1.9) | 583 (2.2) | 368 (1.4) |
| Total | 9,622 | 18,504 (100.0) | 22,188 (100.0) | 24,103 (100.0) | 25,966 (100.0) | 27,009 (100.0) |

Figures in () indicate percent distribution

(Source) Bank of Thailand: Monthly Bulletin.

Table 2-9 Trend in annual amounts of main export items

(in million bahts, thousand tons)

| Year Item | 1966 | | 1967 | | 1968 | | 1969 | | 1970 | | Market of main products |
|-------------------|-------------------|-------|-------------------|-------|-------------------|-------|-------------------|-------|-------------------|-------|-----------------------------------------------------------|
| | Amount (price) | Ton | Amount (price) | Ton | Amount (price) | Ton | Amount (price) | Ton | Amount (price) | Ton | |
| Rice | 4,011 | 1,507 | 4,653 | 1,482 | 3,775 | 1,068 | 2,945 | 1,023 | 2,522 | 1,050 | Hongkong, Singapore, Malaysia, India, Saudi Arabia. |
| Crude rubber | 1,861 | 202 | 1,574 | 211 | 1,816 | 252 | 2,664 | 276 | 2,237 | 277 | Japan, U. S. Britain Malaysia |
| Maize | 1,520 | 1,218 | 1,355 | 1,090 | 1,556 | 1,481 | 1,674 | 1,476 | 1,846 | 1,386 | Taiwan Japan |
| Tin | 1,291 | 17 | 1,820 | 26 | 1,510 | 24 | 1,631 | 23 | 1,617 | 22 | U. S. A. The Nether- land |
| Teak (in cu.m) | 243 | 49 | 194 | 36 | 169 | 29 | 166 | 29 | 155 | 29 | U. S. Britain, West- Germany, Italy |
| Tapioca | 644 | 688 | 726 | 781 | 772 | 889 | 876 | 975 | 1,221 | 1,324 | The Netherland, West Germany, U.S.A. |
| Kenaf and jute | 1,614 | 473 | 866 | 317 | 674 | 289 | 780 | 256 | 719 | 255 | Japan, Belgium, Britain, India. |
| Others | 2,900 | | 2,976 | | 3,407 | | 3,986 | | 4,502 | | |
| Total | 14,099 | | 14,166 | | 13,679 | | 14,722 | | 14,819 | | |

Source: Bank of Thailand: Monthly Bulletin

Table 2-10 Annual amount of main exports by market

(in million bahts)

| Market \ Year | 1966 | 1967 | 1968 | 1969 | 1970 |
|-----------------|----------------|----------------|----------------|----------------|----------------|
| Japan | 2,930 (20.8) | 3,000 (21.2) | 2,874 (21.0) | 3,192 (21.7) | 3,770 (25.5) |
| U. S. A. | 1,752 (12.4) | 2,024 (14.3) | 1,789 (13.1) | 2,168 (14.7) | 1,985 (13.4) |
| Malaysia | 1,166 (8.3) | 1,211 (8.6) | 1,038 (7.6) | 1,079 (7.3) | 830 (5.6) |
| Singapore | 1,021 (7.2) | 962 (6.7) | 1,181 (8.6) | 1,154 (7.8) | 1,018 (6.9) |
| The Nether-land | 373 (2.6) | 708 (5.0) | 967 (7.1) | 1,030 (7.0) | 1,276 (8.6) |
| Hongkong | 932 (6.6) | 1,084 (7.7) | 921 (6.7) | 1,156 (7.9) | 1,112 (7.5) |
| India | 1,374 (9.8) | 763 (5.4) | 782 (5.7) | 530 (3.6) | 102 (0.7) |
| Britain | 521 (3.7) | 403 (2.8) | 435 (3.1) | 406 (2.8) | 305 (2.1) |
| Total | 14,099 (100.0) | 14,166 (100.0) | 13,679 (100.0) | 14,722 (100.0) | 14,786 (100.0) |

2-3 Tourism

2-3-1 Number of tourist

The number of tourist visiting Thailand increased 2.2 times between 1966 and 1970. Especially in 1970, the rate of increase amounted to a high 33% partly due to the influence of the international exposition held in Japan.

However, affected by trade recession in the United States, the pace of growth slacked to 10% in the first season of 1971, compared to the same season of 1970, and in the second season, dropped below the record of the previous year.

The total growth rate for the first two seasons, as a result, barely reached the level of that of the previous year.

Classified by nationality, the greatest number of visitors were citizens of the United States, followed by Malaysian, Japanese, and British in that order. This order has been unchanged since 1968, although the number of visitors from the United States decreased by 5,000 persons in the first half year of 1971 compared to that of 1970, while the number of Malaysian and Japanese visitors remain in the upward trend.

It is noteworthy also that the number of tourists from West Germany which ranked No. 6 in 1970 had become the 4th, next to Japan, in 1971.

2-3-2 Tourism revenue

Tourism revenue increased year by year following the increase of visitors, and recorded 2,175 million Bahts in 1970 (including the 390 million Bahts from R & R of the U.S. military servicemen). Compared to the other foreign exchange earners on the list of exports, this ranked third in amount.

As for average spending per visitor, that of all tourist, excluding military personnel, was 2,800 bahts per person while that of military personnel was 8,800 bahts per person in the same year. The total spending of R & R (the U. S. military personnel) recorded the highest of 460 million bahts in 1969 (26% of the total tourism revenue) and has since then been on a decreasing trend.

Table 2-11 Number of visiting tourists by nationality

(in persons)

| Year Nationality | 1966 | 1967 | 1968 | 1969 | 1970 | 1970 1966 | 1970 Jan.- June | 1971 Jan.- June |
|---------------------|---------|---------|---------|---------|---------|--------------|--------------------|--------------------|
| U. S. A. | 90,300 | 89,486 | 103,592 | 133,327 | 159,216 | 1.76 | 72,785 | 67,007 |
| Malaysian | 38,821 | 46,712 | 47,235 | 59,621 | 105,037 | 2.71 | 50,022 | 58,725 |
| Japanese | 14,935 | 24,124 | 31,548 | 42,872 | 46,952 | 3.14 | 22,736 | 27,156 |
| British | 18,370 | 22,165 | 26,434 | 31,331 | 36,977 | 2.01 | 18,216 | 17,529 |
| Australian | 9,045 | 11,380 | 17,394 | 22,247 | 28,185 | 3.12 | 14,502 | 12,389 |
| West Germans | 7,978 | 11,654 | 15,084 | 20,190 | 28,023 | 3.51 | 15,229 | 18,583 |
| Indians | 5,392 | 7,181 | 9,961 | 11,957 | 21,111 | 3.91 | -- | -- |
| French | 7,770 | 9,823 | 10,443 | 12,725 | 21,059 | 2.71 | -- | -- |
| Laotian | 22,822 | 35,690 | 18,302 | 16,022 | 20,155 | 0.88 | -- | -- |
| Swiss | 4,050 | 5,653 | 7,372 | 8,143 | 12,805 | 3.16 | -- | -- |
| Others | 65,634 | 71,977 | 89,897 | 111,349 | 149,151 | 2.27 | 104,347 | 95,637 |
| Total | 285,117 | 335,845 | 377,262 | 469,784 | 628,671 | 2.20 | 297,837 | 297,026 |

Source: T. O. T.: Statistics on Tourism in Thailand.

Note: Number of U. S. Citizens does not include R. & R.

Table 2-12 Tourism revenues

(in million bahts)

| Item \ Year | 1966 | 1967 | 1968 | 1969 | 1970 |
|-------------|------|------|-------|-------|-------|
| Tourists | 618 | 736 | 812 | 1,310 | 1,784 |
| R & R | 136 | 216 | 408 | 460 | 391 |
| Total | 754 | 952 | 1,220 | 1,770 | 2,175 |

Note: Recent average stay per person is 4.8 days.

Source: T. O. T: Statistics on Tourism in Thailand.

Table 2-13 Total number of hotel accommodations and average rate of utilization

| Item \ Year | 1967 | 1968 | 1969 | 1970 | 1971 Jan./Oct. |
|----------------------------------------------|-------|-------|-------|-------|-------------------|
| Total number of hotel accommodations (rooms) | 6,338 | 7,984 | 7,984 | 8,753 | 8,879 |
| Average rate of utilization | 60.5 | 54.9 | 56.4 | 57.6 | — |

Note: Figure for average rate of utilization of 1970 includes that of first half year only.

Source: T. O. T.

2-4 Future Development Plan

2-4-1 Future Development Plan of the Kingdom

The 'Third National Economic and Social Development Plan' is the fundamental plan for the development of the Kingdom for the period October 1971 - September 1976. The major objectives are as follows:

- (i) to restructure the economic system and to promote economic growth.
 - (ii) to maintain a reasonable level of foreign reserves and price stability and to overcome immediate problems, particularly the decline in the overall economic growth rate.
 - (iii) to promote economic growth in the rural areas and to reduce the income disparities.
 - (iv) to promote social justice.
 - (v) to develop manpower resources and to create employment.
 - (vi) to promote the role of the private sector in economic development.
- (1) Restructuring of the economic system and promotion of economic growth

In the objective, a target is set to increase the GDP at an annual average rate of 7 % per year.

This will result in an average per capital income growth rate of 4.5 % as compared to the 4.02 for the Second Development Plan period. At this rate, the GDP for the year will reach 178.2 billion bahts (in constant 1962 price) or about 1.4 times that of 1871. Table 2-14 gives a

Table 2-14 Gross Domestic Product
(Constant 1962 prices)

(billion baht)

| | 1971 (estimate) | | 1976 (target) | | Average Annual Growth Rate | |
|----------------------------------------|--------------------|-------|------------------|-------|-------------------------------|---------|
| | GDP | % | GDP | % | '67-'71 | '72-'76 |
| 1. Agriculture | 37.3 | 29.5 | 47.8 | 26.8 | 4.1 | 5.1 |
| 1.1 Crop | 26.0 | 20.5 | 32.5 | 18.2 | 2.7 | 4.6 |
| 1.2 Livestock | 3.9 | 3.1 | 4.7 | 2.6 | 2.7 | 3.4 |
| 1.3 Fishery | 4.3 | 3.4 | 6.9 | 3.9 | 17.3 | 10.0 |
| 1.4 Forestry | 3.1 | 2.5 | 3.7 | 2.1 | 6.5 | 3.4 |
| 2. Mining and Quarrying | 2.1 | 1.7 | 2.8 | 1.6 | 8.1 | 6.0 |
| 3. Industry | 21.4 | 16.9 | 31.4 | 17.6 | 9.2 | 8.0 |
| 3.1 Traditional Industry | 6.6 | 5.2 | 8.3 | 4.6 | 5.1 | 4.8 |
| 3.2 New Industry | 14.8 | 11.7 | 23.1 | 13.0 | 11.4 | 9.2 |
| 4. Construction | 8.4 | 6.6 | 11.7 | 6.6 | 8.4 | 6.5 |
| 5. Electricity and Water Supply | 1.9 | 1.5 | 3.9 | 2.2 | 20.7 | 15.0 |
| 6. Communication and Transportation | 8.6 | 6.8 | 11.5 | 6.5 | 7.5 | 6.0 |
| 7. Trade | 20.5 | 16.2 | 29.3 | 16.4 | 7.7 | 7.0 |
| 8. Banking, Ins. and Real Estate | 5.1 | 4.1 | 11.0 | 6.2 | 14.4 | 15.0 |
| 9. Dwelling | 2.4 | 1.9 | 2.7 | 1.5 | 4.1 | 2.5 |
| 10. Public Admin. and Defense | 5.7 | 4.5 | 7.6 | 4.3 | 10.0 | 6.0 |
| 11. Services | 13.0 | 10.3 | 18.5 | 10.3 | 8.8 | 7.0 |
| <u>GDP</u> | 126.4 | 100.0 | 178.2 | 100.0 | 7.2 | 7.0 |

breakdown of the estimated GDP for the year 1976.

(2) Maintenance of Economic Stability

To achieve this objective the major measures to be adopted are as follows.

- (i) Acceleration of export promotion
- (ii) Filling the expenditure gap created by the reduction of U. S. military spending.
- (iii) Encouragement of foreign investment.
- (iv) Maintenance of suitable and stable level of foreign reserves.
- (v) Other fiscal and monetary measures.

(3) Promotion of economic growth in rural area and reduction of income disparities

The targets for this objective are:

- (i) To improve the standard of living of the rural population while reducing income disparities.
- (ii) To provide a basis for reduction of the gap in standard of living and income.

The measures adopted are as follows:

- (i) Acceleration of agriculture production at an annual growth rate of 5.1 % and increasing of agricultural expenditure which will be increased in allotment at an annual increase rate of 10 %.
- (ii) Increment of small investment in the rural area, such as ditches and dikes, feeder roads and village development projects as a means to provide more employment opportunities.

(iii) Reduction of population growth rate through the expansion of family planning program, and the promotion of employment opportunities through promotion of industries dependent on agriculture products.

(4) Promotion of social justice

The targets in this objective are as follows:

- (i) To diversify Government social services to reach more people.
- (ii) To reduce the gap in standard of living.

The measures adopted will be classified into two categories:

- (i) Improvement of the standard of living of urban population

The provision of low cost housing and more complete water supply service and the alleviating of such problems as mass transportation, traffic congestion and sewerage disposal will be steps taken in this direction.

- (ii) Improvement of the standard of living of rural population

Such measures as the construction of feeder roads, the rural electrification project, and improvement of government health and education services will be some of the measures for this purpose.

(5) The development of manpower and creation of employment

The targets will be as follows:

- (i) To reduce the population growth rate from the present 3.0 % to eventually 2.5 % in 1976.

- (ii) To provide 2.6 million job opportunities and to maintain urban unemployment rate at about 3.2 %.
- (iii) To reduce under employment in the rural area, where 75 % of the labour force is located.

The measures adopted will be :

- (i) Family planning
 - (ii) Provision of employment opportunities through promotion of labour intensive industries in both private and public sectors.
 - (iii) Expansion of professional training programs
 - (iv) Extension of support to training course
 - (v) Dispersal of manpower into the rural areas.
- (6) Promotion of the role of private sector in economic development

The target amount of private investment is planned at 131 billion bahts, or 63 % of total investment. This will represent an annual average increase rate of 7.3 %.

The measures adopted towards this objective will be.

- (i) Encouragement of business groupings, especially in the priority sectors so that production and marketing can operate efficiently.
- (ii) Encouragement of close cooperation between the Government and the private professional groups.
- (iii) Encouragement of private investment in business which do not require Government intervention.
- (iv) Development of better planning for decision on policy for state enterprises.

- (v) Change and improvement of the method of operation of the various Government departments so that private investors will not be hampered by organizational limitations or ineffective administration.

The above is a brief summary of the gist of the Third National Economic and Social Development Plan.

To implement this plan, programs for implementation for different sectors were drawn up. Here a summary of only the development program in the 'Transportation and Communication' sector will be made.

In this sector, the programs drawn up cover all phases of transportation and communication. However, highway development has very high priority and will receive the lion share of the development project for the sector.

Emphasis will be on provincial highway and village roads or feeder roads, and on land transport improvement particularly around the Metropolitan area to reduce traffic congestion in Bangkok. The construction and maintenance of 5,974 km of national highways, 7,468 km of provincial highways and 12,200 km of tertiary roads and feeder roads is planned in the program.

2-4-2 Future Development Plan of the Metropolitan area.

The latest long term development plan for the Bangkok Metropolitan area is the Revised Report on the Greater Bangkok Plan, by the Department of Town and Country Planning. Although at the time of this study, the report had not been officially authorized, it can be safely said that the recommendations in the report will form a basis for the development projects of Bangkok area. The plan is basically a revision of the 'Litchfield Plan' (1958), modifying and rectifying the dimensions

planned in the Litchfield Plan, through using up-to-date statistical data. In this sense, the revised Greater Bangkok Plan is much more oriented to suit the existing situation of the Metropolitan. The main points of the plan are summarized as follows.

(1) Area covered in the Plan

The total area covered by the Greater Bangkok Plan is approximately 732 km². This includes the municipal areas of Bangkok, Thonburi, Nonthaburi, Samut Prakan and Prapadaeng as well as some parts of the area in the four changwats out of the boundary of the municipalities. This completely covers the whole economic sphere which is formed through the inter-community economic activities centering around the capital.

(2) Population

The Greater Bangkok Plan has planned the future population of the Greater Bangkok area at 6.5 million for the year 1990. This is considerably lower than that derived through trend projection, which as indicated in the report, may even reach 10 million. However, as again pointed out in the report, such an unchecked expansion will result in a very unhealthy situation in the Metropolitan and prompt the sprouting of other deterring factors. Nevertheless, to guide the population towards the planned figure of 6.5 million for the Greater Bangkok area, there is no doubt that considerable efforts are required of the authorities in relevant policy making. Population control policy is being proposed in the report, which policy, through proper guidance, will certainly be effective to keep the population to the planned size.

In this study, an estimation of the population out of the Greater Bangkok area but within the Metropolitan changwats is made, and the results show that in 1990, the total population for the whole area of the four changwats will be 7.2 million, which comes to a population of 700 thousand for the areas out of the Greater Bangkok Plan.

(3) Land use plan

Land use of the Greater Bangkok area is classified into the following regions:

(a) Residential region

A total area of 260 thousand rai or approx. 416 km² (57 % of total planned area) is planned as residential area to accommodate a population of 5,810 thousand, or 89 % of the total planned population. The residential area is again reclassified into high density, medium density and low density area with respectively 32 km², 224 km² and 160 km² in area and 1,040 thousand, 3,570 thousand, and 1,200 thousand in population. The remaining 690 thousand in population will be accommodated in commercial areas, military facilities and public institution.

(b) Commercial region

The existing commercial activities are largely carried out in small shopping houses scattered all over the area. In the plan it is proposed that commercial activities be intensified through the construction of more comprehensive shopping centres. A total of 27.7 thousand rai (approx. 44.3 km²) is planned for commercial region, of which

20,000 rai (32 km²) will be for shopping centres and 7,000 rai (11.2 km²) for small shopping areas scattered among residential region.

(c) Industrial region

The area for the industrial region is planned at a labor density of 12 persons per rai (7.5 persons per 1,000 m²). The total area planned comes to 52,700 rai or 84.3 km². Industries will be classified into various types. The pollution-prone industries will be collectively located in industrial estate separated completely from residential areas. Other light industries without fear of pollution will be located at places convenient for the manufacturing and transportation process.

Besides the industrial region in the Greater Bangkok area, a recommendation of establishment of 3 large industrial estates within the Metropolitan sphere, (one each at Nakhon Pathom, Samut Sakorn and Chonburi) is also made.

(d) Government and institutional region

In this category are included the facilities for civil services, military, municipal and international agencies including embassies, hospitals, universities and libraries. An area of 39,452 rai (63.1 km²) is planned for this purpose and are scattered over the area for the convenience in execution of the functions of the agencies.

(e) Recreation region

In the Greater Bangkok Plan an area of 39,000 rai (62.4 km²) is planned in this category which includes vacant lands, parks, gardens and playgrounds, which will be

divided into 14 different districts. An area of 26,000 rai (41.6 km²) is also planned within the residential area.

(4) Transportation system

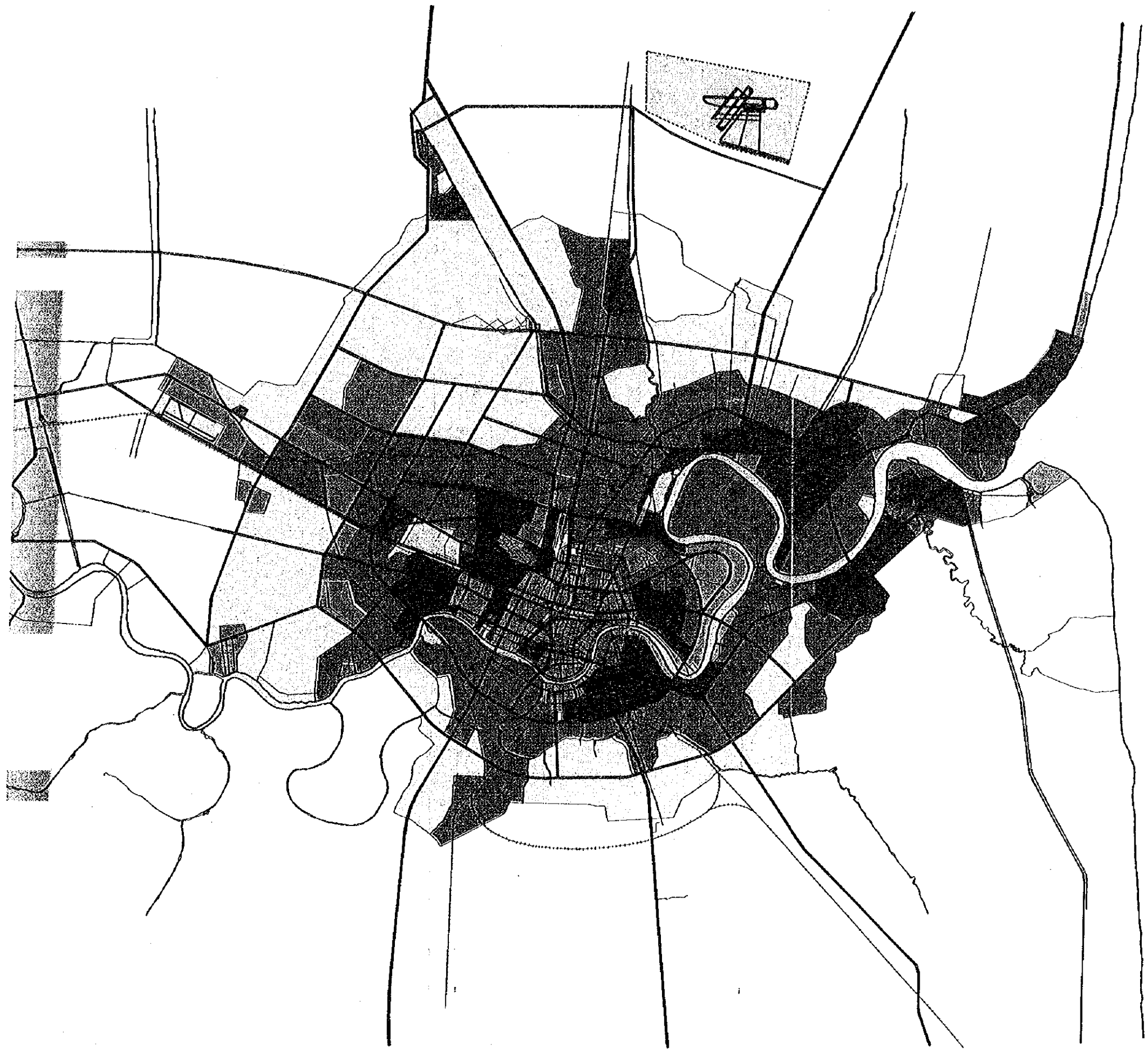
(a) Road system

The 'ring road' is the basic concept in the planning of the future road system in the Greater Bangkok area. Three ring roads are proposed. These are connected by other radial roads and intermediate connecting roads to form a complete net. For the traffic across the Chao Phraya River, three new bridges are proposed, other than the Tachang Bridge or the Sathorn Bridge which are already in the construction or planning stage.

(b) Other transportation system

Total elevation of the railway lines within the Metropolitan area is proposed, a deep-water harbour at Lamchabang is planned, and the early implementation of the planned new international airport is recommended.

Plate No. 2-1 summarizes the 'Land use' plan and the transportation system plan in one drawing, and gives an easy visual understand of the plan.



| | | | | | | | |
|--|---------------------------------|--|-----------------------------------|--|--------------------------|--|---------------|
| | Institution & University Region | | High Density Residential Region | | Industrial Region | | Water Surface |
| | Commercial Region | | Medium Density Residential Region | | Agricultural Region | | |
| | Public Utility Region | | Low Density Residential Region | | Park & Green Belt Region | | |

Future Land Use Plan
Greater Bangkok



CHAPTER 3. TRAFFIC ANALYSIS

3-1 General Description of Transportation System in Thailand

3-1-1 Railway

Thailand's railway system is constructed, run and controlled by "The State Railway of Thailand" under the ministry of communication.

The total length of railroads in 1970 was 3,765 km, of which only 90 km (between Bangkok and Ban Phaen) were double track.

The major routes are:

- (1) The north bound line between Bangkok and Changmai (751 km)
- (2) The north-east bound line connecting Bangkok, Korat and Nongkai (624 km)
- (3) The north-east bound line connecting Bangkok, Korat and Ubon (575 km)
- (4) The east bound line running from Bangkok to the Thai-Cambodian boader.
- (5) The south bound line extending between Bangkok and the Thai-Malaysian border
- (6) The west bound line between Bangkok and Nan Tok (210 km)
- (7) The west bound line between Bangkok and Suphan Buri (157 km)

There are in total 584 railway stations, and the average distance between stations is 6.5 km.

Capacity of terminal stations except that of Bangkok is estimated to be more than enough at present and will be able to meet a 50% increase of the transportation volume.

The volume of passenger transportation increased with the growth of economy between 1962 and 1968, but had slacked down in the rate of growth in recent years. Freight transportation grew in a similar pattern as that of passenger transportation except for a short interval of stagnation in 1963. The decrease of growth rates is attributed to the rapid development of the highway system. (Table 3-1)

Classified by commodity, petroleum products was the largest item being transported followed by clinker, marl, cement in that order. Whereas sea transportation is most suitable for such bulk and heavy goods as petroleum products, it is not possible in Thailand except along the coastline, and railway transportation is expected to become increasingly important in the future. As shown in the Table 3-2, the volume of items 1 - 4 increased between 1968 and 1970 while that of 5 - 10 decreased.

It is anticipated that transportation of bulk and heavy goods will in the future rely on the railway system while the others switch to highway as highway facilities improve.

Classified by place of origin and destination, 60% of the goods on the above mentioned table were transported from or to the central region, 20% the northern region.

The share of railway transportation among all systems of goods transportation is 10% in the southern region, much higher than the 5% for road transportation. This is probably because long-distance transportation is geographically required and railway transportation is better prepared than road transportation.

Table 3-1 Volume and distance of railway transportation

| | Passenger | | | Goods | | |
|------|------------------|--------------------------|------------------------------------|---------------|-----------------------|------------------------------------|
| | Person | Person kilometer | Average distance of transportation | Ton | Ton kilometer | Average distance of transportation |
| Unit | Thousand persons | Million person kilometer | Kilometers | Thousand tons | Million ton kilometer | Kilometers |
| 1965 | 43,914 | 2,847 | 64.8 | 4,435 | 1,534 | 393.1 |
| 1966 | 46,024 | 3,173 | 68.9 | 4,694 | 1,607 | 385.2 |
| 1967 | 48,108 | 3,614 | 75.1 | 5,236 | 1,941 | 413.0 |
| 1968 | 49,729 | 3,884 | 79.7 | 5,462 | 2,083 | 420.6 |
| 1969 | 47,326 | 3,962 | 83.7 | 4,829 | 1,979 | 453.4 |
| 1970 | 48,190 | 4,113 | 85.3 | 5,131 | 2,209 | 470.2 |

SOURCE : Information Booklet

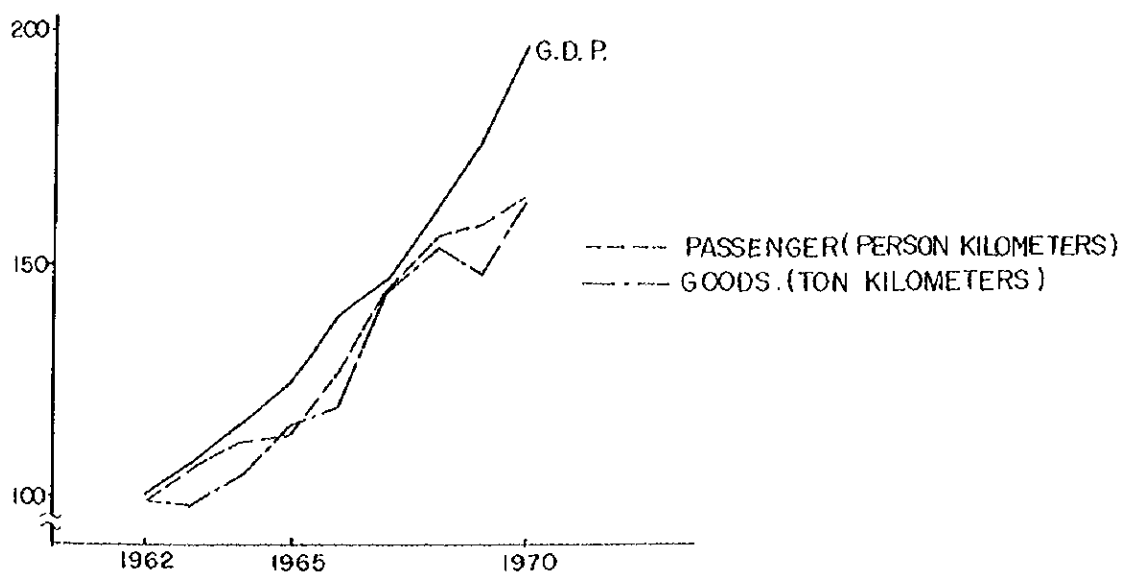


Table 3-2 Breakdown of volume of transportation by item

| Year | 1968 | | | 1970 | | | |
|------|----------------------|-------|-----------------------------|-----------------|-------|-----------------------------|-----|
| | Volume (ton) | % | Average distance (km) | Volume (ton) | % | Average distance (km) | |
| 1 | Petroleum products | 566 | 16.4 | 574 | 856 | 16.7 | 573 |
| 2 | Cement | 639 | 11.7 | 574 | 647 | 12.6 | 533 |
| 3 | Rice products | 420 | 7.7 | 563 | 535 | 10.4 | 530 |
| 4 | Fluorspar | 180 | 3.3 | 735 | 245 | 4.8 | 744 |
| 5 | Lumber, logs & poles | 292 | 5.3 | 531 | 229 | 4.5 | 534 |
| 6 | Clinker & Marl | 1,190 | 21.8 | 94 | 749 | 14.6 | 126 |
| 7 | Maize | 175 | 3.2 | 425 | 160 | 3.1 | 384 |
| 8 | Fertilizer | 83 | 1.5 | 489 | 68 | 1.3 | 473 |
| 9 | Jute & Kenaf | 77 | 1.2 | 476 | 47 | 0.9 | 497 |
| 10 | Livestock | 17 | 0.3 | 325 | 16 | 0.3 | 373 |
| 11 | Other | 1,833 | 33.6 | — | 1,579 | 30.8 | — |
| | Total | 5,462 | 100.0 | 420 | 5,131 | 100.0 | 470 |

Source: Information book

3-1-2 Air transportation

(1) Domestic air system

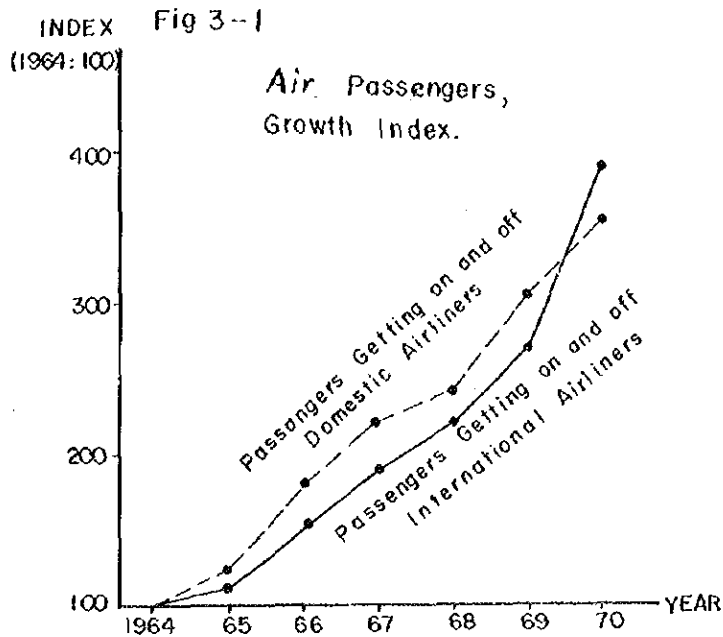
Thailand's domestic air transportation system is operated by the Thai Airway Co. There are 27 domestic airports of which 24 are in operation.

Table 3-3 A list of domestic airports in Thailand

| Name of airport | Dimension | | Runway | | In operation (O) |
|-----------------|-----------|----------------|------------------|----------------|------------------|
| | Runway(m) | Stopway (m) | Surface | Strength (ton) | |
| Chiang Mai | 2,134x38 | 275x38, 150x38 | Concrete | 48.0 | O |
| Chiang Rai | 1,475x30 | 60x30, 60x30 | Asphalt | 13.6 | O |
| Hua Hin | 840x30 | 360x30, 60x30 | " | 13.6 | O |
| Khon Kaen | 1,500x30 | 60x60, 60x60 | " | 13.6 | O |
| Khun Yuam | 1,100x45 | | Laterite & grass | 6.8 | |
| Lumpang | 1,475x30 | 60x30, 60x30 | Asphalt | 13.6 | O |
| Loei | 1,200x30 | 88x30, 50x30 | Laterite | 19.1 | O |
| Mae Hong Son | 1,315x30 | 30x60 | Asphalt | 13.6 | O |
| Mae Sariang | 1,070x30 | 60x30, 60x30 | Laterite & grass | 16.8 | O |
| Mae Sot | 1,100x45 | | " | 16.8 | O |
| Nakhon Phanom | 200x45 | | Laterite | 16.8 | O |
| Nakhon Sawan | 1,200x45 | | " | 16.9 | O |
| Nan | 1,400x30 | | Asphalt | 19.1 | O |
| Nong Khai | 110x40 | | Laterite | 16.8 | O |
| Pai | 700x30 | | Laterite & grass | 6.8 | |
| Pattani | 1,400x40 | | Asphalt | 20.2 | O |
| Phitsanulok | 1,820x45 | 60x45, 60x45 | " | 27.2 | O |
| Phrae | 1,200x40 | | | 16.8 | O |
| Phuket | 1,440x30 | 60x30, 60x30 | Asphalt | 13.6 | O |
| Ranong | 1,080x30 | | Laterite | 16.8 | |
| Sakon Nakhon | 1,400x30 | 100x50, 100x50 | Asphalt | 20.2 | O |
| Songkla | 1,500x45 | | " | 20.2 | O |
| Tak | 1,350x45 | | Laterite | 16.8 | O |
| Ubon | 2,743x38 | 300x38, 300x38 | Concrete | 11.3 | O |
| Udon | 3,048x38 | 300x38, 300x38 | " | 11.3 | O |
| Uttaradit | 1,520x30 | 200x30, 200x30 | Asphalt | 20.2 | O |
| Trang | 1,500x30 | 60x60 | " | 13.6 | O |

As seen from the above table showing name, dimension and runway structure of all domestic airports (excluding Don Muang Airport of Bangkok), a considerable number have runways shorter than 1,400 m. About half of these have laterite finished runway which calls for restraint of utilization under bad weather condition, hence resulting in a high rate of flight cancellation. The busiest airway route is that between Bangkok and Chiangmai, with the frequency of 38 flights/week on both ways. The frequency of the air traffic between Bangkok and Songkla is 14 flights/week. Construction of another 14 airports is now being planned. They include those of Surat Thani, Narathiwat, Surin, Ranong and Lom Sak.

The volume of passenger transported was 200 thousand persons, and 100 million person kilometers in 1970, for an average trip distance of 500 km. The average rate of increase was as high as 20 - 25% during the last 5- 6 years. Such a pace of increase, as illustrated by the graph in Fig. 3-1 is comparable to that of the international airways. Classified by routes, the number of passengers transported between Bangkok and Chiangmai was the largest, being about one third of the total of domestic airlines. The other major routes next to this in volume of passenger transportation are that between Bangkok and Songkla, and that between Chiangmai and Chiang Rai, each flying only about 1/4 of the passengers volume of the Bangkok-Chiangmai route. As much as 80% of domestic air passenger departed from or destined for Bangkok.



(2) International air transportation

Do Muang International Airport, situated 17 km north of Bangkok, is not only the main access of Thailand via air, but also an air centre of southeast Asia. It is on the regular stop of international liners flown by 30 companies, with a total of 30,000 scheduled civilian flights a years. The first flight of Jambo Jet flying here was that of PAA, on Nov. 1, 1971.

The features of Don Muang Airport are given in Table 3-4.

Table 3-4 Features of Don Muang Airport

| Item | Dimension |
|--------------|------------------------------------------------------|
| 1. Elevation | 4 m |
| 2. Runway | |
| Dimension | 3,210 x 60 m 3,000 x 45 m |
| Strength | 34,000 kg |
| Surface | Concrete |
| 3. Stopway | |
| Dimension | 305 x 60 m 305 x 60 m 305 x 45 m 305 x 45 m |
| Surface | Asphalt |
| 4. Apron | |
| Dimension | 847 x 110 m |
| Surface | Concrete |
| 5. Taxiway | |
| Dimension | 24 m |

Don Muang Airport is equipped with two 3000 m class runways parallel to each other. Construction of a new airport terminal building which was started in 1969 is now been completed after some delay. This new building is designed to have the departure & arrived lounges twice as big as the previous one, and various other facilities improved. Nevertheless, with a daily total of 200 - 250 international flights and a daily average of 20 domestic flights, together with non-schedule military aircrafts departing and arriving, the hourly number of departing and arriving flights at the

peak of a day amount to over 140, and the capacity of the airport itself is already almost saturated. In view of this, the construction of a new international airport at Khlong Nong Ngahao, 30 km east of Bangkok, with a total area of 24 km² is being planned.

The annual total number of arriving and departing passengers was 600 thousand persons each, the volume of goods arriving was 10,000 ton, and that departing was 9,000 ton. Classified by season, the volume of passenger during April - August was largest, that between November and February was smallest. Classified by airliners, the Thai Airway accounted for 20%, the largest proportion of the total of passengers, followed by JAL (10%), CPA (8%), MSA (7%) and PAA (6%) in that order.

Table 3-5 International Air Traffic of Bangkok Airport

| Years Oct. to Sept. | Aircraft | | Passengers | | Freight (Metric Ton) | | Increase/ Decrease | |
|---------------------------|----------|-----------------------|------------|---------|-----------------------|----------|-----------------------|--------|
| | Total | Increase/ Decrease | In | Out | Increase/ Decrease | In | | Out |
| 1960 1961 | 19,823 | | 110,662 | 108,967 | | 1,536.09 | 1,952.73 | |
| 1961 1962 | 23,783 | +18.1% | 126,464 | 129,745 | +16.7% | 2,694.72 | 2,070.80 | +36.5% |
| 1962 1963 | 27,135 | +14.9% | 146,316 | 149,976 | +14.8% | 2,118.78 | 2,388.02 | - 5.4% |
| 1963 1964 | 27,711 | + 2.1% | 179,380 | 174,402 | +20.3% | 2,100.45 | 2,521.33 | + 2.5% |
| 1964 1965 | 28,752 | + 3.8% | 182,593 | 189,121 | + 5.7% | 2,388.12 | 2,796.04 | +12.2% |
| 1965 1966 | 21,697 | -24.6% | 246,893 | 248,390 | +33.2% | 3,358.78 | 3,920.41 | +40.4% |
| 1966 1967 | 21,894 | + 1.9% | 326,439 | 325,875 | +31.8% | 4,317.00 | 4,968.00 | +27.6% |
| 1967 1968 | 26,159 | +19.4% | 380,171 | 375,939 | +15.8% | 6,198.00 | 6,324.00 | +34.9% |
| 1968 1969 | 25,237 | + 7.9% | 462,612 | 456,930 | +21.1% | 6,831.58 | 7,443.78 | +14.0% |
| 1969 1970 | 33,515 | +18.6% | 621,442 | 606,097 | +33.4% | 9,645.99 | 8,693.79 | +28.4% |

3-1-3 Coastal waterways

The volume of domestic coastal goods transportation has not been much growing, as seen in the following table.

Table 3-6 Volume of goods handled at harbors
(include only coastwise freight)

in 1,000 tons

| Region | 1960 | 1965 | 1968 | 1969 |
|---------------------------------------------|------|-------|-------|-------|
| Whole Kingdom | 902 | 1,054 | 1,222 | 1,236 |
| Port of Bangkok | 345 | 480 | 592 | 521 |
| Ports on east coast of southern Thailand | 380 | 431 | 556 | 635 |
| Ports on west coast of southern Thailand | 91 | 85 | 27 | 28 |
| Other ports | 86 | 58 | 47 | 52 |

This is because navigable coastline are limited to the southern part of Thailand.

The following table gives the volume of coastal goods transportation divided into two parts: goods transported from or to Bangkok and goods transported between other places. The proportion of the former, in volume against the total is greater than 70%.

Table 3-7 Proportion of goods transported to and from Bangkok against the total of coastal goods transportation

| Name of region & port | Total (ton) | Goods to and from Bangkok (ton) | | Relative weight of goods to and from Bangkok (%) | |
|------------------------------------------|-------------|---------------------------------|----------|--------------------------------------------------|-----------|
| | | To BKK | From BKK | | Sub Total |
| Total of Thailand | 523.6 | 128.6 | 263.3 | 381.9 | 72.9 |
| Ports on east coast of southern Thailand | 395.5 | 106.4 | 261.3 | 367.7 | 93.0 |
| Ports on west coast of southern Thailand | 57.6 | 0 | 0 | 0 | 0 |
| Other ports | 70.5 | 22.2 | 2.0 | 24.2 | 34.3 |
| Major ports | | | | | |
| Songkhla | 126.9 | 34.9 | 84.3 | 119.2 | 93.9 |
| Surat Thani | 89.5 | 42.8 | 38.1 | 80.9 | 90.4 |
| NakhornSiThammarat | 77.8 | 5.8 | 67.1 | 72.9 | 93.7 |
| Pattani | 67.4 | 5.6 | 58.0 | 63.6 | 94.4 |
| Kantang | 31.5 | 0 | 0 | 0 | 0 |
| Phuket | 26.1 | 0 | 0 | 0 | 0 |
| Samut Songkram | 22.0 | — | 0.3 | 0.3 | 1.4 |
| Chantha Buri | 20.3 | 12.9 | 0.2 | 13.1 | 64.5 |
| Ko Samui | 16.7 | 13.5 | 3.0 | 16.5 | 98.8 |
| Narathiwat | 14.9 | 3.8 | 10.9 | 14.7 | 98.7 |
| Trat | 13.7 | 9.1 | 1.3 | 10.4 | 75.9 |

Classified by area, 90% of good transportation between ports on the eastern coast of southern Thailand (coastline of the Gulf of Thailand) was with the port of Bangkok on the other end. On the other hand, goods transportation between ports on the western coast (the coast of Indian Ocean) are not connected to the port of Bangkok, since such connection means a long detour route around the Malay Peninsular. The following table gives the volume of goods transported by coastal vessels, classified into 4 groups of routes. The volume of goods transported between Bangkok and ports on the southern coastline, which was 70% of the whole, was the largest, followed by that between ports of the southern coast, which was 16%.

Table 3-8 Volume of goods transportation on coastal seaway by routes

| Routes | Volume (in 1,000 tons) | % |
|------------------------------------|---------------------------|-----|
| Bangkok & southern Thailand | 184 | 70 |
| Bangkok & other ports | 12 | 5 |
| Between ports of southern Thailand | 43 | 16 |
| Between other ports | 24 | 9 |
| Total | 263 | 100 |

Breakdown of the goods transported from & to Bangkok is shown on the following list. The bulk of goods transported to Bangkok involves primary products and that from Bangkok includes essential goods. For the ports on the western coast which have no connection in goods transportation with Bangkok, essential goods are imported from Penang and Singapore.

Table 3-9 Breakdown of goods transportation to and from the port of Bangkok

| | To Bangkok | | From Bangkok | |
|---------------------------------------------|------------|------|--------------|------|
| | 1,000 ton | % | 1,000 ton | % |
| Total | 128.6 | 100 | 263.3 | 100 |
| Rice | 0 | 0.0 | 6.9 | 2.6 |
| Coconut | 18.2 | 14.1 | 0.3 | 0.1 |
| Fishery products | 0.4 | 0.3 | 4.2 | 1.6 |
| Other food stuff, and tobacco | 4.7 | 3.6 | 77.0 | 29.2 |
| Rubber | 13.6 | 10.6 | 1.2 | 0.5 |
| Timber | 49.4 | 38.4 | 2.6 | 1.0 |
| Building material | 0.7 | 0.5 | 88.9 | 33.7 |
| Machines | 1.0 | 0.8 | 2.7 | 1.0 |
| Fuel oils and gasoline | 1.2 | 0.9 | 55.8 | 21.2 |
| Chemical fertilizers & chemical products | 0.1 | 0.1 | 4.6 | 1.7 |
| Others | 39.3 | 30.7 | 19.1 | 7.4 |

3-1-4 Inland waterways

With the Chao Phraya River penetrating the heart of Thailand, the Mekong River flowing on the east along the border, and a network of khlongs connecting a host of tributaries of these rivers, inland waterways have been the main stay of transportation system in Thailand since early days.

In spite of the development of railway and highway networks, waterways still remain important to-day, and the bulk of this country's goods, including agricultural products, livestock and pontries, are transported on waterways. Transportation of passengers relies also largely on water. The volume of passengers crossing Chao Phraya River by ferries alone amounts to 160 thousand persons a day. Water buses, water taxi and sampans plying the Chao Phraya River and network of khlongs connecting with this river are essentially important in the citizen's daily life, as means of commuting and transportation.

There are 4 main routes of waterways connecting Chainat and the Bangkok-Thonburi metropolitan area:-

- (1) The westmost route including Suphan, Nakhon Chaisi, and Chai rivers which runs a distance of 310 km between Chainat and Samut Sakhon via Suphan Buri.

This route, being deeper than the others, is especially useful during dry season.

- (2) The route next to the first one, involving Nai river, and Bang Sai Khlong, runs a distance of 150 km almost parallel to the Chao Phraya River, from Chainat Southward and meets the Chao Phraya at the southern part of Ayuthaya. Even though reported to be deeper than 2 m during dry season, there are apparently shallower places, since siltation is observed at some parts of the course.

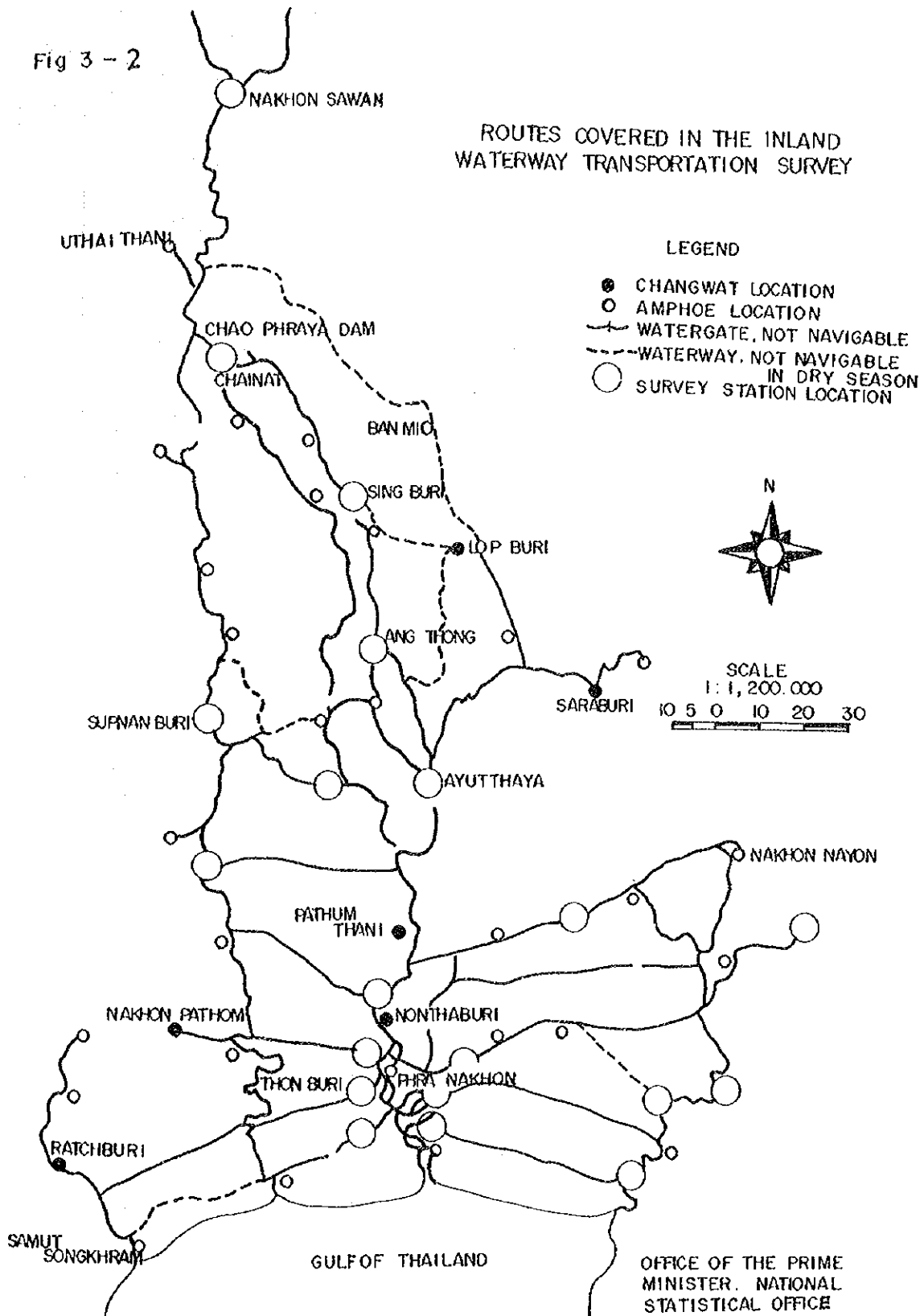
(3) The third route counting from the west, starts from Chainat, runs via Sing Buri, Ayuthaya and straight into Bangkok, is the largest waterway in Thailand. It has a length of 360 km, between the junction of Ping and Wang rivers and the Gulf of Thailand, and carries the largest traffic volume in Thailand.

(4) The eastmost route starts with a detour eastward and turns round to meet Menam Chao Phraya at Ayuthaya. This course involves unnavigable parts during dry season.

In view of the importance of inland waterways, a "Survey of Inland Waterway Transportation" was conducted in 1963 by the government, headed by the Harbour Dept. and cooperated by N. S. O & N. E. D. B., on 30 sampled days between Oct., 1963 & Oct., 1964 at survey stations located as shown on the Fig. 3-2.

It is known that the total number of vessels navigating the inland waterways counts 26,000, of which 25,000 belong to the central region. Out of this 25,000 vessels, 16,000 were covered by the above survey. The remaining apparently includes those only very occasionally used, and those travelling only within a very confined sphere around the owner's dwelling.

Fig 3 - 2



Source " Survey of Inland Waterway Transportation", Harbour Dept.

Table 3-10 Daily traffic volume on khlongs by season

| Survey station | Yearly average (Vessel /day) | Dry season (Jan - May) (Vessel /day) | Rainy season (June - Dec.) (Vessel /day) | Rainy season /Dry season | Proportion of vessels flowing in and out of metro.area (%) |
|--------------------------|---------------------------------|--------------------------------------------|------------------------------------------------|--------------------------|------------------------------------------------------------|
| <u>Capital District</u> | | | | | |
| 1 Nonthaburi | 973 | 886 | 1,039 | 1.17 | 90.0 |
| 2 Wat Suwankiri | 680 | 649 | 694 | 1.07 | 97.8 |
| 3 Phasicharoen | 246 | 264 | 232 | 0.88 | 89.6 |
| 4 Wat Sai | 171 | 161 | 179 | 1.11 | 89.7 |
| 5 Saensaeb | 35 | 36 | 35 | 0.97 | 89.4 |
| 6 Phrakhanong | 57 | 50 | 62 | 1.24 | 81.1 |
| 7 Samrong | 22 | 22 | 22 | 1.00 | 57.7 |
| <u>Northern District</u> | | | | | |
| 8 Ayutthya | 336 | 257 | 397 | 1.54 | 87.9 |
| 9 Sena District | 174 | 219 | 139 | 0.63 | 72.5 |
| 10 Suphan Buri | 82 | 109 | 61 | 0.56 | 56.9 |
| 11 Songphinnong District | 119 | 129 | 110 | 0.85 | 31.7 |
| 12 Ang Thong | 171 | 44 | 268 | 6.09 | 73.6 |
| 13 Sing Buri | 174 | 66 | 256 | 3.88 | 70.7 |
| 14 Chai Nat | 243 | 168 | 300 | 1.79 | 72.7 |
| 15 Nakhon Sawan | 136 | 38 | 210 | 5.53 | 77.9 |
| <u>Eastern District</u> | | | | | |
| 16 Bangpa Kong | 13 | 15 | 12 | 0.80 | 40.3 |
| 17 Chachoengsao | 36 | 35 | 37 | 1.06 | 65.0 |
| 18 Bangkhla | 19 | 18 | 19 | 1.06 | 36.6 |
| 19 Nakhon Nayok | 14 | 15 | 14 | 0.93 | 26.5 |
| 20 Phrachin Buri | 21 | 21 | 21 | 1.00 | 38.3 |
| Average | 186 | 160 | 205 | 1.28 | |

Source: Survey of Inland Waterway Transportation, 1963

As seem in the above table showing traffic volume of each survey station, Nonthaburi has the largest average daily traffic volume of 1,000 return trips, followed by Wat Swankiri with 700 return trips, and Chainat with 300 return trips a day.

Classified by season, traffic volume in the rainy season is larger, being 1.28 times of the dry season.

Table 3-11 Traffic volume and transportation volume to and from the metropolitan area by way of khlongs

| Place | Traffic volume (thousand vessels per year) | | Transportation volume (thousand tons per year) | | Loading efficiency (ton/vessel) | |
|-------------------------------|--------------------------------------------------|-------|---------------------------------------------------------|-------|---------------------------------------|------|
| | To | From | To | From | To | From |
| Total | 368.0 | 351.9 | 5,668.7 | 442.4 | 15.4 | 1.3 |
| Total excluding Nonthaburi | 256.0 | 238.9 | 5,612.2 | 414.6 | 21.9 | 1.7 |
| Capital District | | | | | | |
| Nonthaburi | 112.0 | 113.0 | 56.5 | 27.8 | 0.5 | 0.3 |
| Samut Prakarn | 5.1 | 3.0 | 53.4 | 8.4 | 10.4 | 2.8 |
| Pathum Thani | 58.4 | 54.9 | 1,505.4 | 40.4 | 25.8 | 0.7 |
| Northern District | | | | | | |
| Ayutthya | 48.7 | 51.0 | 1,134.7 | 75.5 | 23.3 | 1.5 |
| Ang Tong | 5.0 | 5.2 | 93.4 | 17.6 | 18.6 | 3.4 |
| Sara Buri | 5.1 | 4.0 | 180.9 | 6.3 | 35.1 | 1.6 |
| Lop Buri | 7.2 | 6.9 | 235.2 | 2.8 | 32.5 | 0.4 |
| Sing Buri | 3.6 | 3.7 | 87.8 | 12.2 | 24.7 | 3.3 |
| Chai Nat | 3.2 | 2.8 | 90.6 | 6.0 | 28.1 | 2.2 |
| Uthai Thani | 2.0 | 1.4 | 44.5 | 9.0 | 22.2 | 6.5 |
| Nakhon Sawan | 9.5 | 9.1 | 262.5 | 8.5 | 27.7 | 0.9 |
| Phichit | 10.0 | 6.9 | 300.0 | 12.5 | 30.1 | 1.8 |
| Phit Sanulok | 2.8 | 2.4 | 100.0 | 2.8 | 35.5 | 1.2 |
| Eastern District | | | | | | |
| Chachoengsao | 7.9 | 7.3 | 101.3 | 24.1 | 12.8 | 3.3 |
| Nakhon Nayok | 0.8 | 0.5 | 9.9 | 0.7 | 12.3 | 1.4 |
| Prachin Buri | 1.9 | 1.8 | 37.7 | 2.7 | 19.8 | 1.5 |
| Western District | | | | | | |
| Nakhon Pathom | 6.9 | 6.0 | 107.1 | 44.9 | 15.4 | 7.5 |
| Samut Sakhon | 24.6 | 21.6 | 193.2 | 31.3 | 13.9 | 1.5 |
| Samut Songkhram | 14.1 | 13.4 | 195.8 | 36.3 | 7.8 | 2.7 |
| Ratchaburi | 27.1 | 25.3 | 645.1 | 24.5 | 23.8 | 1.0 |
| Suphan Buri | 8.9 | 9.2 | 199.7 | 30.9 | 22.5 | 3.4 |
| Others | 3.0 | 3.2 | 34.3 | 16.9 | 11.6 | 5.2 |

The above table shows the volume of goods transported on waterways from and to the metropolitan area. The yearly inflow of goods was 5,670 thousand tons, and the outflow was 440 thousand ton, less than 1/10 of the inflow.

The bulk of goods transported to the metropolitan area via waterways was from Pathun Thani and Ayuthaya areas, the proportion of each against the total influx being 27% and 20% respectively. The average volume of goods transported per vessel was 15 ton for inflowing goods and 1.3 ton for outflowing goods.

Breakdown of inflowing and outflowing goods by commodities are listed on the following table. Among the inflowing goods, building material is the largest in volume, being 50% of the whole, followed by rice with 29%, and maize, with 10% of the same.

Table 3-12 Breakdown of goods transported to and from the metropolitan region by way of khlongs

| Item | To | | From | |
|---------------------------------------|--------------------------|---------|--------------------------|---------|
| | Volume (thousand ton) | Percent | Volume (thousand ton) | Percent |
| Total | 5,670 | 100 | 442 | 100 |
| Material including building materials | 2,809 | 49.5 | 77 | 17.4 |
| Maize | 601 | 10.6 | 10 | 2.3 |
| Fruits, vegetable | 129 | 2.3 | 32 | 7.2 |
| Rice | 1,650 | 29.1 | 61 | 13.8 |
| Other food stuff | 219 | 3.8 | 46 | 10.4 |
| Fuels | 73 | 1.3 | 120 | 27.2 |
| Fertilizer | 15 | 0.3 | 26 | 5.9 |
| Others | 174 | 3.1 | 70 | 15.8 |

3-1-5 Highway

The construction, rehabilitation and maintenance of national and provincial highways and the development of national highway system is under the responsibility of the Department of Highways.

As of 1971, the total length of highway under the control of the Department of Highways and open to traffic came to 17,105 km, of which 10,977 km are national highway and 6,128 km provincial highways. 9,681 km of the 10,977 km of national highway are maintained with asphalt surface, and the remaining 1,296 km maintained with aggregate surface. Of the provincial highways, 1,781 km are with asphalt surface and the remaining 4,347 km with aggregate surface. There were in addition, 17,412 km of highways under construction. The following table shows the trend of increase in length of highway under the Department of Highways.

Table 3-13 Length of highway open to traffic in Thailand (km)

| Year | National Highway | Provincial Highway | Total |
|------|------------------|--------------------|--------|
| 1964 | 9,404 | 2,214 | 11,681 |
| 1965 | 9,482 | 2,793 | 12,275 |
| 1966 | 10,335 | 2,995 | 13,330 |
| 1967 | 9,517 | 3,893 | 13,410 |
| 1968 | 9,744 | 5,209 | 14,953 |
| 1969 | 9,972 | 5,729 | 15,701 |
| 1970 | 10,401 | 5,891 | 16,292 |
| 1971 | 10,977 | 6,128 | 17,105 |

Bangkok is the hub of the major trunk highways in Thailand. The Highway Route No. 1, by far the most important routes of all, starts from Bangkok and forms the main artery connecting the northern part of the Kingdom, passing Saraburi, Nakhon Sawan, Tak, Lampang (where it branches to Chiang Mai), Chiang Rai to end at the Thai-Laos border. The Route No. 2 branches out to the east from Route No. 1 at Saraburi to form the main route to the northeastern region, passing Nakhon Ratchashima, Khon Kaen to end in Nong Khai, the major border town which is separated from the Laotian capital of Vientien by the Mekong River. The Route No. 3, joins Bangkok with the Southeastern region, passing through Samut Prakan, Rayong, Chantaburi, and terminating in Trat. The Route No. 4 is the main artery to the Southern peninsular region, starting from Bangkok and passing through Nakhon Pathom, Petchaburi, Chumpon, Trang, and, at the Thai-Malaysian border, connecting to the Malaysian highway Route No. 1 which runs right up to Singapore. Other highways complement the abovementioned 4 major routes to form a network over the Kingdom.

3-2 Existing Road Network Situation in Bangkok

The present road network system is far from desirable for a metropolitan of three million in population. The expansion of the metropolitan is far too fast for new road construction and road improvement to keep pace with. (Fig. 3-3). A random survey of road area percentage carried out by the Office of Metropolitan Traffic Planning revealed that only around ten percent of the total area in the surveyed area is roadway. The percentage for the whole metropolitan is probably much lower, as the survey was carried out near the old city centre of Bangkok, where roads exist at a higher density than the districts farther away from the city core. This is a very low percentage as compared to other major cities of the world where an average of over twenty percent is common.

The many waterways, which in former days, served as the main arteries of transportation, have also proved to be great hindrance for a road network. Minor waterways have to be filled up, while major waterways which are preserved, have to be spanned by major structures. The major waterways are shown in Fig. 3-4. By far the most important waterway is the Chao Phraya River which still plays a very important role in transportation of Bangkok. The river forms the natural boundary between the municipalities of Bangkok and Thonburi.

Only four bridges span this great river. The Memorial Bridge joins the old city centres of the two municipalities and is the most important cross-river bridge. Some six kilometers upstream is the Krung Thon bridge which is next to the Memorial Bridge in importance. To the south, the Krung Thep bridge forms a connection between the Phrachao Taksin Road on the Thonburi side and the Charoen Krung Road on the Bangkok side.

Traffic volume on this bridge is small, due mainly to the poor running condition of the access roads. The Rama VI Bridge is far to the north and serves as the link between Nonthaburi and Thonburi.

The demand of cross-river traffic is beginning to show sign of exceeding the traffic capacity provided by the bridges and provision of new bridges is underway. The Tachang Bridge at present under construction will no doubt greatly relieve the daily traffic congestion on the Memorial Bridge. The Sathorn Bridge to be shortly implemented should further improve the situation of cross-river traffic.

Major khlongs (canals) also form major obstacles in improvement of road network. The old city centre is encircled by Khlong Lot, Khlong Ong Ang-Khlong Pang Lam Phu, and Khlong Phadung Krung Kasem. In this old city district, the three khlongs are well served by bridges which link all major thoroughfares. However, the widths of the carriageways inevitably narrow at the bridges, resulting in certain falling in capacity.

The Khlong SaenSaeb running east-west-wise from Khlong Krung Kasem to the boundary of Amphoe Bang Kapi and then turning northeast therefrom, virtually divides the city of Bangkok into two parts. The east-west section of the khlong is about ten kilometres in distance. However, there are only eight bridges across the waterway. (including the bridge at Wittayu Road which is not yet open to traffic). There is not a bridge across the three-kilometre section from Soi Asok (Soi 21) to Soi Ekkamai (Soi 63). This lack of connection link between Phetchaburi Road and Sukhumwit Road results in unnecessary concentration of traffic and brings about congestion on these two roads. The other khlongs of importance on the

Bangkok side are Khlong Samsen, Khlong Bang Sue, Khlong Bang Khaen, Khlong Lat Phrao, Khlong Prem Prachakon and Khlong Phranakhong. On the Thonburi side, Khlong Bangkok Yai, Khlong Bangkok Noi, Khlong Bang Kruai and Khlong Mon are the major khlongs which require major structures in road construction. However, the problem here is less prominent due to the present-day comparative low demand for cross-river traffic movement.

The road network pattern is basically different between the old city centre (The Amphoes of Phra Nakhon, Pomprap Suttru Phai and Samphanthawong) and the rest of the Metropolitan. As can be seen from Fig. 3-3, the old city centre is densely linked by grid road network running in the east-west and north-south direction. However, the roads are on the whole narrow, and multi-lane roadways are few. Nevertheless, such grid network provide good accessibility to every part within the centre. Beyond this centre core, the network pattern is basically linear. In each direction, two or three trunk roads run parallel from the centre core to the outskirt. These parallel trunk roads are connected to each other by very few connecting links. As a result, movement from one trunk road to another is confined to very limited number of roads. The trunk roads in different directions are seldom connected to each other, and transfer of traffic from one direction to another usually has to take place in the heavily built-up city area.

By far the most important trunk roads are those running in the east-west direction. The Phetchaburi-New Petchaburi Road, the Sukhumwit Road, and the Rama IV Road, are the three trunk roads running in these direction, the Phetchaburi Road starts from the Mission Hospital near Khlong Krung Kasem and runs eastward along Khlong Saen Saep for a distance of about ten kilometres to meet the Soi Phranakhong-Khlong Tan (Soi 71) which

runs in a north-south direction. The section east of Soi Nana (Soi 3) is referred to as the New Petchaburi Road. This thoroughfare is comparatively new, and, being sandwiched on both sides by Khlong Saen Saep and Khlong Samsen, development along the road has not extended to any depth, although the strips on both sides of the road are completely opened to business and commercial activities.

The Rama IV Road is the southernmost of the three east-west thoroughfares. It starts from the Bangkok Railway Station to join the Sukhumwit Road at Phrakhanong, about ten kilometres from the starting point. This thoroughfare is important as the distributor and collector road for traffic to and from the Amphoes of Bangrak and Yannawa. Skirting the Port of Bangkok, it also offers the access to the port area through Sunthon Kosa Road and At Narong Road.

The Sukhumwit Road lies between the two above-mentioned roads. The section running east-west, namely from Khlong Krung Kasem to the railway line to the port, is named Rama I Road for the first half and then Ploenchit Road for the other half. The name Sukhumwit Road begins from the east of the railway line, and runs in a southeast direction to Phrakhanong and then southwards to Bangna before branching off into national highways Route No. 3 and Route No. 34. Rama I Road and Ploenchit Road form the axle of the new city centre, where business and commercial concerns are concentrated. The districts on both sides of the Sukhumwit Road are the fastest developed districts in recent years. Business, commercial and international tourist facilities line the road sides, while deep behind the road, residential area sprawl at tremendous pace. South of Phrakhanong, industrial establishments are clustered.

For the sections between Klong Krung Kasem and the Port railway line, the three thoroughfares are connected by the roads

Rama VI, Phayathai, Ratchadamri and Wittayu, which run perpendicular to the three roads. Rama IV Road and Rama I Road are further connected by the Henri Dunnant Road. East of the Port railway line, the lack of connecting roads between the three roads is conspicuous. New Petchaburi Road and Sukhumwit Road are connected by Soi Nana (Soi 3), Soi Asok (Soi 21), Soi Ekkamai (Soi 63), and Soi Phrakhanong-Khlong Tan (Soi 71). The connecting links between Sukhumwit Road and Rama IV Road are Soi Ari (Soi 26), Soi Naphasap (Soi 36), Soi Ban Kluai Tai (Soi 40) and Soi Kluai Nam Thai (Soi 42). Except for Soi Asok, which has a carriageway of four lanes, the other sois are all narrow in width, poor in alignment, and low in traffic capacity.

The most important trunk roads in the north-south direction are the Phahon Yothin Road and the Mittraphap Road (Super-highway). The Phahon Yothin Road, starting from the Victory Monument, extends north-northeast to intersect with Mittraphap Road at Lat Phrao and continues in the same direction, finally remeeting Mittraphap Road north of the Changwat boundary. The Phahon Yothin Road is continued to the south by the Phaya Thai Road which ends at the Rama IV Road. The Mittraphap Road begins from Din Daeng, and after twice intersecting with the Phahon Yothin Road, extends north as national highway Route No. 1, which is the sole connecting trunk road with the northern changwats. Along both sides of the road, the Phahon Yothin Road is heavily built-up from the Victory monument to the intersection with Pradiphat Road, and moderately built up beyond this point, except at the vicinity of Don Muang, where some concentration is seen near the airport. The Mittraphap Road is a new highway, and development along the road has not taken place to any extent. However, residential development and some industrial development is fast picking up momentum, especially near Din Daeng.

The Rama V Road and the Samsen Road also vertically traverse Dusit Amphoe from the old city centre, and the extension to the north makes up the main link between Bangkok and Nonthaburi. Pradiphat Road link these two roads to Phahon Yothin Road to provide access to the north for the Dusit Amphoe.

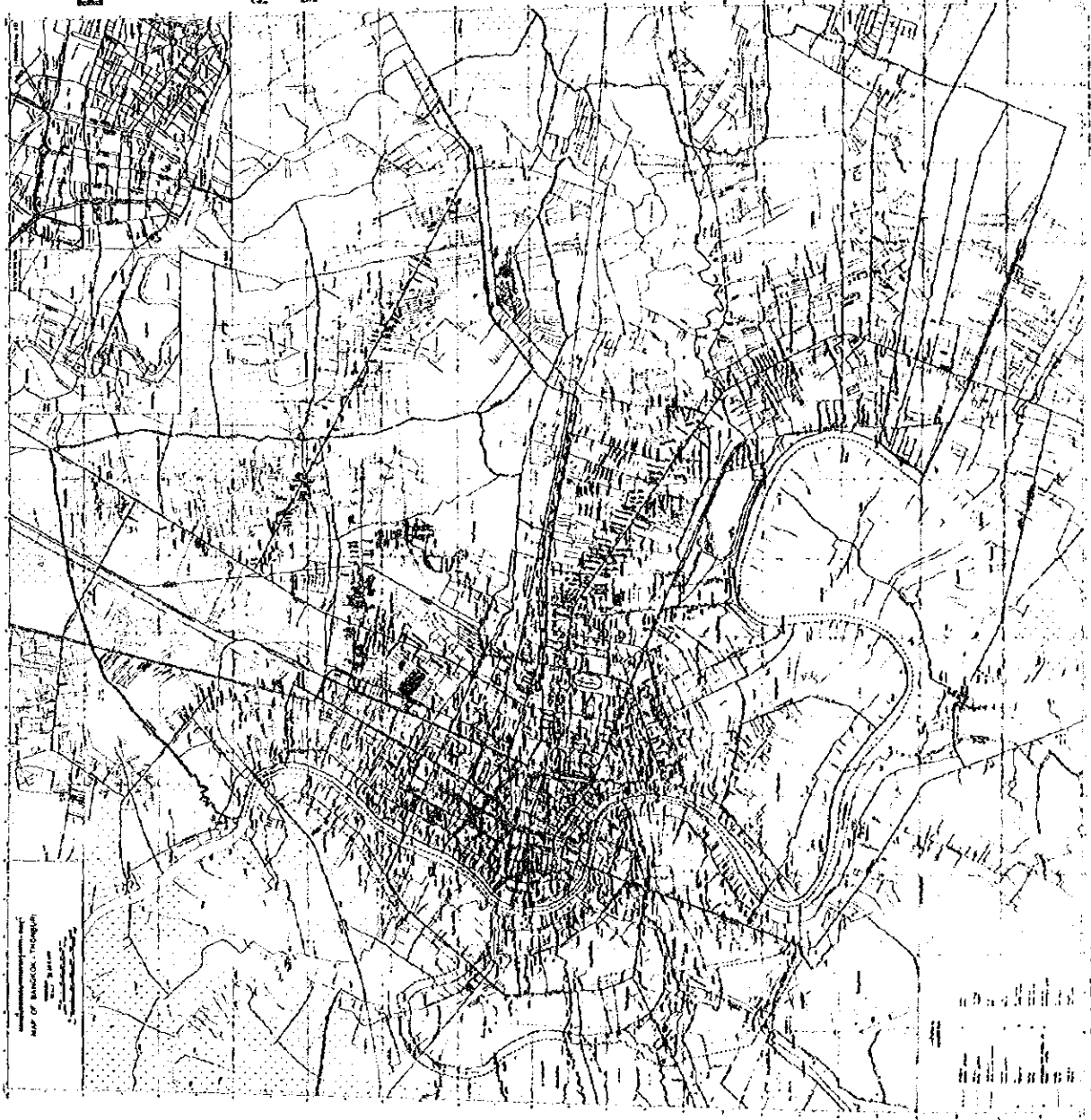
The core on the Thonburi side is small and the road network pattern is more simplified. From the Memorial Bridge, the Prachathipak Road extends south to Wong Wian Yai, from which the road is continued by Phra Chao Taksin Road to Phra Pradaeng. East of the Phra Chao Taksin Road, the Charoen Nakhon Road runs along the Chao Phraya River. From Wong Wian Yai, the Inthara Phithak Road runs west until Tha Phra intersection where it branches into Phet Kasem Road to the west and Charan Sanit Wong Road to the north. The Charan Sanit Wong Road ends at Rama VI Bridge and is the major link between Thonburi and Nonthaburi.

Compared to Bangkok, Thonburi is generally less heavily built up. However, development is taking place at a high pace along the Phra Chao Taksin Road and Charan Sanit Wong Road. To the west, heavy concentration is found up to the Tha Phra intersection.

Fig. 3-5 shows the major trunk roads in the twin municipalities. From this, it can be seen that lack of connection between trunk roads to the east and those to the north is conspicuous. Far beyond the town centre, the Lat Phrao Road and the road via Kitikhachon Stadium come under this category. However, the detour is too great for any practical purpose as a bypass. All traffic transfer between the north and the east take place by once passing through the city area. This results in the daily nightmarish traffic congestion at the Din Daeng area and along the Ratchaprarop Road.

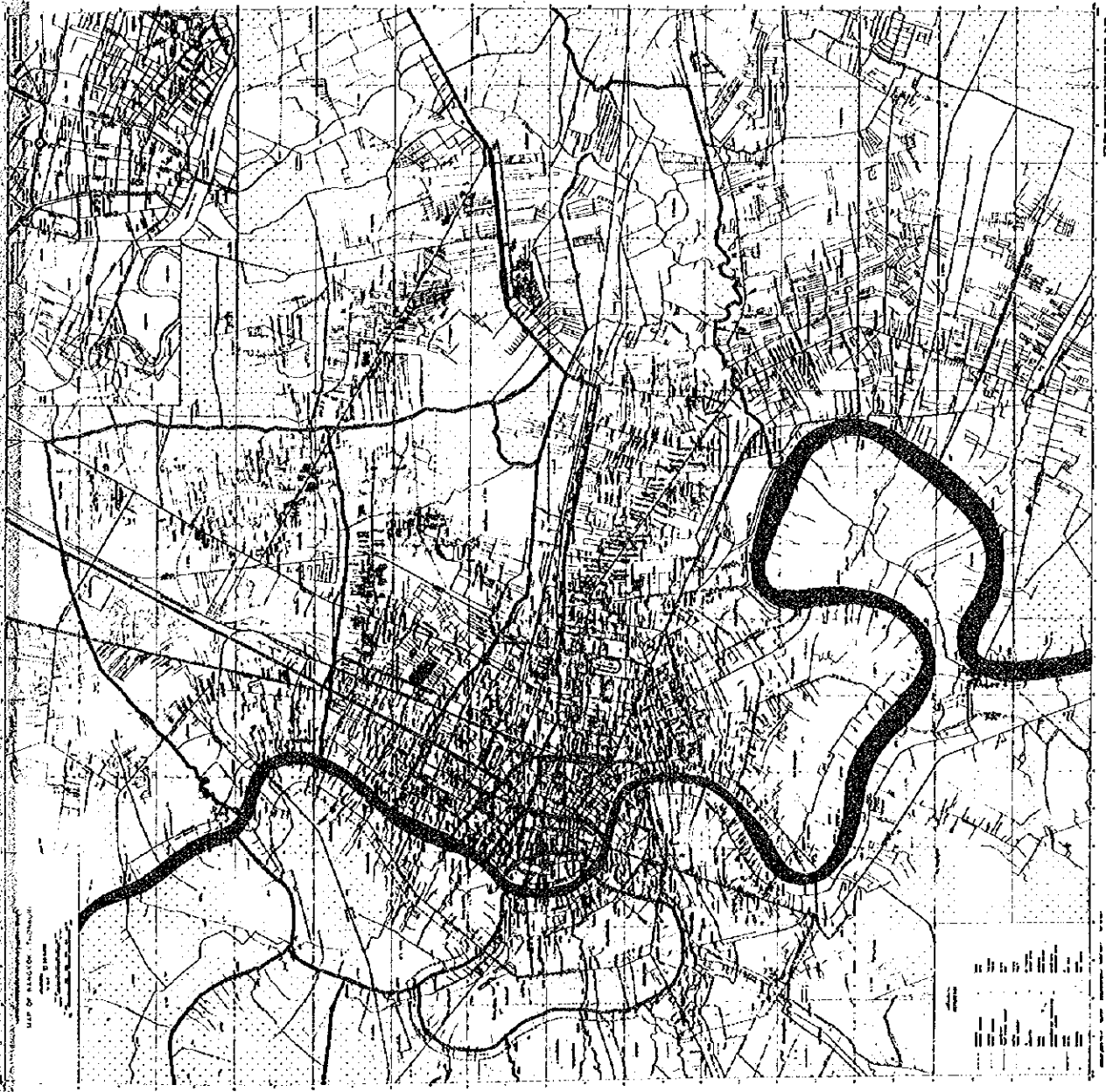
Bangkok-Thonburi Ring Road Part II

Fig 3-3
Existing Road Network in Bangkok



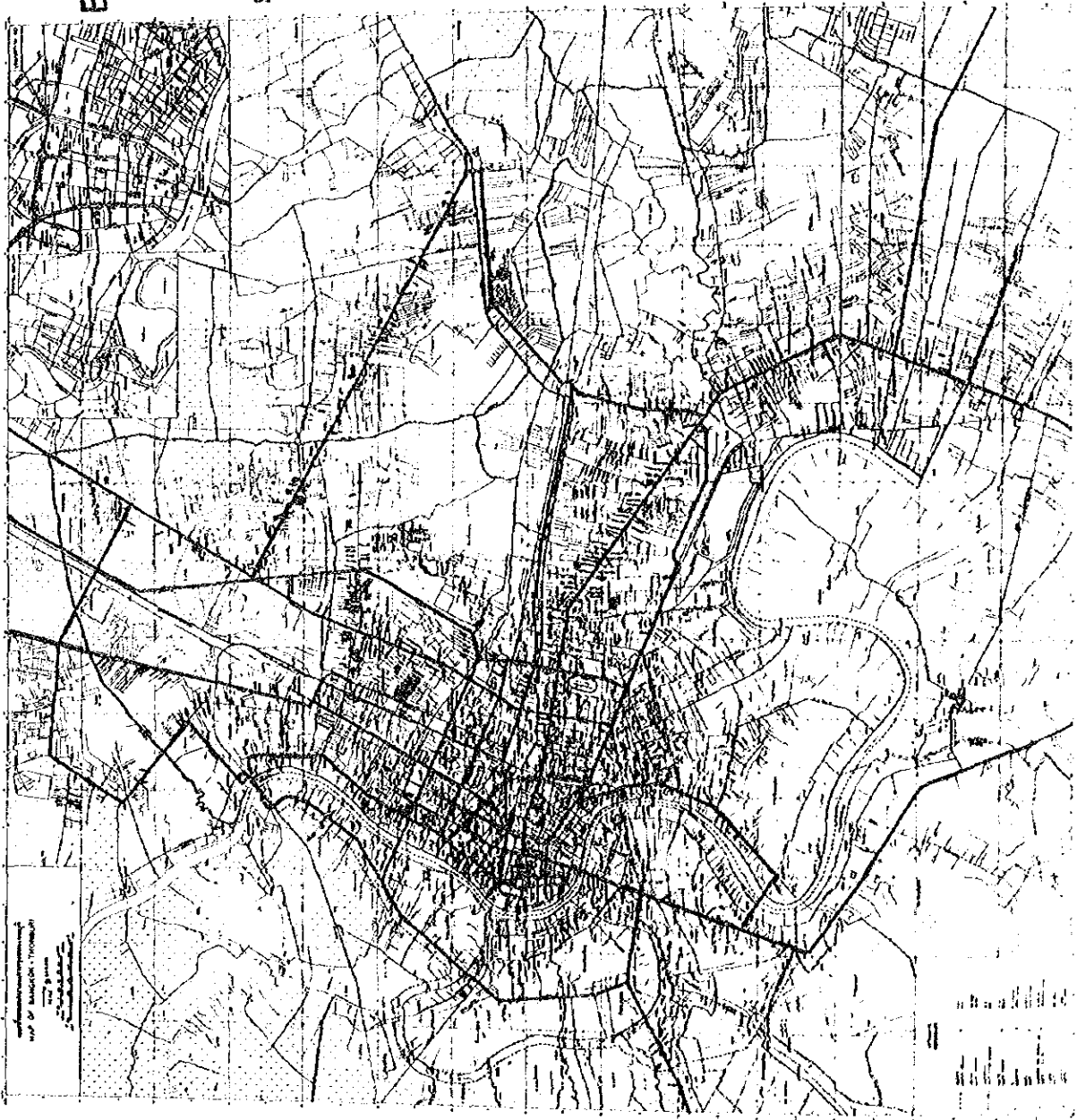
Bangkok-Thonburi Ring Road Part II

Fig 3-4
Major Waterways in Bangkok



Bangkok-Thonburi Ring Road Part II

Fig 3-5
Major Trunk Roads in Bangkok



3-3 Analysis of Present Traffic Situation in Bangkok

Congestion is the general impression received from the traffic situation in the city area of the twin municipalities. However, the extent of the congestion cannot be clearly grasped without some actual surveys. The following surveys are the common surveys usually carried out.

- (1) Travelling time survey : The actual travel time required for a road section is surveyed through travelling with the traffic stream and recording the time consumed for the road section. In this study, an extensive travel time survey was carried out and the details will be described in subsequent sections.
 - (2) Cross-section traffic volume survey: A count of the volume of traffic through a road section is made and then analyses carried out regarding the quality and quantity of the traffic volume. A comparison of the traffic volume with the capacity of the carriageway will provide useful information regarding the traffic situation of the road section. In this study, lack of time did not allow our carrying out a traffic count of our own. However, an extensive survey had been carried out by the Office of Metropolitan Traffic Planning. Analyses were carried out with the valuable survey results kindly provided us by the MTP.
 - (3) Intersection traffic survey : In this survey, the volume of traffic in each direction is counted for the intersection surveyed to establish the pattern of traffic flow at the intersection. In this study, the data of intersections traffic survey were also kindly provided by the MTP.
- In the following sections, a description of the traffic situation in the Bangkok-Thonburi municipalities will be made through analyses of the aforesaid survey results.

3-3-1 Travel time survey

In this study, travel time survey was carried out from August 3 to August 16, 1972 for all major roads in the city area of the metropolitan. (Fig. 3-6 to 8). A passenger car was driven through the surveyed route with the main traffic stream, and the running time, stopping time, and reasons for stopping were recorded for every road section. The travel speed and travel time of a road section varies according to the hourly traffic volume which in turn is variable according to the time of the day. To obtain good survey data, it is necessary therefore to carry out as many surveys as possible, spreading the survey time to different time of the day. In the survey for this study, consideration in this regards is taken and endeavours are made to carry out at least two runs for each road section. However, due to lack of time, it was not possible to carry out two runs for some sections of minor importance. As a result, some sections (e. g. the road sections in Dusit Amphoc) were only covered by one survey run. (Fig. 3-9).

Table 3-14 is the summary of the travel time survey. It took a total of 15 hours and 55 minutes to travel a total distance of 371.9 kilometres, which comes to an average travelling speed of 23.4 kilometres per hours. Of the nearly 16 hours of travelling time, a loss time of four hours and nineteen minutes (or 27.1% of travel time) was accounted for by various reasons. The average running time for the same distance was therefore 32.1 kilometres per hour. A breakdown of the stopping time shows that by far the largest percent of loss time was due to traffic signal at intersections, (55.7%). Bangkok is probably conspicuous for its extraordinary length of traffic signal cycle. Especially during the 'rush hour' when most of the traffic signals at major intersections are manually manipulated by traffic

policemen, it is not uncommon for vehicles to encounter red traffic signal for a period of from three to five minutes. This results in long waiting queues sometimes several hundred metres long. Sometimes the effect of such long queues are extended to the next intersections where the vehicles are unable to move ahead despite the traffic signals being green because of the waiting queues ahead. It is fortunate that such disturbances are not common, due to the extraordinary length between intersections in Bangkok. The accumulated loss to the national economy due to undue waiting time must be great, and we cannot help but feel that a shorter signal cycle is more beneficiary on the whole. 'Congestion', the next biggest item in loss time (28.6%), includes time loss for stopping at an intersection for more than one traffic signal cycle. This phenomenon is especially conspicuous in the old city centre (China Town) where vehicles move ahead at a 'snail pace' for most of day.

During the whole survey, a total stops of 46 times were made for pedestrian crossing the roads. Although the total loss time was comparatively small, such abrupt crossing of pedestrian at unexpected place are annoying to drivers and cause disruption to the smooth flow of traffic, not to mention the peril of traffic accidents. The non-observation of the pedestrian crossing zones and the disrespect of the pedestrian crossing zones by the drivers render such crossing zones meaningless and result in pedestrians crossing at any place considered convenient. Enforcement of pedestrians crossing the roads only at specified places will greatly help in the smooth flow of the traffic stream. 28 stops were made for buses stopping for boarding and descending passengers. While some were due to insufficient width in carriageway, many were due to bus drivers' failure to pull along the side of the road for

passenger loading and unloading. Education of bus drivers in this respect is also important to ensure uninterrupted traffic flow. Other reasons made up about 10% of interruptions during the travel survey and are considerably of less importance compared to the above-mentioned reasons.

Fig. 3-10 shows the average travel time, stopping time and the number of stops for each road section, while Fig. 3-11 shows the average travel speed of each road section as obtained from the survey. On the whole the average travel speed of most of the road sections in the survey area falls between 20 km/hour to 30 km/hour. Most road sections in and around the old city centre recorded an average travel speed of under 20 km/hour. Especially on the two one way streets in the China Town, namely the Yaowarat Road and the Charoen Krung Road, the average travel speed fell below 10 km/hour. The road sections in the Phrakhanong district also recorded extremely low average travel speed. Of the linear trunk roads, the Mittraphap Road and the New Petchaburi Road registered the highest average travel speed. The Mittraphap road, being under partial control, has few interruptions by traffic signals. The section from Din Daeng to the Inthramara intersection average nearly fifty kilometres hourly speed while the section further north could be travelled at over sixty kilometres in average speed. The New Petchaburi Road, by virtue of being less densely built-up on both sides of the road, has a smaller traffic volume, and was able to record an average travel speed of well over 40 km/hour. On the Thonburi side, The section of Charan Sanitwong from Tha Phra junction to the Krung Thon Bridge intersection shows a relatively high average speed, while the rest of the network averaged around or below 30 km/hour.

Table 3-14 Summary of Travel Time Survey

(Survey Carried out during 3rd Aug. - 16 Aug. 1972)

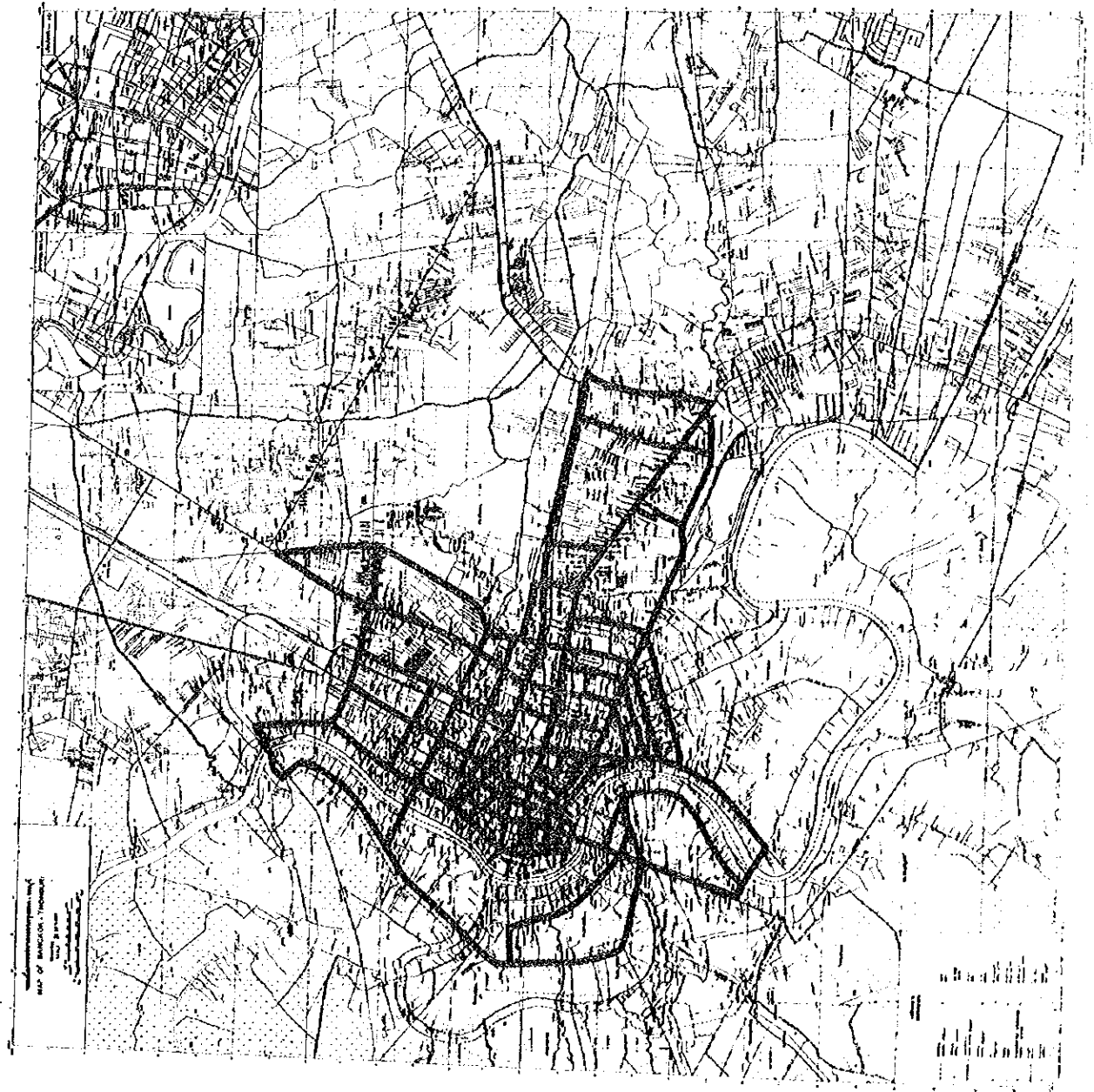
| | | |
|---------------------------------|---|-----------------------------------------------------------|
| Total Distance Travelled | : | 371.9 km |
| Total Travel Time | : | 15 hours 55 min. 15 sec. |
| Average Travel Speed | : | 23.4 km/hr. |
| Total Stopping Time (543 stops) | : | 4 hours 19 min 09 sec. (27.1% of Total Travel Time) |
| Total Running Time | : | 11 hours 36 min. 06 sec |
| Average Running Speed | : | 32.1 km/hr. |

Details of Stopping Time

| | <u>Reason for Stopping</u> | <u>No. of Stops</u> | <u>Total Stopping Time</u> | <u>%</u> |
|-----|-------------------------------------------------------|---------------------|----------------------------|----------|
| | Total | 543 | 4 hr. 19 min. 09 sec. | 100.0% |
| 1. | Traffic Light | 224 | 2 hr. 24 min. 23 sec. | 55.7% |
| 2. | Traffic Accident Ahead | 3 | 1 min. 21 sec. | 0.5% |
| 3. | Pedestrian Crossing | 46 | 6 min. 04 sec. | 2.3% |
| 4. | Due to Bus Stopping | 28 | 3 min. 37 sec. | 1.4% |
| 5. | Vehicle Entering Traffic Stream from side roads | 23 | 2 min. 53 sec. | 1.1% |
| 6. | Stop at Police' Direction (for vehicle or Pedestrian) | 9 | 6 min. 36 sec. | 2.6% |
| 7. | Congestion | 152 | 1 hr. 14 min. 05 sec. | 28.6% |
| 8. | Vehicle Crossing | 18 | 2 min. 47 sec. | 1.1% |
| 9. | Weaving at Circus (Rotary) | 19 | 6 min. 43 sec. | 2.6% |
| 10. | Construction Work ahead | 10 | 3 min. 20 sec. | 1.3% |
| 11. | Railway Crossing | 3 | 5 min. 10 sec. | 2.0% |
| 12. | Stopping for Right Turn | 8 | 2 min. 10 sec. | 0.8% |

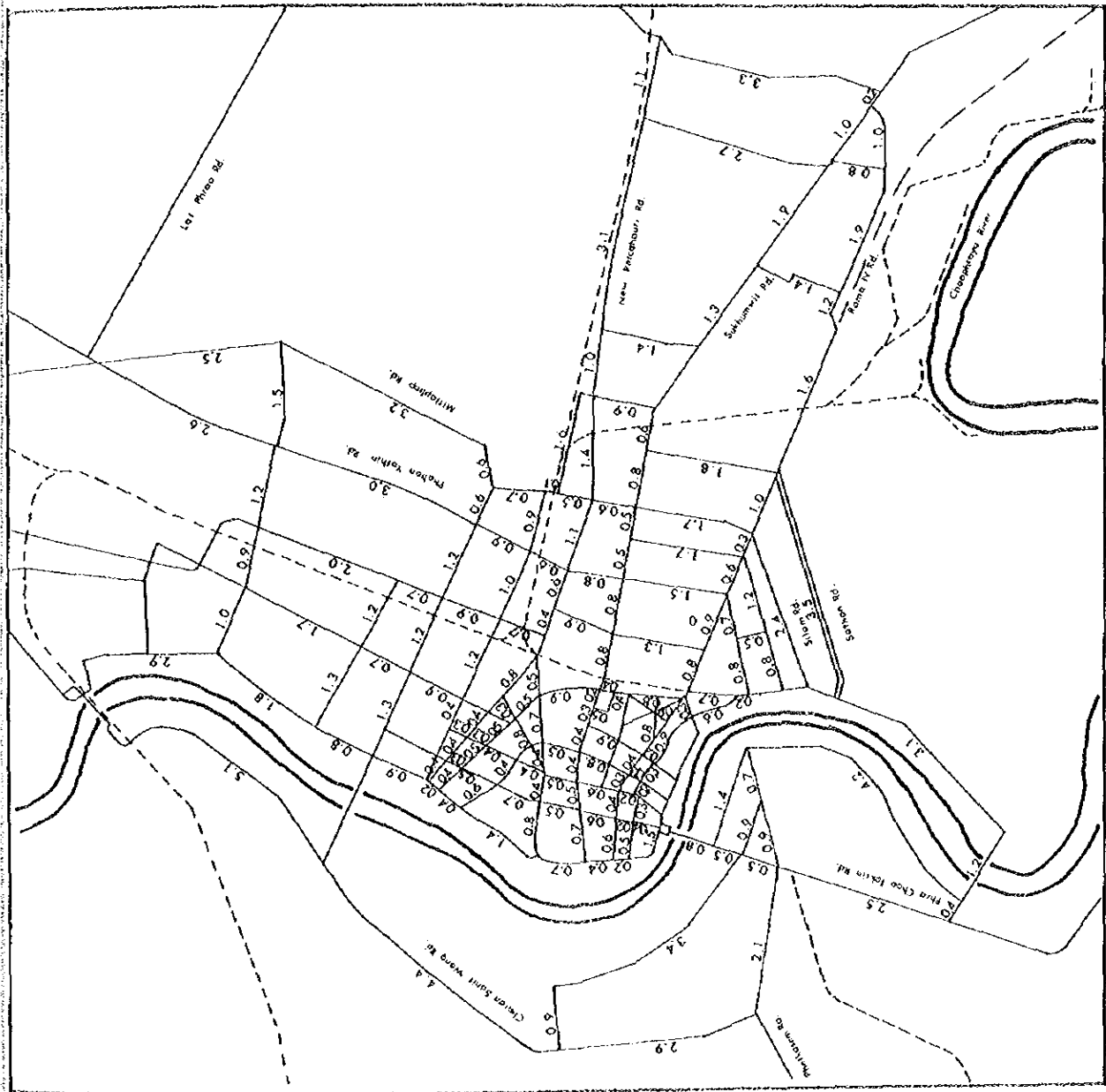
Bangkok-Thonburi Ring Road Part II

Fig 3-6
Roads Covered in Travel Time Survey



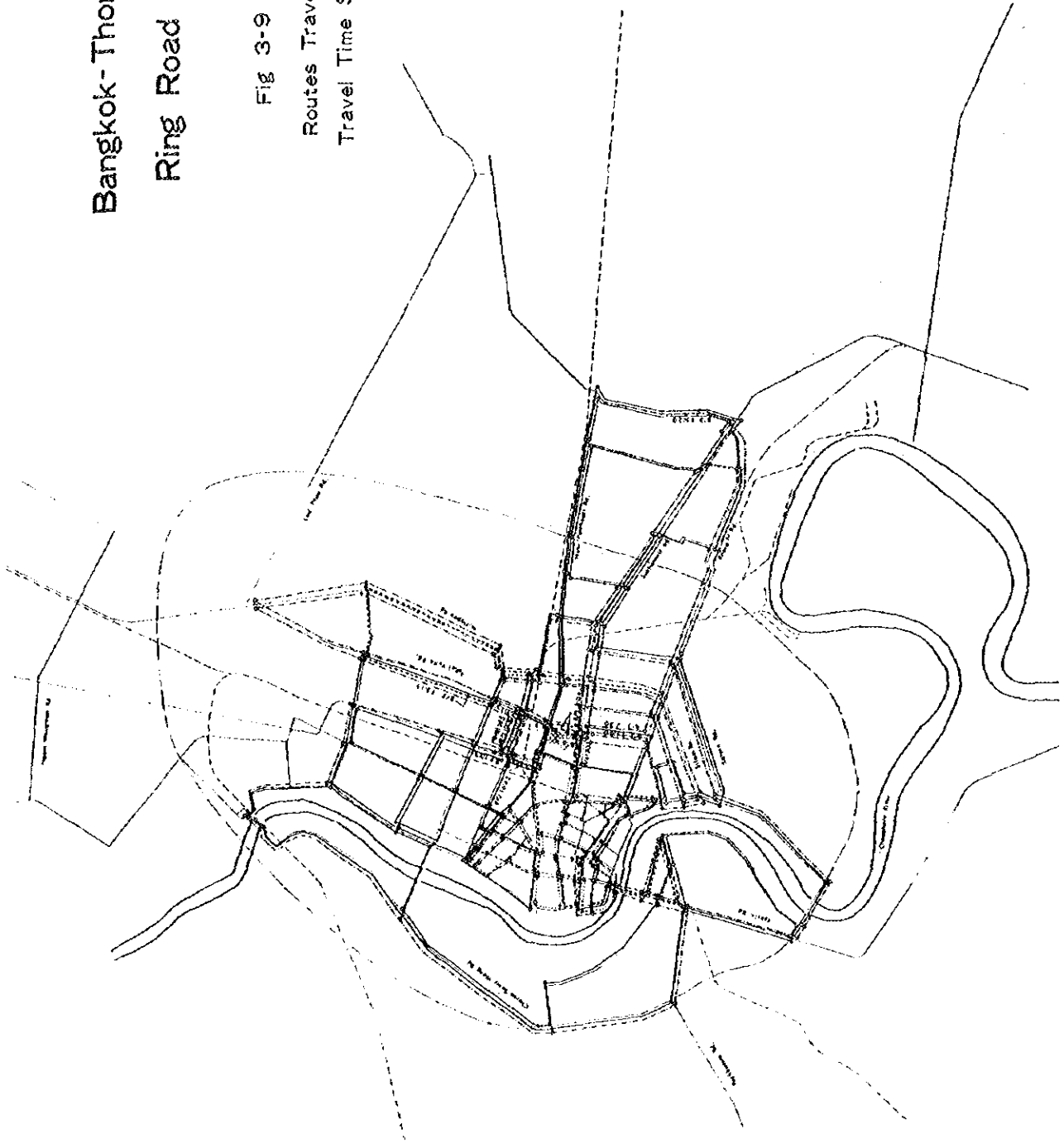
Bangkok-Thonburi Ring Road Part II

Fig 3-7
Link Distance for Roads
Covered in Travel Time Survey



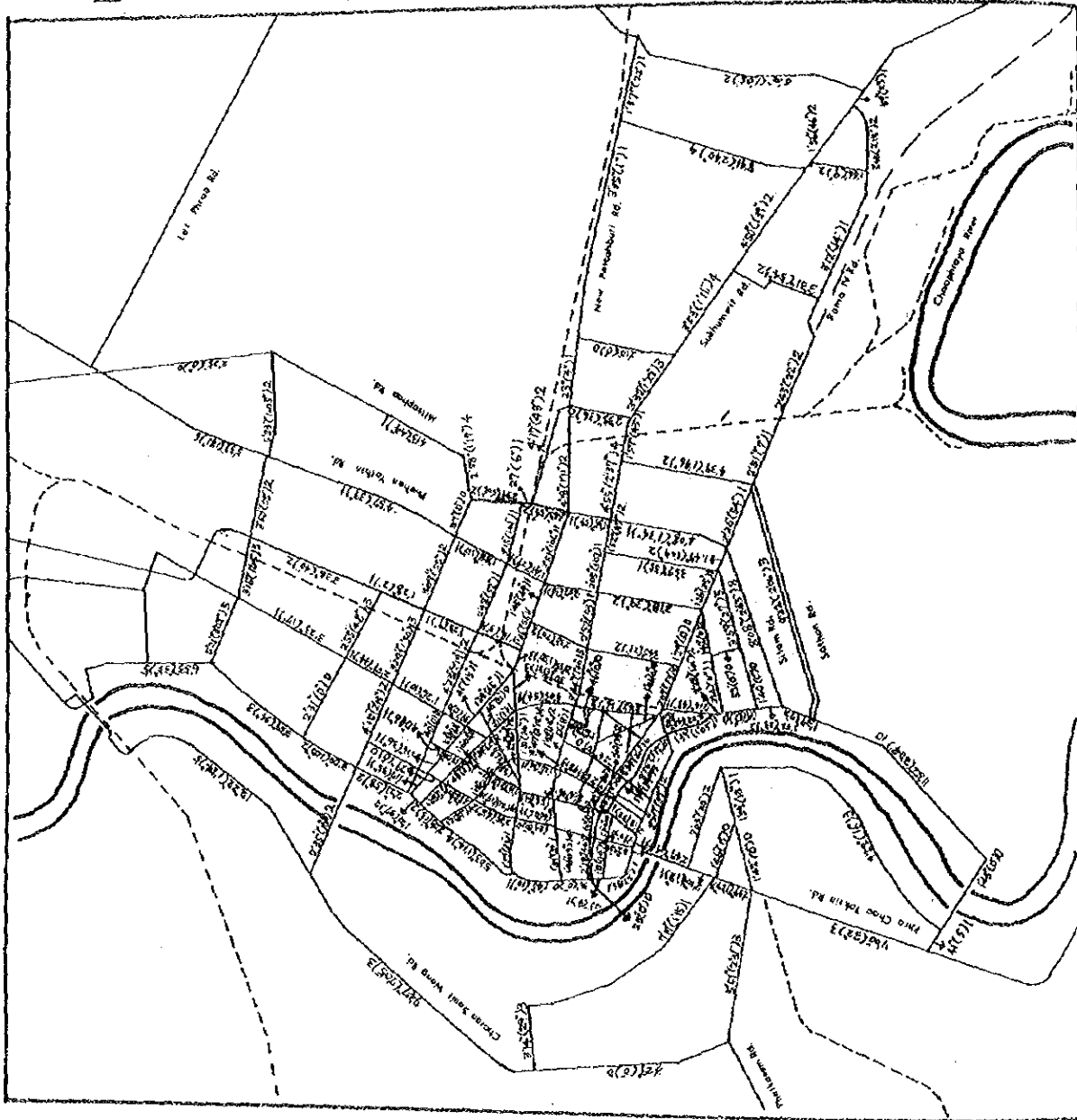
Bangkok-Thonburi
Ring Road Part II

Fig 3-9
Routes Travelled in
Travel Time Survey



Bangkok-Thonburi Ring Road Part II

Fig 3-10
Results of Travel Time Survey
Travel Time (Stopping Time) &
No. of Stops



Bangkok-Thonburi Ring Road Part II

Fig 3-111

Average Travel Speed



3-3-2 Cross section traffic volume survey:

A comprehensive cross section traffic volume survey was carried out by the Office of Metropolitan Traffic Planning on thirty three major road sections from 28 January to 8 February, 1972. (Fig. 3 - 12). The survey was for a sixteen-hour period from 6 a.m. to 10 p.m. when hourly traffic classified into 8 classes of vehicle types were counted. Detailed data of the survey together with results of analyses are to be found in the appendices of this report.

- (1) The four road sections from the north to the old city centre, namely stations No. 10 (Samsen Rd), No. 11 (Pracha Thipathai Road), No. 12 (Ratchadamnoen Nok Road), and No. 13 (Nakhon Sawan Road), registered almost equal daily traffic volume of around thirty five thousand vehicles. This even distribution of traffic volume on parallel alternative roads indicate that the traffic volumes are nearing the capacity of the roads, which are all utilized to the fullest extent. In reality, except for the Ratchadamnoen Nok Road, which is ten lanes in width, the other three roads are all weak four lane roads, and a traffic volume of thirty five thousand vehicles is almost the upper limit.
- (2) Three cross section counts were taken on the most important east-west trunk road, the Sukhumwit Road and its extension up to Khlong Krung Kasem, the first at the vicinity of Soi Ari (Soi 26), the second at Ploenchit Road and the third at Rama I Road. The traffic volume recorded in each case remained at around seventy thousand vehicles per day, indicating that for a total distance of over ten kilometres, there was no conspicuous increase or decrease in traffic volume.

In most city roads, the traffic volume tend to be highest near the city centre, and taper off proportional to its distance from the city centre. The phenomenon on Sukhumwit Road is therefore extraordinary and is worth special attention. It can easily be seen that this phenomenon is due to the fact that the Sukhumwit Road is loaded up to capacity throughout the whole length and does not allow any increase in traffic volume to any great extent.

The Sukhumwit Road is a weak six lane road where normally only four lanes are fully utilized while the two outermost lanes are usually occupied by parking vehicles. It is difficult to imagine any further increase in traffic without causing total paralysis to the whole road.

- (3) The Rama IV Road had a traffic volume of over 100,000 vehicles per day at the vicinity of the Bangkok Railway Station, but gradually decreased to ninety thousand near Lumpini Park, and then to around sixty thousand at the vicinity of Soi Ari. The New Petchaburi had a traffic volume of slightly under fifty thousand near Soi Klang (Soi 49), but increased as it approached the city centre, to sixty thousand near Soi Nana, and eventually to around seventy thousand at Lan Luang Road. These figures are more normal for a city trunk road, showing that some increase in traffic volume can be tolerated at the end away from the city centre.

Unfortunately, due to lack of sufficient connecting link between these roads and the Sukhumwit Road, diversion of traffic from the Sukhumwit Road to these two roads is difficult, and they cannot be of any major help in relieving the congestion on the Sukhumwit Road.

- (4) With a traffic volume of about forty thousand per day, the Mittraphap Road (Superhiway) is comparatively under-capacity compared to the Phahon Yothin Road, which runs parallel and is with a traffic volume twice that of the Mittraphap. Here again, diversion is difficult without provision of more connecting links. The Mittraphap, lowly built up along the route, however, has a special purpose of serving as the access road to the airport and the northern regions of the nation.
- (5) Soi Rongphayaban and Nang Linchi Road provided the access to the deep southern part of Yannawa Amphoe. Despite the poor condition of alignment and low traffic capacity of these two-lane roads, traffic volumes counted were respectively twenty and thirty thousand vehicles per day. The lack of adequate road network in this area results in heavy concentration of traffic on the limited number of access roads. Considering the future development earmarked for this district, improvement of the road network is of utmost importance.
- (6) The Memorial Bridge, with a daily traffic volume of over one hundred thousand vehicles, accounted for over half the traffic volume across the Chao Phraya River. The Krung Thon Bridge registered a traffic volume of about fifty thousand, which was about a quarter of the total cross river traffic. Despite the favourable condition of being near the new town centre of the Bangrak Amphoe, the Krung Thep Bridge had an unexpectedly low daily traffic of just over thirty thousand. The poor condition of the access roads is the main reason for this low traffic volume, and the improvement of the road network in the Yannawa Amphoe will no doubt be of help in putting this bridge into better use.

3-3-3 Intersection traffic stream survey

An intersection traffic stream survey was also carry out by the Office of Metropolitan Traffic Planning in early 1972 on seven intersections, most of which are situated away from the town centre. (Fig. 3-13). A few comments on the traffic pattern of each intersection is given below.

- (1) Station No. 1 (Charan Sanitwong - Krung Thon extension intersection)

The major flow at this intersection was the left or right turn traffic from the Charan Sanitwong Road to cross the Krung Thon Bridge, and the north-south through traffic formed only a small percentage. This shows that the characteristic of the Charan Saintwong Road is that of an access road for Thonburi to enter Bangkok. As the densely built up area of Thonburi is much farther to the south of the intersection, it clearly shows that the Charan Sanitwong Road serves as a bypass for traffic to the northern part of Bangkok.

- (2) Station No. 2 (Wong Sawan Road-Phibun Songkhram Road intersection)

This is the intersection where the traffic from Nonthaburi branches to Bangkok and Thonburi via Samsen Road and Rama VI Bridge respectively. The survey results show that the major traffic stream is in the direction of the Rama VI Bridge. Coupled with the results of station No. 1 it can easily be deduced that a large portion of the traffic from Nonthaburi to Bangkok travels via Rama VI Bridge-Charan Sanitwong Road-Krung Thon Bridge route, and the percentage of traffic via the Samsen Road is small.

Here again, the northern section of the Charan Sanitwong Road serves as the bypass for traffic from Nonthaburi to Bangkok.

(3) Station No. 3 (Pahol Yothin Road-Mittraphap Road intersection)

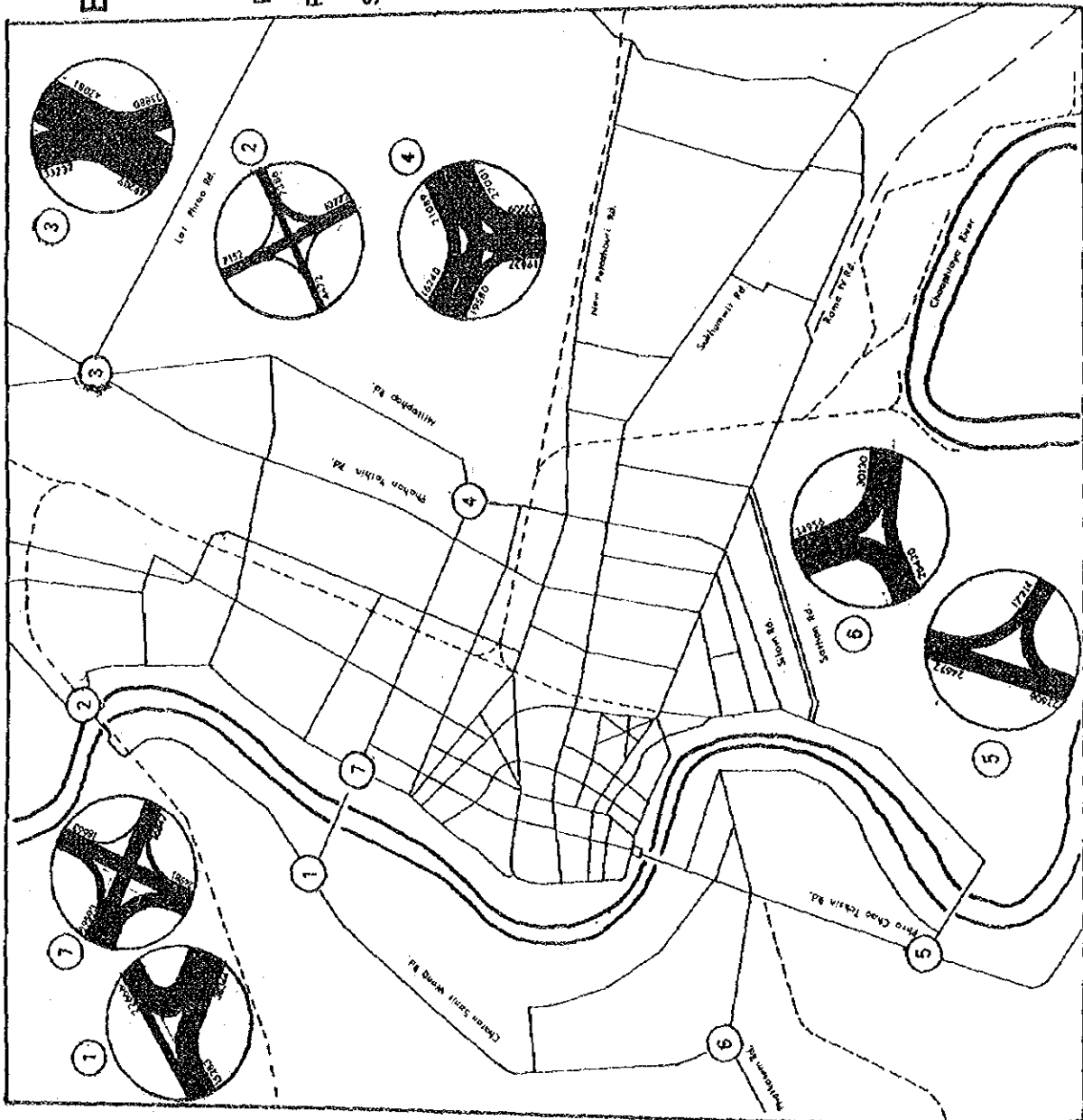
At this junction, the traffic from the north branches into either the Pahol Yothin Road or the Mittraphap Road. It is interesting to note that while traffic from either direction of the Mittraphap Road divided almost equally into the two roads after crossing the intersection, while the major share of the traffic on the Pahol Yothin Road continued ahead along the same road. The fact that the Pahol Yothin was far densly built up than the Mittraphap along the road, thus contributing to the generation of traffic from along the road, was a major reason for this traffic pattern. However, the perennial congestion at Din Daeng, the terminal of Mittraphap Road, certainly did not offer any encouragement to the use of the Mittraphap Road.

(4) Station No. 4 (Din Daeng Junction)

The Mittraphap Road terminates here, where traffic diverges into Ratwithi Road and Ratchaprarop Road. This is the major junction, through which traffic between the northern part and the southeast part of Bangkok mainly traverses, and congestion is a common phenomenon. While traffic was generally heavy in all direction, the Mittraphap Road-Ratchaprarop Road direction formed the major flow. The relief of congestion at this junction is one of the major problems for the improvement of traffic situation in Bangkok.

Bangkok-Thonburi Ring Road Part II

Fig 3-13
Results of Intersection Traffic
Stream Survey
(6.00a.m~6.00p.m)



(5) Station No. 5 (Taksin Road-Krung Thep Bridge Extension).

This is the major access to the Krung Thep Bridge.

However, survey results showed that the Krung Thep Bridge direction was a minor stream of traffic flow. The poor running condition of Charoen Krung Road, the access to the bridge on the Bangkok side, was no doubt the major reason for the low usage.

(6) Station No. 6 (Samyeak Tha Phra Junction)

The Charan Sanitwong Road and, the Phetkasem Road meet at this point. It is significant that the Charan Sanitwong direction was the major stream for both the traffic from the west and from the town centre of Thonburi. It is evident again, that a large portion of the traffic goes into Bangkok via Krung Thon Bridge.

(7) Station No. 7 (Ratwithi Road-Samsen Road Intersection)

This is an intersection in the town centre and is also the entrance to the Krung Thon Bridge on the west. At this intersection the traffic volume in the southern direction was almost twice that in the east direction, showing that the gravity of the southern direction, which leads to the city centre is greater than the eastern direction. Traffic to Krung Thon Bridge came evenly from all directions.

3-3-4 Origin-destination traffic survey

Another traffic survey that will usually provide very valuable informations regarding the traffic situation of a region is the origin-destination traffic survey (O-D Survey). In this survey, the starting point and destination of each vehicle trip is verified through either road-side interview of vehicle driver

at specific cordon stations, or through requesting the cooperation of a sampled portion of vehicle owners to provide informations regarding their activities at a specific date by answering a prepared questionnaire. An analysis of the results of an O-D survey will reveal the pattern of the vehicle owners' desire to travel irrespect of the actual traffic pattern on the existing road network. Provision of future road network to fulfil the actual desired of the owners may be planned.

However, an O-D survey, whether through roadside interview or home interview, and the subsequent analyses, are very costly and time consuming. Moreover, constant survey of the same region at regular interval is necessary in order to obtain the trend of change in travel desire. For these reasons, the availability of complete O-D data is limited to very few countries of high intensity in highway traffic. In Bangkok, no regular O-D survey had been made in the past. A few limited surveys were carried out in the process of planning of specific projects. (e. g. O-D survey of Chao Phraya River cross river traffic for the Memorial Bridge Project.) However, such limited data are not sufficient for the overall transportation planning of the Metropolitan. In early 1972, the Office of Metropolitan Traffic Study carried out an overall O-D survey of the whole Metropolitan area. To date, the data are under scrutinization and analysis, and the final results will not be ready for some time to come. Should these data be finally made available, they will be most valuable for any future plans of the Metropolitan road network.

In this study, unfortunately, the full data of the O-D survey carried out by the Office of Metropolitan Traffic Study could not be made available. Through the effort of the Office, however, some parts of the interim results were provide us. They are:

- (a) Outbound traffic in car-units per 24 hours (cordon-line 1972)
- (b) Outbound traffic-person trips per 24 hours (cordon-line 1972)

Although incomplete, the above data were most valuable as reference materials.

In lieu of the results of actual O-D survey, theoretical O-D matrices were made for the purpose of this study. This was done through first estimating the number of registered vehicles in each zone, and assuming the average number of trips per vehicle to obtain the volume of traffic generated from each zone and then distributing the zone generated traffic volume by the gravity method. The O-D matrices thus obtain were then checked for credibility through comparing the theoretical results with the actual cross-section traffic volumes at specific cordon lines, and also through comparing the general distribution pattern of the vehicle type with the limited data available from the Office of the Metropolitan Traffic Study.

A detailed description of the steps taken for the preparation of the theoretical O-D matrices will be made in the subsequent section.

3-3-5 Theoretical synthesis of present O-D matrices

As mentioned above the only O-D traffic survey carried out for the Metropolitan area was the one made by the Office of Metropolitan Traffic Planning in early 1972. This survey, which covered the Changwats of Phranakhon, Thonburi, Nonthaburi and Samut Prakan, was made for the private passenger traffic, public passenger traffic and goods traffic.

The data of the survey are in the process of being sorted, checked and analysed. The part which covered the outbound traffic had been made available and been made use of in this study. However, analysis for the internal traffic will not be completed in time for this study. For this reason, theoretical O-D matrices had been synthesized and the results checked against the actually surveyed traffic volume of various cross sections for reliability of the theoretical figures.

The steps taken in the establishment of the O-D matrices are as follows:

(1) Zone division

The proposed Ring Road is situated within the Metropolitan boundary, and is shown as the Middle Ring Road in the Greater Bangkok Plan. For this reason it is possible that traffic volume that originates within the ring may make use of the Ring Road as a bypass to circumvent the traffic congestion in the city centre. This was taken into consideration in the process of zone division, and the size of the zones were decided such that unnatural movement of the traffic will not result when the zones are represented by centroids in traffic assignment. Also, correspondence of traffic volume of the zones with zone economic factors is necessary in estimation and analysis of future traffic.

Taking these facts into consideration, the zones are divided to coincide with the Amphoe boundary in principle, and subdivided into smaller units where necessary.

Away from the city centre, the zone boundaries are drawn along the amphoe boundaries at the vicinity of the Ring Road, and along the Changwat boundary for the districts at a distance from the Ring Road. Further away from the Metropolitan sphere, two or more changwats are

combined to make one zone wherever the road network pattern allows. As a result, the Changwat of Phranakhon is divided into 30 zones, and the Changwat of Thonburi into 14 zones. The rest of the kingdom is divided into 18 zones, making a total of 62 zones in all. The zoning code list and the zone map are shown respectively in Table 3-15 and Fig. 3-14 - 3-16.

Table 3-15 Zone division

| Zone No. | Enclosed area | |
|----------|----------------------------|----------------------------------------|
| 01 | Amphoe Phra Nakhon | |
| 02 | Amphoe Pomprap Sattru Phai | |
| 03 | Amphoe Samphanthawong | |
| 04 | Pathum Wan | Wang Mai, Rong Muang |
| 05 | | Pathum Wan |
| 06 | | Suan Lumpini |
| 07 | Bang Rak | Sripraya, Bangrak, Mahapruotaran |
| 08 | | Sriwong, Silom |
| 09 | Amphoe Yannawa | Yannawa, Tung Wad Don, Wad Phraya Kai, |
| 10 | | Tung Mahamek |
| 11 | | Bang Kor laem, Bang Klou |
| 12 | | Bang Pong Prang, Chongnonaree |
| 13 | | Chongnonaree |

| Zone No. | Enclosed area | |
|----------|---------------------|---------------------------------------------------------|
| 14 | Amphoe Dusit | Bang Sue |
| 15 | | Tanon Nakornchaisri |
| 16 | Amphoe Dust | Vachirapayabarn, Dusit, Suanchitlada, Siyark Mahanark, |
| 17 | Amphoe Phraya Tai | Sam Sen nai |
| 18 | | Huay Kwang |
| 19 | | Tung Phraya Tai, Petchburi, Tanon Phraya Tai, Makasan, |
| 20 | | Bang Kapi |
| 21 | Amphoe Bang Khen | Lad Yao |
| 22 | Amphoe Bang Kapi | |
| 23 | Amphoe Phra Khanong | Klong Toey |
| 24 | | Klong Ton |
| 25 | | Phra Khanong |
| 26 | | Suan Luang, Prawes, Nong Born, Dok mai, |
| 27 | | Bang Jark, Bang Na, |
| 28 | Amphoe Rat Burana | |
| 29 | Amphoe Bangkok Noi | Bang Or, Bang Plad, Bang Yec Khan, Bang Barnru, Siriraj |
| 30 | | Bang Khur Non, Bang Khur Sri |
| 31 | | Buan chang Lor, Siriraj |
| 32 | Amphoe Bangkok Yai | Wat Tah Phra |
| 33 | | Wat Aroon |

| Zone No. | Enclosed area | |
|----------|-----------------------------------------------------------|------------------------------------------------------------------------------------------------------------------------------------|
| 34 | Amphoe Muang | Wat Kalaya, Hiranruchee |
| 35 | | Bang Yee Ruer, Talare Plu, |
| 36 | | BuKhalo |
| 37 | Amphoe Klon San | Klon San, Klong Ton Srai, Somdej Chao Phraya, |
| 38 | | Bang Lampu-lang |
| 39 | Amphoe Bang Khen | Sri Gun, Thoong Song Hong, Talad Bang Khen, |
| 40 | | Sai Mai, Kling Tanon, Or Ngenn, Anusaowari, TaRoeng |
| 41 | Amphoe Min Buri, Amphoe Nang Chog, Amphoe Lad Krabang, | |
| 42 | Phra Pradaeng | Sang Kanong, Bang Kasorp, Bang, Korbua, Bang Ka Chao, Bang Nam Pueng, Bang Pueng, Talad, Bang Yor, Bang Kru, Bang Jark |
| | Amphoe Muang | Nah Kluer, Nai Klong Bang Pla-Kod, Laem Fah Pah, Klong Bang Pla-Kod |
| 43 | Amphoe Amphoe Bang Plee | Bang Plee Yai, Bang Chalong, Bang Pla, Bang Sao Tong, RaJathewa, Nong Prue Srisa Charak Nai, Srisa Charak Yai |
| | Amphoe Bang Bor Amphoe Muang | Bang Poo |
| 44 | Amphoe Taling Chan | |
| 45 | Amphoe Phasi Charoen, Amphoe Nong Khaem | |
| 46 | Amphoe Bang Khun Thien | |

| Zone No. | Enclosed area | |
|----------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------|
| 47 | Phra Pradaeng | Bang Yah Praeg, Bang Hang Hua Suor, Samrong Tai |
| | Amphoe Muang | Bang Prong, Pak Nam, Bang Duam, Samrong Nueh, Bang Muang, Tai Baan, Bang Poo Mai, Praeg Sa, Bang Kaew |
| 48 | Amphoe Srai Noi, Amphoe Bang Yai, Amphoe Bang Kreuy, Amphoe Bang Bua Tong, Amphoe Pak Kred | |
| 49 | Amphoe Muang | Shai Mar, Bangrak Nai, Bang Krang, Bang Pai, Bang Sri Muang, Bang Kasor, Tah Srai, Talad Kwan, Bang Khen, Suan Yai |
| 50 | Samut Sakhon | |
| 51 | Ratchaburi, Samut Songkhram | |
| 52 | Kanchanaburi | |
| 53 | Nakhon Pathom, Suphan Buri | |
| 54 | Pathum Thani, Pra Nakhon Si Ayutthaya, Ang Thong, Sing Buri | |
| 55 | Lop Buri, Saraburi | |
| 56 | Nakhon Nayok, Prachin Buri | |
| 57 | Chachoengsao | |
| 58 | Chon Buri, Rayong, Chanthaburi, Trat | |
| 59 | Phetchaburi, Prachuap Khiri Khan, Chumphon, Ranong, Surat Thani, Phangnga, Krabi, Nakhon Si Thammarat, Phuket, Trang, Phatthalung, Satun, Songkhla, Pattani, Yala, Narathiwat | |
| 60 | Chiang Rai, Mae Hong Son, Nan, Chiang Mai, Lamphum, Lampang, Phrae | |

Bangkok-Thonburi Ring Road Part II

Fig 3-14
Zone Division
(Bangkok-Thonburi
Municipal District



Bangkok-Thonburi Ring Road Part II

Fig 3-15

Zone Division (Metropolitan Area)

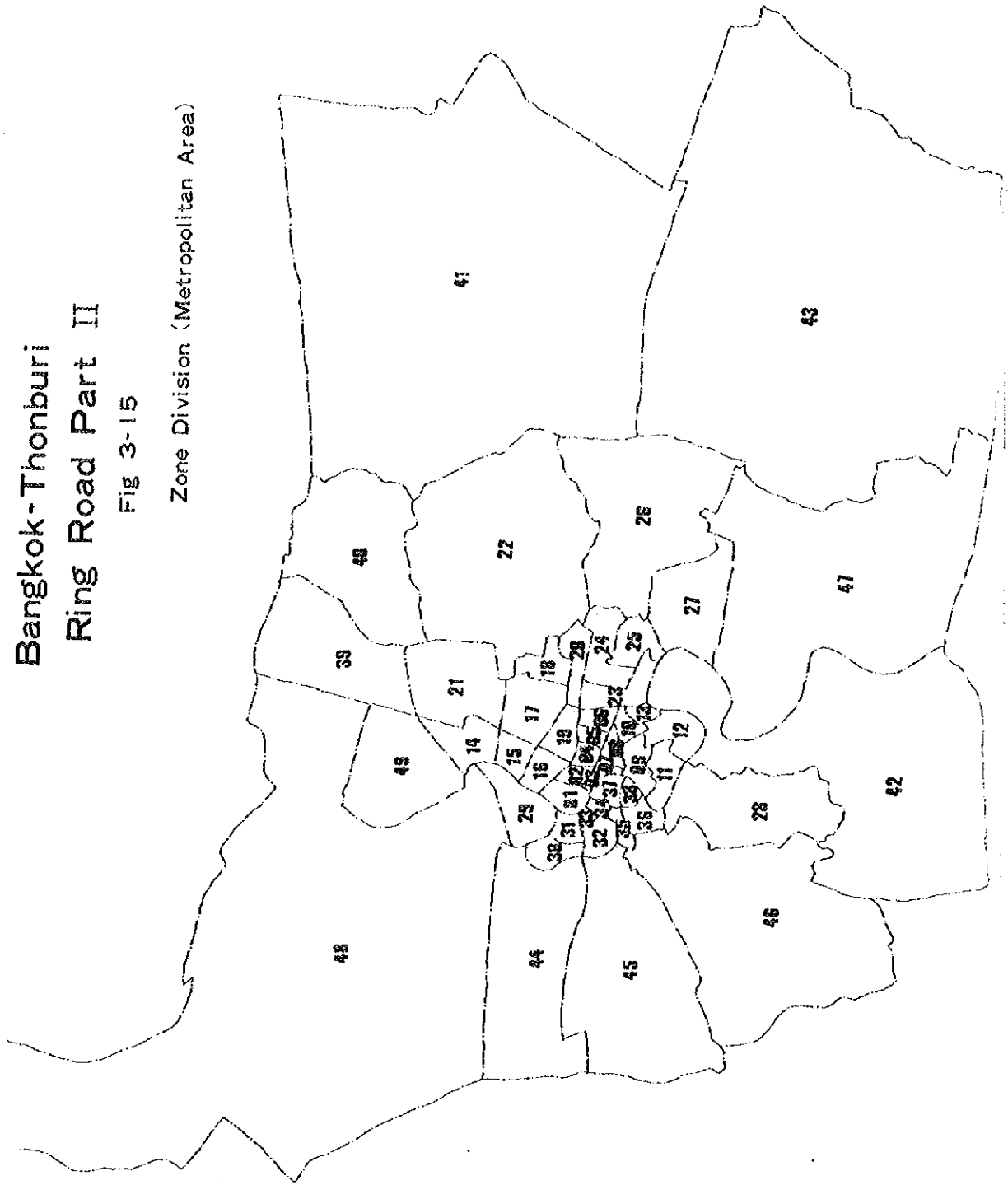
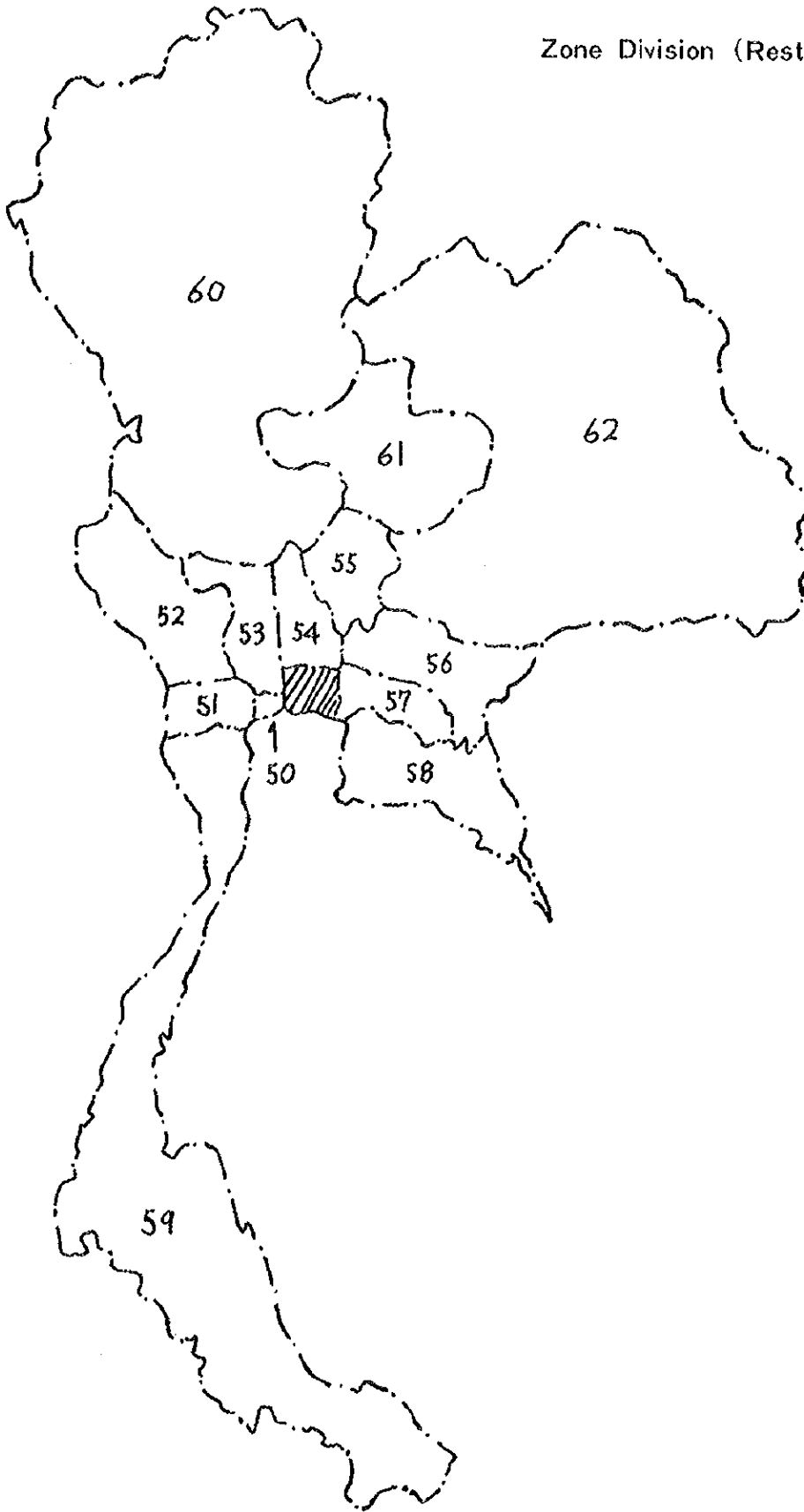


Fig 3-16

Zone Division (Rest of Kingdom)



| Zone No. | Enclosed area |
|----------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 61 | Phetchaburi, Phichit, Chaiyaphum |
| 62 | Nong Khai, Loei, Udon Thani, Kalasin, Sakon Nakhon, Khon Kaen, Maha Sarakham, Roi Et, Nakhon Phanom, Ubon Ratchathani, Nakhon Ratchasima, Buri Ram, Surin, Si Sa Ket |

(2) Estimation of the registered number of vehicles for the traffic zones

The data on the registered number of vehicles which were available to us were the figures by Changwats for the past years. To estimate the generated traffic volume of the traffic zones, an estimation of the registered number of vehicles for the traffic zones was first carried out.

Of all the statistical data for the Metropolitan area, most were only figures by Changwats, and the only figures which were broken down into Amphoe level are the population statistics for the past years. In the estimation of present registered number of vehicles, it was assumed that the rate of vehicle ownership for the whole of the twin capital Changwats was uniform and the registered number of vehicles for the Amphoes were distributed in the proportion of the population of the Amphoes. Where Amphoes are further divided into smaller zones the registered number of vehicles were further divided in proportion to the area of the zones to the whole Amphoe. However, minor adjustments were made, where the degree of development in an Amphoe varies very greatly from zone to zone.

(3) The estimation of average number of trips per vehicle

In the traffic survey carried out by the Office of Metropolitan Traffic Study the traffic volume was classified into the three following vehicle types. (1) private passenger traffic, (2) public passenger traffic and (3) goods traffic. In this study, for facilitation of future coordination, the O-D matrices were classified into the same three vehicle types. To estimate the daily average number of trips per vehicle, the results of the O-D surveys carried out in Japan for the major cities in 1965 and 1968 by the City Bureau, Ministry of Construction, Japan, were used as reference.

In Japan, the daily average number of trips per vehicle in the years 1965 and 1968 for cities over 1,000,000 in population are as shown in Tables 3-16 and 3-17.

Table 3-16 Average number of trips per vehicle in Japan, 1965
(For cities over 1,000,000 in population)

| Type of vehicle | Trip/day |
|--------------------|----------|
| Passenger vehicles | 14.8 |
| Goods vehicles | 5.6 |
| All vehicles | 8.0 |

Table 3-17 Average number of trips per vehicle in Japan, 1968
(For cities with population 1,000,000 - 3,000,000)

| Type of vehicles | | Trip/day |
|------------------|---------------|----------|
| Passenger sedan | Private owned | 3.7 |
| | Company owned | 5.3 |
| | Business | 69.3 |
| Bus | Private | 6.9 |
| | Public | 11.9 |
| Goods vehicles | | 5.4 |
| All vehicles | | 7.2 |

The analysis results of Japan cannot be adopted outright for application in Bangkok but some modifications have to be made to suit the actual local conditions. In general it can be said that due to the extensive provision of mass transportation system in the major cities in Japan (commuting trains, subway trains, tram cars or monorail) the average number of trips per vehicle tends to be lower as compared to Bangkok where road transport is the absolute major means of transportation. Unfortunately, this reasoning cannot be verified with empirical figures. In this study, we have therefore assumed an increase of about fifty percent for private passenger vehicle, buses and goods vehicle. The situation is different for public passenger cars in Japan, there being a general shortage of taxis in major cities, resulting in an abnormally high number of average trips per vehicle. (Statistical data show that public passenger vehicles made up 2.5% of the total registered number of vehicles in the cities of over

1,000,000 in population in Japan while the same for Bangkok is 7.4%). For this reason, it is reasonable to assume that the average number of trips in Bangkok should be much lower. In this study, we have assumed a figure about forty percent lower than the average in Japan. With the above assumptions, the figures adopted for Bangkok are as shown in Table 3-18. The resultant average number of trip for all vehicles is 8.5 trips per day, which figure being slightly higher than the average for the major cities in Japan which was 8.0 for the year 1965 and 7.2 for 1968.

Before the above assumed average daily number of trips per vehicle is put into practical use, a check is made by calculating the total number of vehicle-kilometres and comparing the figures with the actually surveyed cross-section vehicle composition. The average trip length per vehicle by types of vehicle for the major cities in Japan are adopted in the calculation and the resultant composition of total vehicle-kilometres obtained. It is assumed that this will represent the vehicle type composition for cross section traffic volume.

In comparison with the actual cross section traffic composition, it was found that while the compositions of the private passenger vehicles and buses are very close to the actually surveyed figures, the composition for the public passenger vehicles is lower than surveyed figures while that for goods vehicles higher than surveyed cross section composition. The share of goods traffic for the

cross section traffic volume is only 4.6% according to the MTP cross section traffic count. This is unduly low, considering the fact that goods vehicle has a share of 15% of the registered number of vehicles. It is due to the fact that many light trucks being used as mini-buses, all light truck traffic are being categorized as mini-bus traffic. Under this assumption, then a comparison has to be made of the total of public passenger traffic (which includes mini-bus traffic) and goods traffic. In this case, the calculated share is 47%, which is fairly closed to the actually surveyed share of 44.8%.

Table 3-18 Estimated average number of trips per vehicle in Bangkok

| | | Private Passenger Vehicle | Public Vehicle | | | Goods Vehicle | Total |
|-------------------------------|------------------|---------------------------|-----------------|-------|--------|---------------|---------|
| | | | Passenger Sedan | Buses | Total | | |
| Registered number of vehicles | Bangkok Thonburi | 218,637 | 21,611 | 4,871 | 26,482 | 41,587 | 286,706 |
| | Nonthaburi | 1,906 | 140 | 200 | 340 | 924 | 3,170 |
| | Samut Prakan | 1,857 | 58 | 300 | 358 | 851 | 3,066 |
| | Total | 222,400 | 21,809 | 5,371 | 27,180 | 43,362 | 292,942 |
| Trip per Vehicle Trips/day | | 5.6 | 40.0 | 20.0 | | 7.0 | |
| | | 5.6 | 35.0 | | | 7.0 | |
| | | 8.8 | | | | 7.0 | |
| | | 8.5 | | | | | |

(4) Estimation of zone generated traffic volume

The zone registered number of vehicles for the year 1970 and the average number of trips per vehicle per day were established as shown in the above-mentioned sections (1) and (2). The zone generated traffic volume was then derived through the multiplication of the figures.

The total generated traffic volume for the Metropolitan Changwats of Bangkok, Thonburi, Nonthaburi and Samut Parakan are summed up as follows:

Total generated traffic volumes by type of vehicles
(year 1970)

| | |
|---------------------------|-------------|
| Private Passenger Traffic | 1, 245, 440 |
| Public Passenger Traffic | 951, 300 |
| Goods Traffic | 303, 534 |
| Total | 2, 500, 274 |

(5) Estimation of distributed traffic volume

The traffic distribution between zones is estimated through the use of the gravity model, with the travel time between zone centroids as the resistance factor.

The general formula of the gravity model method is as follows:

$$T_{ij} = T_i \times T_j \times \frac{k}{D_{ij}^n}$$

whereby T_{ij} : Distributed traffic volume between zones i and j

T_i, T_j : Zone generated traffic volumes for zones i and j

D_{ij} : Resistance factor between zones i and j (travel time between i and j)

n, k : empirical coefficient

The travel time between zones (D_{ij}) was established through a route search by the computer for the shortest route between zones centroids in travel time. The empirical coefficient was obtained through the regressive analysis of the outbound traffic volume surveyed by the Office of the Metropolitan Traffic Study. The results of the regression calculation are as follows :

| | <u>n</u> | <u>k</u> |
|---------------------------|----------|--------------------------|
| Private passenger traffic | 1.64367 | 0.24305×10^{-4} |
| Public passenger traffic | 1.26255 | 0.14179×10^{-6} |
| Goods traffic | 1.73565 | 0.44722×10^{-4} |

The resultant O-D matrices for the year 1970 as prepared through the above mentioned method are as in Table 3-19 - 3-21.

(6) Verification of the theoretical O-D matrices for 1970

A check of whether the theoretical O-D matrices do represent the actual traffic situation is made through comparison of cross section traffic volumes at suitable cordon lines. The cordon line that is most easily comparable is that drawn across the Chao Phraya River. Table 3-22 shows the cross river traffic volume carried out in several previous surveys against the theoretical cross river traffic volume for 1970 as estimated in this study. The table clearly shows that while there were some minor fluctuations between surveys due probably to the difference in months of the surveys carried out, the cross river movement remained at an almost constant volume of between 175,000 and 195,000/24 hours for the period 1969 to 1972. While the other road sections showed an annual increase of 10 to 15% in traffic

volume, the cross river traffic volume did not show any marked increase from year to year. This is evidently due to the fact that the bridges across the river, especially the Memorial Bridge, had traffic volumes nearing the capacity of the bridges, and increase in traffic volumes were physically almost impossible. The estimated cross river traffic volume fell squarely within the range of past survey results, showing that as far as cross river movements are concerned, the theoretical O-D matrices made a very good representation.

Several other cordon lines were also drawn at several cross sections within the municipality area.

While, as different from the cross river traffic, direct comparisons are difficult, due to difference in year, as well as existence of local or intra-zonal movement in actual survey results. However, endeavours had been made to convert the figures into the same level.

The results of all the comparisons shows that the estimated cross section traffic volumes fairly represented the actual traffic situation for the various cordon lines, and the computed O-D matrices for 1970 may be adopted with quite high degree of reliability.

Table 3-22 Comparison of traffic movements across the Chao Phraya River

(a) Surveyed traffic volume in March 1969 (Sathorn Bridge Feasibility Study Report)

| | | |
|--------------------------------------|----------------------------------------------|---|
| i) Memorial bridge (March 12) | 74,618 vehicle/12 hours (700 to 1900 hr.) | |
| ii) Krung Thon bridge (March 11) | 27,196 | " |
| iii) Krung Thep bridge (March 18) | 17,657 | " |

iv) Rama VI bridge 6,501 vehicle 12 hours
 (March 19) (700 to 1900 hr.)

Total for 12 hours 125,972 vehicles

Conversion into Total 192,649 vehicles (x1.5293*)
 for 24 hours

*the conversion rate vehicles per 24 hr/vehicles per
 12 hr = 1.5293 was derived from the 24 hour survey
 carried out for the Memorial Bridge

Report for the three bridges of Krung Thon, Memorial
 and Krung Thep.

(b) Surveyed traffic volume in May/June 1971
 (Memorial bridge report)

| | | |
|------------------------|--------------------------|----------|
| i) Memorial bridge | 81,885 vehicles/24 hours | |
| (28 May-2 June) | | |
| ii) Krung Thon bridge | 52,322 | " |
| (26 May) | | |
| iii) Krung Thep bridge | 28,534 | " |
| (29 May) | | |
| sub-total | 162,741 | " |
| iv) Rama VI bridge | 11,450 | " (6.6%) |
| (estimated)* | | |
| Total for 24 hours | <u>174,242</u> | " |

(c) Surveyed traffic volume in February, 1972 (Office of
 Metropolitan Traffic Planning)

| | |
|------------------------|---------------------|
| i) Memorial bridge | 72,204 veh/16 hours |
| (8 February) | (0600-2200 hr.) |
| ii) Krung Thon bridge | 41,645 " |
| (7 February) | |
| iii) Krung Thep bridge | 23,890 " |
| (8 February) | |

| | |
|---------------------------------------|----------------------------------------|
| iv) Rama VI bridge (7 February) | 12,324 veh/16 hours (0600-2200 hr) |
| Total for 16 hours | 150,063 " |
| Conversion into total for 24 hours | <u>179,941</u> vehicles (x 1,1991)* |

* the conversion rate of 1,1991 was derived from the 24 hour survey for the Memorial Bridge Report.

(d) Estimated traffic volume for the year 1970 (This study)

| | |
|-------------------------------------------|----------------------------|
| i) Memorial bridge | 80,875 vehicles per 24 hr. |
| ii) Krung Thon bridge & Rama VI bridge | 61,154 " |
| iii) Krung Thep bridge | 34,448 " |
| Total for 24 hours | <u>176,377</u> vehicles |

(7) The limitations of the theoretical O-D matrices

The purpose of a theoretical present O-D matrices is to produce, through theoretical calculations, a traffic pattern which will represent as close as possible, the actual traffic pattern which existed in the year under study.

In this study, checks with result of other traffic surveys have been made to show that the O-D matrices do show good representation of certain traffic phenomenon at specific points, and the O-D matrices were adopted as practical. However, theoretical calculations are made with assumptions, and the products of such calculations naturally have their short comings.

In the calculations made in this study, due to lack of data, assumption have to be brief, and the zone generated traffic volume was assumed to be proportional to the population. Such a method usually results in the uniformity of generated

traffic pattern for all zones. An analysis of the variation from zone of land use, zone average income, zone rate of vehicle ownership, or other peculiarities in economic activities would have produced a more favorable outcome, if it had been possible.

In the calculation of traffic distribution, again the gravity model is applied uniformly for each single type of vehicle. This again could have been improved if different models were made applicable for different trip purpose (business, shopping, commuting etc.) provided sufficient data are available.

Although the screen section traffic volume of the theoretical O-D matrices (which is an accumulation of individual zone pair traffic volume) had been checked and found to be close to the actually surveyed volume, it is not impossible that the pattern of distribution may be quite different from the actual pattern. This has to be always borne in mind and the O-D matrices should be treated more as a reference in lieu of surveyed O-D matrices and should not be treated as infallible substitutes. However, we can also safely assume that allowing for some deviation in the pattern of distribution, the assigned traffic volume can represent a fairly good picture of the traffic situation on the road network in Bangkok.

As soon as the survey data of the Office of Metropolitan Traffic Planning are available, a check of the theoretical results against the surveyed results is necessary, and necessary rectification made if deviation results in conspicuous distortion of the actual traffic pattern.

3-4 Estimation of Future O-D Matrices

3-4-1 Future zone generated traffic volume

The future zone generated traffic volume of Bangkok was also estimated through forecasting the future registered number of vehicles in the relevant zones.

(1) Forecast of future registered number of vehicles in the Metropolitan area

In the forecast of the future number of vehicles in the Metropolitan area, an attempt was first made with analysis of the correlation between rate of ownership and average per capita income. However, it was found that the results were unfavourable, and the future number of vehicles were eventually estimated through trend projection.

(a) The trend of vehicle registration in the Metropolitan area

The number of vehicles registered in the 4 Changwats of the Metropolitan area are as in Table 3-23 (i) - (iv).

Table 3-23 Registered number of vehicles (1965 - 1970)

(i) Bangkok and Thonburi

| | 1965 | 1966 | 1967 | 1968 | 1969 | 1970 |
|---------------------------|--------|---------|---------|---------|---------|---------|
| Private passenger | 45,338 | 78,584 | 101,579 | 116,969 | 164,968 | 218,637 |
| Public passenger | 17,542 | 18,205 | 18,820 | 18,896 | 24,411 | 26,482 |
| Goods | 19,081 | 28,142 | 30,991 | 34,961 | 36,521 | 41,587 |
| Total | 81,961 | 124,931 | 151,390 | 170,826 | 225,900 | 286,706 |
| Vehicle/ 1,000 persons | 28.3 | 41.6 | 48.3 | 52.6 | 66.8 | 81.5 |

(ii) Nonthaburi

| | 1965 | 1966 | 1967 | 1968 | 1969 | 1970 |
|------------------------|-------|-------|-------|-------|-------|-------|
| Private passenger | 714 | 809 | 1,036 | 1,218 | 1,511 | 1,906 |
| Public passenger | 206 | 213 | 238 | 283 | 321 | 340 |
| Goods | 446 | 577 | 635 | 628 | 892 | 924 |
| Total | 1,366 | 1,599 | 1,909 | 2,129 | 2,724 | 3,170 |
| Vehicle /1,000 persons | — | — | — | — | — | 12.5 |
| (iii) Samut Prakan | | | | | | |
| | 1965 | 1966 | 1967 | 1968 | 1969 | 1970 |
| Private passenger | 568 | 569 | 969 | 940 | 703 | 1,857 |
| Public passenger | 210 | 204 | 137 | 194 | 426 | 358 |
| Goods | 319 | 319 | 357 | 433 | 1,407 | 851 |
| Total | 1,097 | 1,092 | 1,463 | 1,567 | 2,536 | 3,066 |
| Vehicle /1,000 persons | — | — | — | — | — | 9.4 |

(iv) Total of four Changwats

| | 1965 | 1966 | 1967 | 1968 | 1969 | 1970 |
|------------------------|--------|---------|---------|---------|---------|---------|
| Private passenger | 46,620 | 79,962 | 103,584 | 119,127 | 167,182 | 222,400 |
| Public passenger | 17,958 | 18,622 | 19,195 | 19,373 | 25,158 | 27,180 |
| Goods | 19,846 | 29,083 | 31,983 | 36,022 | 38,820 | 43,362 |
| Total | 84,424 | 127,622 | 154,762 | 174,522 | 231,160 | 292,442 |
| Vehicle /1,000 persons | — | — | — | — | — | 71.5 |

(b) Per capita national income

The past statistics of per capita national income for the Kingdom and the Central Region are as in Table 3-24. The per capita national income for the Central Region was 5,550 bahts/year, which is about 1.8 times the national average of 3,160 bahts/year.

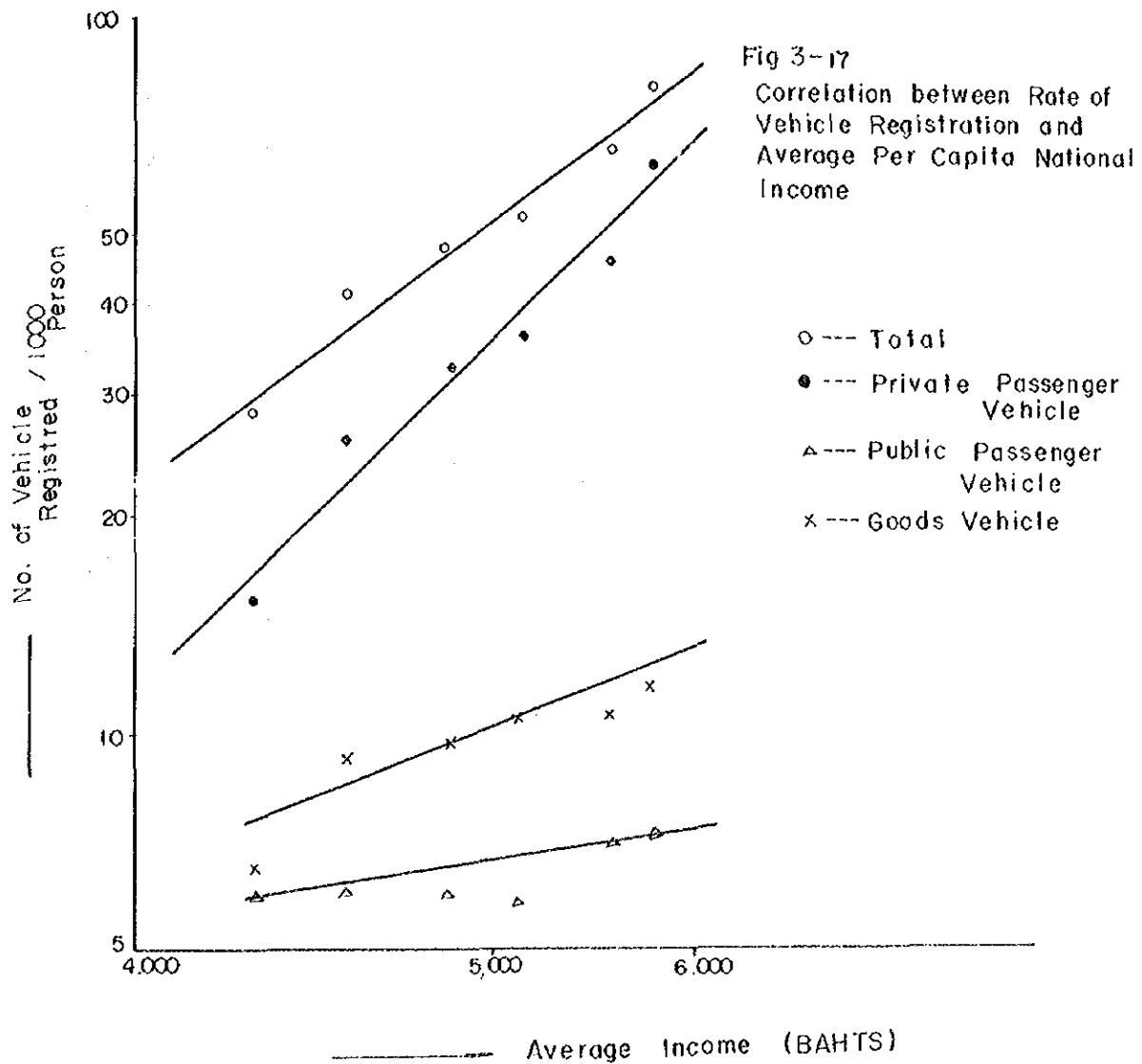
The increase by 1.29 times for Central Region from 1965 to 1970 was also slightly higher than the national figure of 1.27.

Table 3-24 Trend of increase in per capita national income (Baht/year)

| | Central area | Whole Kingdom |
|-----------|--------------|---------------|
| 1965 | 4,290 | 2,480 |
| 1966 | 4,560 | 2,690 |
| 1967 | 4,850 | 2,760 |
| 1968 | 5,100 | 2,900 |
| 1969 | 5,400 | 3,080 |
| 1970 | 5,550 | 3,160 |
| 1970/1965 | 1.29 | 1.27 |

(c) Correlation between rate of vehicle registration and average per capita national income

The correlation between the rate of vehicle registration and average per capita national income for the metropolitan region may be graphically shown as below.
(Fig. 3-17)



The points in the graph show rather good correlation, and the following equations are obtained through regression.

Private passenger vehicle : $Y = 1.5^{-16} \cdot X^{4.79}$ (R = 0.982)

Public passenger vehicle : $Y = 1.5^{-2} \cdot X^{0.81}$ (R = 0.735)

Goods vehicle : $Y = 1.05^{-6} \cdot X^{1.9}$ (R = 0.908)

All vehicles : $Y = 6.8^{-11} \cdot X^{3.66}$ (R = 0.982)

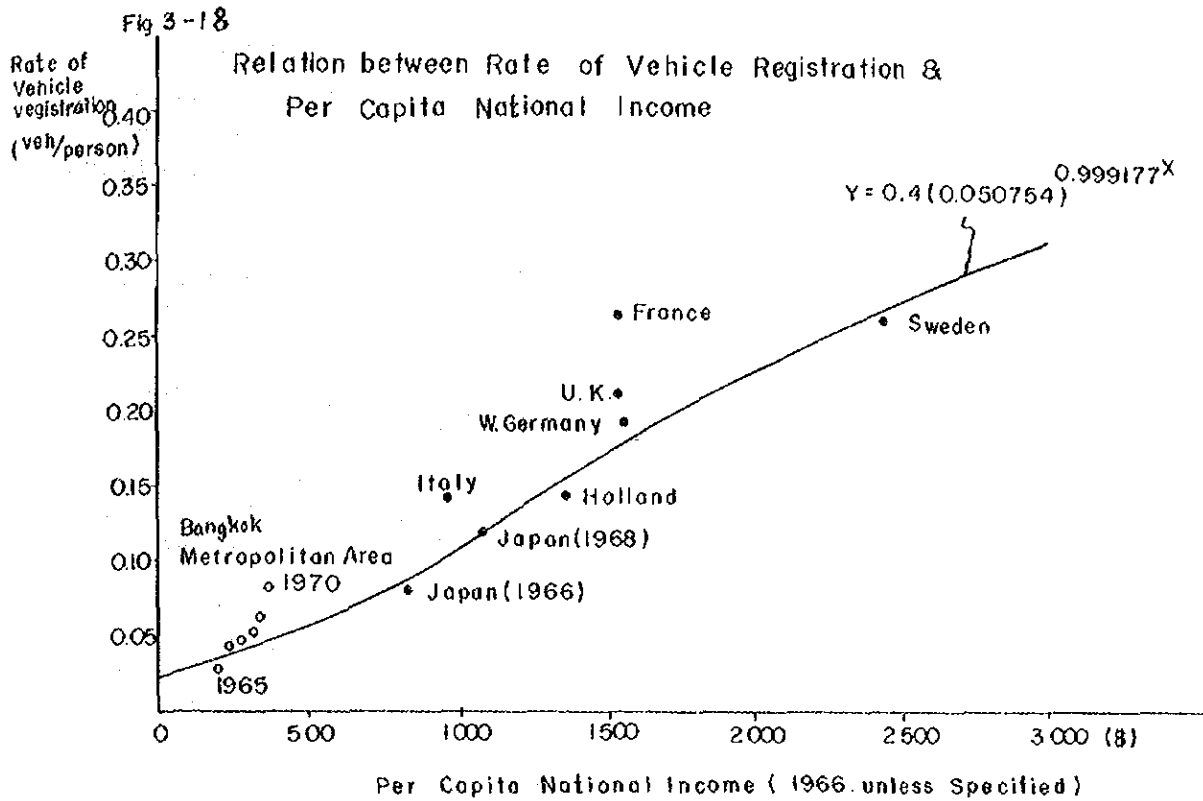
The average per capita national income in the year 1990 for the Metropolitan area was estimated at 13, 100 baht/year.

When this figure is adopted in the equation obtained, an unrealistically high rate of vehicle registration of 1740 veh/1000 population would be derived, showing that the correlation could not be adopted for the forecast of future registered number of vehicles.

It is found that the past rate of ownership of vehicles for the Metropolitan area had been exceptionally high despite a rather low per capita national income.

An analysis of the rate of ownership of various west European nations and Japan against that of Thailand (1965 - 1970) was made and the results are as in Fig. 3-18. It is evident that while there are some ups and downs, a general trend was followed in relation to the per capita national income.

However, the figures for the Metropolitan area were far above the general trend, and a projection of the past trend would be unrealistic. The adoption of the correlation was thus abandoned.



(d) Forecast through trend projection

A trend projection of the registered number of vehicles was made for the 4 Metropolitan Changwats and the results are as in Table 3-25.

Table 3-25 Estimated future registered number of vehicles by trend projection

(i) Bangkok & Thonburi

| | Correlation equation | Estimated number of vehicles, 1990 |
|---------------------------|---------------------------------|------------------------------------|
| Private passenger vehicle | $Y=32600X - 64020420$ (R=0.980) | 859,350 |
| Public passenger vehicle | $Y=1810X - 3541010$ (R=0.904) | 57,670 |
| Goods vehicle | $Y=4045X - 7926750$ (R=0.971) | 126,865 |
| Total | | 1,043,885 |

X : year

(ii) Nonthaburi

| | Correlation equation | Estimated number of vehicles, 1990 |
|---------------------------|------------------------------|------------------------------------|
| Private passenger vehicle | $Y=236X - 462460$ (R=0.980) | 7,230 |
| Public passenger vehicle | $Y=29.7X - 58140$ (R=0.983) | 912 |
| Goods vehicle | $Y=95.1X - 186400$ (R=0.952) | 2,943 |
| Total | | 11,085 |

(iii) Samut Prakan

| | Correlation equation | Estimated number of vehicles, 1990 |
|---------------------------|-------------------------------|------------------------------------|
| Private passenger vehicle | $Y=195X - 382330$ (R=0.752) | 5,760 |
| Public passenger vehicle | $Y=41.8X - 81980$ (R=0.702) | 1,138 |
| Goods vehicle | $Y=171.4X - 336670$ (R=0.733) | 4,562 |
| Total | | 11,460 |

(iv) Total of 4 Changwats

| | Correlation equation | Estimated number of vehicles, 1990 |
|---------------------------|---------------------------------|------------------------------------|
| Private passenger vehicle | $Y=33030X - 64863750$ (R=0.979) | 872,340 |
| Public passenger vehicle | $Y=1880X - 3681590$ (R=0.904) | 59,720 |
| Goods vehicle | $Y=4310X - 8443930$ (R=0.978) | 134,370 |
| Total | $Y=39230X - 77001270$ (R=0.983) | 1,066,430 |

The estimated registered number of vehicles for the 4 Changwats in the Metropolitan area in 1990 is about 1,066 thousand vehicles. On the other hand, the "Greater Bangkok Plan" has an estimated population of 6.5 million for the Greater Bangkok area.

Together with the regions excluded from the greater Bangkok area, the total population for the 4 Changwats in 1990 is estimated at 7.2 million. The average rate of vehicle registration is 150 vehicle per 1000 population, which is about 2 times the present rate of ownership. The figure is quite high as compared to that of other western European nations when the average per capita national income is taken into consideration. (Table 3-26). However, considering that this is a comparison between figures for a region (the Metropolitan area) against the national average rate of vehicle registration for all other nations, and that the present rate is already above that of the general trend of other nations, this cannot be considered an overestimate. In want of other better methods of analysis available against existing data, these rather sophisticated figures are adopted as the estimated future registered number of vehicles for the Metropolitan area.

Table 3-26 Comparison of vehicle registration situation

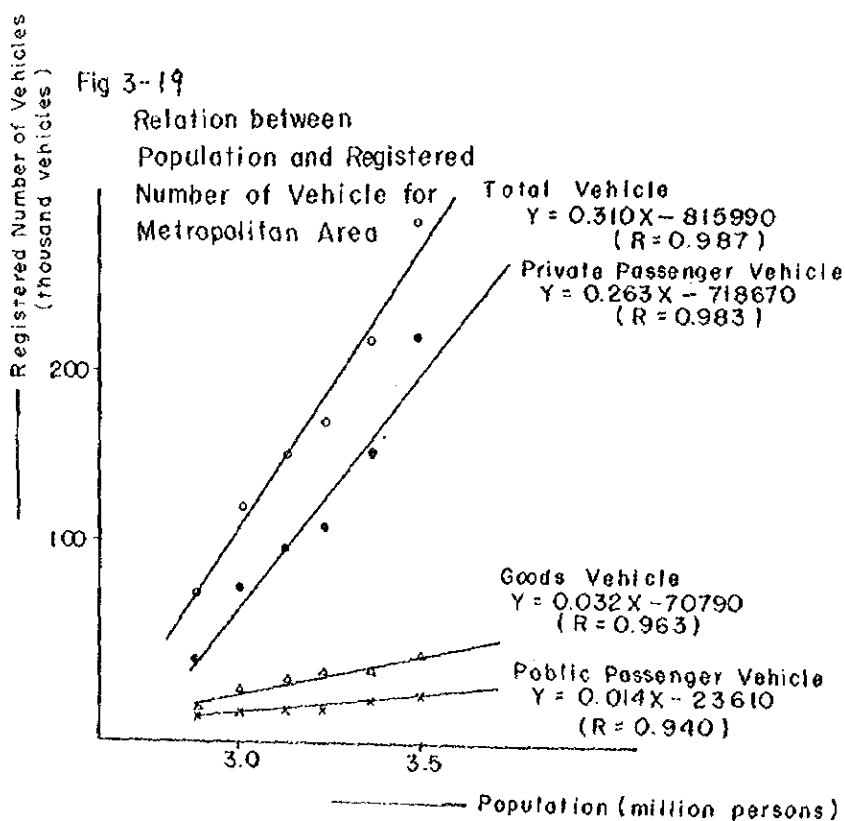
| | Rate of Vehicle Ownership (vehicle /person) | Per capita national income (\$) | Registered No. of Vehicles (1,000 veh.) | Population (1,000 persons) |
|----------------------------------|---------------------------------------------|---------------------------------|-----------------------------------------|----------------------------|
| France | 0.262 | 1,542 | 12,965 | 49,400 |
| W. Germany | 0.189 | 1,574 | 10,881 | 57,485 |
| Italy | 0.136 | 944 | 7,047 | 51,859 |
| Netherland | 0.141 | 1,362 | 1,756 | 12,455 |
| Sweden | 0.259 | 2,392 | 2,024 | 7,808 |
| Switzerland | 0.182 | 2,077 | 1,104 | 6,050 |
| U. K. | 0.207 | 1,515 | 11,350 | 54,953 |
| Canada | 0.348 | 1,979 | 6,926 | 19,919 |
| U. S. A. | 0.471 | 3,154 | 92,843 | 196,920 |
| Japan, 1966 | 0.081 | 791 | 8,015 | 98,865 |
| Japan, 1968 | 0.115 | 1,077 | 11,690 | 101,408 |
| Metropolitan area Thailand, 1990 | 0.150 | 655 | 1,066 | 7,200 |

all figures for 1966 unless stated otherwise

(2) Estimation of future zone registered number of vehicles

The total future registered number of vehicles for the whole Metropolitan area had been estimated. For the calculation of zone generated traffic volume, it is necessary to break this total into zone future registered number of vehicles. The relation between population and registered number of vehicles for the Metropolitan area is graphically as in Fig. 3-19.

It can be seen that for a sufficiently large district, the two show very good correlation. When the district is further divided into smaller zones, the same relation may not necessary apply. However, in this study, there were insufficient data for an analysis of the interzonal difference in characteristics, and a bold assumption was made that the number of vehicles was distributed proportional to the zone population. The 1,066 thousand vehicles was thus allocated to the zones by types of vehicles in proportion to the future estimated zone population, which totals 720 million for the 4 Metropolitan Changwats and 650 million for the Greater Bangkok area.



(3) Future average number of trips per vehicle

The average number of trips per vehicle vary according to the level of economic activities and social activities of the society. It is difficult to estimate the trend of change of the average number of trip in relation to an increase in the rate of vehicle ownership.

An increase in private passenger vehicles will result in the increase of idle vehicles and consequently a decrease in average number of trips on one hand, but also in the increase of short-distance trip on the other. Increase in economic activities tends to increase the trip length of goods vehicle trip, thus reducing the average number of trips, but this may be offset by an improvement in the efficiency in the use of the vehicles through rationalization and better planning.

Past data for Japan shows that the general trend is towards a gradual decrease in the average number of trips per vehicle, although the decrease is by no means significant and may be neglected over a short period. In this study, in want of a substantial argument for or against a decrease in the average number of trips per vehicle in Bangkok, the same figures for 1970 was adopted for 1990.

(4) Establishment of future zone generated traffic volume

The zone generated traffic volume by types of vehicles for the year 1990 was obtained by a multiplication of the zone registered number of vehicles by the average number of trips per vehicle for each type of vehicle.

The zone generated traffic volume for the years 1975 and 1980 were subsequently obtained through interpolation.

3-4-2 Establishment of future O-D matrices

The future traffic distribution was also computed with the gravity model method, applying the same values for the parameters n and k as that for the 1970 traffic analysis. The future interzonal travel time (D_{ij}) for the different years were separately derived with consideration for the future improvement and new construction to the road network in Bangkok. As the future generated traffic volumes for the external zones were not established, the future internal-external and external-external zonal traffic was derived through proportional increase of the existing interzonal traffic volume with the growth factor of registered number of vehicles for the relevant zones. The future O-D matrices thus obtained are shown in Tables 3-27 - 3-35.

K. PUBLIC PASSENGER TRAFFIC-1990

Table with 62 columns (01-62) and 62 rows (01-62). Each cell contains numerical data representing public passenger traffic. The table is a lower triangular matrix where the diagonal elements are the highest values, and values decrease as they move away from the diagonal. The total for the first row (row 01) is 712626.

3-5 Estimation of Traffic Volume on the Ring Road

The future O-D matrices having been established in the previous section, the assignment of the inter-zonal traffic volume to the road network will provide the future traffic on the Ring Road as well as the other road sections in the network.

3-5-1 Methodology

In this study, the quantity-volume (Q-V) traffic simulation method is adopted for the traffic assignment. The bench mark travel speed of each road section varies according to the geometric structure and the environmental conditions of the road section. The bench mark speed can only be maintained when the traffic volume is low. With the increase in traffic volume, the travel speed begins to fall until it reaches the maximum possible volume for the road section, beyond which a decrease of traffic capacity will occur. The relation between travel speed and traffic volume is generally a curve. This principal is adopted in Q-V traffic simulation. Network as well as the road traffic capacity at the bench mark speed is set. However, for the facility in actual calculation, the Q-V curve is simplified into a linear form as shown in Fig. 3-20. In this figure, the speed ' V_1 ' is the bench mark speed of the road section, and the capacity ' Q_1 ' is the maximum capacity of the road section when the bench mark speed may be maintained. Capacity ' Q_2 ' is the maximum possible capacity of the road section and speed ' V_2 ' the speed estimated at the maximum possible capacity. The distributed traffic volume of each zone-pair is divided into many partial fractions and assignment made to the road network, each fraction at a time. At the initial assignment, the bench mark speed is used. After the first assignment, the travel speed of each road section is calculated basing on the traffic volume of the first

assignment. This process is reiterated until all distributed volumes are assignment to the network. In the selection of the optimum route, the travel time of all road sections are calculated and the route with the shortest travel time selected as the best route.

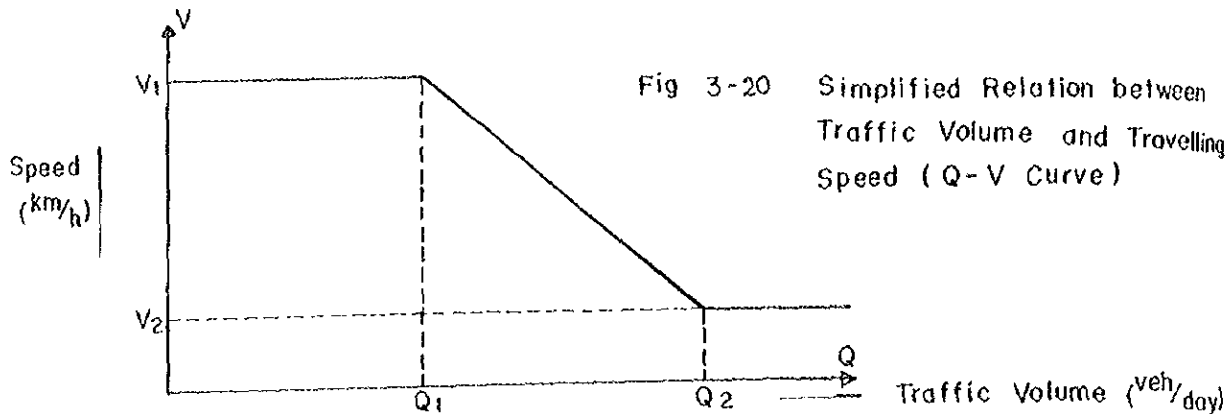


Fig 3-20 Simplified Relation between Traffic Volume and Travelling Speed (Q-V Curve)

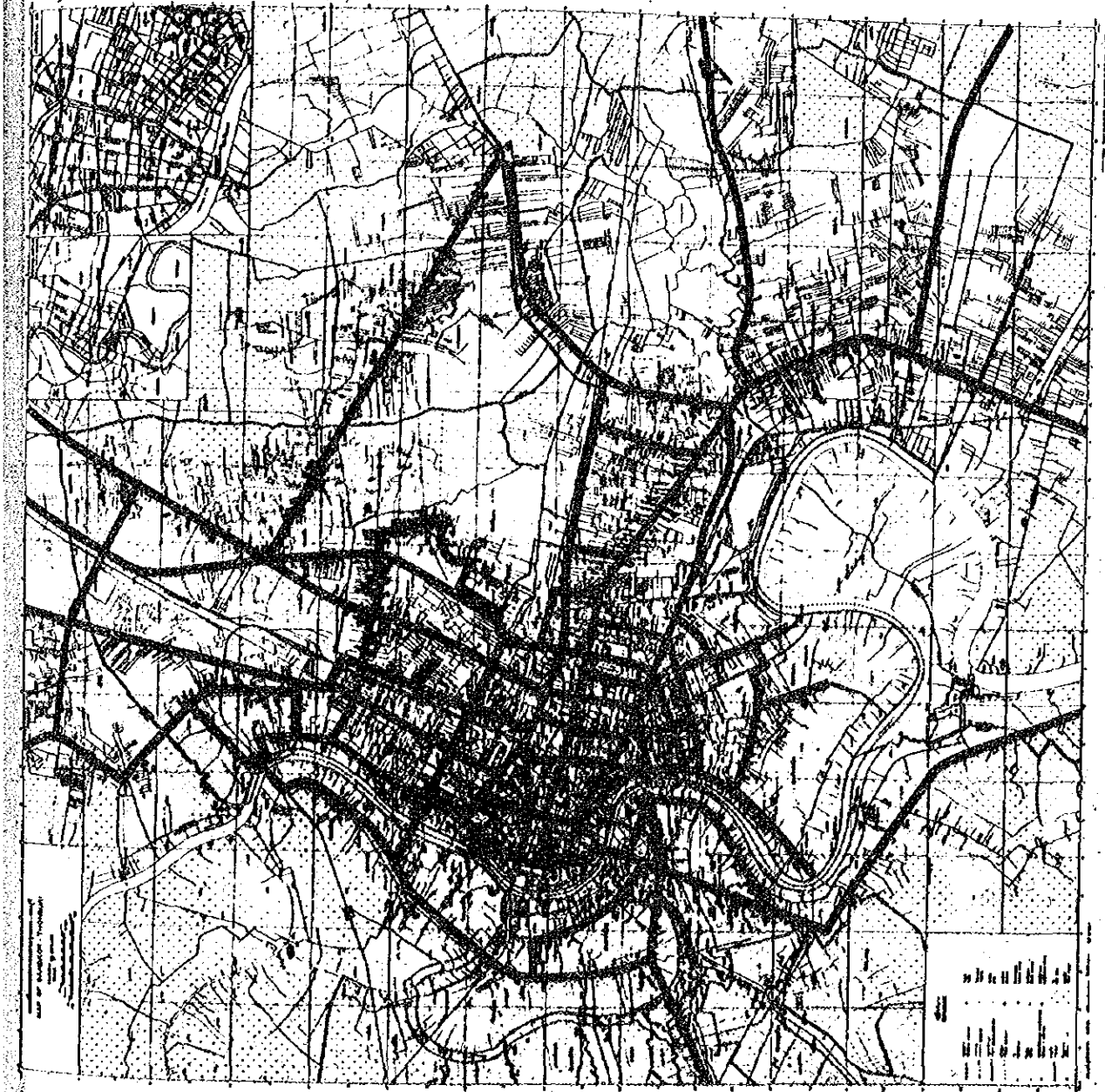
3-5-2 Establishment of the relevant road network

Traffic assignment will be made for all the years at which O-D matrices have been established. Needless to said, the running conditions will be different for the various years as it can be expected that constant improvement will be made to the existing road network to increase the running speed and traffic capacity of the road sections. Also, as years pass by, construction of new roads can also be expected.

It will not be practical to put all road sections of the existing road network into the relevant road network used in traffic assignment, as many of these are local roads serving mainly short-distance local traffic. In the decision of the relevant network, therefore, only the roads section which cater for interzonal traffic are selected. Along this line, the relevant road network for 1970 was decided as in Fig. 3-21.

Bangkok-Thonburi Ring Road Part II

Fig 3-21
Relevant Road Network, 1970



For the year 1975, the several addition which are made are limited to the projects which are at present under construction (such as the Tha Chang Bridge, the Asok-Din Daeng Municipal Road) or in the design stage (the Ring Road Part I, the Riverside Municipal Road etc.).

The decision on the relevant road network for 1980 and 1990 is more difficult. Although in the "Greater Bangkok Plain, (which is not officially authorized), a future highway network plan is included, the programme of completion of the new roads is neither shown in the report nor has it been drawn up by any relevant departments. However, being the only existing plan available, the relevant road network has to be decided basing on the "Greater Bangkok Plan".

A brief study is made, and an assumption made of the roads which will be completed if they are implemented without delay and with the maximum efficiency by the year 1990. This forms the 'optimum road network' relevant to the traffic assignment in this study, and the network contains a majority of the roads planned in the 'Greater Bangkok Plan'. For the year 1980, only minor additions are made to the 1975 relevant road network.

Even though the road network for 1990 thus screened is considerable reduced in number of new roads, it is still difficult to envisage that all the new roads included will be duly completed, since their completion by 1990 will probably call for a sharp increase in the annual budget allotted for new road construction, not to mention that the man power and mechanical power capable of undertaking the construction work will probably be greatly strained. Also, many of the new roads in the network are further away from the town centre than the Ring Road under study. In an analysis for the Ring Road, it will be unnatural to assume that the roads that are planned further away than the Ring Road

should be completed earlier than the Ring Road. A second network for 1980 and 1990 is therefore set whereby it is assumed that the new roads outside of the Ring Road will not be completed by 1990. (The normal network).

Traffic assignment in this study, was made for both the two cases of road networks for 1990. However, for subsequent economic analyses, only the latter case, where new roads outside of the Ring Road were omitted, was taken into consideration.

3-5-3 Setting of Q-V conditions for the relevant road networks

All the junctions (nodes) and road sections between junctions (links) are coded for the purpose of computer calculation. There are in total 184 nodes and 301 links. (Fig. 3-22 to 24)

The links are classified into 15 road classification and the Q-V condition for each class of road set. (Table 3-36). The distance of each link is also provided. The full list of all nodes and links are as in Table 3-37.

In the assignment, the traffic volume was assigned by 10 stages, and the percentage of traffic volume assigned at each stage was at follows:

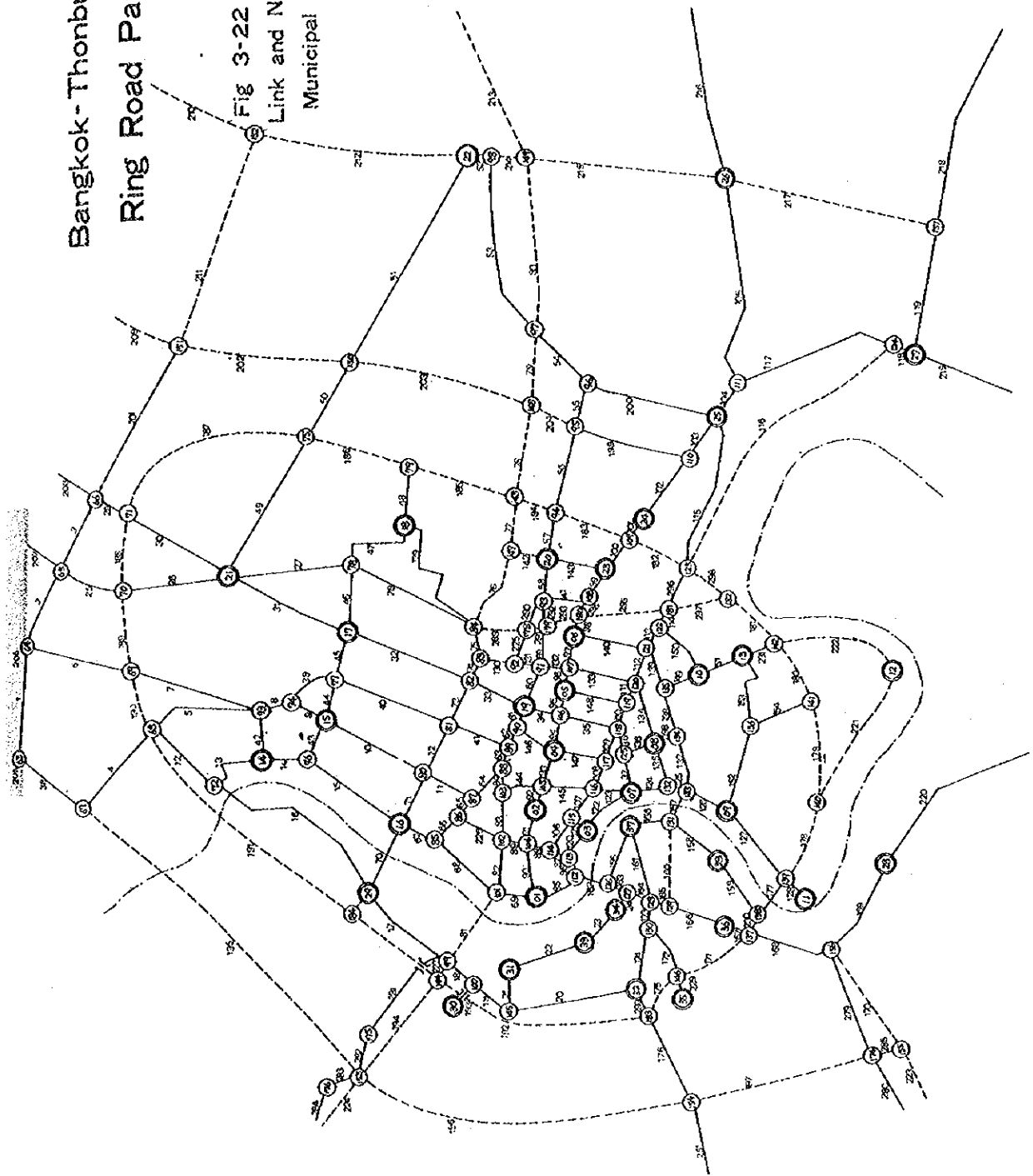
| Stage | 1st | 2nd | 3rd | 4th | 5th | 6th | 7th | 8th | 9th | 10th |
|---------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|------|
| Percent of traffic volume | 20 | 15 | 15 | 10 | 10 | 10 | 5 | 5 | 5 | 5 |

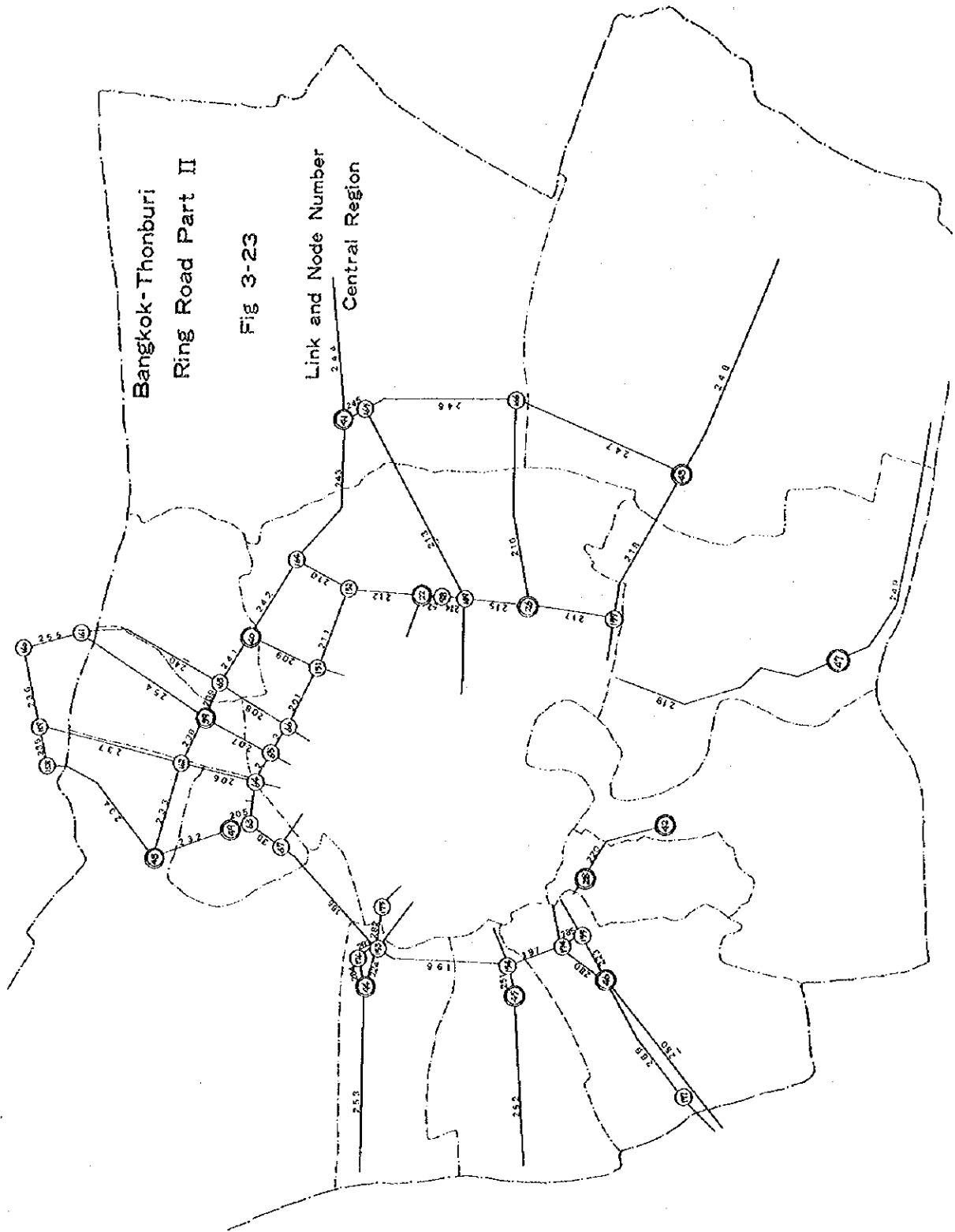
Table 3-36 Q-V Condition by road classification

| Road Classification | | (V ₁) km/h | Q ₁ Capacity Veh/day | (V ₂) km/h | Q ₂ Capacity Veh/day |
|---------------------|--------------------------------------|---------------------------|---------------------------------------|---------------------------|---------------------------------------|
| 1 | Type No. 1 2 Lanes | 45 | 5,500 | 20 | 15,000 |
| 2 | Type No. 2 4 Lanes | 50 | 26,000 | 10 | 110,000 |
| 3 | Type No. 3 4 Lanes | 60 | 27,000 | 15 | 65,000 |
| 4 | Type No. 4 6 Lanes | 55 | 36,000 | 20 | 80,000 |
| 5 | Type No. 5 6 Lanes | 55 | 35,500 | 25 | 71,000 |
| 6 | Type No. 6 6 Lanes | 60 | 37,000 | 20 | 83,000 |
| 7 | Type No. 7 6 Lanes | 65 | 38,000 | 20 | 85,000 |
| 8 | Type No. 8 8 Lanes | 60 | 48,000 | 30 | 96,000 |
| 9 | Type No. 9 8 Lanes | 60 | 52,000 | 20 | 120,000 |
| 10 | Type No. 10 10 Lanes | 50 | 65,000 | 25 | 130,000 |
| 11 | Ring road 6 Lanes | 100 | 49,000 | 30 | 98,000 |
| 12 | Urban planning road 4 Lanes | 70 | 27,000 | 30 | 54,000 |
| 13 | Express way 4 Lanes | 100 | 32,000 | 30 | 64,000 |
| 14 | Express way 2 Lanes | 100 | 6,500 | 30 | 13,000 |
| 15 | Type No. 11 4 Lanes | 40 | 22,500 | 5 | 45,000 |

Bangkok-Thonburi Ring Road Part II

Fig 3-22
Link and Node Number-
Municipal Region





Bangkok-Thonburi Ring Road Part II

Fig 3-24

Link and Node Number-
Rest of Kingdom

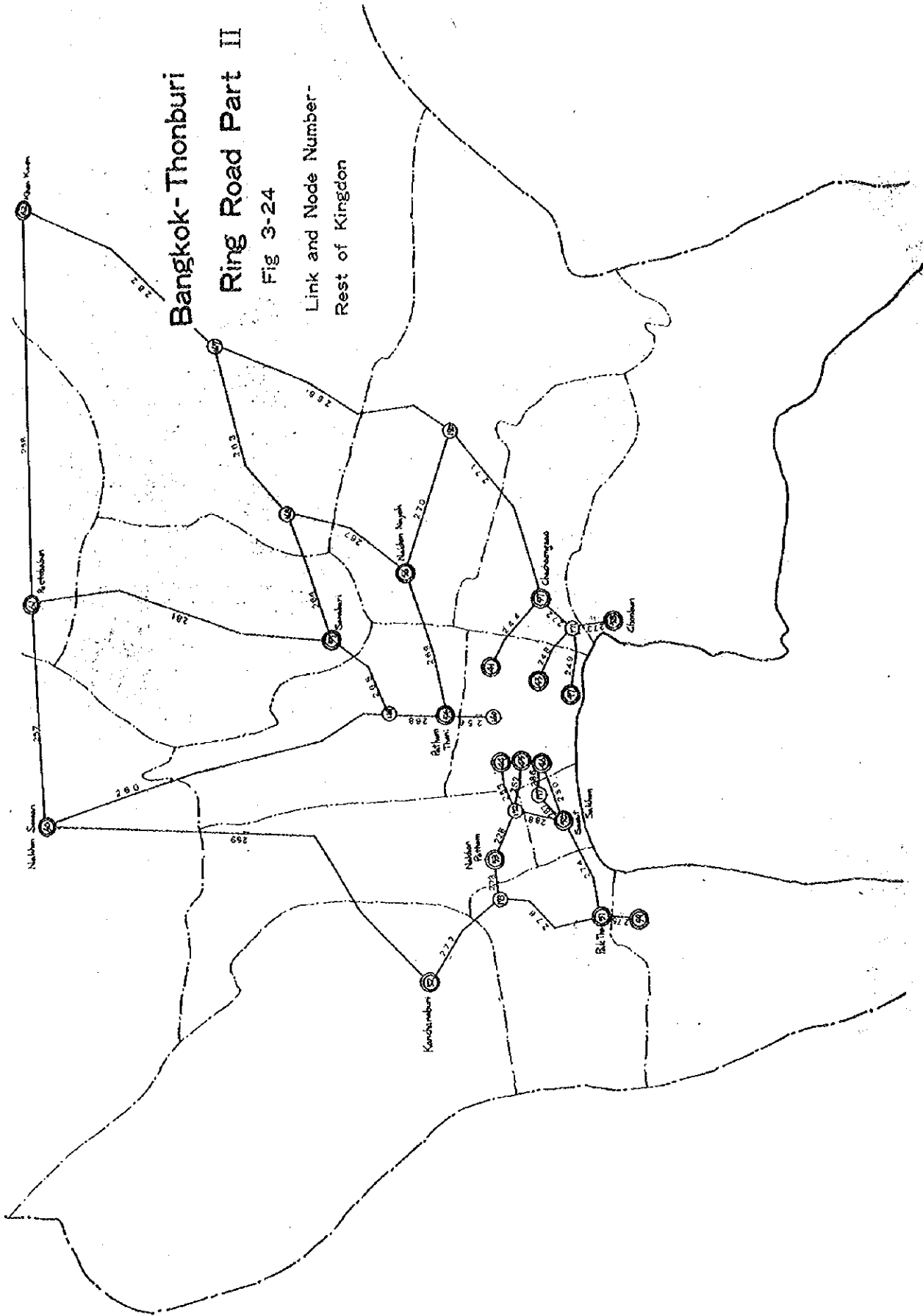


Fig. 3-37 List of nodes and links

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 1 | 63-64 | 2.5 | 1 | 23 | 33-34 | 1.7 | 2 |
| 2 | 64-65 | 1.9 | 1 | 24 | 34-127 | 0.5 | 2 |
| 3 | 65-66 | 1.8 | 1 | 25 | 65-70 | 1.6 | 13 |
| 4 | 67-68 | 2.4 | 1 | 26 | 70-21 | 2.3 | 13 |
| 5 | 68-73 | 2.6 | 1 | 27 | 21-78 | 2.5 | 13 |
| 6 | 64-69 | 2.7 | 3 | 28 | 78-84 | 3.2 | 13 |
| 7 | 69-73 | 3.0 | 3 | 29 | 66-71 | 0.9 | 3 |
| 8 | 73-74 | 0.7 | 2 | 30 | 71-21 | 2.6 | 3 |
| 9 | 74-15 | 0.8 | 3 | 31 | 21-17 | 2.6 | 7 |
| 10 | 15-80 | 2.4 | 3 | 32 | 17-82 | 3.0 | 7 |
| 11 | 80-87 | 1.3 | 3 | 33 | 82-19 | 1.5 | 8 |
| 12 | 68-72 | 1.8 | 1 | 34 | 19-106 | 0.8 | 4 |
| 13 | 72-14 | 1.9 | 1 | 35 | 106-118 | 1.5 | 8 |
| 14 | 14-76 | 1.0 | 1 | 36 | 118-125 | 0.7 | 3 |
| 15 | 76-16 | 2.6 | 2 | 37 | 125-07 | 0.8 | 3 |
| 16 | 72-29 | 5.1 | 1 | 38 | 63-67 | 2.1 | 1 |
| 17 | 29-99 | 2.1 | 6 | 39 | 74-77 | 1.1 | 3 |
| 18 | 99-100 | 0.8 | 6 | 40 | 77-81 | 2.7 | 3 |
| 19 | 100-145 | 1.5 | 6 | 41 | 81-89 | 1.6 | 8 |
| 20 | 145-32 | 2.9 | 6 | 42 | 14-73 | 1.1 | 1 |
| 21 | 145-31 | 0.9 | 2 | 43 | 76-15 | 1.0 | 2 |
| 22 | 31-33 | 1.2 | 2 | 44 | 77-17 | 0.9 | 1 |

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 45 | 77-17 | 1.2 | 3 | 67 | 85-16 | 0.9 | 3 |
| 46 | 17-78 | 1.5 | 1 | 68 | 85-101 | 2.0 | 3 |
| 47 | 78-18 | 2.1 | 1 | 69 | 101-01 | 0.7 | 2 |
| 48 | 18-79 | 1.3 | 1 | 70 | 29-16 | 1.6 | 2 |
| 49 | 21-75 | 3.5 | 3 | 71 | 16-80 | 1.3 | 2 |
| 50 | 75-150 | 1.9 | 3 | 72 | 80-81 | 1.2 | 6 |
| 51 | 150-22 | 5.3 | 3 | 73 | 81-82 | 1.2 | 6 |
| 52 | 22-98 | 0.5 | 3 | 74 | 82-83 | 0.6 | 6 |
| 53 | 98-97 | 4.2 | 3 | 75 | 83-84 | 0.6 | 3 |
| 54 | 97-96 | 1.7 | 3 | 76 | 84-147 | 1.9 | 3 |
| 55 | 96-95 | 1.1 | 3 | 77 | 147-143 | 1.2 | 12 |
| 56 | 95-94 | 2.2 | 3 | 78 | 143-148 | 2.1 | 12 |
| 57 | 94-20 | 0.9 | 3 | 79 | 148-97 | 1.7 | 12 |
| 58 | 20-93 | 1.0 | 6 | 80 | 97-149 | 3.9 | 12 |
| 59 | 93-91 | 1.4 | 6 | 81 | 99-101 | 1.7 | 6 |
| 60 | 91-19 | 1.1 | 6 | 82 | 101-102 | 1.2 | 9 |
| 61 | 19-90 | 0.6 | 6 | 83 | 102-103 | 1.1 | 6 |
| 62 | 90-89 | 0.4 | 6 | 84 | 103-88 | 0.5 | 6 |
| 63 | 89-88 | 0.3 | 6 | 85 | 01-112 | 0.3 | 2 |
| 64 | 88-87 | 1.1 | 7 | 86 | 112-113 | 0.6 | 3 |
| 65 | 87-86 | 0.4 | 3 | 87 | 113-114 | 0.3 | 3 |
| 66 | 86-85 | 0.8 | 2 | 88 | 114-104 | 0.6 | 3 |

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 89 | 104-102 | 0.5 | 3 | 111 | 119-120 | 0.3 | 8 |
| 90 | 01-104 | 1.2 | 1 | 112 | 120-121 | 1.0 | 8 |
| 91 | 104-02 | 0.8 | 3 | 113 | 121-122 | 0.5 | 9 |
| 92 | 02-105 | 0.5 | 2 | 114 | 122-123 | 1.5 | 9 |
| 93 | 105-4 | 0.8 | 3 | 115 | 123-25 | 3.7 | 6 |
| 94 | 04-106 | 0.8 | 3 | 116 | 123-124 | 7.4 | 11 |
| 95 | 106-05 | 0.5 | 6 | 117 | 111-124 | 4.0 | 6 |
| 96 | 05-107 | 0.5 | 6 | 118 | 124-27 | 0.5 | 6 |
| 97 | 107-06 | 0.8 | 6 | 119 | 27-157 | 3.0 | 3 |
| 98 | 06-108 | 0.6 | 7 | 120 | 113-03 | 0.5 | 4 |
| 99 | 108-23 | 1.1 | 7 | 121 | 03-115 | 1.2 | 2 |
| 100 | 23-109 | 0.8 | 7 | 122 | 03-07 | 1.7 | 4 |
| 101 | 109-24 | 0.6 | 7 | 123 | 116-07 | 1.0 | 2 |
| 102 | 24-110 | 1.8 | 7 | 124 | 07-132 | 0.8 | 3 |
| 103 | 110-25 | 1.2 | 6 | 125 | 132-133 | 0.4 | 3 |
| 104 | 25-111 | 0.9 | 6 | 126 | 133-09 | 1.0 | 6 |
| 105 | 111-26 | 4.8 | 3 | 127 | 09-139 | 2.1 | 6 |
| 106 | 114-115 | 0.7 | 2 | 128 | 139-11 | 0.6 | 6 |
| 107 | 115-116 | 1.0 | 2 | 129 | 18-84 | 3.5 | 1 |
| 108 | 116-117 | 0.9 | 8 | 130 | 83-92 | 0.8 | 6 |
| 109 | 117-118 | 0.9 | 8 | 131 | 92-91 | 0.5 | 15 |
| 110 | 118-119 | 0.6 | 8 | 132 | 91-107 | 0.6 | 6 |

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 133 | 107-120 | 1.7 | 6 | 155 | 126-37 | 1.4 | 2 |
| 134 | 120-08 | 1.4 | 3 | 156 | 37-131 | 1.1 | 2 |
| 135 | 08-132 | 1.0 | 3 | 157 | 131-133 | 0.7 | 6 |
| 136 | 125-08 | 0.7 | 3 | 158 | 131-38 | 1.1 | 2 |
| 137 | 133-134 | 1.4 | 3 | 159 | 38-138 | 2.0 | 2 |
| 138 | 134-135 | 1.3 | 3 | 160 | 129-131 | 1.8 | 6 |
| 139 | 135-121 | 0.8 | 3 | 161 | 128-37 | 1.6 | 3 |
| 140 | 121-06 | 1.8 | 5 | 162 | 112-126 | 0.8 | 2 |
| 141 | 108-93 | 0.9 | 2 | 163 | 126-127 | 0.5 | 8 |
| 142 | 20-147 | 0.8 | 3 | 164 | 127-128 | 0.5 | 8 |
| 143 | 20-23 | 1.4 | 2 | 165 | 128-129 | 0.4 | 6 |
| 144 | 103-105 | 0.9 | 1 | 166 | 129-36 | 1.5 | 6 |
| 145 | 105-116 | 1.2 | 3 | 167 | 36-137 | 0.6 | 6 |
| 146 | 90-04 | 0.9 | 2 | 168 | 137-156 | 2.0 | 12 |
| 147 | 04-117 | 1.3 | 3 | 169 | 156-28 | 2.3 | 12 |
| 148 | 05-119 | 1.7 | 2 | 170 | 156-155 | 2.8 | 13 |
| 149 | 135-110 | 0.8 | 1 | 171 | 137-146 | 1.9 | 11 |
| 150 | 10-122 | 1.3 | 1 | 172 | 146-130 | 1.3 | 12 |
| 151 | 10-13 | 1.1 | 1 | 173 | 130-128 | 0.7 | 6 |
| 152 | 09-136 | 2.0 | 2 | 174 | 130-32 | 1.4 | 6 |
| 153 | 136-13 | 1.9 | 1 | 175 | 32-146 | 1.0 | 11 |
| 154 | 136-141 | 1.5 | 1 | 176 | 32-154 | 2.8 | 6 |

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 177 | 138-139 | 1.2 | 3 | 199 | 95-110 | 2.7 | 2 |
| 178 | 139-140 | 1.9 | 11 | 200 | 96-25 | 3.3 | 2 |
| 179 | 140-141 | 2.3 | 11 | 201 | 66-151 | 3.9 | 12 |
| 180 | 141-142 | 1.6 | 11 | 202 | 151-150 | 3.8 | 12 |
| 181 | 142-123 | 2.7 | 11 | 203 | 150-148 | 4.3 | 12 |
| 182 | 123-109 | 1.6 | 11 | 204 | 148-95 | 1.1 | 12 |
| 183 | 109-94 | 1.7 | 11 | 205 | 63-49 | 1.2 | 1 |
| 184 | 94-143 | 1.0 | 11 | 206 | 64-162 | 4.6 | 3 |
| 185 | 143-79 | 2.5 | 11 | 207 | 65-39 | 4.5 | 13 |
| 186 | 79-75 | 2.4 | 11 | 208 | 66-163 | 4.7 | 3 |
| 187 | 75-71 | 4.9 | 11 | 209 | 151-40 | 4.5 | 12 |
| 188 | 71-70 | 1.8 | 11 | 210 | 152-164 | 3.6 | 12 |
| 189 | 70-69 | 1.8 | 11 | 211 | 151-152 | 5.0 | 12 |
| 190 | 69-68 | 1.3 | 11 | 212 | 152-22 | 4.8 | 12 |
| 191 | 68-144 | 8.6 | 11 | 213 | 149-165 | 12.3 | 12 |
| 192 | 144-145 | 1.9 | 11 | 214 | 98-149 | 0.8 | 12 |
| 193 | 30-100 | 0.7 | 3 | 215 | 149-26 | 4.7 | 12 |
| 194 | 144-153 | 2.8 | 13 | 216 | 26-166 | 12.1 | 3 |
| 195 | 153-67 | 8.7 | 12 | 217 | 26-157 | 5.0 | 12 |
| 196 | 153-154 | 7.8 | 12 | 218 | 157-43 | 9.3 | 3 |
| 197 | 154-155 | 4.9 | 12 | 219 | 27-47 | 6.5 | 3 |
| 198 | 08-134 | 0.5 | 3 | 220 | 28-42 | 6.0 | 12 |

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 221 | 140-12 | 3.5 | 11 | 243 | 164-41 | 9.8 | 1 |
| 222 | 12-142 | 3.2 | 11 | 244 | 41-57 | 37.2 | 14 |
| 223 | 155-46 | 2.9 | 13 | 245 | 41-165 | 1.2 | 12 |
| 224 | 153-44 | 2.0 | 13 | 246 | 165-166 | 9.6 | 12 |
| 225 | 92-93 | 1.6 | 1 | 247 | 166-43 | 10.5 | 12 |
| 226 | 86-102 | 1.1 | 10 | 248 | 43-171 | 31.3 | 14 |
| 227 | 99-144 | 0.5 | 6 | 249 | 47-171 | 47.4 | 14 |
| 229 | 53-172 | 25.4 | 14 | 250 | 46-50 | 22.6 | 13 |
| 229 | 35-146 | 0.5 | 12 | 251 | 154-45 | 1.6 | 3 |
| 230 | 137-138 | 0.4 | 6 | 252 | 45-172 | 17.2 | 3 |
| 231 | 13-142 | 0.8 | 1 | 253 | 44-172 | 17.4 | 13 |
| 232 | 48-49 | 4.9 | 1 | 254 | 39-161 | 9.2 | 13 |
| 233 | 48-162 | 6.0 | 1 | 255 | 160-161 | 3.5 | 3 |
| 234 | 48-158 | 9.5 | 1 | 256 | 160-54 | 1.8 | 14 |
| 235 | 158-159 | 2.3 | 1 | 257 | 60-61 | 165.4 | 14 |
| 236 | 159-160 | 4.8 | 1 | 258 | 61-62 | 224.6 | 14 |
| 237 | 159-162 | 5.0 | 3 | 259 | 52-60 | 245.7 | 14 |
| 238 | 162-39 | 3.0 | 1 | 260 | 60-169 | 189.2 | 14 |
| 239 | 39-163 | 2.3 | 1 | 261 | 61-55 | 230.5 | 14 |
| 240 | 161-163 | 9.5 | 3 | 262 | 62-167 | 187.8 | 14 |
| 241 | 163-40 | 3.2 | 1 | 263 | 167-168 | 81.2 | 14 |
| 242 | 40-164 | 5.2 | 1 | 264 | 168-55 | 57.3 | 14 |

| Link No. | Node-pair | Distance | Type | Link No. | Node-pair | Distance | Type |
|----------|-----------|----------|------|----------|-----------|----------|------|
| 265 | 55-169 | 59.7 | 14 | 284 | 176-44 | 1.1 | 1 |
| 266 | 167-170 | 119.1 | 14 | 285 | 155-174 | 1.0 | 12 |
| 267 | 168-56 | 58.3 | 14 | 286 | 46-177 | 12.3 | 2 |
| 268 | 169-54 | 21.4 | 14 | 287 | 177-50 | 12.3 | 2 |
| 269 | 54-56 | 58.6 | 14 | 298 | 50-172 | 24.8 | 2 |
| 270 | 56-170 | 65.2 | 14 | 289 | 84-178 | 1.5 | 2 |
| 271 | 170-57 | 86.2 | 14 | 290 | 178-93 | 0.7 | 1 |
| 272 | 57-171 | 18.3 | 14 | 291 | 178-179 | 0.4 | 2 |
| 273 | 171-58 | 22.7 | 14 | 292 | 179-93 | 0.5 | 6 |
| 274 | 50-51 | 55.8 | 13 | 293 | 180-179 | 1.0 | 2 |
| 275 | 51-59 | 10.4 | 13 | 294 | 180-108 | 0.3 | 7 |
| 276 | 51-173 | 60.5 | 14 | 295 | 180-181 | 2.6 | 2 |
| 277 | 173-52 | 51.2 | 14 | 296 | 181-123 | 1.0 | 9 |
| 278 | 173-53 | 19.1 | 14 | 297 | 181-182 | 1.5 | 2 |
| 279 | 156-174 | 2.6 | 2 | 298 | 182-123 | 1.4 | 11 |
| 280 | 174-46 | 2.9 | 2 | 299 | 183-32 | 0.5 | 6 |
| 281 | 99-175 | 1.8 | 1 | 300 | 144-184 | 2.4 | 11 |
| 282 | 175-153 | 1.8 | 1 | 301 | 184-29 | 0.6 | 2 |
| 283 | 153-176 | 1.1 | 1 | | | | |

3-5-4 Results of traffic assignment

Traffic assignment were made for the years 1975, 1980, 1990 both for 'with' and 'without' the Ring Road. For the year 1990, the assignment for both the 'optimum network' and the 'normal network' were carried out. The results of the assignments are graphically shown in Fig. 3-25 - Fig. 3-32.

Various cases where only part of the Ring Road Part II were completed was also assumed and assignment made. These are shown in Fig. 3-33 - Fig. 3-35 for reference.

It can be seen from the results of the traffic assignment that the traffic volume on the Ring Road reach the requirement for a six lane highway in all cases from as early as 1975. Purely from the traffic point of view, the total construction of the whole Ring Road from the initial stage is necessary.

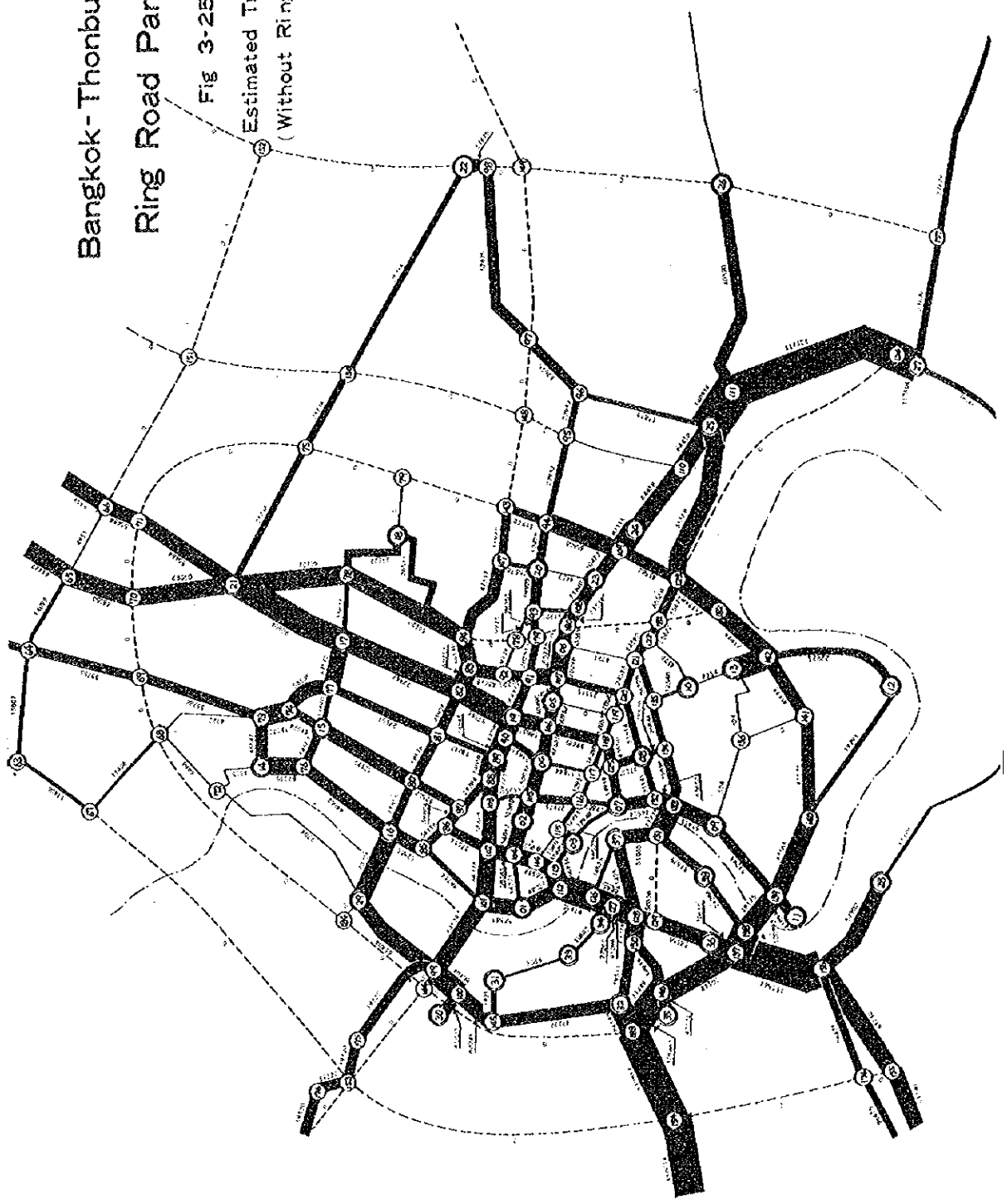
Also, the traffic volumes on other road sections are also, as expected, very heavy, and by 1990, most of them will have surpassed the capacity of the roads. Early steps to improve the existing road network are also matters of emergency.

3-5-5 Intersection traffic stream analysis

In the assignment, analysis for the traffic stream at interchanges of the Ring Road were also made. The results for the 1990 are as in Fig. 3-36 - Fig. 3-37. It will be seen that at most intersection planned with diamond interchanges, the traffic volume will reach the capacity of the intersections, and appropriate channelization at the interchanges will be required to increase the capacity at interchanges. Total grade separation of these intersections, however, cannot be recommended, as these intersections are within the built-up area, and despite the enormous cost required for grade separation, the effectiveness is dubious without coordination with all other intersections in the city.

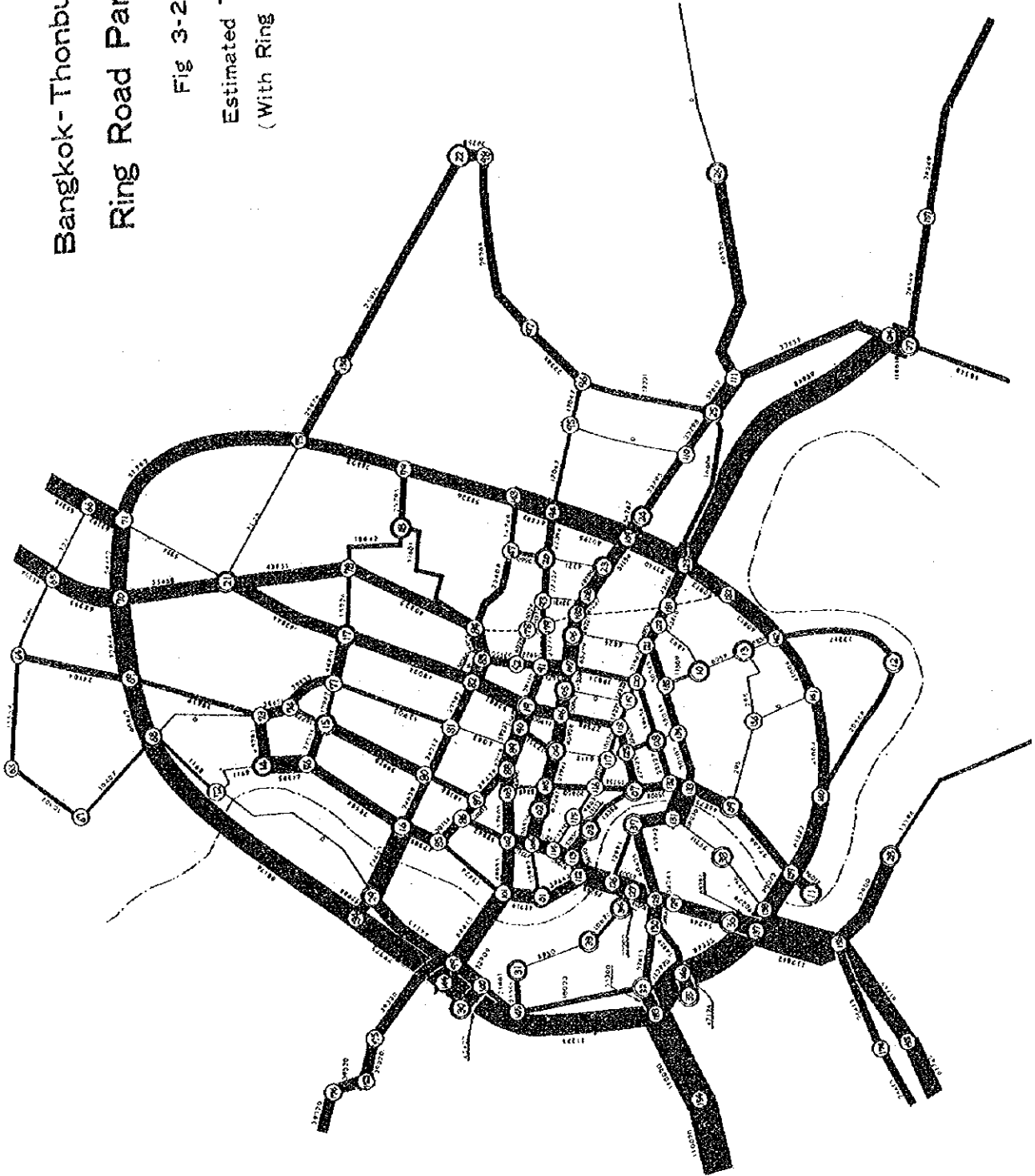
Bangkok-Thonburi Ring Road Part II

Fig 3-25
Estimated Traffic Flow, 1975
(Without Ring Road)



Bangkok-Thonburi
Ring Road Part II

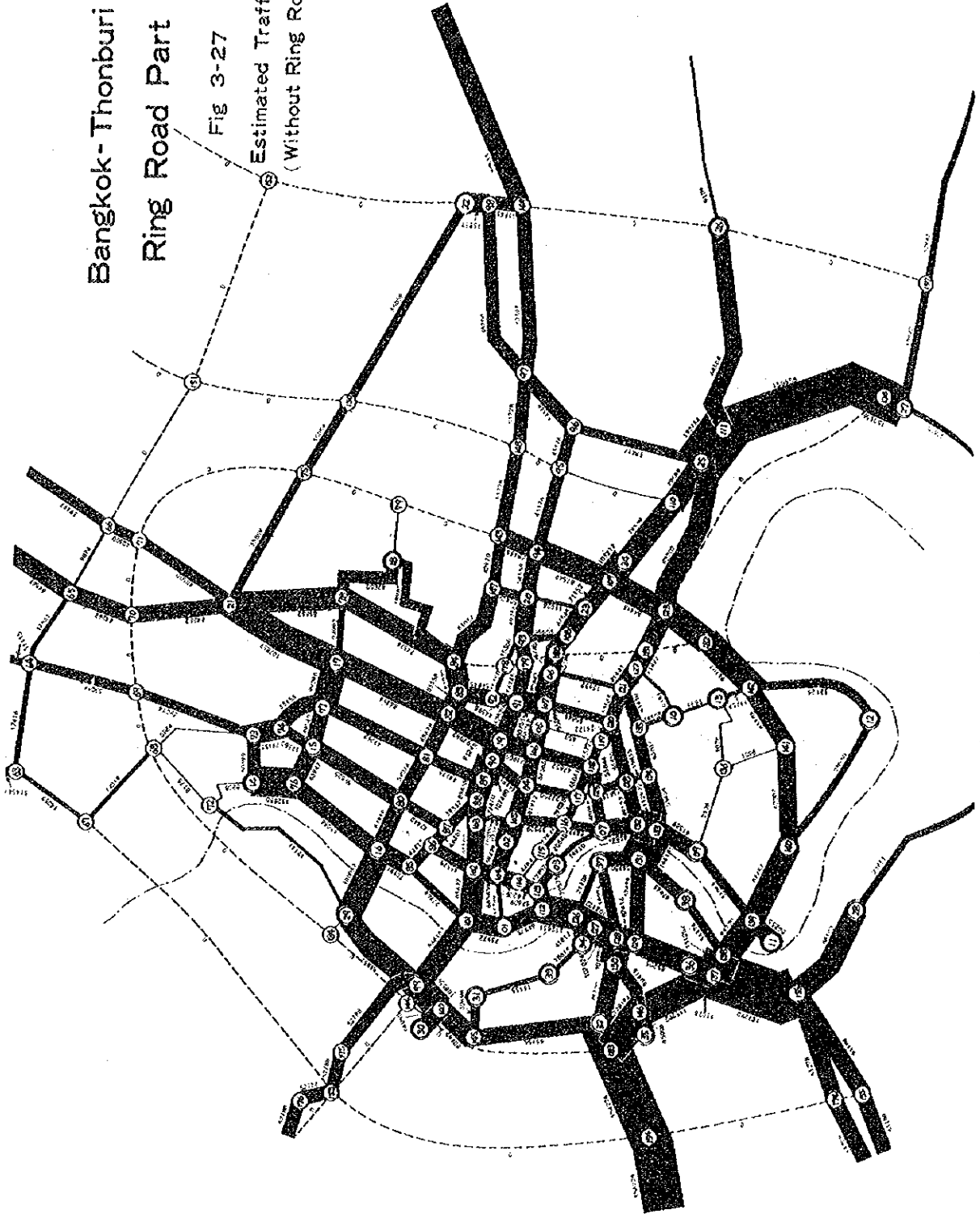
Fig 3-26
Estimated Traffic Flow, 1975
(With Ring Road)



Bangkok-Thonburi Ring Road Part II

Fig 3-27

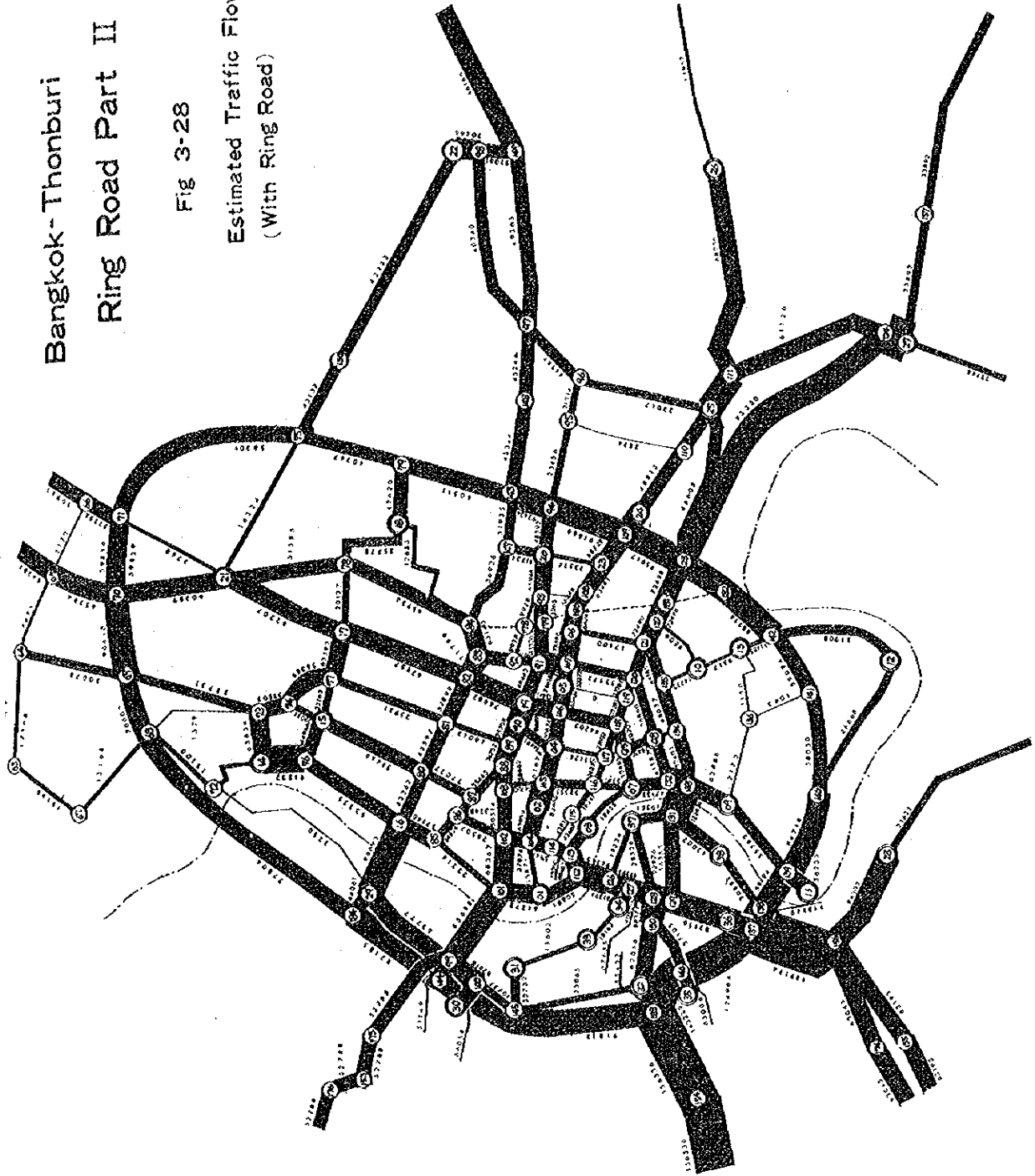
Estimated Traffic Flow, 1980
(Without Ring Road)



Bangkok-Thonburi Ring Road Part II

Fig 3-28

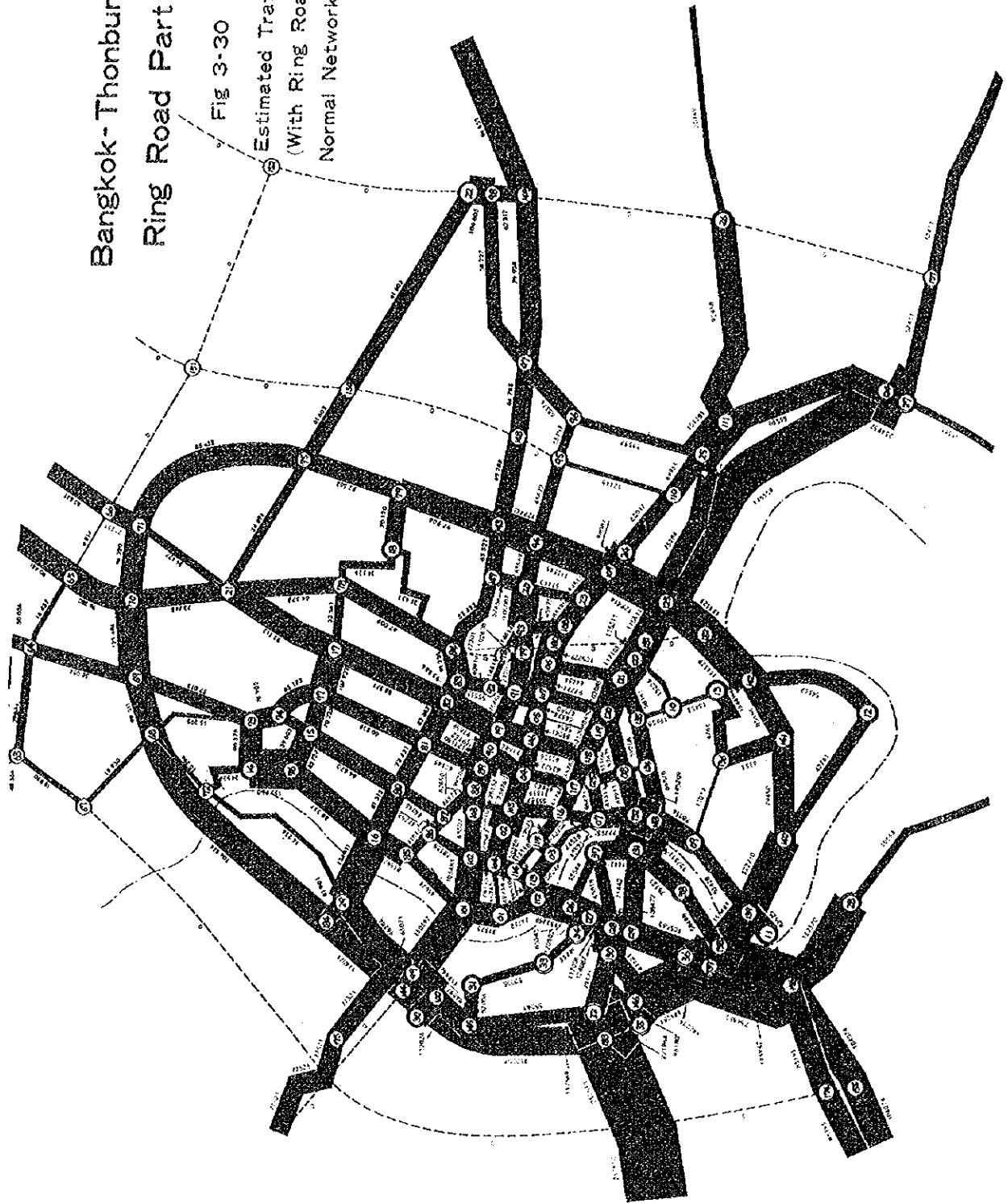
Estimated Traffic Flow, 1980
(With Ring Road)



Bangkok-Thonburi Ring Road Part II

Fig 3-30

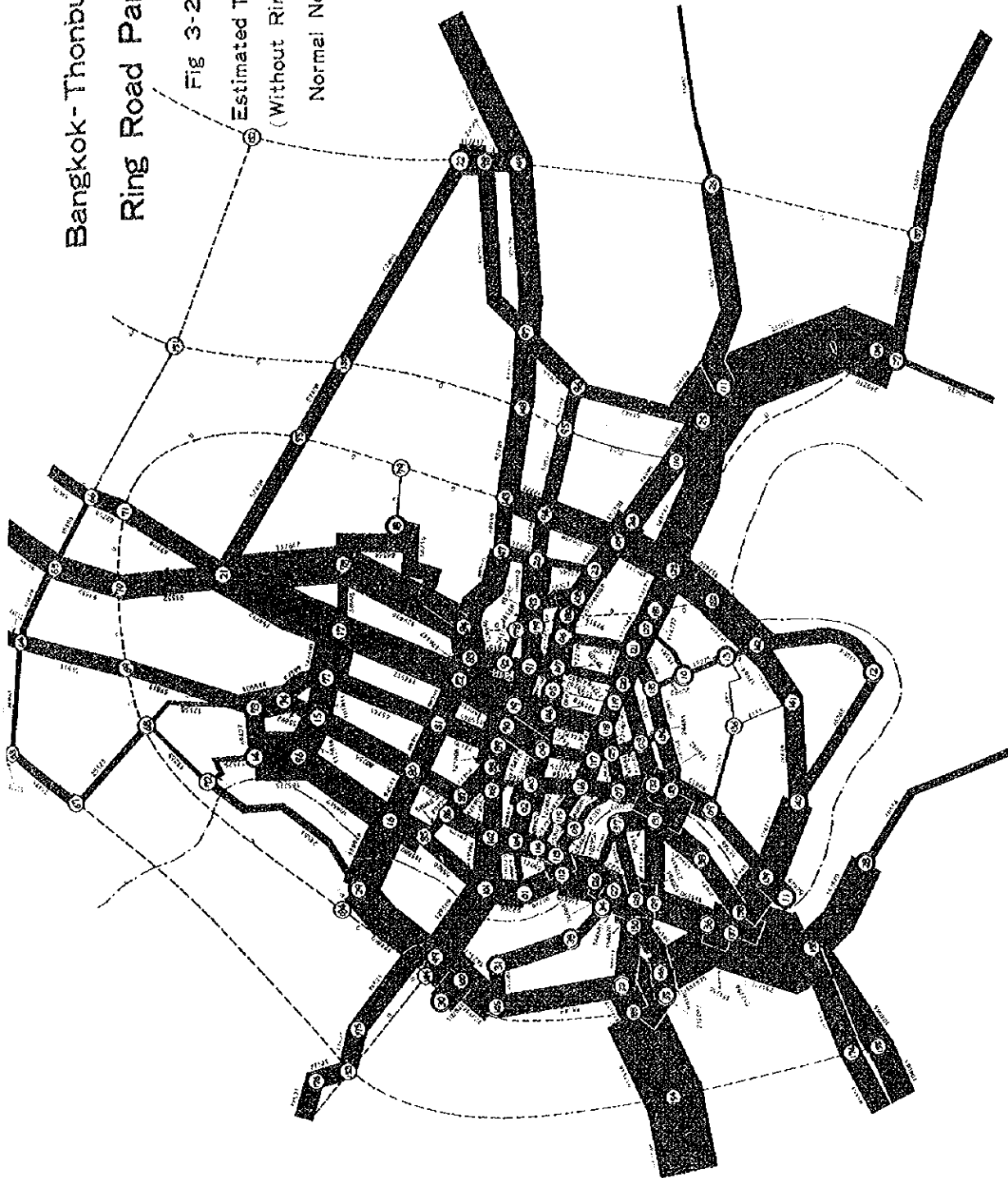
Estimated Traffic Flow, 1990
(With Ring Road,
Normal Network)



Bangkok-Thonburi Ring Road Part II

Fig 3-29

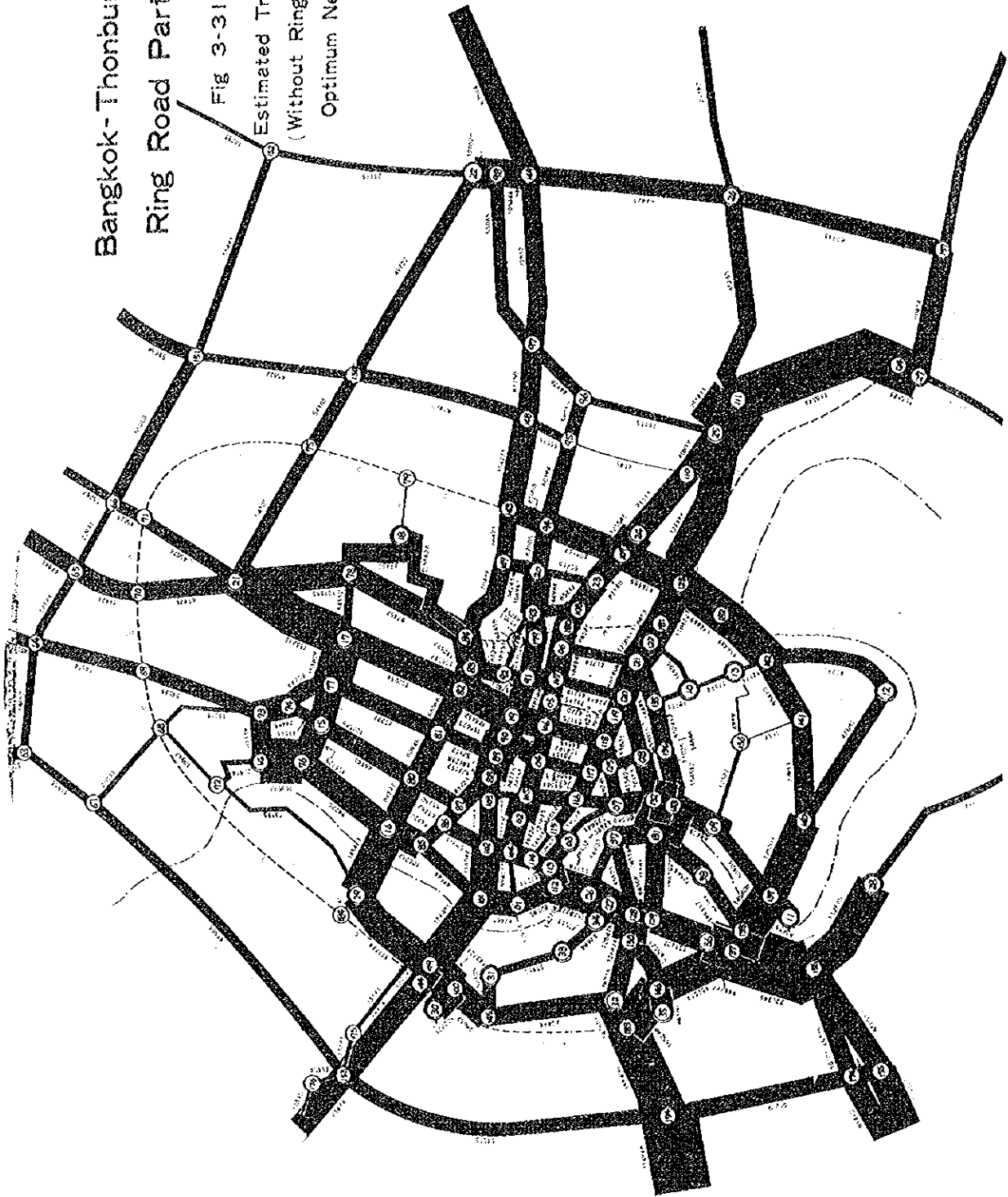
Estimated Traffic Flow, 1990
(Without Ring Road,
Normal Network)



Bangkok-Thonburi Ring Road Part II

Fig 3-31

Estimated Traffic Flow, 1990
(Without Ring Road,
Optimum Network)

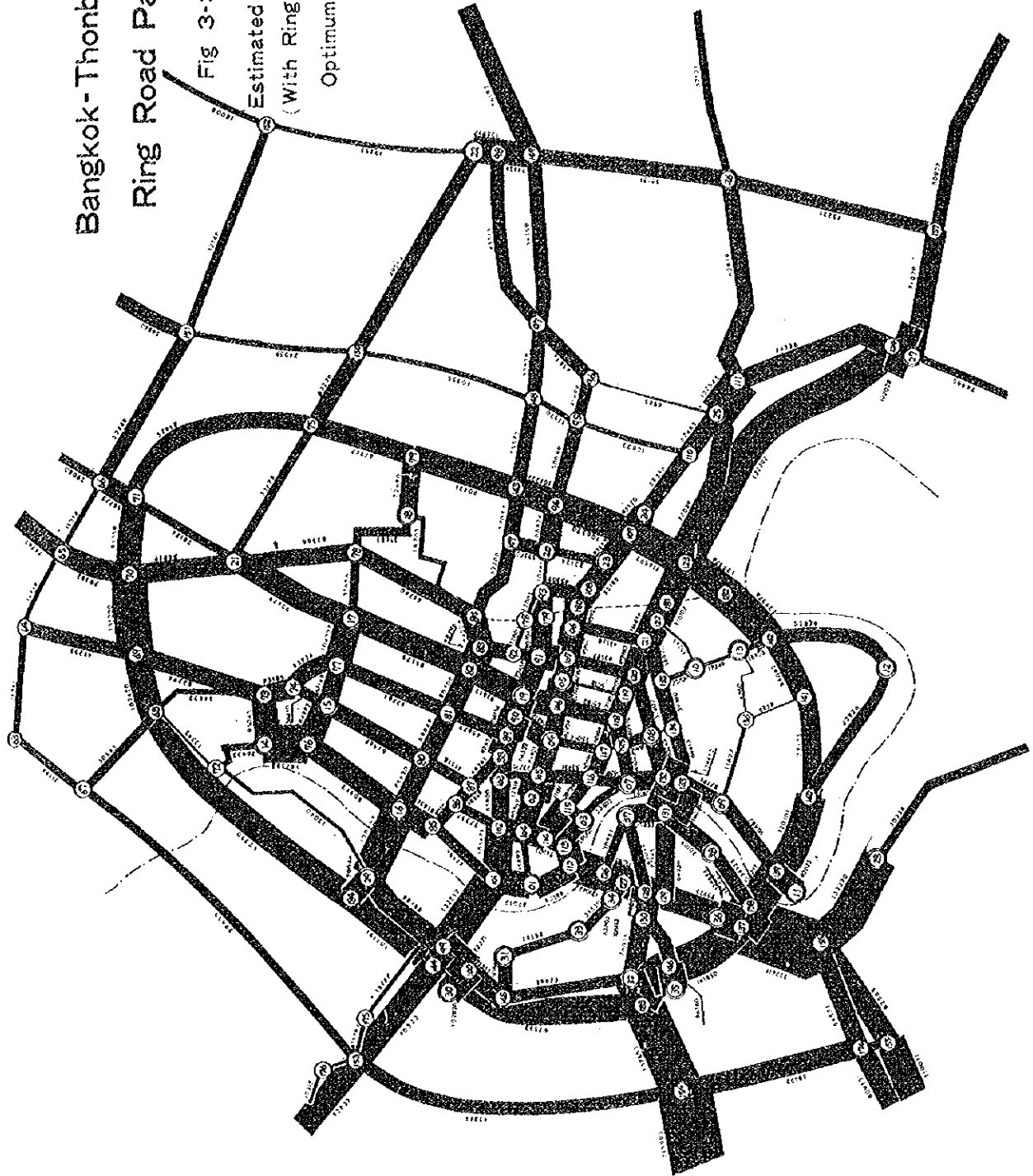


Bangkok-Thonburi

Ring Road Part II

Fig 3-32

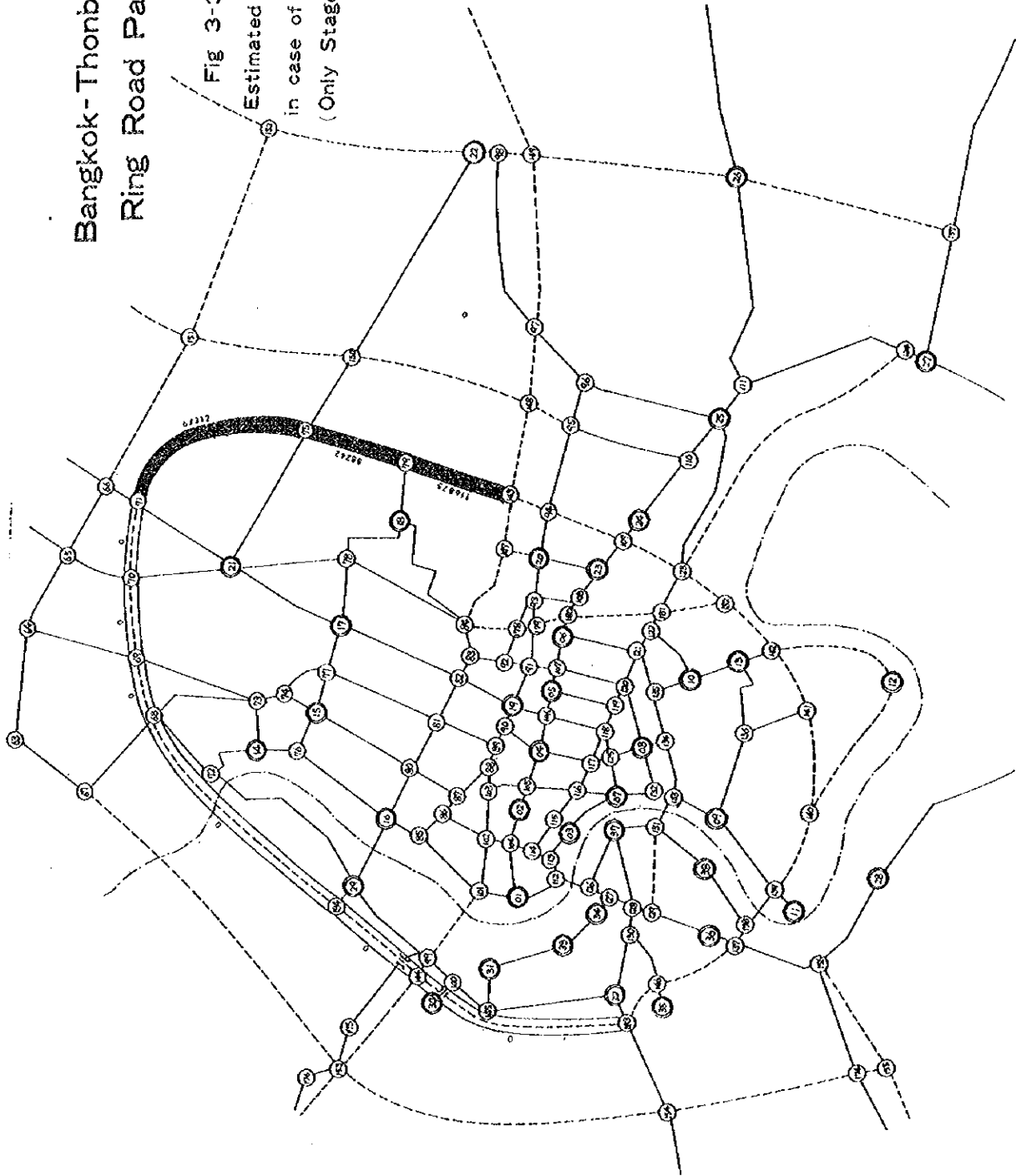
Estimated Traffic Flow, 1990
(With Ring Road.
Optimum Network)



Bangkok-Thonburi Ring Road Part II

Fig 3-33

Estimated Traffic Flow, 1990
in case of Stage Construction.
(Only Stage 1 being Completed)
vehicles/day

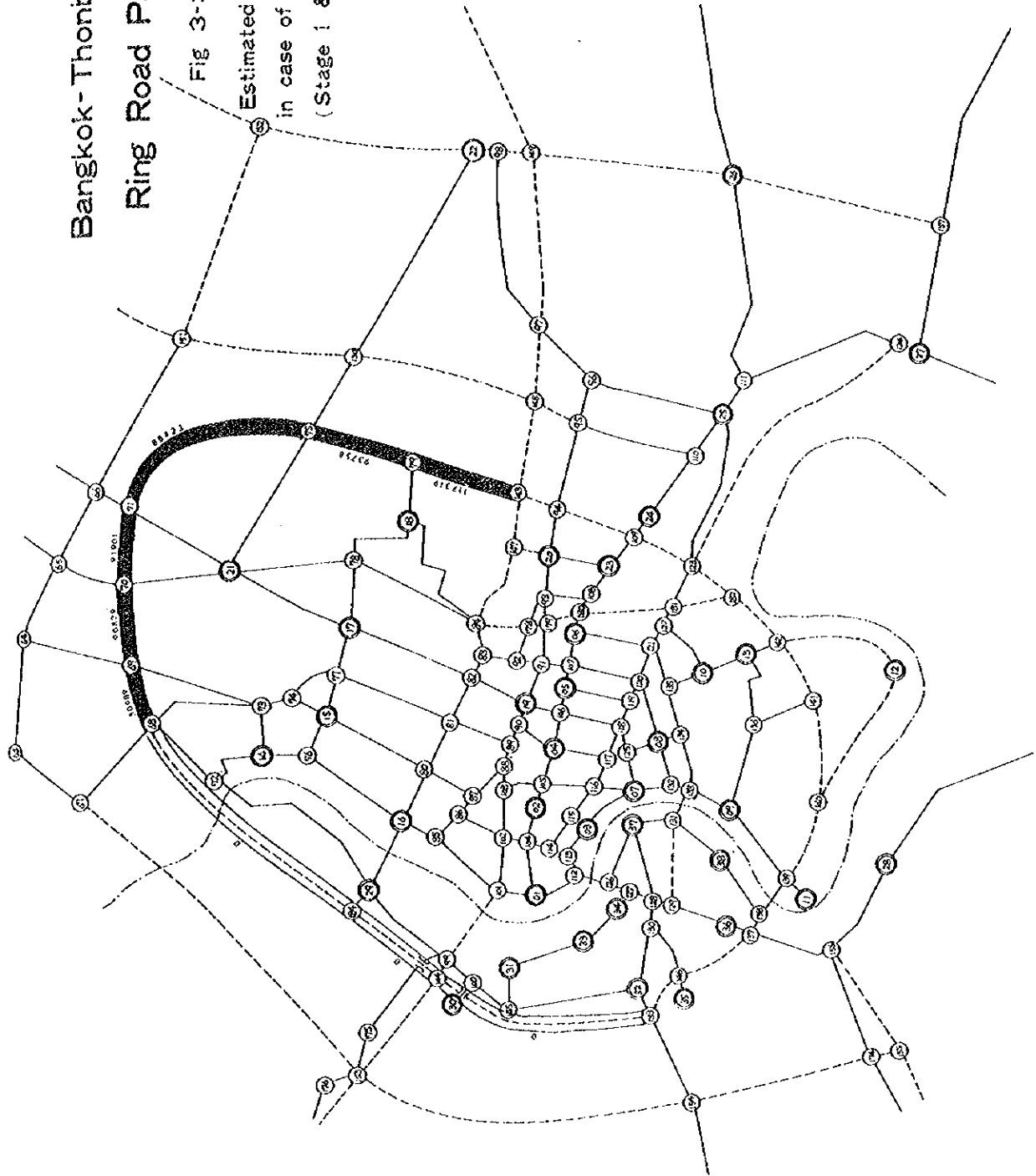


Bangkok-Thonburi Ring Road Part II

Fig 3-34

Estimated Traffic Flow, 1990
in case of Stage Construction
(Stage 1 & 2 being completed)

vehicles/day



Bangkok-Thonburi Ring Road Part II

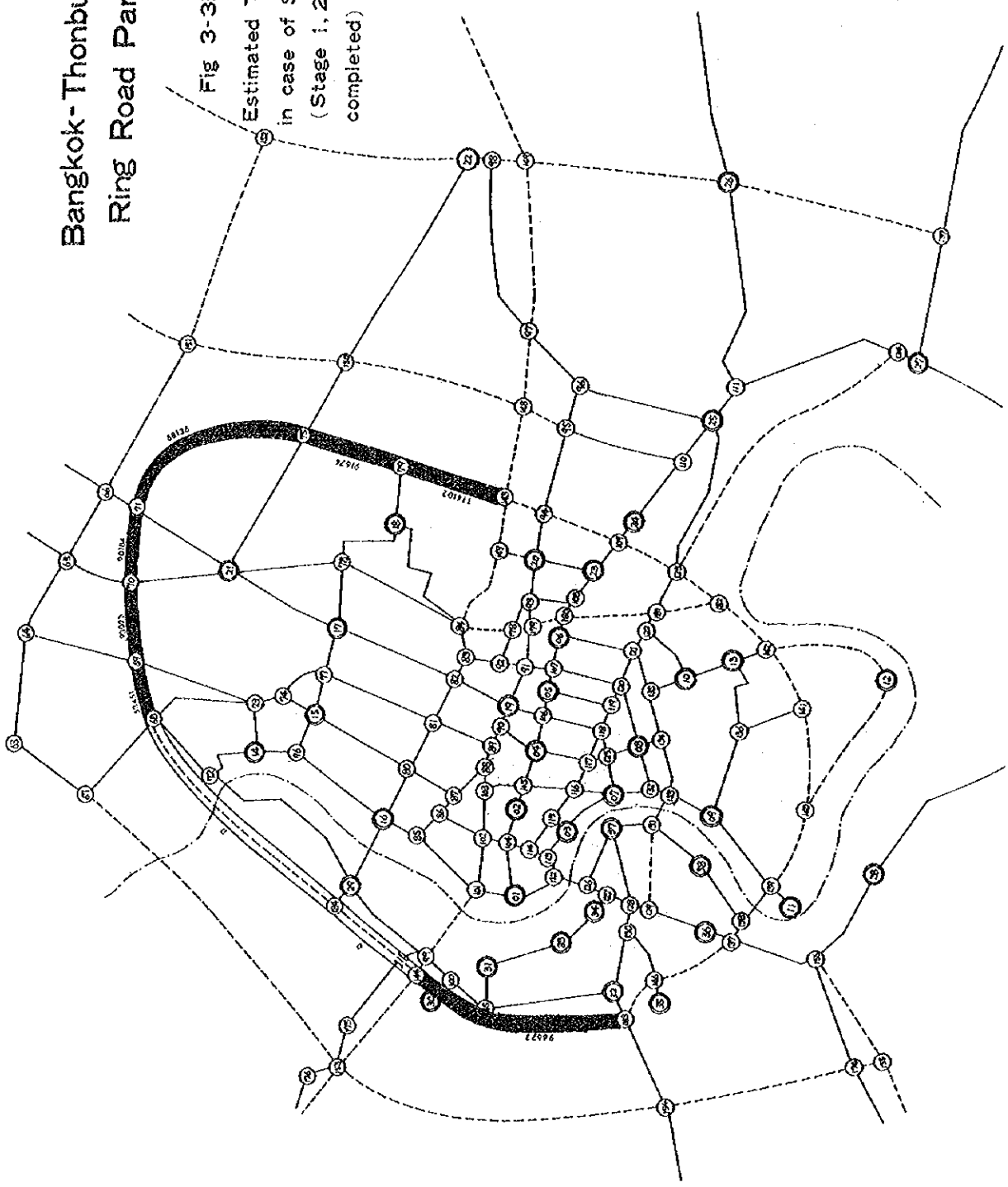
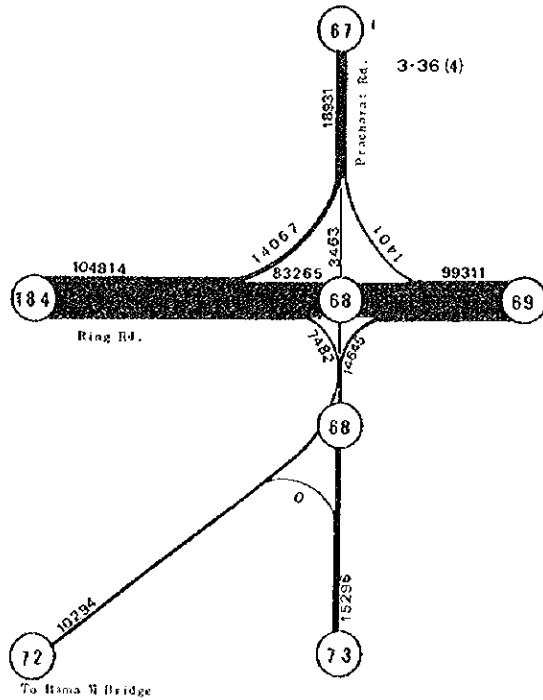
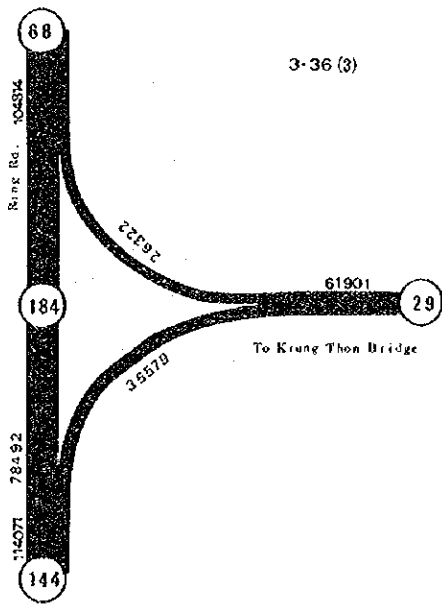
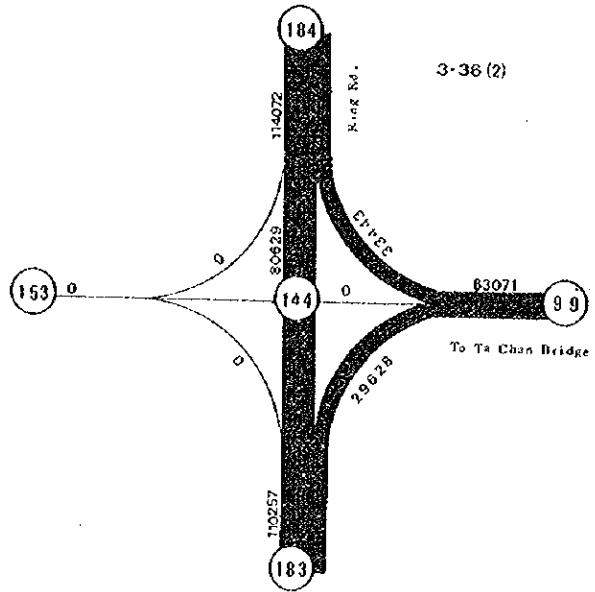
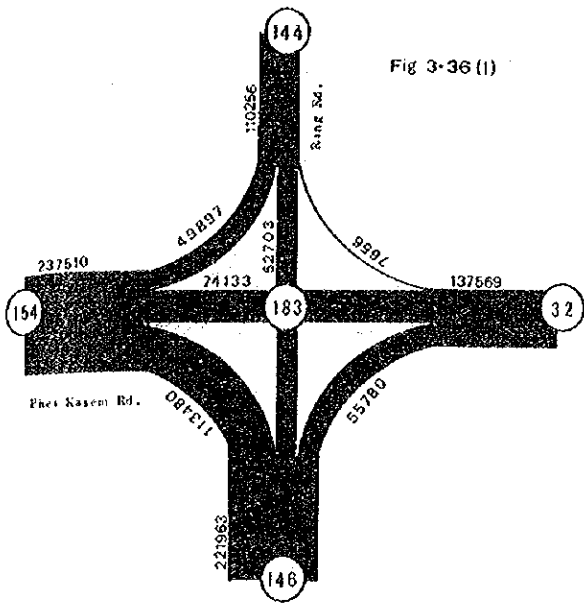
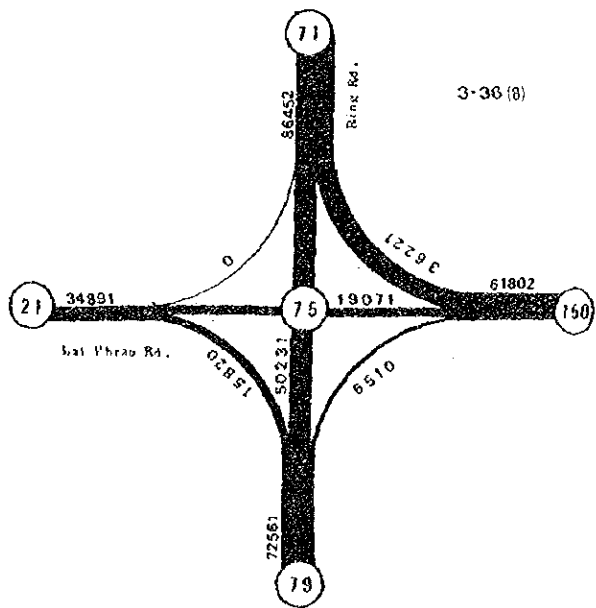
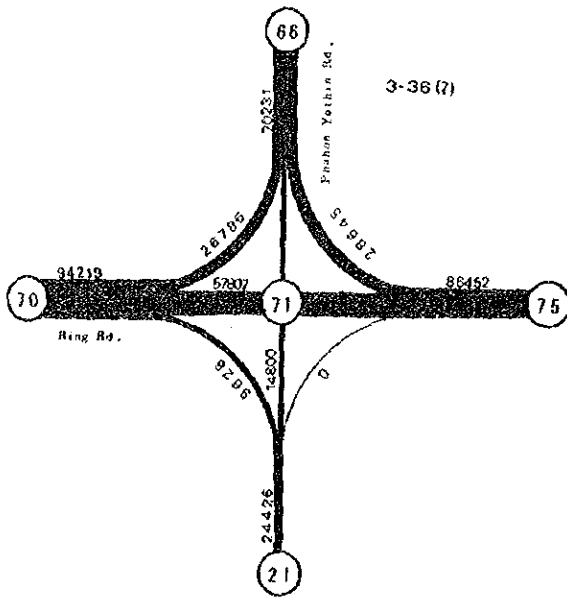
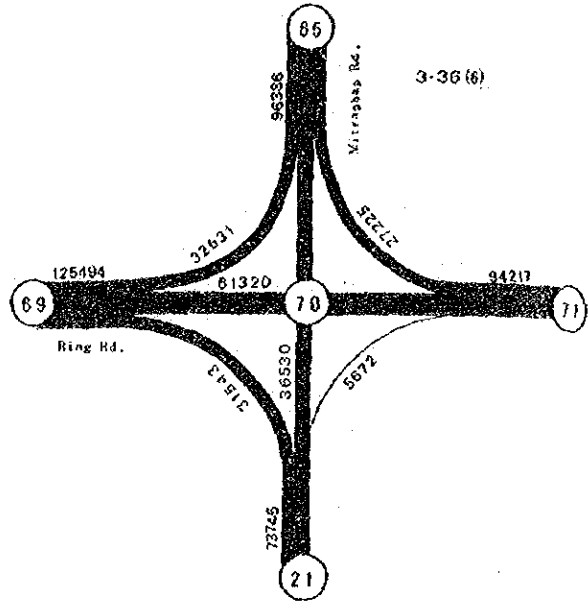
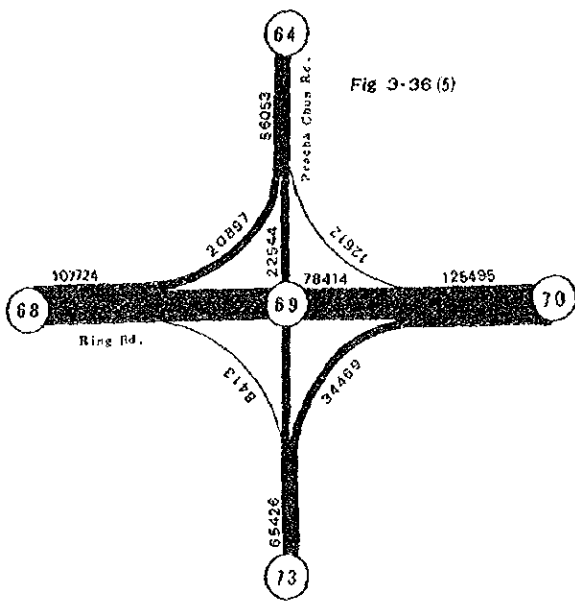


Fig 3-35
 Estimated Traffic Flow, 1990
 in case of Stage Construction
 (Stage 1, 2 & 3 being
 completed) vehicles/day

Estimated Intersection
 Traffic Stream, 1990
 (Normal Network)

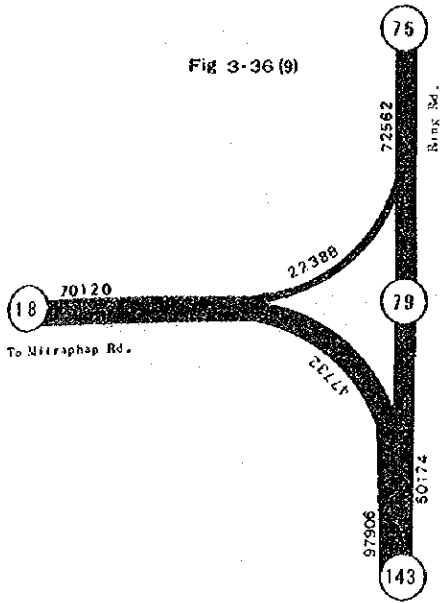


Estimated Intersection
 Traffic Stream, 1990
 (Normal Network)

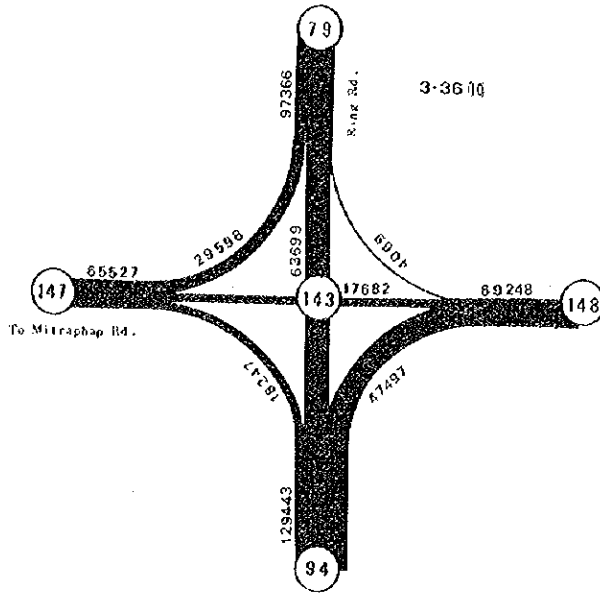


Estimated Intersection
 Traffic Stream, 1990
 (Normal Network)

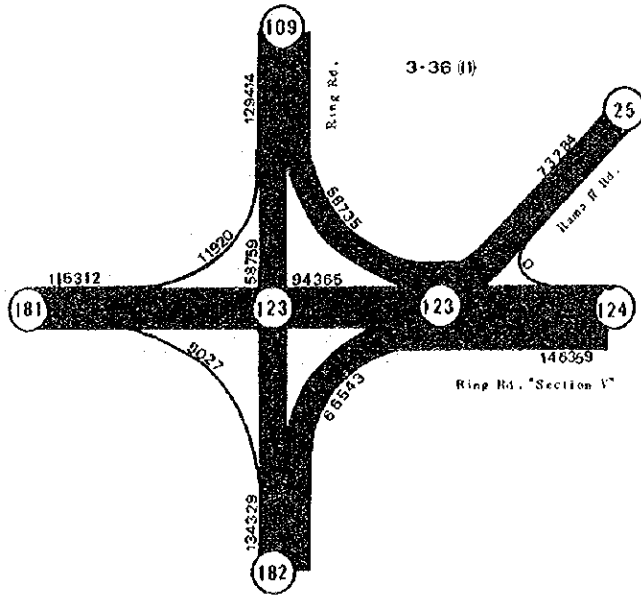
Fig 3-36 (9)



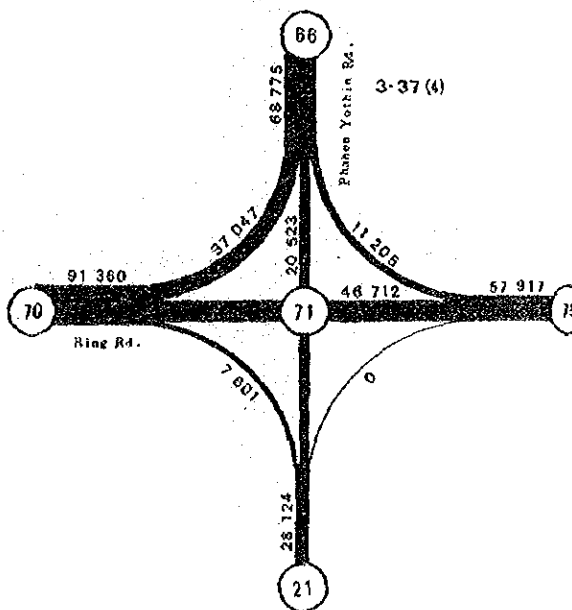
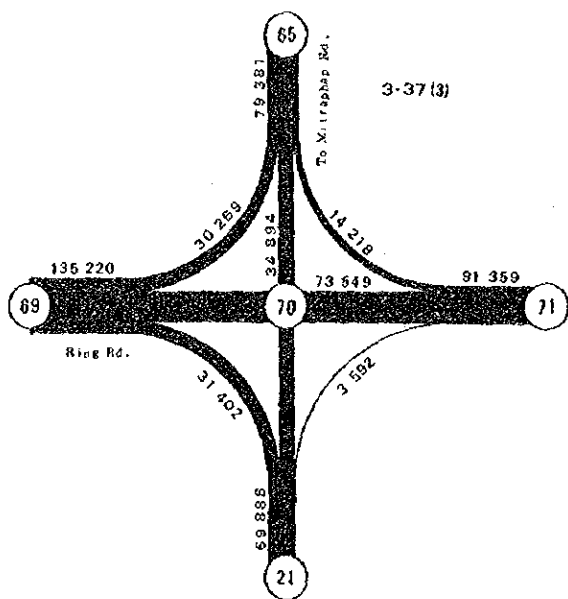
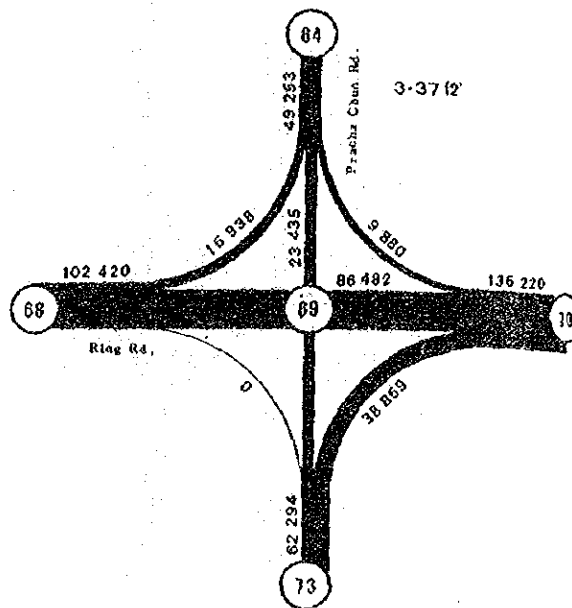
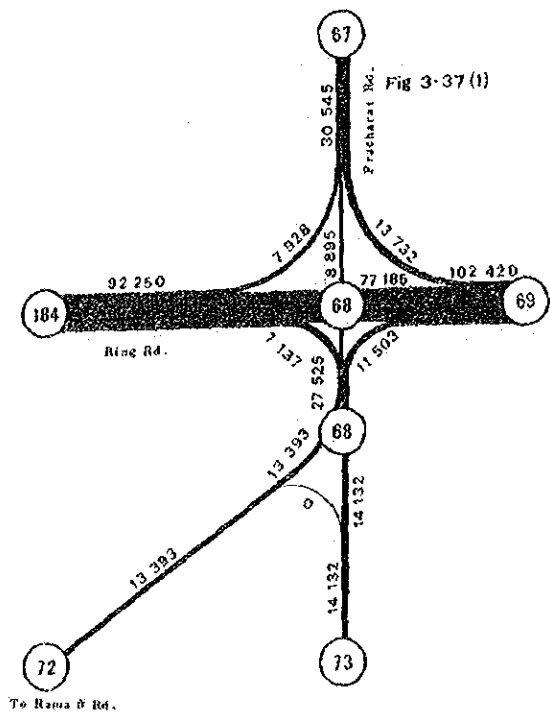
3-36 (10)



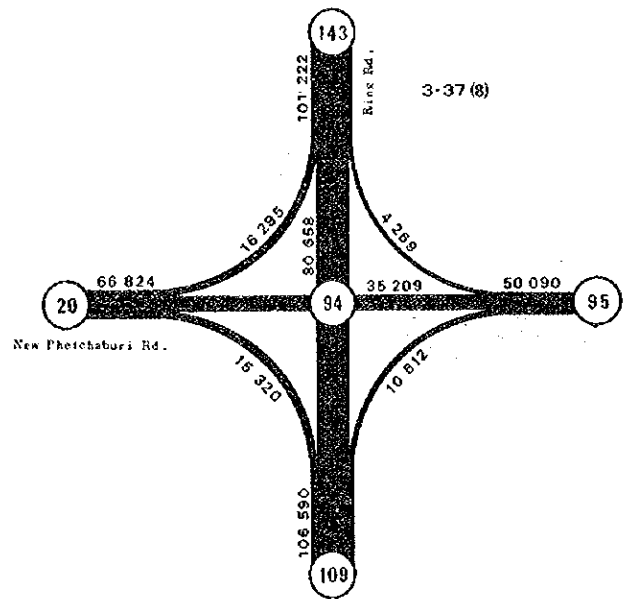
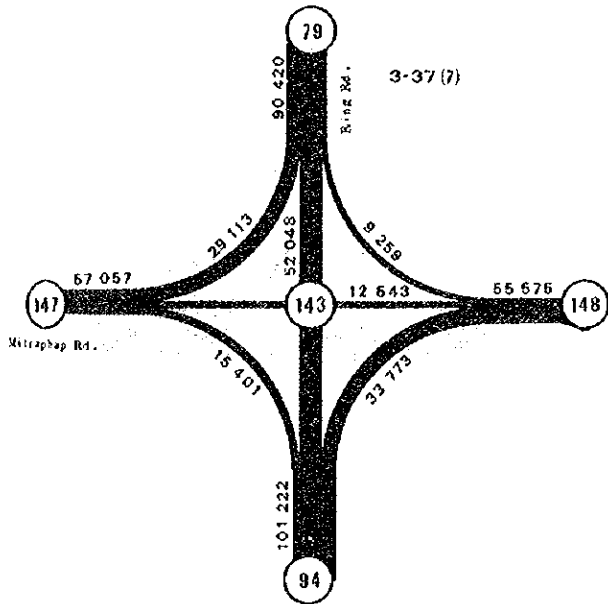
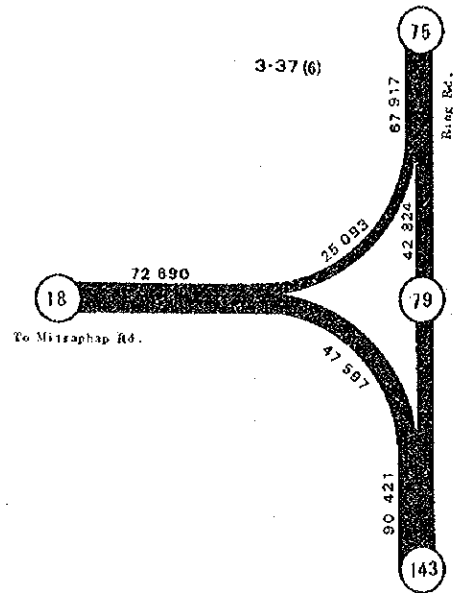
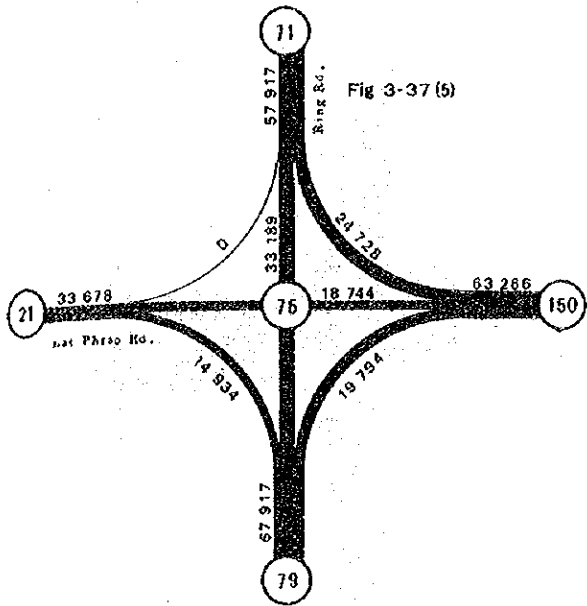
3-36 (11)



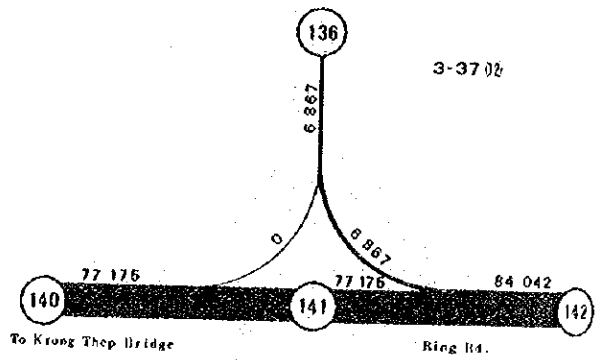
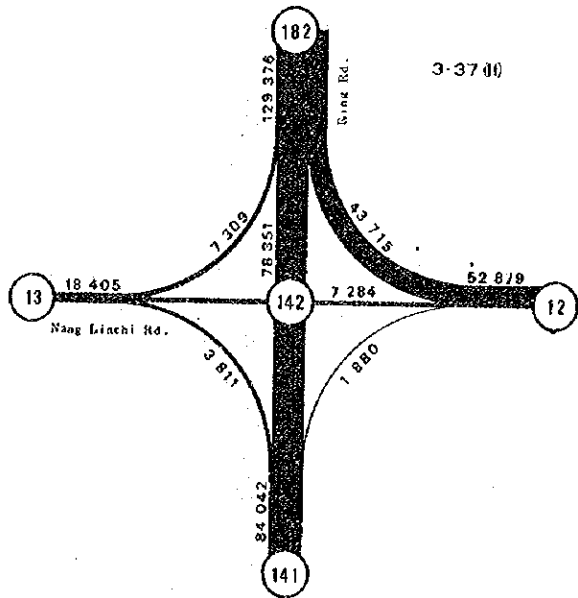
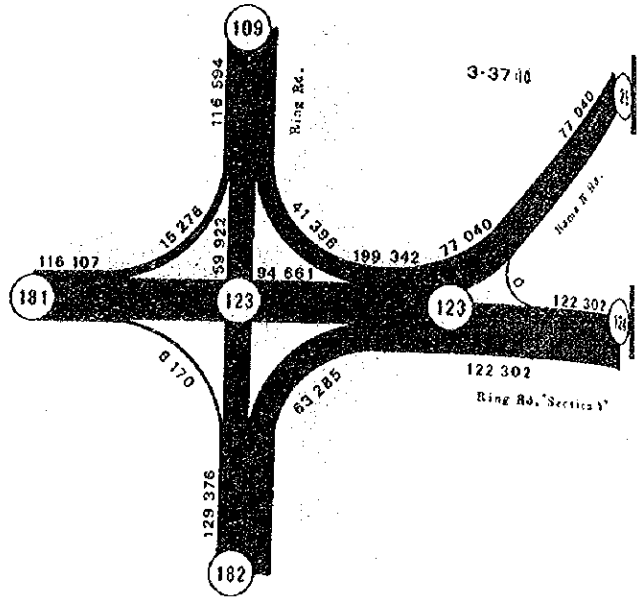
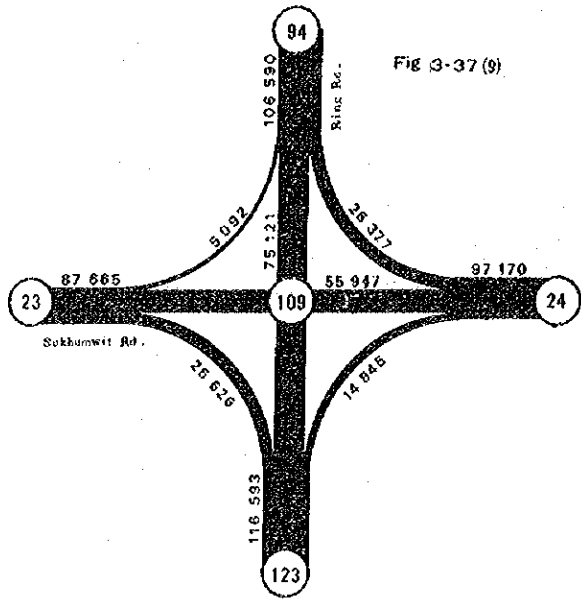
Estimated Intersection
 Traffic Stream, 1990
 (Optimum Network)



Estimated Intersection
 Traffic Stream, 1990
 (Optimum Network)

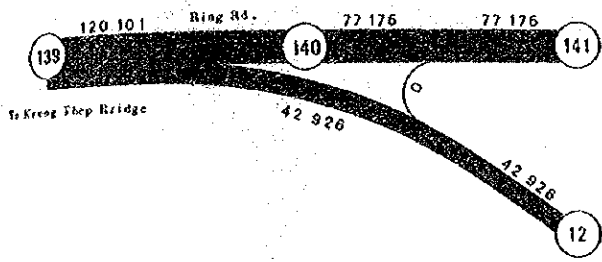


Estimated Intersection
 Traffic Stream, 1990
 (Optimum Network)

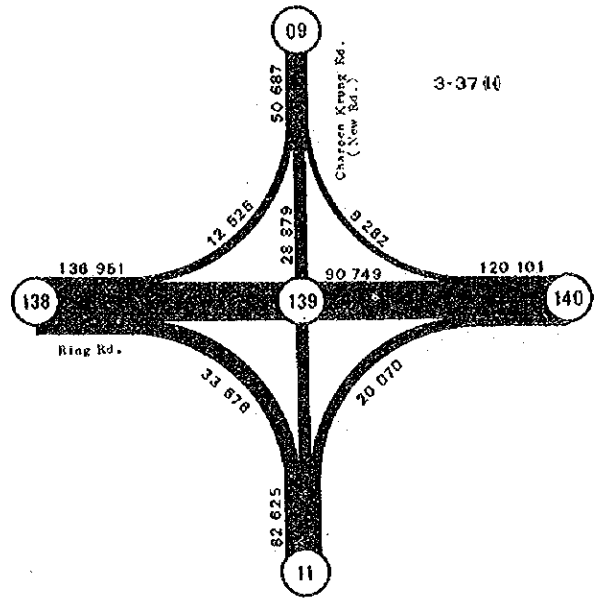


Estimated Intersection
 Traffic Stream, 1990
 (Optimum Network)

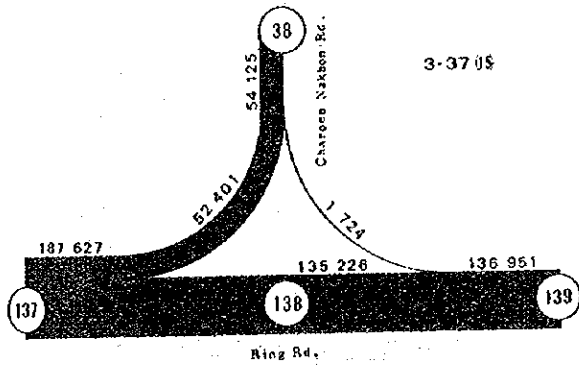
Fig 3-3705



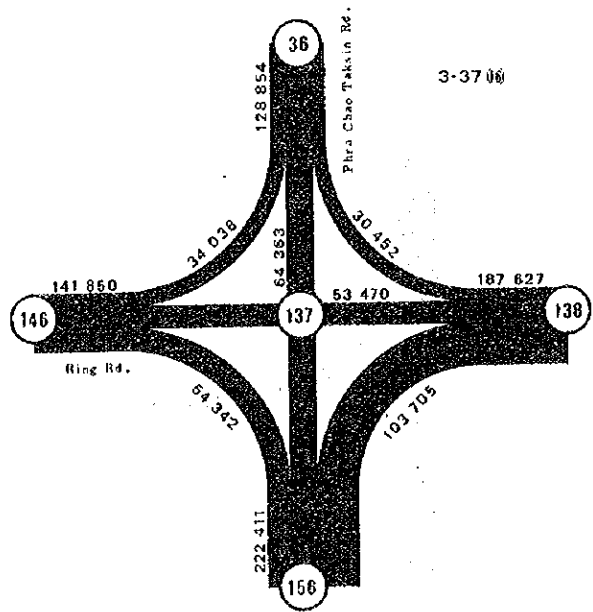
3-3710



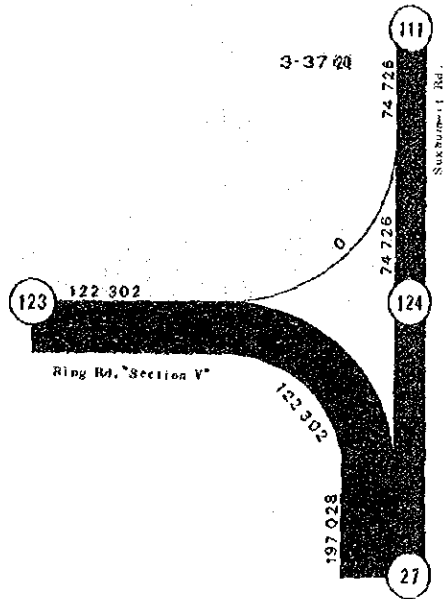
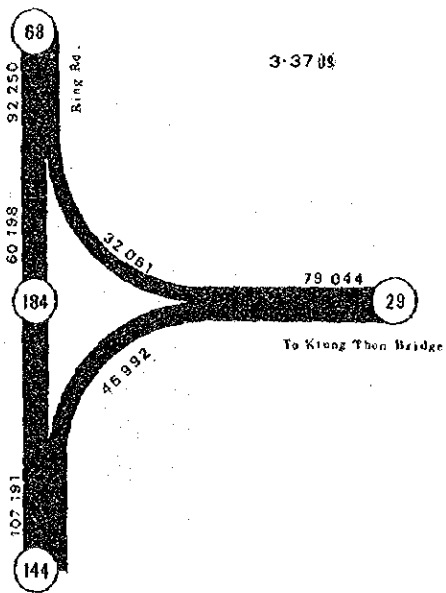
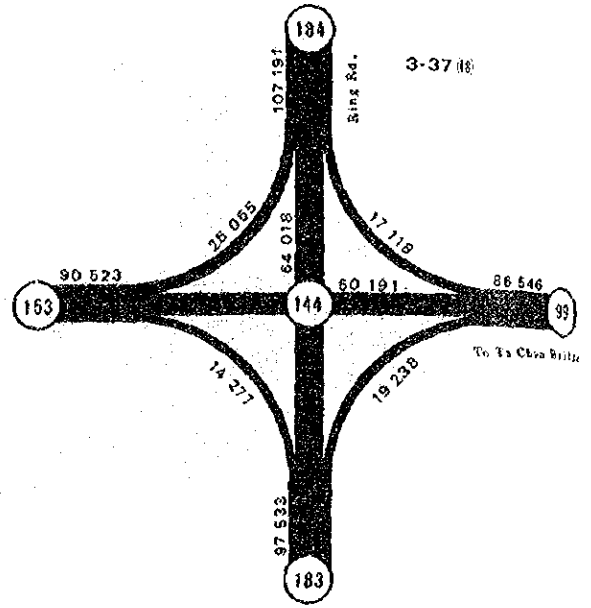
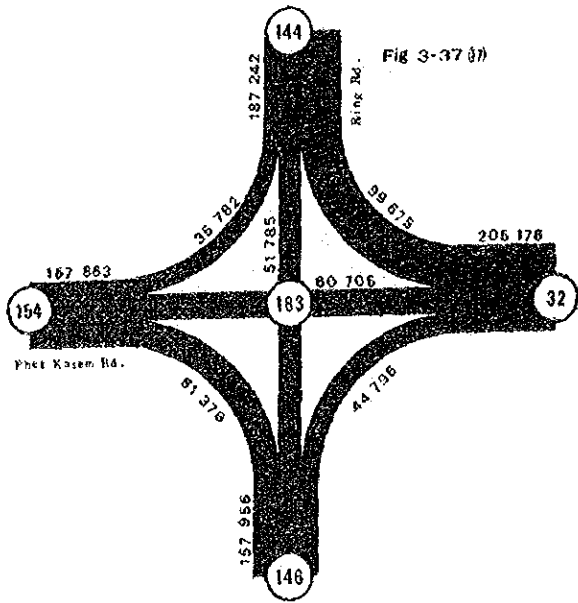
3-3705



3-3706



Estimated Intersection
 Traffic Stream, 1990
 (Optimum Network)



CHAPTER 4. ECONOMIC ANALYSIS

4-1 Introduction

4-1-1 About economic analysis

Economic analysis is necessary in new highway construction to determine the economic viability or feasibility of the project. There are many methods of analysis commonly in use. The following are a few of the most commonly adopted methods.

(1) Equivalent uniform annual cost method

This is a method whereby all investment costs and all annual expenses are combined into one single annual sum that is equivalent to all disbursement for the whole analysis period if distributed evenly over the period. In examining more than one alternatives, the one with the lowest equivalent uniform annual cost is the most economical. If the value is converted into the present worth, then the answer derived will be the same as that obtained by the present worth of costs method.

(2) Present worth of cost method

In this method, all investment costs and all annual expenses are combined into a single sum converted into present worth, representing the totals sum necessary at the initial year to finance the total disbursements over the analysis period. The one with the lowest present worth is the most economical of the alternatives compared.

(3) Equivalent uniform annual net return method

This is a method whereby an income factor or benefit factor is added to the equivalent uniform annual cost method. The result of the analysis shows the amount by which the equivalent uniform annual income is more or less than the

equivalent uniform annual cost. The alternative which has the greatest equivalent uniform annual net return is the one economically most feasible.

(4) Net present value method

In this method, the difference between the present worths of outward cash flows and inward flows of incomes or benefits is computed. It is the same in principle as the present worth of costs method, except that the factor of annual income is included. The alternative with the greatest net present values is considered economically most feasible.

(5) Benefit/cost ratio method

The benefit/cost ratio is the ratio between the equivalent annual benefit (or its present worth) and the equivalent uniform annual cost (or its present worth). An alternative with a benefit/cost ratio higher than 1.0 is economically feasible, and of the alternatives compared, the one with the greatest benefit/cost ratio has the highest economic preference.

(6) Internal rate of return method

In this method, the discount rate which will produce an equal value for both the negative costs and the positive incomes or benefits is computed. Usually, in highway projects, two or more alternatives are compared to determine the differential benefits. The higher the internal rate of return the greater the economy of the alternative is indicated.

Although there are many methods in economic analysis, no single method may be considered absolutely advantageous over the other methods as each method has its limitations

and its advantages when applied to different types of projects. For this reason, in many cases, more than one method are adopted in order to provide more field for making a final decision.

In all the above-mentioned methods, there is a common objective of comparing the future stream of costs and incomes/benefits in such a way that for the analysis period the results will indicated the probable net return on the proposed investment, or the most economical alternative for producing the returns. It is evident therefore, that in such an analysis, the analysis of the costs and incomes/benefits for the project is necessary. A description will be made of the cost and benefits analysis to be adopted in this study in the subsequent sections.

4-1-2 The general concept of economic analysis in the study

In the economic analysis of a project within a comprehensive transportation system plan, the usual practice will be to evaluate the economic viability, feasibility, or priority vis-a-vis other projects, mutually exclusive or inclusive. This will involve the analysis of all projects under contemplation. However, such an analysis will be entirely out of the scope of this study, whereby the target is set only at the Ring Road. The short period of time allowed for the study will in any way rule out the possibility of probing into so deep an analysis. The analysis here, therefore, will involve only the evaluation of the feasibility of the Ring Road as against the case of 'without' the Ring Road.

The relative feasibility of the Ring Road project in comparison to other transportation projects will have to be left pending a study at a higher level.

The problem left for this study, will therefore boil down to only answering the question, whether it is worthwhile to implement the Ring Road project. A simple way to answer this question will be as follows.

Assume a most pessimistic case, whereby the minimum of benefits is taken into consideration, and the investment of the whole route for the complete cross section of the road carried out right from the beginning. (This is usually an uneconomical way of investment.) On evaluation, if it is found that such an investments pays (or is feasible), then any accruement of unaccounted-for benefits, and any savings on the method of investment will further enhanced the feasibility of the project. This method is adopted to ascertain that the Ring Road project is feasible even under the most pessimistic conditions, and then the evaluation for a suggested more realistic schedule of investment also made.

4-2 Cost Analysis

In the economic analysis of a new highway project, the cost involved may be largely classified into the following three categories:

- i) Initial investment costs
- ii) Maintenance and management expenses
- iii) Road users costs

Full description of the above items will be offered in the ensuing sections.

(1) Initial investment cost

The main items in this category are the costs involved in the construction of the proposed road. They are the cost of land acquisition, the cost of construction, the other miscellaneous cost related to the implementation of the project. When any reinvestments are planned during the analysis period, the costs of reinvestment are also taken into consideration.

The costs in this category are directly related to the engineering aspects of the project, and are usually computed during the cost of the engineering study of the project. In this study, the engineering aspects of the proposed road discussion are made in details in Part II of this report. The cost estimated in the engineering study are then adopted in the economic evaluation.

(2) Maintenance and management expenses

This item includes the disbursements involved annually during the analysis period to keep the highway in good operating condition.

They are the costs of repairs, improvements, maintenance, management of the road during the analysis period.

In the case of a toll road, the direct and indirect costs

involved in the operation of the toll system are also included. In this study, the expenses are estimated in the engineering study and the results adopted in economic analysis.

(3) Road users costs

These are the costs involved by the road users in the course of using the new road. The major items are the vehicle operating costs and the time value of the vehicles and the passengers. However, these items are seldom taken up independently as the costs of the project. Rather, comparisons are made between these cost involved in the case 'with' and the case 'without' the proposed project, and the saving in costs computed as benefits of the new project.

The operating cost of a vehicle is composed of many items. These items may be roughly classified under the following five headings:

- (a) Distance elements: The body, the mechanical and the electrical components, consumption of gasoline and oils, tyres, depreciation and others.
- (b) Time elements: Accidents, depreciation, insurance, licence, interest, parking and others.
- (c) Ownership elements: Garage, property tax, vestcharge and others.
- (d) Commercial elements: Operators wages and benefits, overheads, business taxes and others.
- (e) Value of travel time of vehicle occupants

These elements vary in cost according to the conditions of the country, the prices of the component items, the rate of duty and taxes of the country, as well as the alignment,

road surface condition, gradient and other geometric conditions of the road being used. Careful detailed analyses are necessary to establish the per unit operating cost of a vehicle in a district or a road under study.

The consulting engineers T. P. O. Sullivan & Partner had, in early 1971, carried out a very comprehensive analysis of the operating cost of vehicles in Thailand, and the results are presented in the report "Road User Costs in Thailand".

In this report the road user costs at different travelling speed were analysed by vehicle types and class of roads.

The vehicle type and the road class classifications are as follows:

(a) Vehicle type:

- i) Car
- ii) Light Bus
- iii) Light truck
- iv) Heavy bus
- v) Heavy truck

(b) Class of road:

- i) First class road: well paved roads
- ii) Second class road: well engineered and maintained laterite road.
- iii) Third class road: Earth road and inferior laterite road.

As the report was based on data for rural roads, modifications have to be made for the application of the results to urban road networks. In a subsequent report on "Memorial Bridge" also by the said T. P. O Sullivan & Partners, the consultants had duly made modifications to establish the costs applicable to Bangkok.

The study made was very detail, and it was concluded that the findings could be adopted to this study without qualification. However, the classification of vehicles in this study is different from that of the said cost analysis, and the findings had to be modified to suit the purpose of this study.

The final vehicle sperating costs as adopted in this study are shown in Table 4-1, and Fig. 4-1 is a graphical representation of the same.

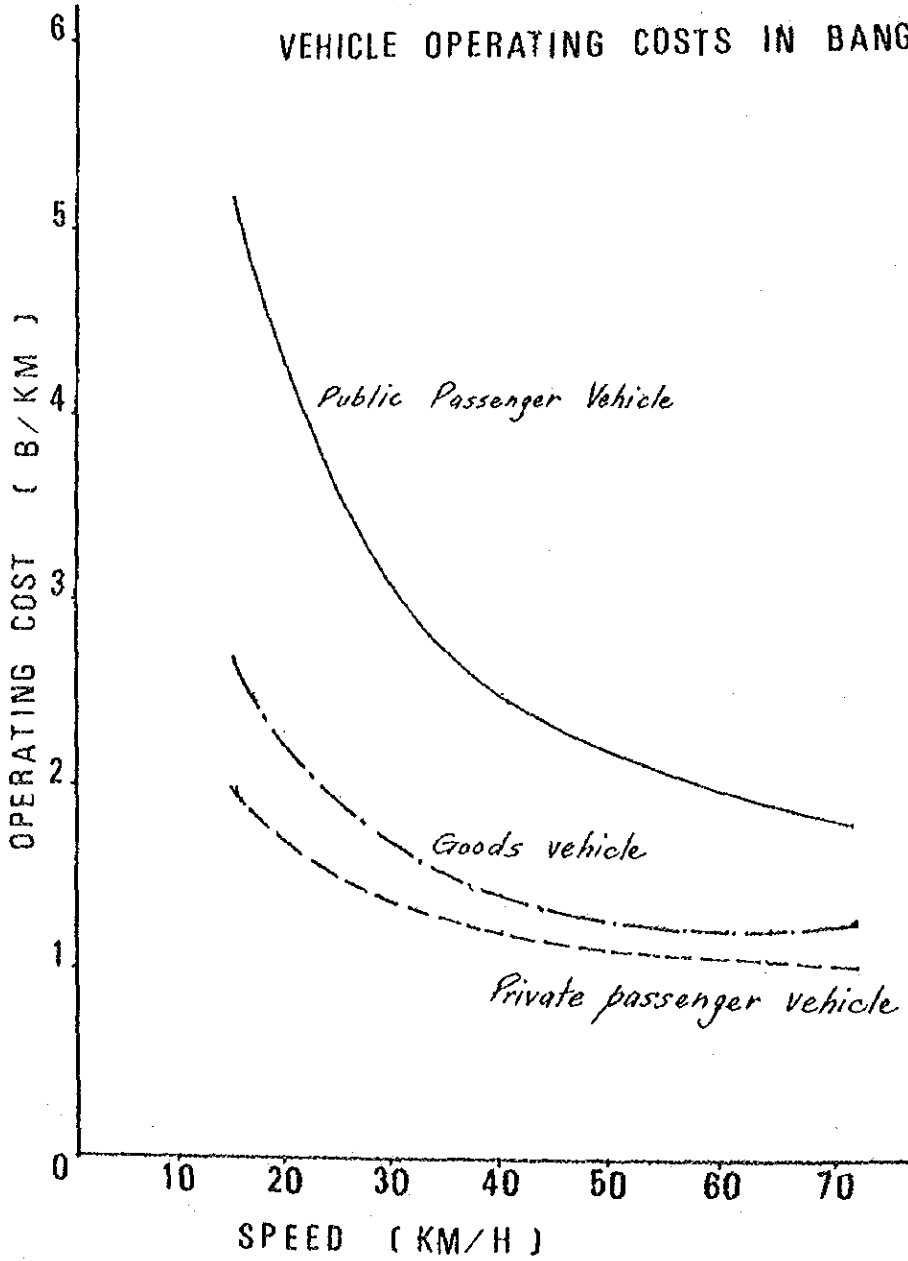
Table 4-1 1970 vehicle operating costs in Bangkok
with leisure time unvalued

| Speed Km. p. h. | *Operating cost in Baht/Km | | |
|--------------------|--------------------------------------------|-------------------------------------------|--------------------------------|
| | Vehicle Type | | |
| | Private passenger vehicle (Weighted) | Public passenger vehicle (Weighted) | Goods vehicle (Weighted) |
| 16 | 1.953 | 5.101 | 2.676 |
| 20 | 1.713 | 4.257 | 2.286 |
| 25 | 1.520 | 3.560 | 1.946 |
| 30 | 1.386 | 3.082 | 1.704 |
| 36 | 1.275 | 2.678 | 1.509 |
| 40 | 1.217 | 2.475 | 1.415 |
| 64 | 1.079 | 1.889 | 1.225 |
| 72 | 1.064 | 1.803 | 1.235 |

* Passenger time values included.

FIG 4-1

VEHICLE OPERATING COSTS IN BANGKOK



4-3 Benefit Analysis

The completion of a new transportation project will bring about various economic benefits to the community, some readily quantifiable, others so very interrelated to their factors of economic activities that the methods for quantification will be highly sophisticated and the credibility of the outcomes dubious. In this study, following the general concept of evaluating the 'minimum' benefits, only the factors which are easily quantifiable will be computed, while other economic benefits narrated qualitatively.

4-3-1 Computation of quantifiable benefits

The most readily quantifiable item of all benefits will be the saving of the road users cost, which includes both the operating cost of the vehicles and the time value of the occupants. This comes from two general streams. The one is the saving of time and cost by the vehicles diverted to the new highway as compared to the conventional route, and this is usually referred to as the direct benefit. The other is the saving of time and cost by the remaining vehicles on the existing road network, which, through the diversion of part of the traffic to the new route, enjoys some relieve in traffic congestion, and will be able to provide traffic with a better service. This is usually termed the indirect benefit. In this study, the total saving at different year of all traffic on the whole network, covering both the direct at the indirect benefit, can be computed on one calculation by comparing the total road users cost 'with' and 'without' the Ring Road. This will be done through multiplication of the total vehicle-kilometer in each case with the unit road user cost for the

respective case. Table 4-2 shows the total travel time and travel distance of the whole network in various years under different conditions and Table 4-3 summarizes the benefits of these cases.

The annual benefit derived from the Ring Road in 1975, if the whole route were completed in that year, would have come to 425.0 million bahts. The same for 1980 and 1990 (normal network) are respectively 630.8 million and 898.2 bahts. This represents an increase to 1.48 and 2.11 times the benefit in 1975. The increase in traffic volume, as compared to 1975, are estimated at 1.41 and 2.13 times for 1980 and 1990 respectively. The growth rate in benefit is almost parallel to that in traffic volume. This shows that the increase in traffic volume is the main factor in the increase in benefits, and that the deterioration in traffic congestion is but a minor factor, if any contribution is made at all.

The benefits for the intermediate years and subsequent years are derived through linear interpolation and extrapolation, and the year by year benefits are shown in Table 4-4 and Fig. 4-2.

One of the possible programmes for stage construction is discussed in Chapter 7, whereby only a portion of the Ring Road will be available for traffic at the initial stage. In this case, the benefit in 1975 will have been reduced to 118.8 million. A table of annual benefits according to the stage construction programme, which calls for the opening of the first stage in 1977, will be as shown in Table 4-5, and the graphical presentation as in Fig. 4-3.

Table 4-2 Summary of total daily travel time & travel distance in whole network

| Year | Road Network | Total Travel Time (vehicle-minute) | | | | Total Travel Distance (vehicle kilometer) | | | |
|------|---------------------------------|------------------------------------|--------------------------|---------------|------------|-------------------------------------------|--------------------------|---------------|------------|
| | | Vehicle Type | | | | Vehicle Type | | | |
| | | Private Passenger Vehicle | Public Passenger Vehicle | Goods Vehicle | Total | Private Passenger Vehicle | Public Passenger Vehicle | Goods Vehicle | Total |
| 1975 | Without Ring Road | 12,031,611 | 7,447,021 | 4,915,767 | 24,394,399 | 10,635,262 | 6,442,567 | 5,506,268 | 22,584,097 |
| | With Ring Road | 10,987,804 | 6,732,209 | 4,634,676 | 22,354,689 | 10,595,440 | 6,425,555 | 5,510,868 | 22,531,863 |
| 1980 | Without R. R. | 16,871,332 | 9,993,619 | 6,711,406 | 33,576,357 | 14,472,187 | 8,356,261 | 7,393,494 | 30,222,397 |
| | With R. R. | 15,163,346 | 8,883,196 | 6,346,824 | 30,393,366 | 14,475,410 | 8,369,098 | 7,374,891 | 30,219,399 |
| 1990 | Without R. R. (optimum network) | 29,190,454 | 15,383,922 | 11,199,027 | 55,773,403 | 23,270,565 | 11,950,587 | 11,132,808 | 46,353,960 |
| | With R. R. (optimum network) | 26,336,066 | 13,743,973 | 10,659,542 | 50,739,581 | 22,689,535 | 11,880,891 | 11,077,439 | 45,647,865 |
| | Without R. R. (normal network) | 32,277,331 | 16,891,052 | 11,868,578 | 61,036,961 | 22,916,740 | 11,934,640 | 11,115,702 | 45,967,082 |
| | With R. R. (normal network) | 28,926,131 | 14,950,720 | 11,209,696 | 55,086,547 | 22,962,210 | 11,966,480 | 11,107,642 | 46,036,332 |

Table 4-3 Benefits through saving in vehicle operating cost

| Year | Without Ring Road | | | | With Ring Road | | | | Benefits | | |
|-------------------------|---------------------------------|--------------------------|---------------|------------|---------------------------|--------------------------|---------------|------------|------------|-------------------|---------|
| | Private passenger vehicle | Public passenger vehicle | Goods vehicle | Total | Private passenger vehicle | Public passenger vehicle | Goods vehicle | Total | Baht/day | Million baht/year | |
| 1975 | Unit operating cost (Baht/km) | 1.142 | 2.184 | 1.229 | --- | 1.115 | 2.055 | 1.234 | --- | | |
| | Total operating cost (Baht/day) | 12,145,469 | 14,070,566 | 6,767,203 | 32,983,238 | 11,813,916 | 13,204,515 | 6,800,411 | 31,818,842 | 1,164,395 | 425.0 |
| 1980 | Unit cost | 1.151 | 2.226 | 1.228 | --- | 1.118 | 2.072 | 1.232 | --- | | |
| | Total cost | 16,657,487 | 18,601,037 | 9,079,769 | 44,338,293 | 16,183,508 | 17,340,771 | 9,085,866 | 42,610,145 | 1,728,148 | 630.8 |
| 1990 (optimum net-work) | Unit cost | 1.172 | 2.314 | 1.260 | --- | 1.150 | 2.184 | 1.238 | --- | | |
| | Total cost | 27,273,102 | 27,653,658 | 14,027,338 | 68,954,098 | 26,092,965 | 25,947,866 | 13,713,869 | 65,754,700 | 3,199,398 | 1,167.8 |
| 1990 (normal net-work) | Unit cost | 1.202 | 2.416 | 1.287 | --- | 1.173 | 2.280 | 1.261 | --- | | |
| | Total cost | 27,545,921 | 28,834,090 | 14,305,908 | 70,685,920 | 26,934,672 | 27,283,574 | 14,006,737 | 68,224,983 | 2,460,937 | 898.2 |

Table 4-4 Annual benefits

| Year | Annual benefits (million bahts) |
|------|---------------------------------|
| 1975 | 425.0 |
| 76 | 466.2 |
| 77 | 507.3 |
| 78 | 548.5 |
| 79 | 589.6 |
| 80 | 630.8 |
| 81 | 657.5 |
| 82 | 684.3 |
| 83 | 711.0 |
| 84 | 737.8 |
| 85 | 764.5 |
| 86 | 791.2 |
| 87 | 818.0 |
| 88 | 844.7 |
| 89 | 871.5 |
| 90 | 898.2 |
| 91 | 924.9 |
| 92 | 951.7 |
| 93 | 978.4 |
| 94 | 1,005.2 |
| 95 | 1,031.9 |
| 96 | 1,058.6 |
| 97 | 1,085.4 |
| 98 | 1,112.1 |
| 99 | 1,138.9 |
| 2000 | 1,165.6 |

Fig 4-2 Annual Benefits

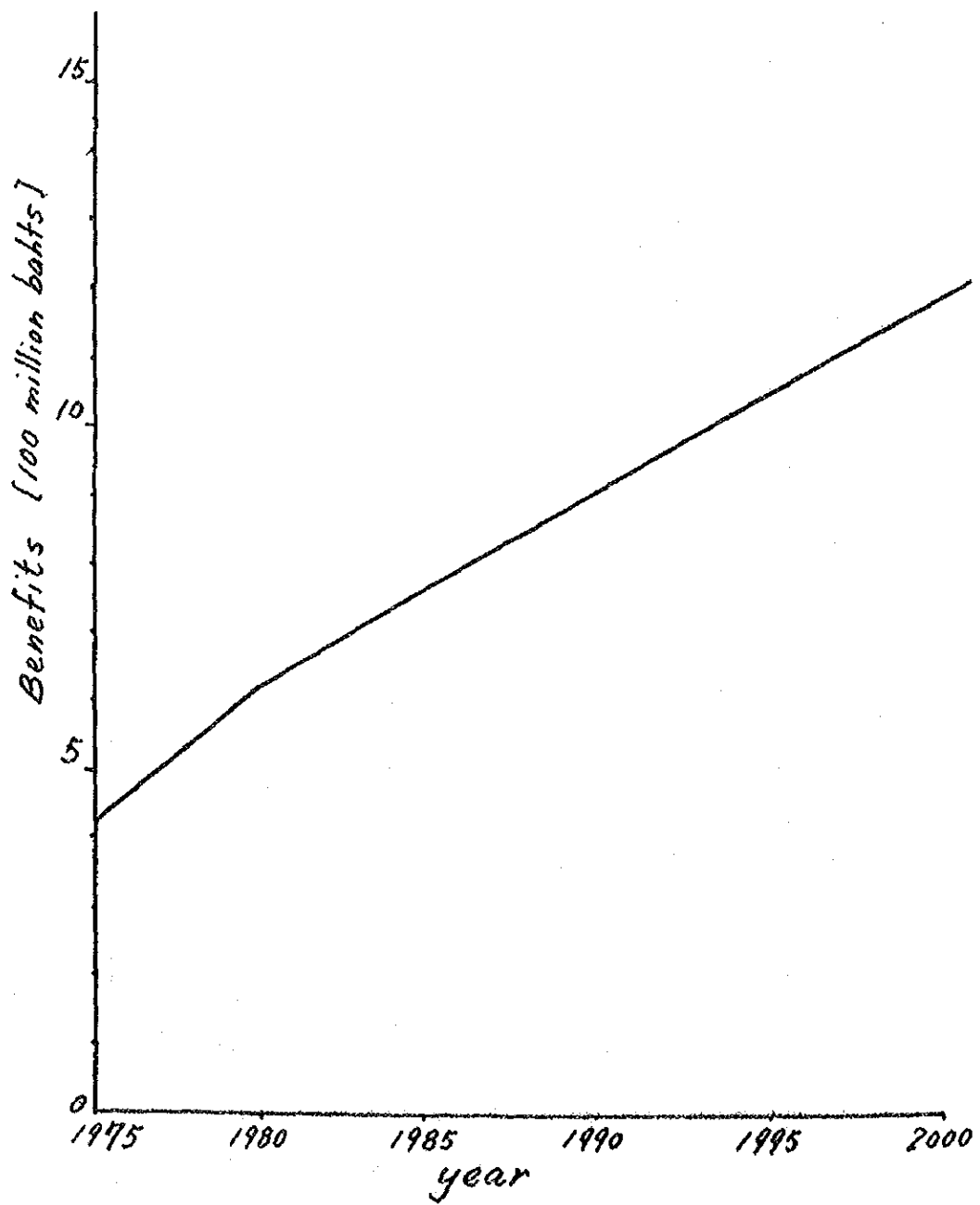
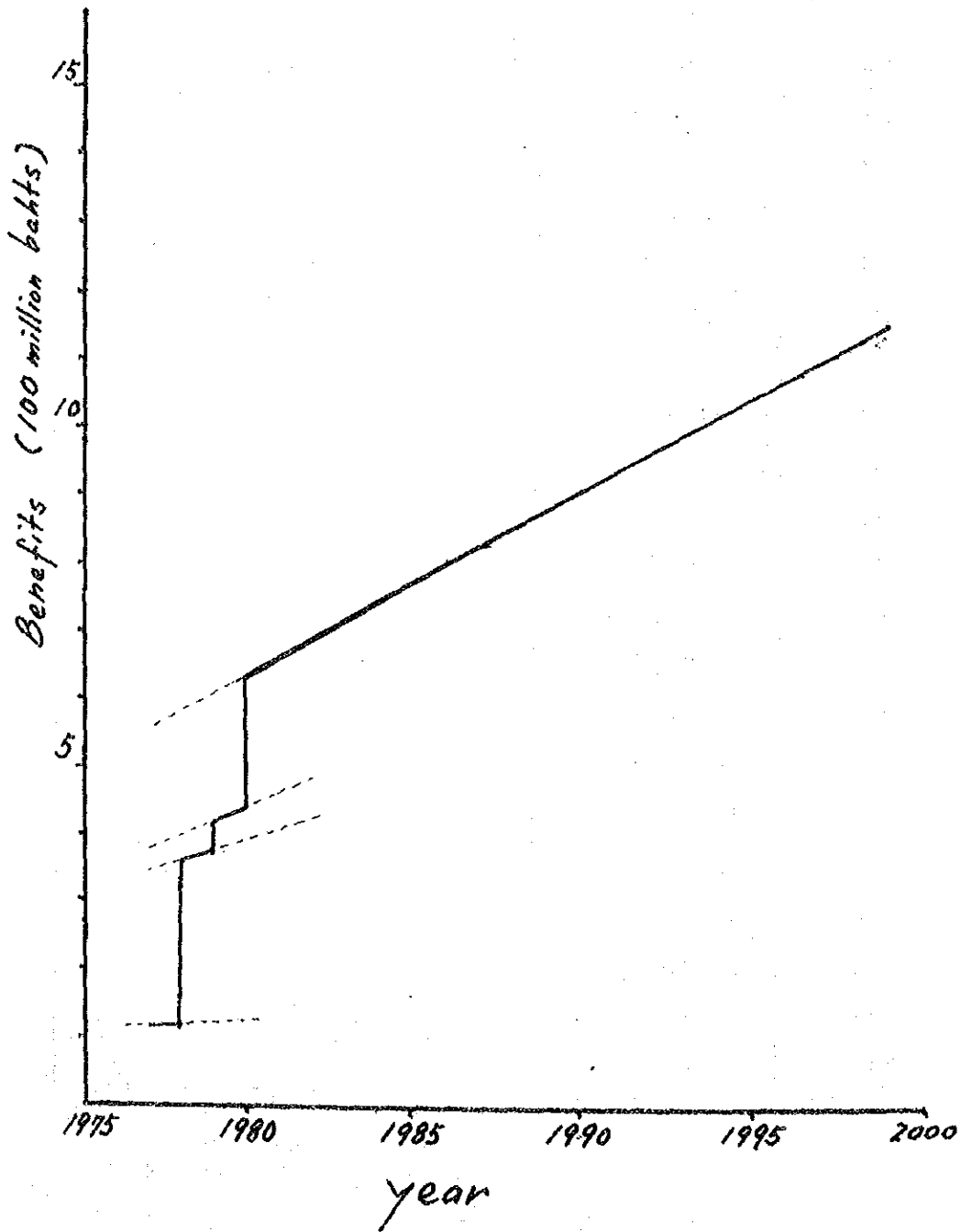


Table 4-5 Annual benefit in case of stage construction

| Year | Benefit | Remarks |
|------|---------|----------------------------|
| 1975 | -- | |
| 76 | -- | acquisition & construction |
| 77 | 120.2 | Completion of 1st section |
| 78 | 361.7 | " 2nd " |
| 79 | 421.1 | " 3rd " |
| 80 | 630.8 | Ring completed |
| 81 | 657.5 | |
| 82 | 684.3 | |
| 83 | 711.0 | |
| 84 | 737.8 | Project completed |
| 85 | 764.5 | |
| 86 | 791.2 | |
| 87 | 818.0 | |
| 88 | 844.7 | |
| 89 | 871.5 | |
| 90 | 898.2 | |
| 91 | 924.9 | |
| 92 | 951.7 | |
| 93 | 978.4 | |
| 94 | 1,005.2 | |
| 95 | 1,031.9 | |
| 96 | 1,058.6 | |
| 97 | 1,085.4 | |
| 98 | 1,112.1 | |
| 99 | 1,138.9 | |
| 2000 | 1,165.6 | |

Fig 4-3 Increase in Annual Benefits
in case of stage construction



4-3-2 Other economic effects

The completion of a new road will not only result in the benefits enjoyed by the vehicles within the road network through saving in time and cost, but will also bring about many other economic effects to the community. The quantification of these is difficult without a thorough analysis into the economic structure of the community, or without more detail data. Only brief comments on these economic effects will be made below.

(1) Intensification of land use

The building up of a city develops along transportation route, whatever the mode of the transportation.

Where land transportation facilities are not available, residence and commerce will take place along a waterway.

When a railway line is built, building up will begin centering around a railway station. When a new road is completed, development activities will sprout along the route.

A major part of the Ring Road is planned to traverse through regions where economic activities are limited to agriculture.

The completion of the Ring Road will no doubt serve as a catalyst to stimulate more intensified economic activities along the route. Residential areas, commercial areas, and industries can be expected to hang to the new road.

However, it is difficult to assess quantitatively the effects of such anticipated development, nor can the pace of development be forecast. However, an indication may be seen from the building up which followed the completion of the Mittraphap Road and the New Phetchaburi Road extension. Fig. 4-4 shows the trend of building up in Bangkok since the beginning of this century.

It is seen that within a short span of seven decades, the city area has expanded by more than 10 times, increasing at a pace of more than 3 times every thirty years. Centering around the old city centre, the expansion has been in all directions, the east being the most prominent. The two new roads of Mittraphap and New Phetchaburi came into the picture after 1958. The development along these two roads within a short span of time was most phenomenal, as can be seen from the state of building up in the 1967 map. The railway line which runs along the New Phetchaburi extension, however, forms a barrier and development along this road does not go into any depth beyond the railway line. From the figures, the lack of development on the north-eastern portion of Bangkok is most conspicuous. Despite the region's relative nearness to the city center (as compared to Don Muang, Bangna or even to Bangsu region), the lack of transportation facilities is fatal. The Lat Phrao Road is the only major trunk road within this region, and the route of Lat Phrao Road certainly does not contribute towards a rapid access to the city center. The Ring Road, which is proposed to run right through the heart of this 'desert' region in a north-south direction, will no doubt contribute greatly to the future development of the region.

(2) Effect of reduction in traffic accidents

Lack of sufficient data bars the possibility of making a comparison of the rate of vehicle accident on an expressway as against that on an ordinary roadway, and it will be rash to conclude that a road as the Ring Road will be safer than other roads. However, the appearance of the Ring Road will reduce the congestion on the whole road network, and

will, indirectly contribute to the safety on these existing roads.

(3) Rationalization of goods transportation

Heavy goods vehicles are at present curbed from entering the Municipal boundaries from six in the morning to nine in the evening. The vehicles from upcountry will have to wait outside the boundaries for the daily lifting of the restriction, resulting in undue waste of time.

The completion of the Ring Road will enable these heavy trucks to come into the city area during daylight hours without making major disturbance to the traffic of the existing road network. This will indirectly contribute to the future rationalization of goods transportation in and out of Bangkok.

(4) Others

There are as doubt many other indirect benefits as a result of the completion of the new road, since the improvement of the transportation system will not only stimulate economic activities but will also indirectly provide incentive to cultural, civic, sports and recreational activities.

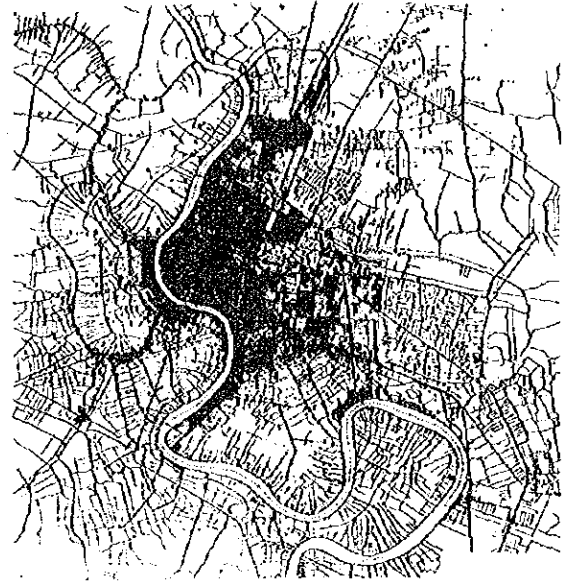
Such unquantifiable benefits to the community shall also be a major consideration in making a decision towards the implementation of the Ring Road.

Fig 4-4 Trend of Building Up in Bangkok



year:1906

Estimated area 13.3km²



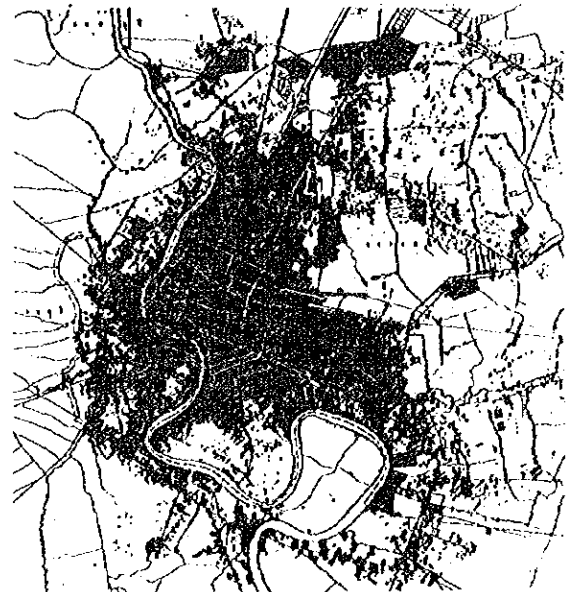
year:1936

Estimated area 43.2km²



year:1958

Estimated area 96.4km²



year:1967

Estimated area 141.5km²

4-4 Cost-Benefit Analysis

As has been mentioned before, in the cost-benefit analysis here, the sole purpose is to study the feasibility of the investment on the Ring Road as against the quantifiable benefits that may be expected of the Ring Road. Calculation will be made of both the benefit-cost ratio and the internal rate of return when the whole investment is made in the initial year, for the best alternative routes decided in Chapter 7.

(a) Construction cost

The construction costs are estimated in Chapter 7, and the summary is given below:

(i) Ring Road Part II

| | | |
|-----------------------------------------|----------------------------------|---|
| Cost of land acquisition and demolition | | |
| | 693.3 million bahts (1975 value) | |
| Cost of construction | | |
| | 2,165.0 | " |
| Total | 2,858.3 | " |

(ii) Section 5 (excluding junction with Ring Road)

| | | |
|---------------------------------------|---------------------|---|
| Cost of land acquisition & demolition | | |
| | 177.8 million bahts | |
| Cost of construction | 566.7 | " |
| Total | 744.5 | " |

(iii) Junction of Section 5 with Ring Road

| | | |
|---------------------------------------|---------------------|---|
| Cost of land acquisition & demolition | | |
| | 195.0 million bahts | |
| Cost of construction | 150.0 | " |
| Total | 345.0 | " |

(iv) Whole Project

Cost of land acquisition & demolition

1,066.1 million bahts (1975 value)

Cost of construction 2,881.7 "

Total 3,947.8 "

(b) Maintenance cost

The annual maintenance cost are estimated as follows:

(i) Ring Road Part II 10.8 million bahts/year (1975 value)

(ii) Section 5 2.8 "

(iii) Junction of Section 5 with Ring Road

1.5 "

(iv) Total 15.1 "

(c) Benefits

The benefit are as shown in Table 4-4 in the preceding section.

(d) Benefit-cost ratio

Benefits-cost ratio are calculated under the following conditions

(i) Discount rates : three cases were considered, which are 10%, 12%, and 15%

(ii) Redemption period 20 years

Under the above mentioned conditions, the accumulated discounted costs and benefits are as in table 4-6.

Basing on the above results, benefit-cost ratios and the internal rates of return are calculated and the results are as below.

Table 4-6 Annual Discounted Benefits and Maintenance Cost

Unit: million baht

| Year | Construc- tion Cost (1975 price) | Benefits | | | | Maintenance Cost | | | |
|--------------------------|-------------------------------------------|------------------|------------|---------|---------|------------------|------------|-------|-------|
| | | Current price | 1975 price | | | Current price | 1975 price | | |
| | | | 10% | 12% | 15% | | 10% | 12% | 15% |
| 1975 | 3,947.8 | 425.0 | 425.0 | 425.0 | 425.0 | 15.1 | 15.1 | 15.1 | 15.1 |
| 76 | | 466.2 | 423.8 | 416.2 | 405.4 | " | 13.7 | 13.5 | 13.1 |
| 77 | | 507.3 | 419.3 | 404.4 | 383.6 | " | 12.5 | 12.0 | 11.4 |
| 78 | | 548.5 | 412.1 | 390.4 | 360.6 | " | 11.3 | 10.7 | 10.0 |
| 79 | | 589.6 | 402.7 | 374.7 | 337.1 | " | 10.3 | 9.6 | 8.6 |
| 80 | | 630.8 | 391.7 | 357.9 | 313.6 | " | 9.4 | 8.6 | 7.5 |
| 81 | | 657.5 | 371.1 | 333.1 | 284.3 | " | 8.5 | 7.7 | 6.5 |
| 82 | | 684.3 | 351.2 | 309.5 | 257.3 | " | 7.7 | 6.8 | 5.7 |
| 83 | | 711.0 | 331.7 | 287.2 | 232.4 | " | 7.0 | 6.1 | 4.9 |
| 84 | | 737.8 | 312.9 | 266.1 | 209.7 | " | 6.4 | 5.4 | 4.3 |
| 85 | | 764.5 | 294.7 | 246.1 | 189.0 | " | 5.8 | 4.9 | 3.7 |
| 86 | | 791.2 | 277.3 | 227.5 | 170.1 | " | 5.3 | 4.3 | 3.2 |
| 87 | | 818.0 | 260.6 | 210.0 | 152.9 | " | 4.8 | 3.9 | 2.8 |
| 88 | | 844.7 | 244.7 | 193.6 | 137.3 | " | 4.4 | 3.5 | 2.5 |
| 89 | 871.5 | 229.5 | 178.3 | 123.2 | " | 4.0 | 3.1 | 2.1 | |
| 90 | 898.2 | 215.0 | 160.4 | 110.4 | " | 3.6 | 2.8 | 1.9 | |
| 91 | 924.9 | 201.3 | 150.9 | 98.8 | " | 3.3 | 2.5 | 1.6 | |
| 92 | 951.7 | 188.3 | 138.6 | 88.4 | " | 3.0 | 2.2 | 1.4 | |
| 93 | 978.4 | 176.0 | 127.2 | 9.1 | " | 2.7 | 2.0 | 1.2 | |
| 1994 | | 1,005.2 | 164.4 | 116.7 | 70.6 | 15.1 | 2.5 | 1.8 | 1.1 |
| Total for 20 years | 3,947.8 | | 6,093.3 | 5,313.8 | 4,428.8 | 302.0 | 141.3 | 126.5 | 108.6 |
| 1995 | | 1,031.9 | 53.4 | 107.0 | 63.0 | 15.1 | 2.3 | 1.6 | 0.9 |
| 96 | | 1,058.6 | 143.0 | 98.0 | 56.2 | " | 2.1 | 1.4 | 0.8 |
| 97 | | 1,085.4 | 133.3 | 89.7 | 50.1 | " | 1.9 | 1.2 | 0.7 |
| 98 | | 1,112.1 | 124.2 | 82.1 | 44.7 | " | 1.7 | 1.1 | 0.6 |
| 1999 | | 1,138.9 | 115.6 | 67.0 | 39.8 | 15.1 | 1.6 | 1.0 | 0.5 |
| Total for 25 years | 3,947.8 | | 6,762.8 | 5,757.6 | 4,682.6 | 377.5 | 150.9 | 132.8 | 112.1 |

(e) Benefit-cost ratio under stage construction

The investment of the whole 3,947.8 million baht in one year is not practical, as this will increase the budget for the highway projects by many times and will adversely affect other highway projects. Moreover, a construction work of such a scale at such a short time will require concentrated supply of man-power and materials, which will be beyond the capacity of existing supplies.

Technically, it will also be practically impossible to safely carry out such a construction work. A suggestion as one of the practical methods of stage construction is made in Chapter 7. Here the economic analysis is made in the case that the suggested stage construction schedule is implemented.

(i) Construction cost

The annual allotment of investment amount as well as the accumulated amount will be as in table 4-7.

(ii) Benefits

According to the schedule, the benefits will not be produced until 1977, when part of the Ring Road is completed. The benefits for the first several years will be considerably lower than that which available when the whole project is completed, and the accumulated benefits will be as in table 4-8.

(iii) Maintenance cost

The annual maintenance cost will be as shown in table 4-9.

(iv) The results of the economic analysis are as follows:

- a) at discount rate of 10% benefit cost ratio = 1.54
- b) at discount rate of 12% benefit cost ratio = 1.33
- c) at discount rate of 15% benefit cost ratio = 1.09
- d) Internal rate of return = 16.4%

Construction by stage, whereby the Ring Road will be partially in operation 1977, is more practical.

The feasibility of the project also remains unchanged despite the fact that the benefit for the first year will not be available until 1977, and the first year amount will be reduced to one-fourth.

Table 4-7 Annual Allotment in Construction Cost in case of Stage Construction.

unit: million baht

| No. of year after operation | Year | Annual Investment | Discounted Value | | |
|-----------------------------|-------|-------------------|------------------|---------|---------|
| | | | 10% | 12% | 15% |
| -- | 1974 | 551.6 | 551.6 | 551.6 | 551.6 |
| -- | 1975 | 610.9 | 610.9 | 610.9 | 610.9 |
| -- | 1976 | 441.5 | 401.4 | 394.2 | 383.9 |
| 1 | 77 | 518.5 | 428.5 | 413.3 | 392.1 |
| 2 | 78 | 489.4 | 367.7 | 348.3 | 321.8 |
| 3 | 79 | 353.0 | 241.1 | 224.3 | 201.8 |
| 4 | 80 | 351.0 | 217.9 | 199.2 | 174.5 |
| 5 | 81 | 363.8 | 205.4 | 184.3 | 157.3 |
| 6 | 82 | 194.0 | 99.6 | 87.7 | 72.9 |
| 7 | 83 | 75.0 | 35.0 | 30.3 | 24.5 |
| | Total | 3,948.7 | 3,159.1 | 3,044.2 | 2,891.3 |

Table 4-8 Annual Benefits in case of Stage Construction

unit: million baht

| No. of year after operation | Year | Annual Investment | Discounted Value | | |
|-----------------------------------|-------|----------------------|------------------|---------|---------|
| | | | 10% | 12% | 15% |
| 1 | 1977 | 120.2 | 99.3 | 95.8 | 90.9 |
| 2 | 78 | 361.7 | 271.8 | 257.5 | 237.8 |
| 3 | 79 | 421.1 | 287.6 | 267.6 | 240.8 |
| 4 | 80 | 630.8 | 391.7 | 357.9 | 313.6 |
| 5 | 81 | 657.5 | 371.1 | 333.1 | 284.3 |
| 6 | 82 | 684.3 | 351.2 | 309.5 | 257.3 |
| 7 | 83 | 711.0 | 331.7 | 287.2 | 232.4 |
| 8 | 84 | 737.8 | 312.9 | 266.1 | 209.7 |
| 9 | 85 | 764.5 | 294.7 | 246.1 | 189.0 |
| 10 | 86 | 791.2 | 277.3 | 227.5 | 170.1 |
| 11 | 87 | 818.0 | 260.6 | 210.0 | 152.9 |
| 12 | 88 | 844.7 | 244.7 | 193.6 | 137.3 |
| 13 | 89 | 871.5 | 229.5 | 178.3 | 123.2 |
| 14 | 90 | 898.2 | 215.0 | 160.4 | 110.4 |
| 15 | 91 | 924.9 | 201.3 | 150.9 | 98.8 |
| 16 | 92 | 951.7 | 188.3 | 138.6 | 88.4 |
| 17 | 93 | 978.4 | 176.0 | 127.2 | 79.1 |
| 18 | 94 | 1,005.2 | 164.4 | 116.7 | 70.6 |
| 19 | 95 | 1,031.9 | 153.4 | 107.0 | 63.0 |
| 20 | 96 | 1,058.6 | 143.0 | 98.0 | 56.2 |
| | Total | | 4,965.5 | 4,129.0 | 3,205.8 |

Table 4-9 Annual Maintenance Cost in case of Stage Construction

unit: million baht

| No. of years after operation | Year | Annual Maintenance Cost | Discounted Value | | |
|------------------------------|-------|-------------------------|------------------|------|------|
| | | | 10% | 12% | 15% |
| 0 | 1976 | 1.4 | 1.3 | 1.2 | 1.2 |
| 1 | 77 | 3.7 | 3.1 | 2.9 | 2.8 |
| 2 | 78 | 6.2 | 4.7 | 4.4 | 4.1 |
| 3 | 79 | 8.7 | 5.9 | 5.5 | 5.0 |
| 4 | 80 | 10.4 | 6.5 | 5.9 | 5.2 |
| 5 | 81 | 12.2 | 6.9 | 6.2 | 5.3 |
| 6 | 82 | 13.0 | 6.7 | 5.9 | 4.9 |
| 7 | 83 | 14.4 | 6.7 | 5.8 | 4.7 |
| 8 | 84 | 15.1 | 6.4 | 5.4 | 4.3 |
| 9 | 85 | " | 5.8 | 4.9 | 3.7 |
| 10 | 86 | " | 5.3 | 4.3 | 3.2 |
| 11 | 87 | " | 4.8 | 3.9 | 2.8 |
| 12 | 88 | " | 4.4 | 3.5 | 2.5 |
| 13 | 89 | " | 4.0 | 3.1 | 2.1 |
| 14 | 90 | " | 3.6 | 2.8 | 1.9 |
| 15 | 91 | " | 3.3 | 2.5 | 1.6 |
| 16 | 92 | " | 3.0 | 2.2 | 1.4 |
| 17 | 93 | " | 2.7 | 2.0 | 1.2 |
| 18 | 94 | " | 2.5 | 1.8 | 1.1 |
| 19 | 95 | " | 2.3 | 1.6 | 0.9 |
| 20 | 96 | " | 2.1 | 1.4 | 0.8 |
| | Total | | 92.0 | 77.2 | 60.7 |

PART 2

ENGINEERING STUDY

CHAPTER 5. SOILS INVESTIGATIONS

5-1 Introduction

This chapter presents the results of a preliminary study for general subsurface conditions, foundations and pavement for the proposed Ring Road Project, Part II, in Bangkok, Thailand. This study consists of literature survey, interviews of engineers in this area and field reconnaissance, but includes no exploratory borings nor laboratory testing specifically conducted for this project.

In the Bangkok area a wealth of information is available concerning its general geology and subsurface characteristics. The results of a comprehensive study are also available on the engineering properties of the subsurface soils, particularly of the Bangkok clay which is now well known for its low strength and high compressibility.

Much of the information useful for this preliminary study was derived from published data and incorporated in this report. A bibliography relative to this subject is compiled and included in the appendices.

5-2 Geology

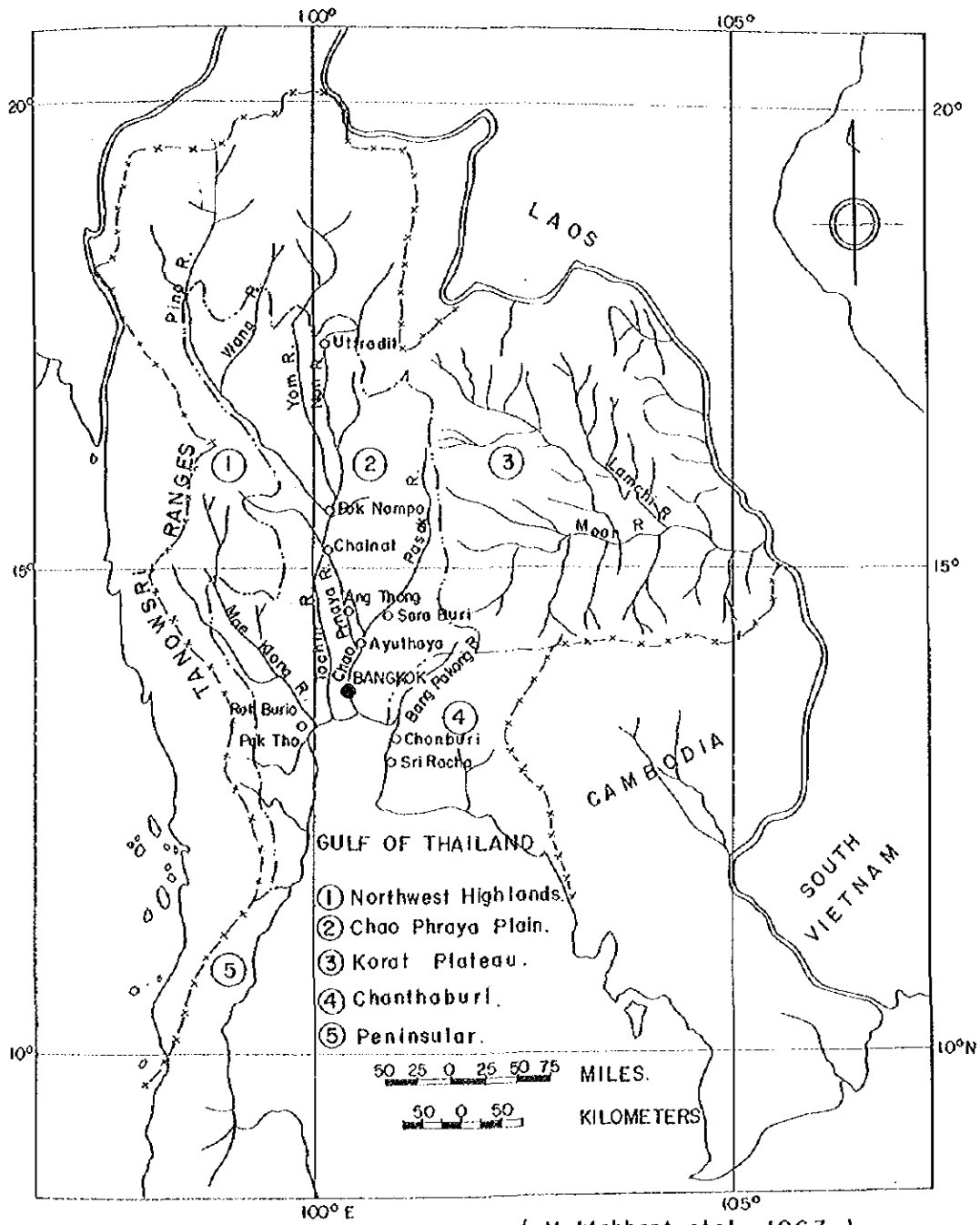
Bangkok is located on the bank of Chao Phraya River, some 20 km north of its estuary to the Gulf of Thailand. This river with many tributaries is the main drainage of the Northwest Highlands and the Chao Phraya Plain, Fig. 5-1. It is a basin filled with marine and alluvial deposits, forming an extremely flat plain about 300 km long and 100 km wide, and being completely surrounded by highlands except on the south.

It is considered that the depression of the Chao Phraya Basin is the result of the late Tertiary structural movements of the earth crust. The earlier shore of the gulf could have been as far north as Uttradit, 250 km away from the present one (Muktabhant, et al 1963).

It is speculated that, in the last 20,000 years, the rate of rise in sea level generally exceeded the rate of deposition from Chao Phraya River and invasion of the sea over the land occurred until 8,000 to 4,000 years ago, during which time the marine clays were deposited. Then land building began and an extensive deltaic plain has been formed. The marine clays were subjected to wetting and drying cycles from tidal movement of approximately ± 1.0 m during emergence of the land, and then built up above mean sea level by the terrestrial deposition of silt and clay size particles in the annual flood waters (Cox, 1968).

The alluvial deposits thus accumulated gradually moved the shore southward, presently at an annual rate of 4 to 5 m and formed a very flat plain; + 18 m near Chainat at the head of the delta, +4 m at Ayuthaya and in the average + 1.80 m (1.0 to 2.0 m) in Bangkok all above the mean sea level.

The situation described as above is best illustrated by geologic sections through and across Chao Phraya Delta, Figs. 5-2 and 5-3.



(Muktabhant et al, 1967)

FIG. 5-1 Physiographic Provinces of Thailand

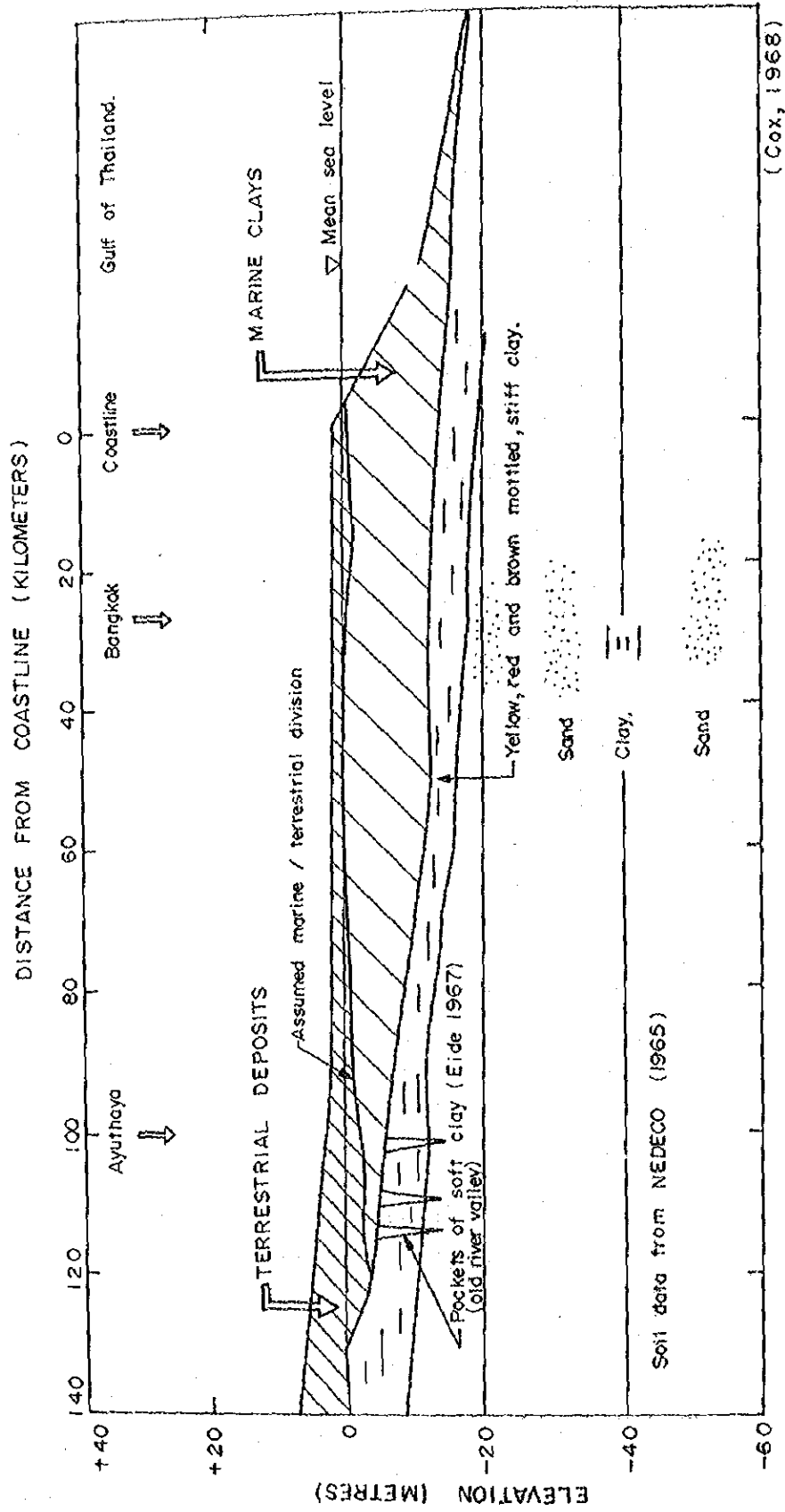


FIG. 5-2 CROSS SECTION THROUGH CHAO PHRAYA DELTA, THAILAND.

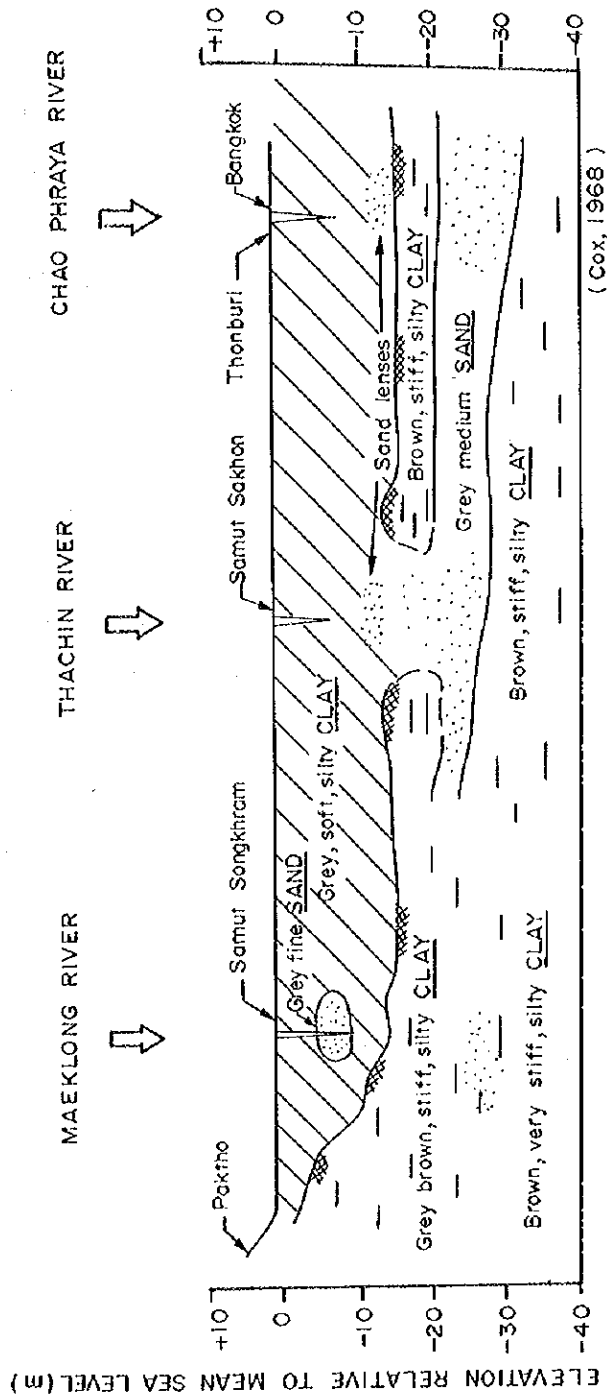


FIG. 5-3 CROSS SECTION ACROSS CHAO PHRAYA DELTA, THAILAND.

5-3 Subsurface Conditions

The general subsurface conditions of Bangkok area are illustrated typically by three soil profiles, each covering the distances of 6 to 17 kilometers (Muktabhant, 1967). The first profile, Fig. 5-5, is along Pahol Yothin Highway, approximately north-south, while Figs. 5-6 and 5-7 show soil profiles approximately in the east-west direction. Their approximate locations are indicated in the area plan, Fig. 5-4.

For engineering purposes, the soil profile in Bangkok can generally be divided into four layers and described as follows:

- (1) A weathered crust of 2 ± 1 m, usually composed of mottled gray-brown clay having cracks due to alternate cycles of wetting and drying. In some areas, miscellaneous fill, generally clayey, is encountered. The water table is at El. + 1.0 to 1.5 m above M. S. L.
- (2) Very soft to medium dark gray clay, referred to as the soft Bangkok clay, that usually extends to El. -12 ± 2 m.
- (3) Stiff to hard, gray and yellow brown, clay of variable thickness.
- (4) Dense sand and gravel layers, with some sandy clay, that occur alternately at El. -22 ± 2 m to an indeterminate depth of at least 300 m. (Moh, et al 1969).

Near the bank of Chao Phraya River, a thick deposit of loose clayey sand may underlie the soft Bangkok clay.

In the upper 200 m of the deposits, six separate confined aquifers have been identified at depths of approximately 35, 65, 85, 115, 150 and 200 m. The ones at 85 m and 150 m are most heavily used at the present time. These aquifers consist of poorly sorted mixtures of fine sand, coarse sand and gravel, and are of great areal extent having an average thickness of about 20 m (Camp, Dresser and Mckee, 1969).

Some evidences of subsidence and the possibility of future general subsidence in the Bangkok area have been pointed out in view of extensive deep-well pumping now under way in this area (Brand, et al, 1971).

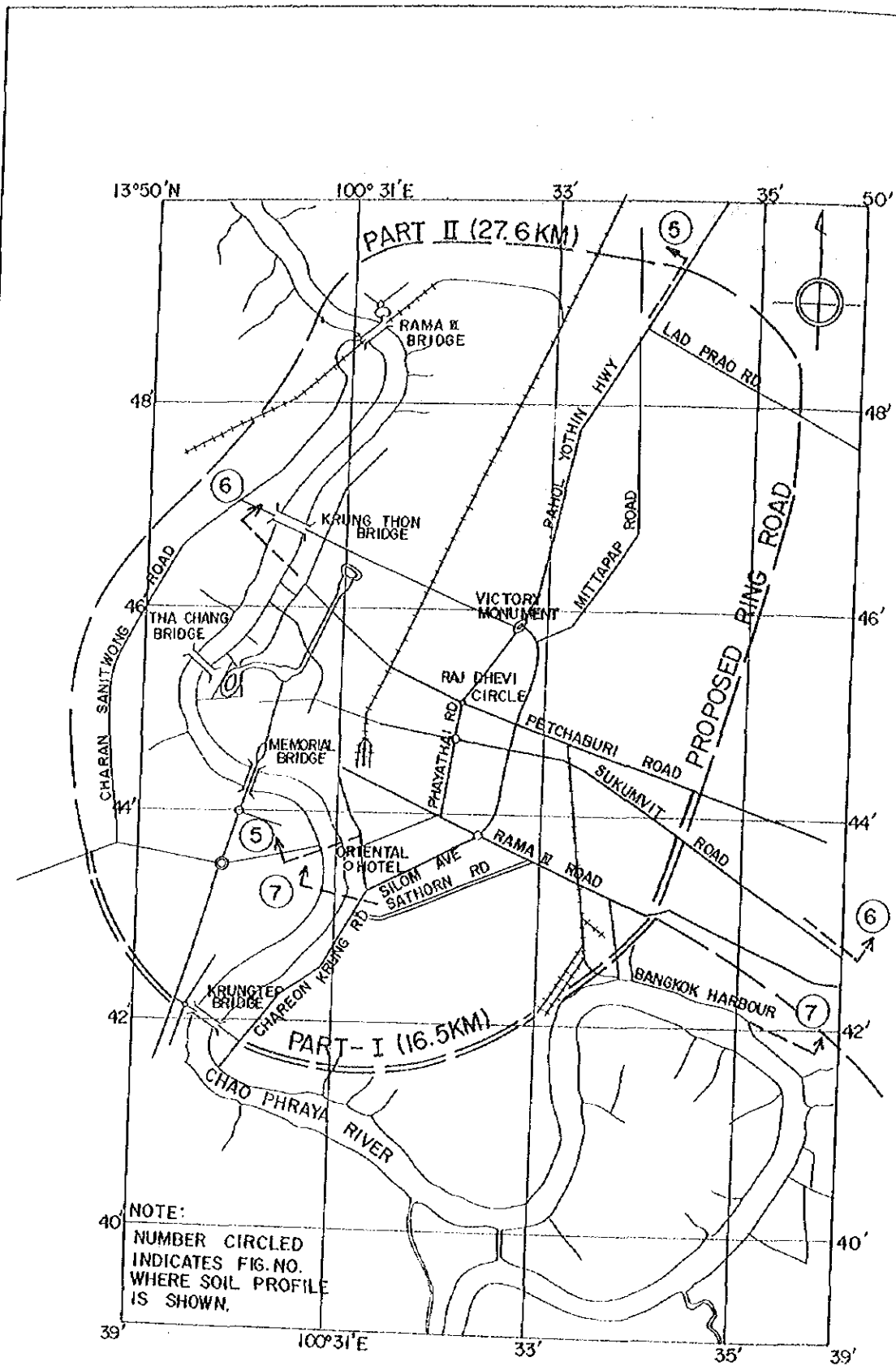
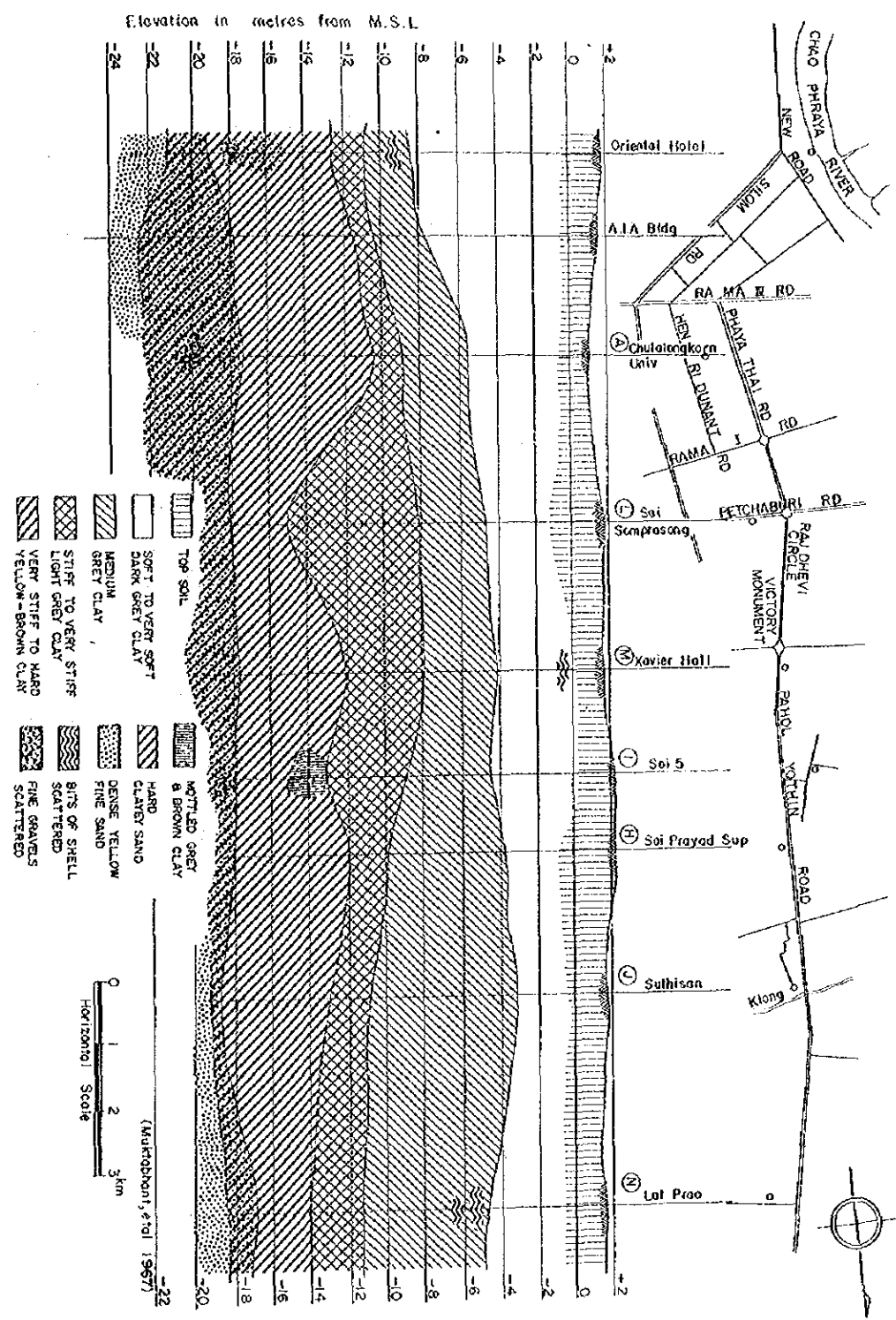


FIG. 5-4 LOCATION MAP OF BANGKOK AREA.

FIG. 5-5 SOIL PROFILE ALONG PAHOL YOTIN HIGHWAY



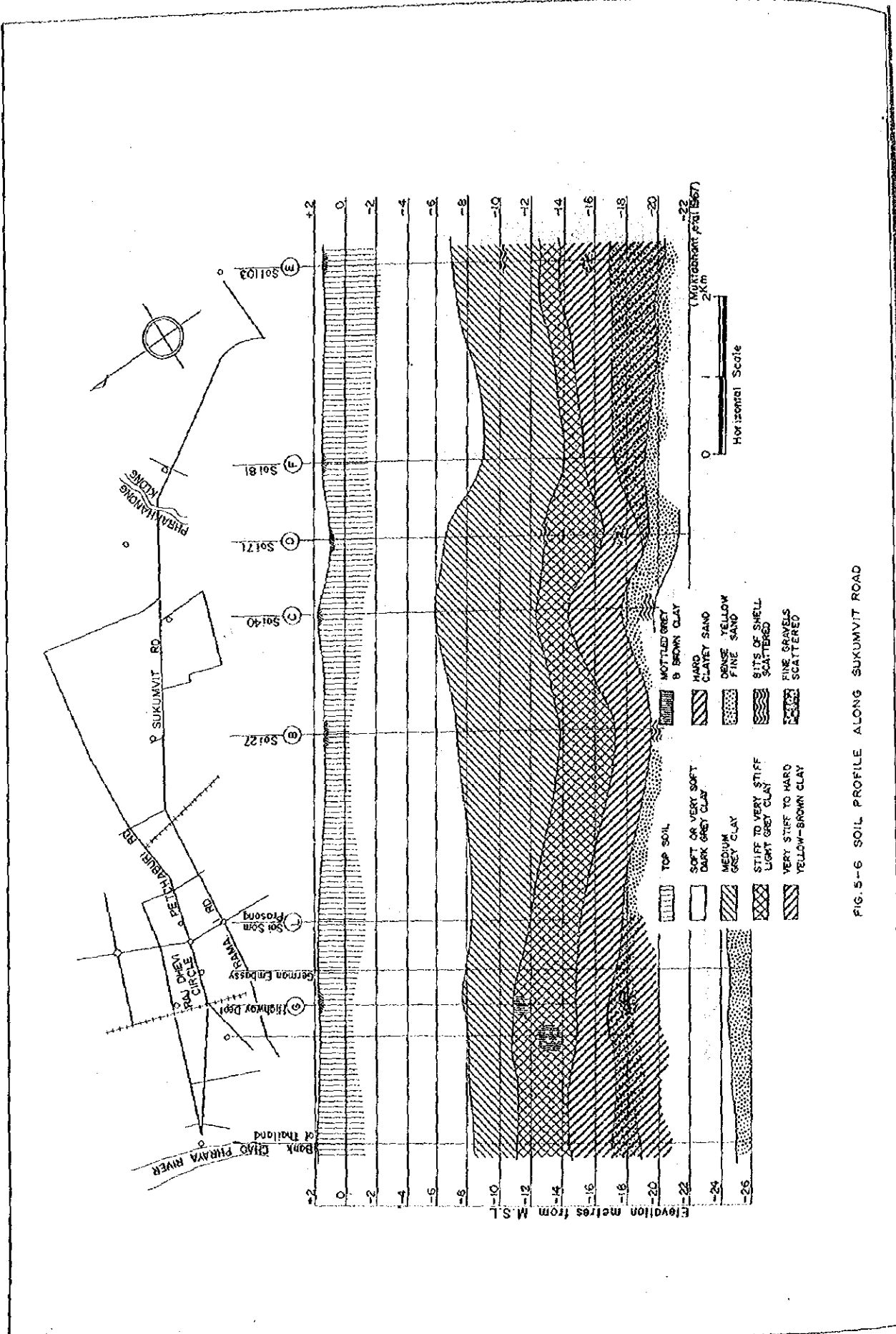


FIG. 5-6 SOIL PROFILE ALONG SUKUMVIT ROAD

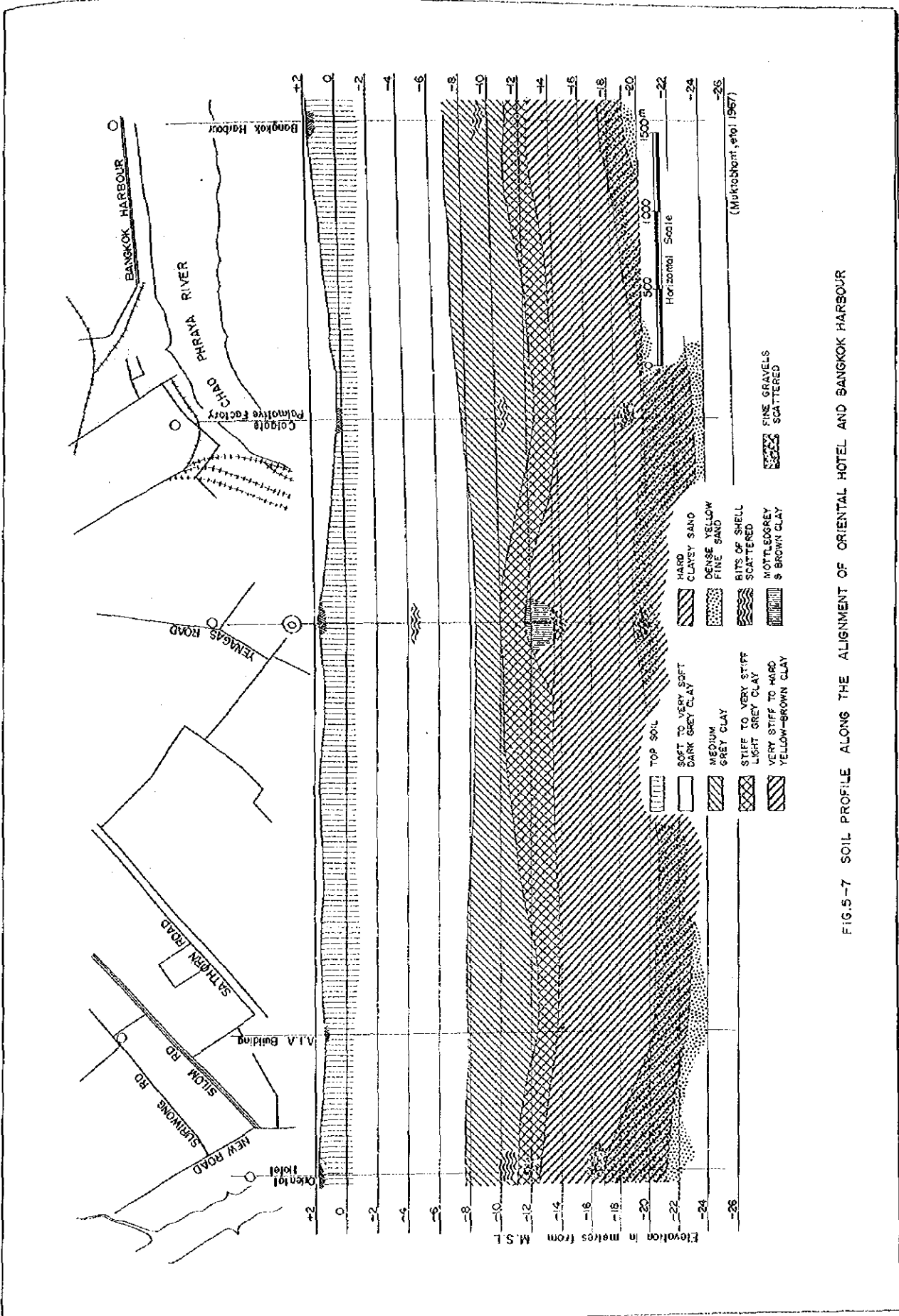


FIG.5-7 SOIL PROFILE ALONG THE ALIGNMENT OF ORIENTAL HOTEL AND BANGKOK HARBOUR

5-4 Engineering Properties of Subsurface Soils

5-4-1 Soft Bangkok clay, layers (1) and (2)

(1) Physical properties:

Bangkok clay is primarily a normally consolidated marine clay which occurs as a fairly homogeneous and isotropic deposit, but contains some fine sand and silt lenses and occasional shell fragments. Typical physical properties of this clay is summarized in Table 5-1. The clay is composed, in decreasing order of abundance, of quartz, feldspar and the clay minerals of montmorillonite, illite and kaolinite.

(2) Strength characteristics:

Some typical undrained strength data are shown versus depth in Fig. 5-8 and 5-9. Fig. 5-8 summarizes the results of unconfined compression tests and field vane shear tests from nine exploratory borings. These holes are located widely, covering the City of Bangkok fairly well. In the upper 4 to 6 m of the Bangkok clay, linear relationships between depths and undrained shear strength do not hold as generally indicated for greater depths by Figs 5-8 and 5-9. This weathered crust is thought to be due to desiccation, ion exchange reactions and weathering of clay minerals by leaching water. A set of shear strength parameters for Bangkok clay is shown in Table 5-2, which also includes typical strength envelopes in terms of total and effective stresses.

(3) Compressibility characteristics:

The Bangkok clay has not been subjected to a greater vertical stress than the existing overburden pressures, but appears slightly overconsolidated throughout the depth, Fig. 5-10. This overconsolidation is considered to be due to the pronounced secondary consolidation of the sediments after

deposition as the clay has high plasticity and appreciable organic contents, the rate of secondary consolidation being quite large. Some typical e - $\log p$ curves are indicated in Fig. 5-11.

The coefficient of consolidation, c_v , determined from routine oedometer tests varies from 0.5 to 2.0×10^{-4} cm^2/sec .

It appears however that, below a consolidation pressure of approximately 15 t/m^2 , the coefficient is primarily governed by the stress history of the clay, being higher in the range of 5 to 18×10^{-4} m^2/sec .

5-4-2 Stiff to hard Bangkok clay, layer (3)

This zone consists of an overconsolidated clay having a maximum past pressure in the neighborhood of 45 t/m^2 . Overconsolidation of the clay has in general been attributed to desiccation. Typical physical properties of the stiff Bangkok clay is summarized also in Table 5-1. Two series of shear strength parameters are indicated below, which suggests that this clay possesses similar shear parameters as those of the overlying soft clay.

| Series | <u>Total Stress</u> | | <u>Effective Stress</u> | |
|--------|---------------------|---------------------|-------------------------|---------------------------|
| | $c(\text{tsm})$ | $\phi(\text{deg.})$ | $\bar{c}(\text{tsm})$ | $\bar{\phi}(\text{deg.})$ |
| I | 5.0 | 13.3 | 2.8 | 22.5 |
| II | 7.1 | 12.2 | 4.5 | 21.7 |

(Nelson, et al, 1970)

This zone offers fairly high resistance against driving and even the upper portion has been commonly utilized as a bearing stratum for medium sized buildings in this area.

According to the results of 15 borings (Muktabhant, 1967), the standard penetration resistance (N-values) ranges from 6 to more than 50, averaging about 25, and the unconfined compressive strength varies from 12 to 53 t/m².

5-4-3 Dense sand and gravel, layer (4)

The hard clay is underlain normally by strata of yellow dense fine sand at an elevation between -20 and -24, M. S. L. The top boundary is not always clearly defined but usually encountered in a transition stage of yellow brown clayey sand.

This zone is capable of supporting piles and caissons for large heavy structures.

Table 5-1 Typical physical properties of Bangkok Clay

| | Soft Bangkok Clay Layers (1) and (2) | Stiff Bangkok Clay Layer (3) |
|---------------------------------------|--------------------------------------|------------------------------|
| Color | Dark gray | Mottled brown and grey |
| Consistency | Very soft to Medium | Stiff |
| Natural Water Content | 50 - 88 (60 - 70) | 20 - 30 |
| Liquid Limit | 55 - 95 (75) | 53 - 65 (59) |
| Plastic Limit | 23 - 33 (28) | 21 - 24 (23) |
| Plasticity Index | 20 - 60 (47) | 32 - 42 (37) |
| Liquidity Index | 0.7 - 1.0 (0.85) | 0.1 |
| % Finer than 2 μ | 40 | 44 |
| Activity | 0.6 - 1.1 (0.8) | 0.83 |
| Soluble Salt Content (gr/liter) | 1.5 - 15 (2.5 - 10) | 5.6 |
| Organic Matter (%) | 1 - 5 (3) | 0.8 |
| Specific Gravity | 2.65 - 2.75 (2.70) | 2.74 |
| Wet Density : Above 2±1 m depth | 1.65 - 1.80 (1.74) | - |
| (t/m ²) Below 2±1 m depth | 1.45 - 1.75 (1.65) | - |
| Dry Density | 0.84 - 1.13 (1.13) | 1.61 |
| Void Ratio | 1.4 - 2.2 | (less than 1) |
| Sensitivity | 3 - 7 (5) | 1.3 |
| Main Reference | Ladd, et al (1971) | Nelson, et al (1970) |

Note: Numbers in parentheses indicate representative values.

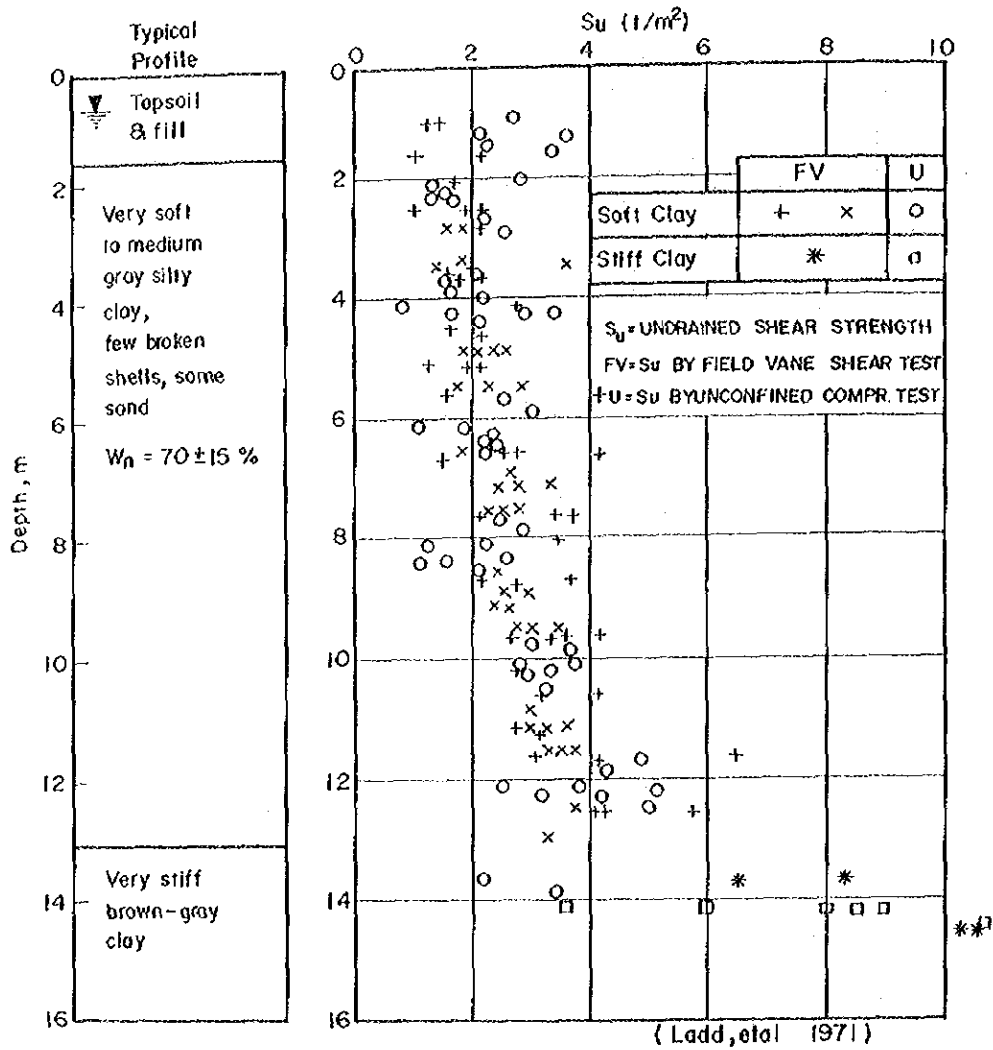


FIG. 5-8 TYPICAL FIELD VANE AND UNCONFINED STRENGTH DATA ON BANGKOK CLAY.

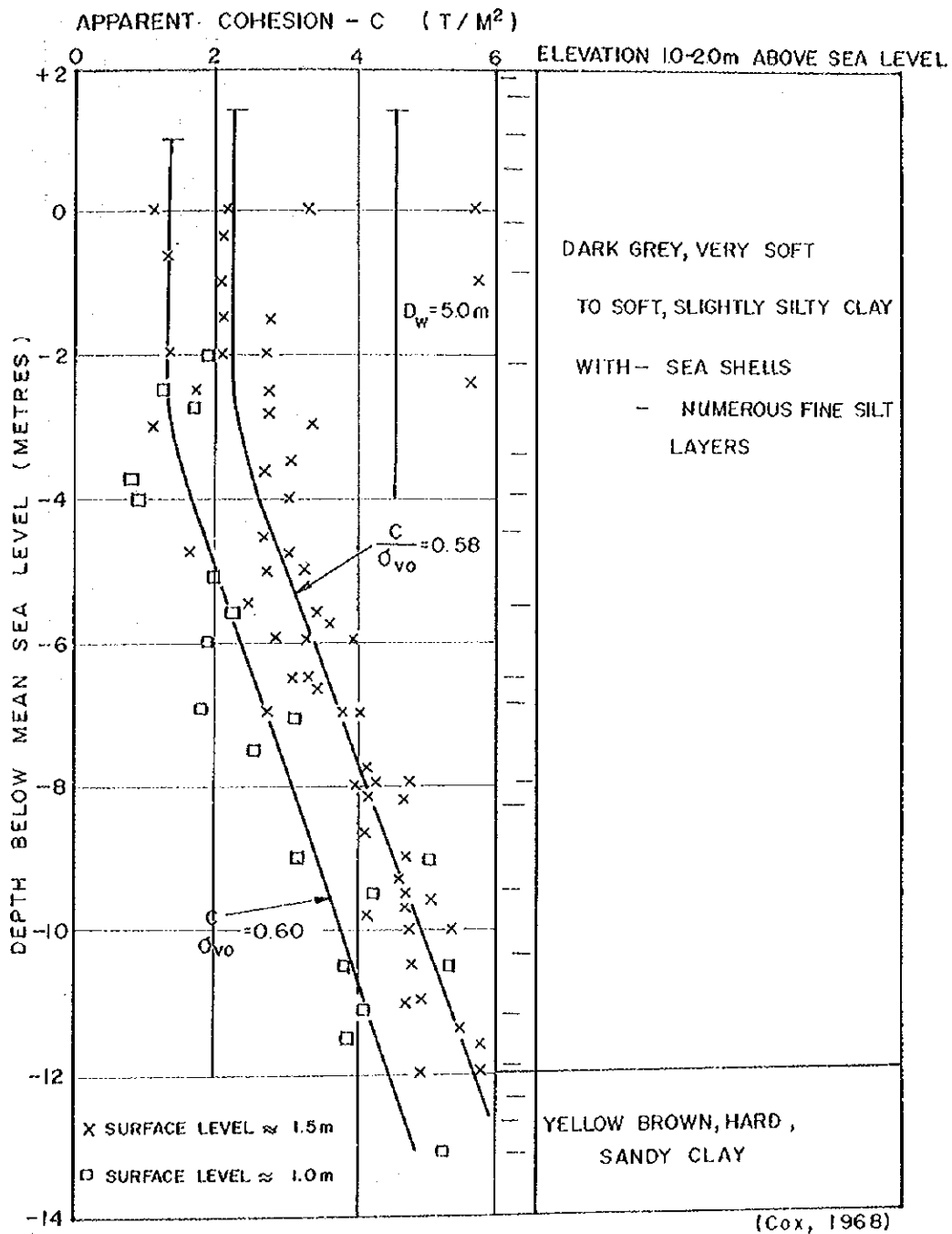
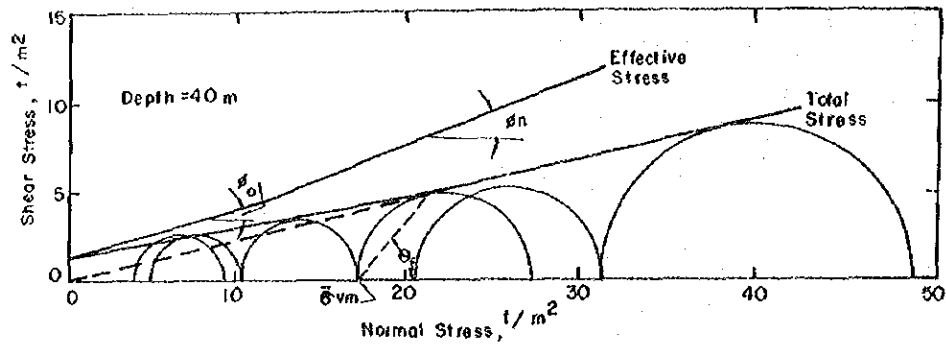


FIG. 5-9 SHEAR STRENGTH PROFILES FOR BANGKOK CLAY.

Table 5-2 Shear Strength Parameters for Bangkok Clay (Moh, et al 1969)

| Depth metres | $\bar{\sigma}_{vm}$ ton/sqm | Total Stresses | | | Effective Stresses | | |
|-----------------|--------------------------------|----------------|---------------------|---------------------|---------------------------|---------------------|---------------------|
| | | c ton/sqm | ϕ_n degrees | ϕ_o degrees | $\bar{\sigma}$ ton/sqm | ϕ_n degrees | ϕ_o degrees |
| 1.1 | 27.7 | 0.6 | 13 | - | 0.5 | 20 | - |
| 1.5 | 25.6 | 2.0 | 11 | 9 | 1.8 | 18 | 13 |
| 2.5 | 24.6 | 0.8 | 11 | 10 | 0.7 | 20 | 19 |
| 4.0 | 17.5 | 1.4 | 13 | 9 | 1.6 | 21 | 16 |
| 5.3 | 0 | 0 | 11 | - | 0 | 20 | - |
| 10.5 - 11.7 | 6.1 | 6.1 | 15 | 10 | 3.7 | 24 | 20 |



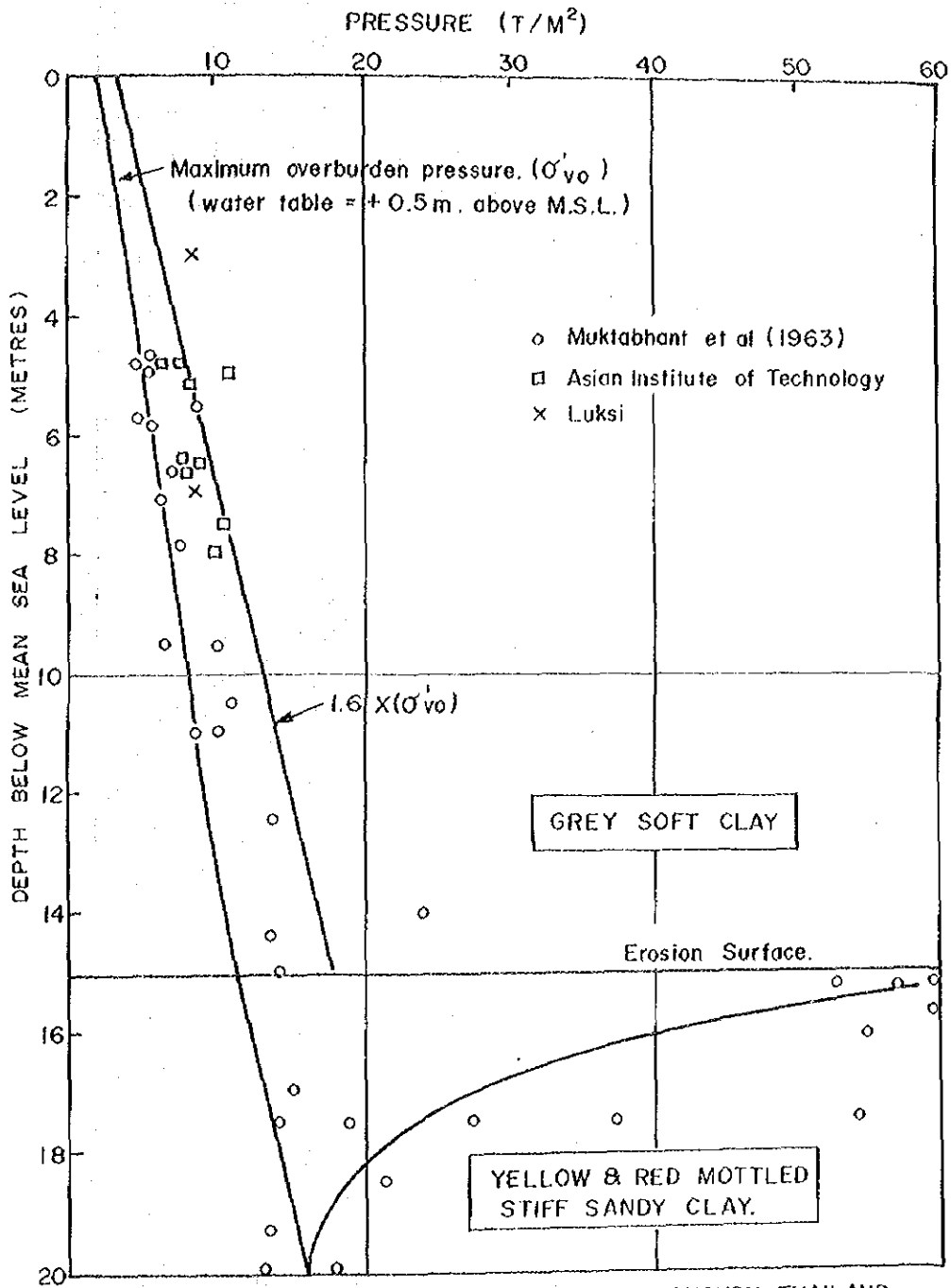


FIG. 5-10 CRITICAL PRESSURE PROFILE AT BANGKOK, THAILAND.
(Cox, 1968)

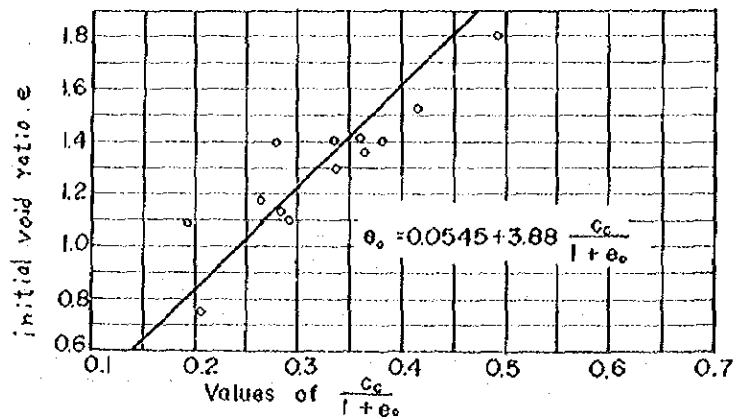
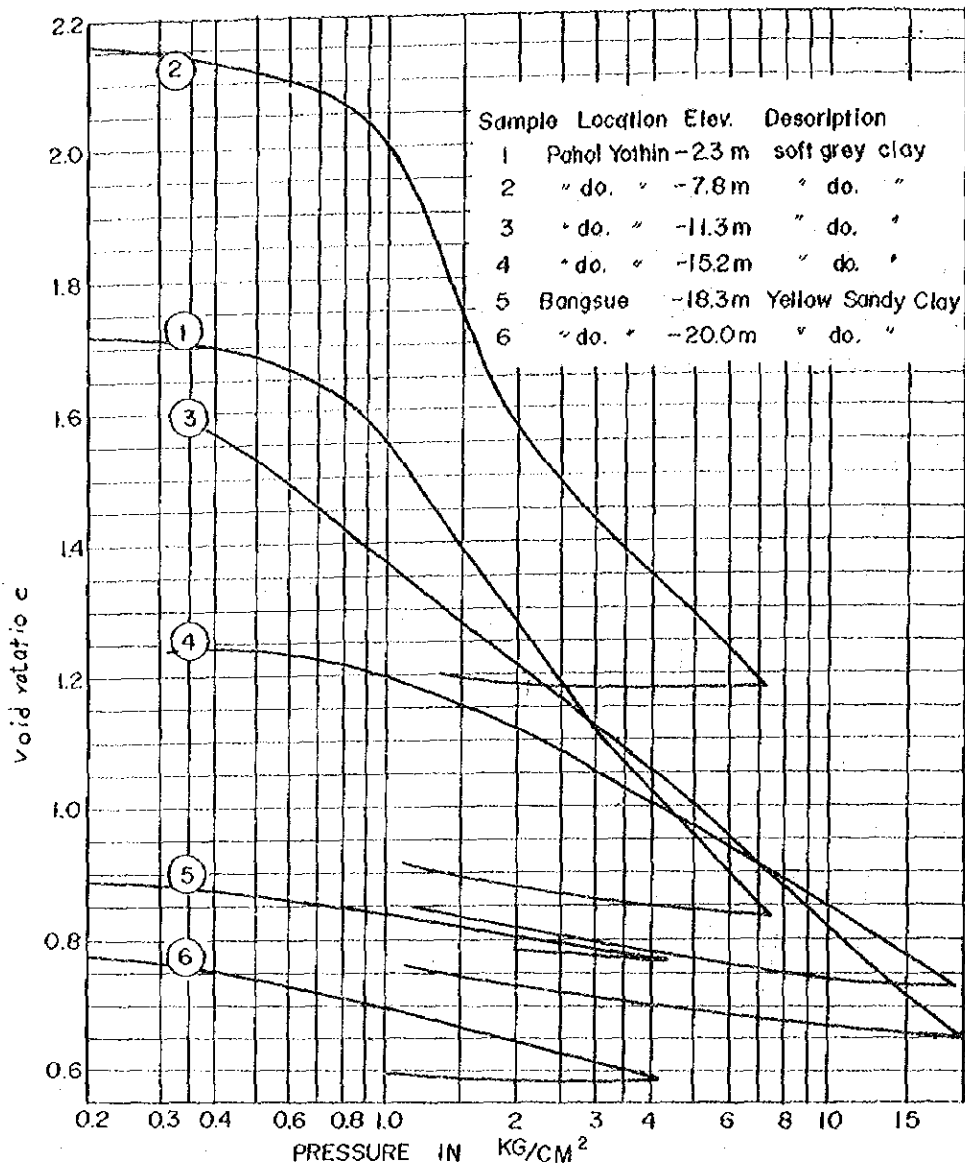


FIG.5-11. TYPICAL RESULTS OF CONSOLIDATION TESTS ON BANGKCK CLAY

5-5 Engineering Problems

5-5-1 Embankment stability

Because of the extremely soft and highly compressible nature of the Bangkok clay, it poses a serious threat to stability of even low embankments which are only a few meters in height, and makes it difficult to predict their settlement behavior on the basis of laboratory test results. Although the stability and settlement of embankments are inseparable in nature for a soft clay such as the Bangkok clay, each subject is treated separately herein.

In connection with the Bangkok-Sri Racha Highway project, Eide (1967) had analysed several failures of trial embankments and found that recalculation of these slides based on the conventional Swedish circle method employing vane shear strength yielded an average factor of safety of 1.5. A safety factor as large as 2.0 was calculated for a failure which extended from the embankment to a side borrow pit. Cox (1968) pointed out that the conventional slope analysis did not take into account horizontal forces between embankment and natural soils, and inclusion of these forces would indicate a factor of safety close to unity at failure.

Considerable lateral stresses are developed by arching in the embankment during settlement and act outwards from the center toward the toe. Fissures and slickensides noted in the Bangkok clay could also result in a lower in-situ shear strength than is measured by either laboratory compression tests or field vane shear tests.

Undrained strength analyses may, however, have a factor to make the results too conservative when appreciable consolidation takes place during construction. In the Thonburi-Paktho Highway project, vane shear tests were carried out

before and after construction in the weathered crust beneath the trial embankment. It was revealed that the vane shear strength in this crust had increased from its initial value of 1.2 t/m² to an average value of 2.2 t/m² in several months. For this project three large trial embankments were constructed with extensive instrumentation a few years before the highway construction began in 1970, Table 5-3. The embankments which were deliberately built to failure for a stability study failed when the height reached 2.6 m on a coconut plantation field and 4.2 m on a rice field (Cox, 1970). Another embankment failure was reported near Bangkok when it was built to a height as low as 1.6 m.

Brand (1971) gave an interesting account for an embankment failure which took place some 20 km north of Bangkok. Concrete piles, 15 m long and 30 by 40 cm in cross-section, were driven in an attempt to improve the marginal stability of a retaining wall for a roadway embankment located along a canal, and two days after pile driving the embankment slid into the canal together with the wall and piles. This failure was attributed to excess porewater pressures developed in the soft Bangkok clay due to pile driving.

5-5-2 Embankment settlement

Chandrangsu (1967) reports his experience in 1939 about the embankment construction of Sukhumvit Highway in the Bangkok area: the actual quantity of material dumped to form embankment of about 1 m in height was twice as much as the quantity calculated from the cross-section. He recalls similar excessive settlements observed during road construction in the vicinity of the city including the Bangkok-Don Muang Highway and pointed out that the settlements were far greater than those

due to elastic compression and consolidation of the soil layers underneath.

According to Eide (1967), a 3 m high railway embankment located 6 km south of Ayuthaya, settled 2.5m. (It was first built as a single track some 65 years ago and then expanded to a double track about 15 years ago). His investigation showed that only 1.2 m of the total settlement was due to consolidation and the remaining 1.3 m due to shear deformation in the form of plastic flow in the clay at constant water content. He also estimated the settlement during construction was 0.7 to 0.8 m. At this location it was found that the soft Bangkok clay filled an old river bed being about 14 m deep. Therefore, the condition may be considered comparable to that in the Bangkok area.

Construction difficulty was encountered in a section near Bangkok (Circeo, 1968) when low fills experienced settlements over 30% of their heights in one year period. The most critical section, only 1.5 m high, settled a total of about 1 m during the construction period of 1.5 years.

Table 5-3 summarizes the results of an extensive trial embankment program constructed for the Thonburi-Paktho Highway project (Cox, 1971). The locations of test sites are given in Fig. 5-12 and some of the settlement data are shown in Figs. 5-13 and 5-14. It is to be noted that settlements for a relatively short period are sizable and the rates of secondary consolidation very considerable.

As shown in Table 5-3, a 4.2 m high embankment settled 1.8 m for a period of 3.7 years and was only 2.4 m above the ground level as of February, 1971. It was also found that the pore pressure parameters, μ and A , depended upon the over-consolidation ratio of the silty clay beneath the embankment.

At high stress levels μ varied from 0.4 to 0.5 in the weathered crust to 0.8 below this zone. The coefficient A_f at similar stress levels varied from -0.2 to +0.6 in the weathered crust to 0.8 below this zone and correlated well with the laboratory test results for Bangkok clay (Cox, 1968).

In addition to the fact that large shear flows make settlement predictions next to impossible, presence of thin lenses of silt and sand in the soft Bangkok clay complicates the settlement rates considerably.

The actual settlements, however, do not always exceed the predicted ones. In connection with the Bangkok-Sri Racha Highway project (N. G. I., 1967), a trial embankment of 1.7 m in height was constructed near Bangkok. The settlement was calculated to be on the order of 1 m, approximately 20 cm of which should occur during the first year. As it turned out, however, the settlements actually measured, were only 7 cm in the first few months and practically no more settlements were observed for the subsequent six months. It was reasoned that the average embankment load 3.3 t/m^2 was very close to, but still too small to exceed the critical load (apparent pre-consolidation load) and the Bangkok clay behave as an over-consolidated clay, although it was normally consolidated in terms of its stress history.

All the indications are, therefore, that the values from laboratory results should not be applied directly to field cases. Forecasting settlement and its rate for the soft Bangkok clay is still at present such a complex and unsubstantiated process that it is much safer to make the use of empirical, observed time-settlement relationships on a similar subsurface condition with similar loading than to estimate them by analytical means alone. In this regard, it is highly recommended for

the proposed Ring Road project that at least one trial embankment be constructed at each representative site as early as possible and such relations be established for the most economical construction.

5-5-3 Embankment materials

Most of the roads and highways in this area have been constructed on low embankments consisting of topsoil scooped from roadside borrows within the right of way, which in turn serve as canal or ditch after completion of roadways. The current practice does not necessarily follow this traditional method, however, due to increasing cost and difficulty in land acquisition and construction requirement for better embankment material. In the Ring Road project, which is proposed in most part through highly developed urban areas, it is expected that practically all the embankment materials will have to be brought in from other sources located a considerable distance away from the project site.

The Bangkok clay at shallow depths is generally classified as A-7-5 or A-7-6 in terms of the AASHO classification system and possesses poor to fair compaction characteristics. The optimum moisture content ranges from 18 to 28%, the modified AASHO maximum density from 1.4 to 1.8 t/m³ and the CBR value from 1 to 7. However, the water content in situ is usually so high that it is difficult to attain such compaction in the field, and the surfacial soil in general must therefore be classified as poor to very poor embankment material. This highly plastic heavy clay, after compaction, may attain a maximum wet in-situ density of about 2.3 t/m³, and does not dry up to the optimum moisture content except the top 20 cm or so.

Lateritic soils and river sand are now often used for embankment construction in this area. The available data on lateritic soils from the Saraburi area indicate the modified AASHO maximum dry density of 1.94 to 2.17 t/m³ at the optimum moisture content of 4 to 12% with the 4 day-soaked values of CBR ranging from 4 to 87. Lateritic soils available in the Paktho-Ratburi-Kanchanaburi area also show good compaction characteristics; the modified AASHO maximum dry density of 2.05 to 2.30 t/m³ at the optimum moisture content of 7.6 to 10.6% with CBR varying from 15 to 94. The lateritic soils possess excellent compactability responding well to higher compaction efforts, but the test results scatter considerably even if the soils are from the same source.

River sand, fine to medium in grain size, is dredged in quantities from Chao Phraya River near Bang Pa-in, Ayuthaya and Ang Thong, and may be transported by barges to the Bangkok area. Medium to coarse sand may be found in the river bed of Mae Klong River near Ratburi and upstream of it. Typically, the maximum dry density is 1.68 t/m³ and the optimum moisture content around 17% on the average. The design CBR value of the river sand used for the Don Muang-Saraburi Highway was 15.

Although gravel is not available in this area, limestone and andesite are quarried and crushed commercially. There are a number of rock quarries within a 100 km radius of the project site; the Ratburi Highland foothill areas to the west, the Saraburi area to the north and the Cholburi area to the southeast. In testing these rocks, the percentage of wear in the L. A. abrasion tests ranges roughly between 20 and 40%.

Rice husk ash has been used as lightweight embankment material in Thailand on approach embankments of railway and

highway bridges. Most rice mills are equipped with steam engines using rice husk as fuel, and the resulting ash may be found in adequate quantities at scattered locations. Laboratory tests indicate that the rice husk ash is composed chemically of silicon dioxide (about 90%) and the dry density under repeated loading is on the order of 0.8 t/m^3 showing high frictional resistance but no cohesion (Williams, et al, 1971), and that effective compaction may also be attained by mixing clay with lime and the ash (Lazaro et al, 1971). According to the results of a field test (General Engineering Co., 1967), the ash stabilized with 6% of hydrated lime showed the maximum dry density of 0.64 t/m^3 at the optimum moisture content of 90% and it was suggested that the maximum wet density in-situ may reach 1.35 t/m^3 .

5-5-4 Deep foundations

The Bangkok clay is so soft and compressible that the building code permits no shallow foundations designed for a pressure more than 2.0 t/m^2 in the Bangkok area. In consequence, practically all structures require deep foundations which transfer the loading to the more competent, less compressible underlying bearing strata.

In addition to extensive use of timber piling, composite piles used to be popular for relatively light loading and consisted of a timber pile section driven into the clay and the reinforced concrete section above the groundwater level. For instance, steel towers for the power transmission lines in the vicinity of Bangkok rest on composite piles consisting typically of a 16 m wood section and a precast concrete section, 40 cm square and about 4 m long. These were considered as friction piles and designed for 20 to 25 tons per pile (Miller, 1963).

Reinforced concrete piles are extensively used in this area, and are typically 25 to 35 cm square and up to 26 m in length with design capacity of 20 to 45 tons per pile. In the recently completed portion of the Asian Highway which starts 52 km north of Bangkok, for example, bridge piers are supported mainly by two types of reinforced concrete piles; 35 cm square solid piles designed for 30 to 45 tons per pile, and octagonal hollow piles with the inside diameter of 38 cm and the wall thickness of 10 cm designed for 55 to 110 tons per pile (Holmberg, 1970).

Cast-in-place concrete piles and piers of various types have also been frequently used. One of the most notable cases is the Thachang Bridge which is to be completed in 1974. The main piers are supported by cast-in-place, reinforced concrete piers, 1.5 m in diameter, 45 m in length and designed for 430 tons per pier. The other bridge piers and abutments are founded on prestressed concrete piles, about 25 m in length. These piles are 40 cm octagonal and 35 cm square with the design load ranging from 40 to 60 tons per pile. The Bangkok Noi Bridge, also presently under construction, employs prestressed concrete piles, 35 cm square, 24 m long and designed for 40 tons per pile.

Caissons have been used for bridges across Chao Phraya River. The main piers of the Memorial Bridge and the Rama VI Bridge were constructed under compressed air, while those of the other two, Krung Thep and Krung Thon on open caissons (Prakobyantrakich, 1963)

Fig. 5-15 shows a representative boring log for the location of each crossing as well as its foundation level.

According to a recent investigation (O'Sullivan, 1971), the caissons of the Memorial Bridge which were constructed in 1932 were actually founded at EL. -28.7 and EL. -29.7, M. S. L.,

on the Thonburi side and the Bangkok side, respectively.

It was pointed out that only 0.4 m of river bed remains at the point of maximum scour around the base of the Thonburi pier and the scour rate could be as high as 0.5 m per year. It is to be noted that the Thonburi pier is located on the outside of the meander, resting in alternate layers of stiff silty clay and silty sand with trace of clay which indicate standard penetration resistance ranging from 20 to 30.

5-5-5 Approach embankment

In the Bangkok area, evidences of excessive differential settlements are observed at practically every connection between the bridge abutment and the approach embankment, characterized by damages of various degrees caused in sidewalks and railings at the end of the bridge. Although the pavement in such transition is usually covered by thick asphalt overlays, the design vertical curve has been so badly impaired that vehicles approaching a bridge have to slow down in order to avoid possible driving hazards, or at least unpleasant feeling of bumps every time they get on and off the bridge.

In an attempt to cope with this problem, two schemes have been employed for some of the recent highways constructed to a higher standard. These measures consist of the use of relief piling beneath the approach embankment and the use of lightweight fill material for the embankment after preloading.

In a test fill program for the Bangkok-Sri Racha Highway project, sand drains, 20 cm in diameter, were installed to depths ranging from 3 to 13 m beneath a 2.1 to 2.3 m high embankment which was built on a stratum of the Bangkok clay, 14.5 m in thickness. Where the sand drains were deepest, it settled as much as 1.3 m in a period of 15 months but was

still settling at a rate of 3 cm a month. It was concluded that the sand drains were generally effective to accelerate the settlement, but not effective enough to limit its rate to a tolerable level within the expected construction period. The use of relief piling was then recommended for bridge approaches in this project.

Also in the Asian Highway project, relief piles were employed for bridge approaches in areas where the soft Bangkok clay was present. The purpose of relief piling is to minimize differential settlements between the embankment and the bridge abutment and also to increase stability of such relatively high embankment.

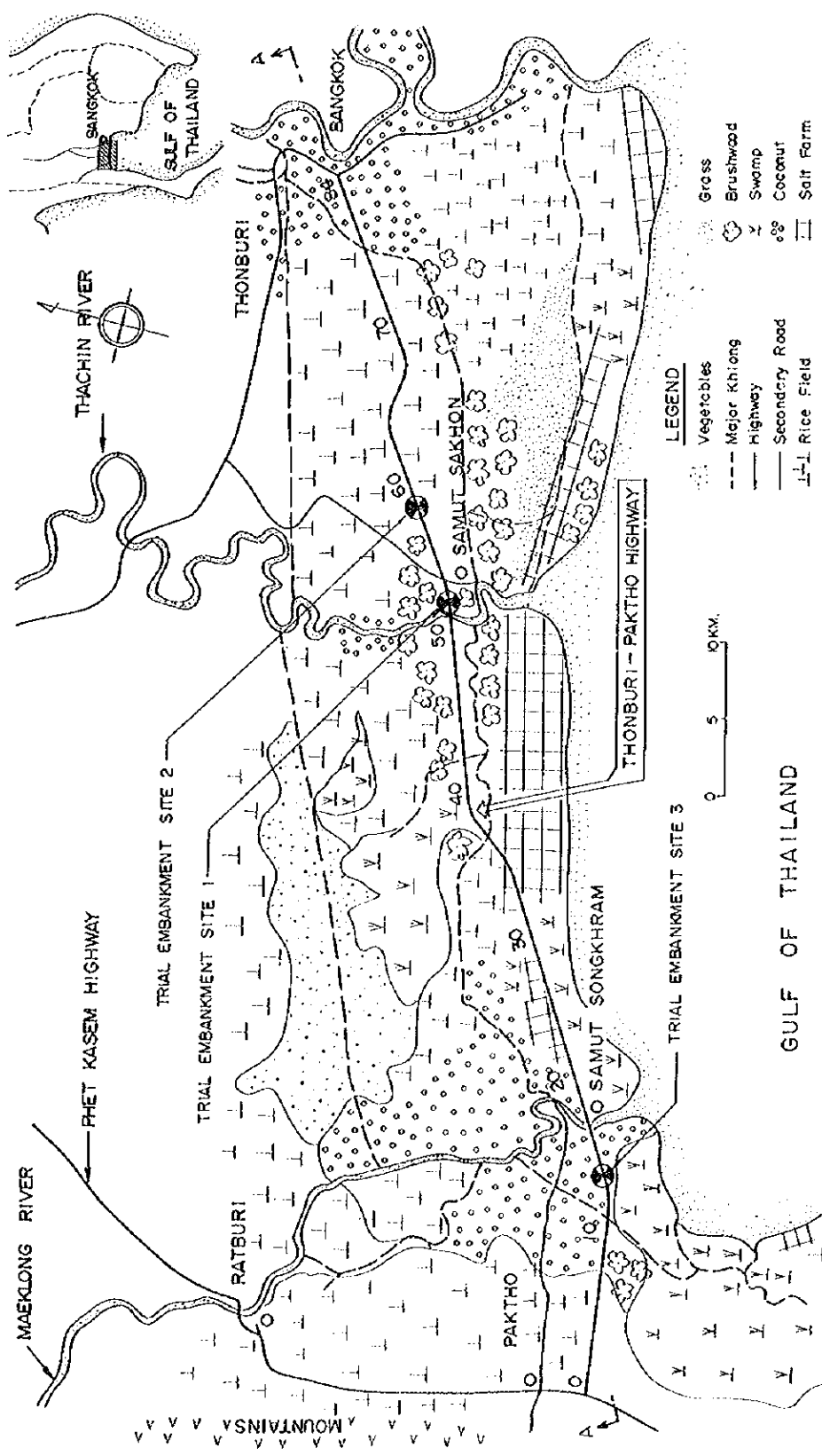
The piles put into use were 20 to 30 cm diameter timber piles for pile length less than 16 m, and 22 cm square prestressed concrete piles for lengths greater than 16 m. Fig. 5-16 gives typical cross-sections showing the relief piling for the Asian Highway designed by KAMPSAX in 1968.

To take the maximum advantage of the preloading effect, it appears very effective to replace a heavy fill material after an adequate preloading period by a lightweight material such as rice husk ash, which has been discussed as an embankment material in Section 5-5-3. The rice husk ash has been used apparently successfully on approach embankments to railway bridges on the Eastern Line and the high approaches to the Rama VI Bridge. The Department of Highways has also used this material at least on three bridge approaches before it was decided to employ the combined scheme of sand preloading and lime-stabilized rice husk ash for the Thonburi-Paktho Highway presently under construction. This scheme is reported to have been successful in reducing post-construction settlements of the approach embankments.

Besides these two methods, stabilization by means of sand compaction piles or chemico-lime piles (consisting of quick lime) would also prove to be effective to reduce the settlements. It is recommended, therefore, to include such treatment in the trial embankment program to verify their effectiveness.

Table 5-3 Summary of Trial Embankment Results
Thonburi-Pak Tho Highway (Cox, 1971)

| Site | Station | Section No. | Information Required | Height (m) | Maximum Pressure (t/m ²) | Start of Construction (settlement Period to Feb. 71)(yrs) | Immediate Settlement (cm) | Settlement to Feb. 71 (cm) | Settlement Rate - cm/cycle of 0.1 yr (secondary coefficient-Cr) | Maximum Lateral Movement (cm) |
|------|-----------------------------------|-------------|--------------------------|----------------|--------------------------------------|-----------------------------------------------------------|---------------------------|----------------------------|-----------------------------------------------------------------|-------------------------------|
| 1 | 54 + 800 | 1 | Stability and settlement | 4.20 | 7.22 | 15/6/67(3.7) | 30.0 | 168 | - | 12.4 |
| 2 | 63 + 600 (Rice Field) | 1 | Stability | 4.70 (Failure) | 7.31 | 11/6/69 | 34.2 | - | - | 9.0 |
| | | 2 | Settlement | 0.97 | 1.65 | 1/6/69(1.7) | 2.0 | 7 | 7.2 (0.005) | 0.8 |
| | | 3A | Settlement | 2.45 | 4.21 | 5/9/69(1.5) | 8.2 | 47 | 55.0 (0.038) | 3.5 |
| | | 3B | Settlement | 1.38 | 2.31 | 9/8/69(1.6) | 1.7 | 13 | 16.5 (0.011) | 1.0 |
| | | 3C | Settlement | 0.75 | 1.21 | 28/8/69(1.5) | 1.0 | 4 | 2.4 (0.002) | 0.4 |
| 3 | 13 + 700 (Old coconut plantation) | 1 | Stability | 2.64 (Failure) | 4.96 | 4/9/69 | 25.0 | - | - | 8.5 |
| | | 3A(a) | Settlement | 1.83 | 3.29 | 19/11/69(1.3) | 8.4 | 45 | 51.0 (0.040) | 4.0 |
| | | 3A(b) | Settlement | 1.93 | 3.30 | 19/11/69(1.3) | 8.6 | 43 | 51.0 (0.040) | 5.0 |
| | | 3B | Settlement | 1.09 | 1.57 | 12/11/69(1.3) | 2.2 | 18 | 22.5 (0.017) | 1.2 |
| | | 3C | Settlement | 0.70 | 1.17 | 3/11/69 (1.4) | 1.3 | 8 | 8.5 (0.006) | 0.4 |



(General Engineering Co., Ltd)

FIG 5-12 LOCATION MAP OF TRIAL EMBANKMENTS,
THONBURI - PAKTHO HIGHWAY

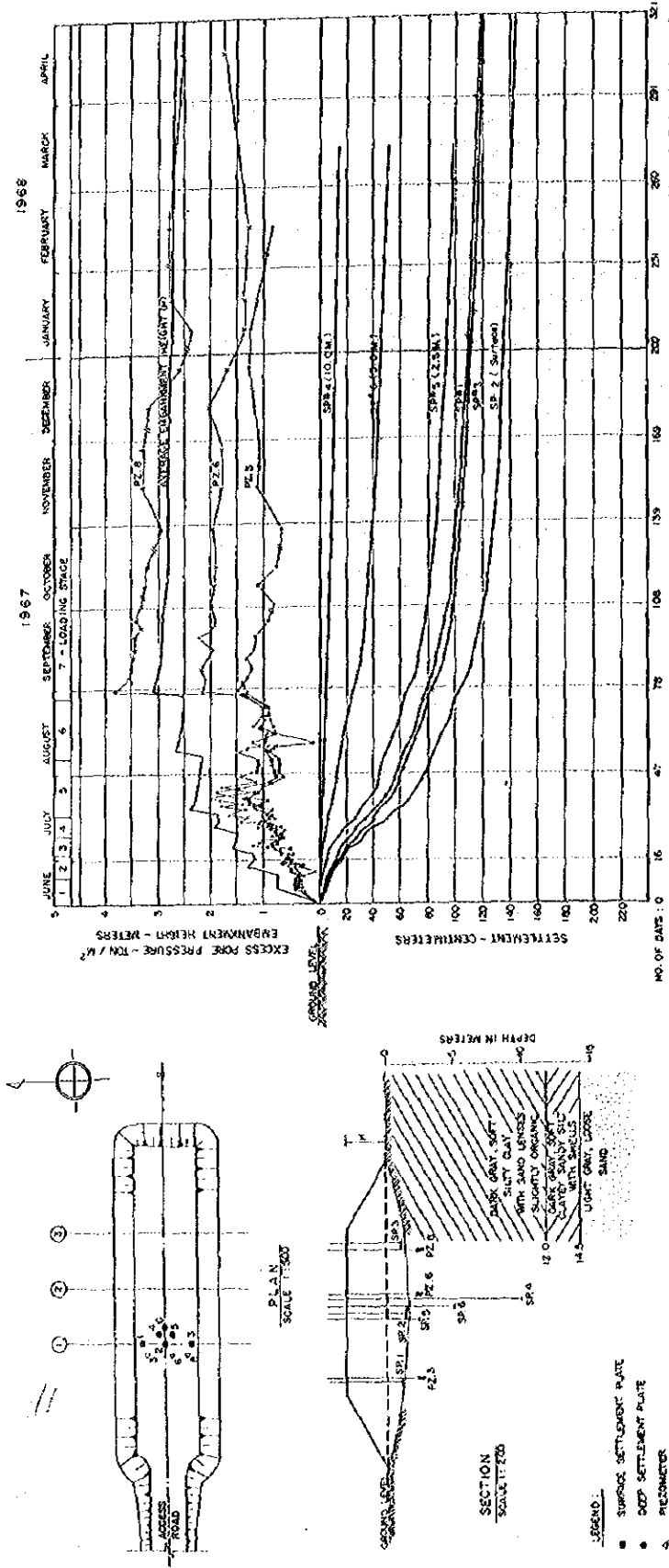
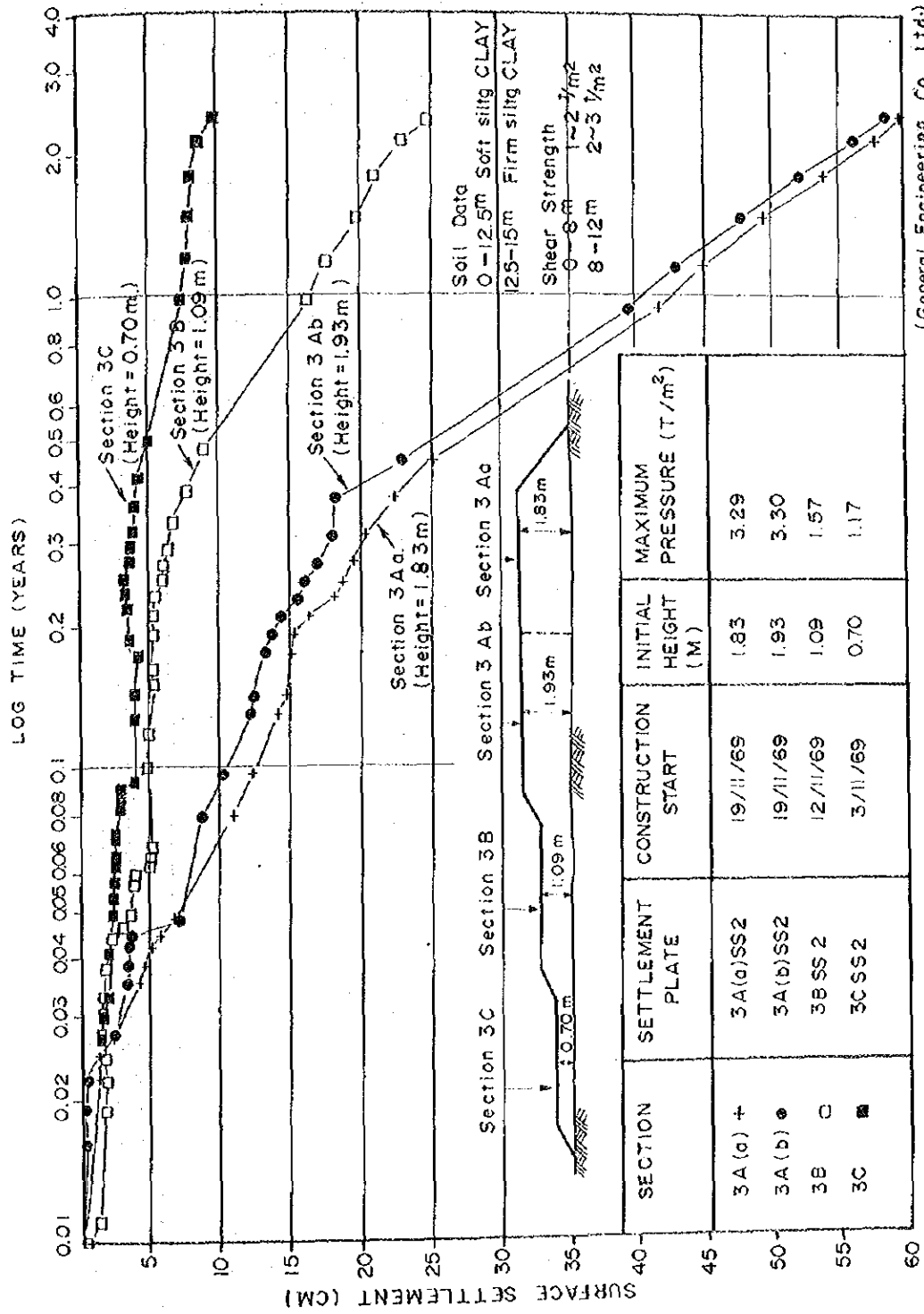


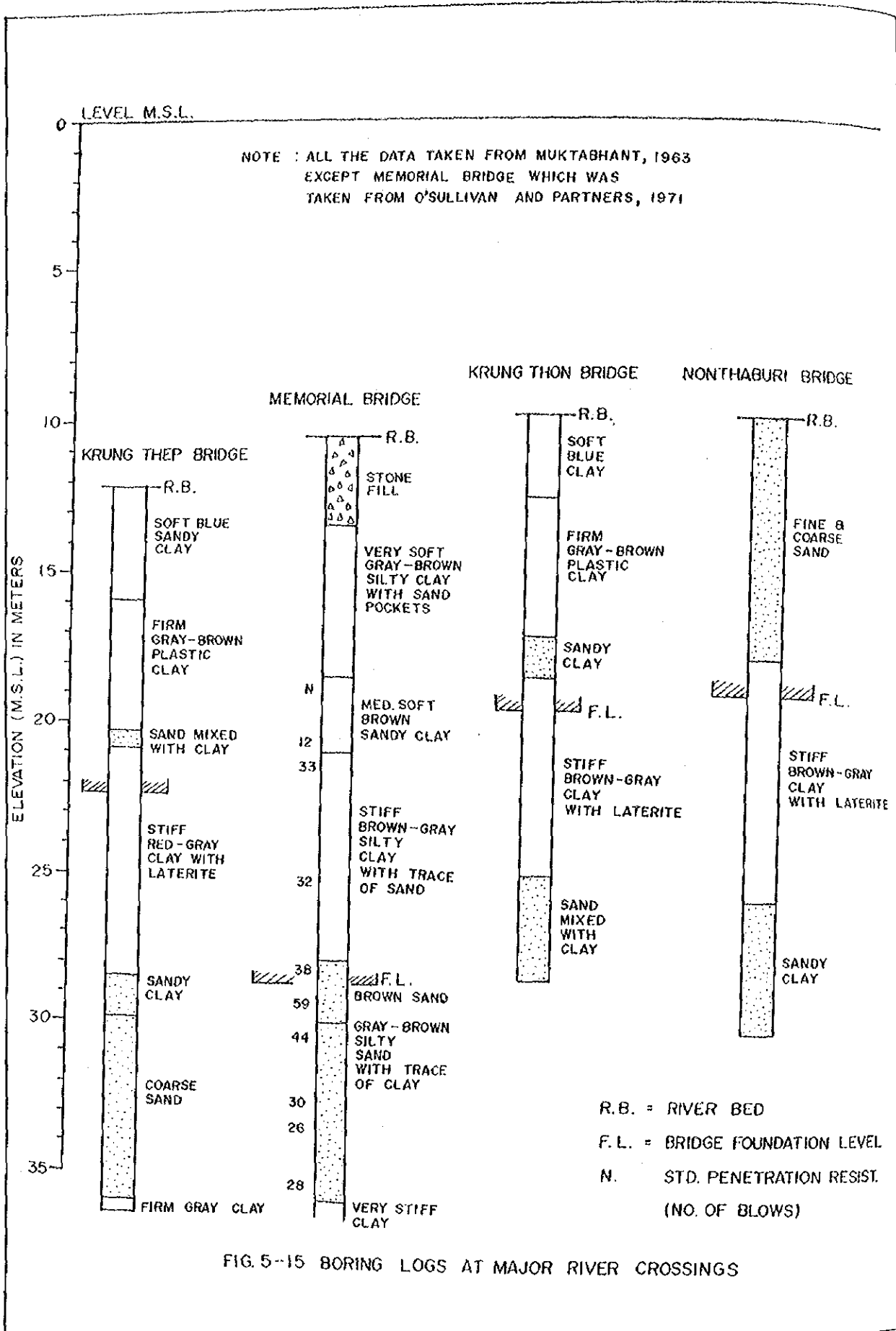
FIG. 5-13 SETTLEMENTS AND POREPRESSURES
 TRIAL EMBANKMENT, SITE 1
 THORBURN - PAK THO HIGHWAY PROJECT

(General Engineering Co., Ltd.)



(General Engineering Co., Ltd.)

FIG. 5-14 SURFACE SETTLEMENT, TRIAL EMBANKMENT SITE 3, THONBURI - PAKTHO HIGHWAY



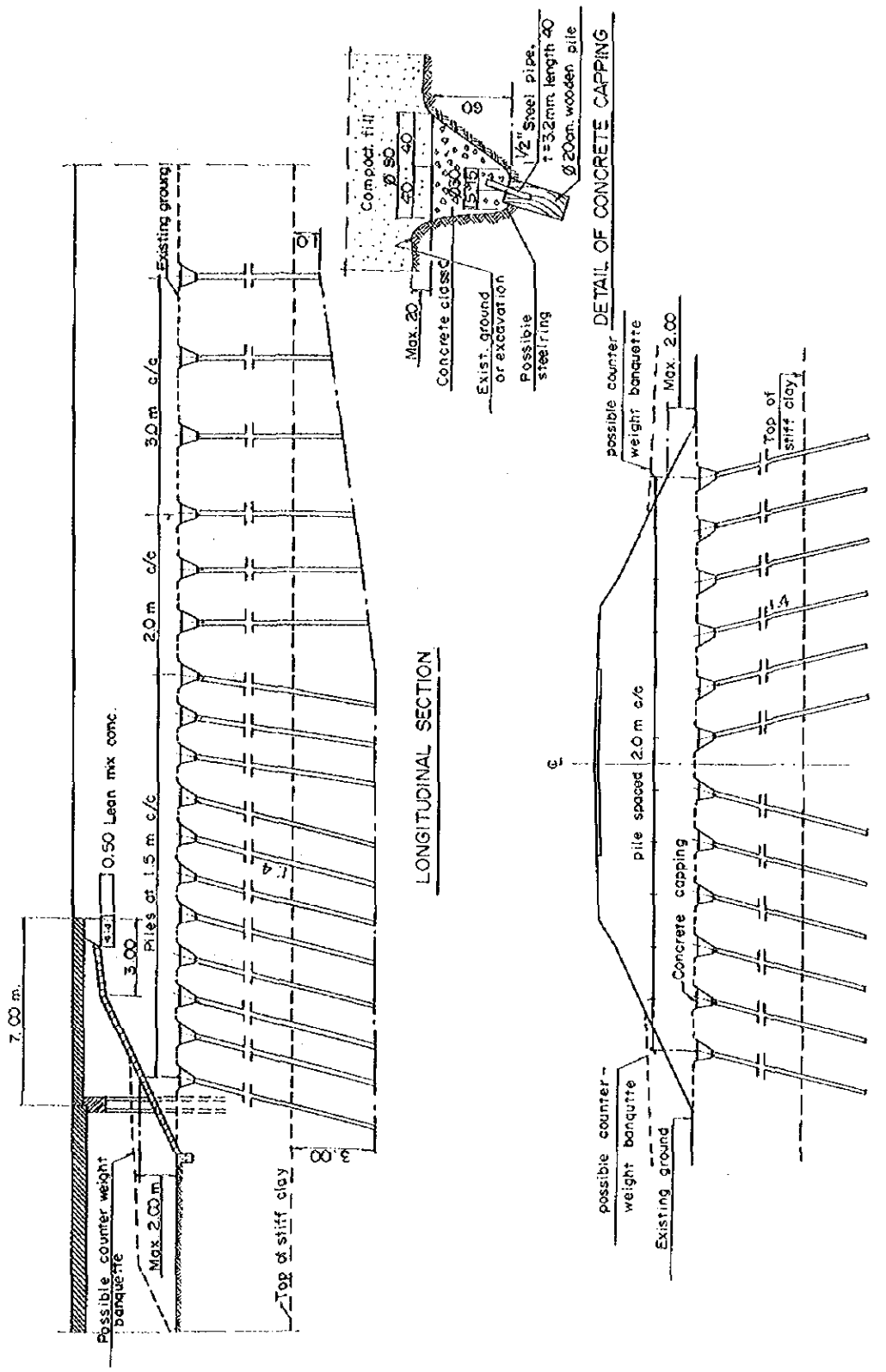


FIG. 5-16 RELIEF PILING (KAMPSAX , 1968)

5-6 Pavement

As of 1971 the Department of Highways had under its responsibility a total of 17, 105 km of highways open to traffic; 10, 977 km of National Highways and 6, 128 km of Provincial Highways. The surface type of these highways are tabulated as follows:

| | <u>National Highway</u> | <u>Provincial Highway</u> |
|----------------|-------------------------|---------------------------|
| Asphalt paved | 9, 571 | 1, 770 |
| Concrete paved | 110 | 11 |
| Unpaved | 1, 296 | 4, 347 |
| Total | 10, 977 | 6, 128 |

As shown above the overwhelming majority of the highways in Thailand are surfaced with asphalt pavement. In the Bangkok area, however, concrete pavement appears to be predominant for heavily-travelled arterials and city streets within and near the city limits. It is noted that even on these roads, asphalt is applied to intersections, approaches to structures and shoulders. Both asphalt and cement are produced in abundance in Thailand and local contractors have experience in both types of pavement.

Typical cross sections of pavement structures are given in "Highways in Thailand, 1971" published by the Department of Highways and the ones for primary highways are shown in Fig. 5-17 (a) and (b). The concrete pavements in the Bangkok area are typically 23 cm in thickness; but the asphalt pavement throughout the country is mostly only 5 cm in thickness underlain by various undercourses. It is to be noted that the 23 cm thick concrete pavement and the 5 cm thick asphalt pavement on undercourses such as shown in Fig. 5-17 (a) and (b) are not comparable in terms of stability and durability; the former is far more

competent than the latter. As a matter of fact, in the Bangkok area the asphalt pavement breaks down relatively easily due to heavy traffic or floods which often submerge the pavement section during the rainy season. It is by this reason that the asphalt pavement is generally believed in Thailand to be of inferior quality requiring constant maintenance as compared with the concrete pavement.

The 23 cm concrete pavement of the Super Highway to Don Muang Airport, for instance, is in excellent shape except approaches to bridges. The concrete pavement of the original two lanes of National Highway No. 4 between Thonburi and Nakorn Pathom shows poor performance manifested by numerous cracks and unpleasant gaps at almost every joint of concrete slabs due probably to pumping. The additional two lanes of No. 4, now almost complete, are paved with asphalt surface of 10 cm in thickness, which is believed to be one of the first thick flexible pavements in Thailand. Also under construction is a 1.5 km section of Suksawat Road in Thonburi with a 12 cm thick surface of asphalt concrete with a pavement section of 50 cm in total, placed on a 65 cm subgrade of compacted sand.

Properly designed and constructed surfaces of the asphalt concrete pavements are capable of carrying almost unlimited volumes of traffic, provided only that they are supported by adequate foundations. The majority of these surfaces may be expected to have an economic life of 20 years or more. It is also true that concrete surfaces are sometimes economical because of their low cost of maintenance and their relative permanency.

While each type has its merits and demerits for the conditions prevailing in the Bangkok area, the particularly important points to be considered are as follows:

1. Where considerable post-construction settlements are expected, it is easier to apply corrective measures to asphalt surfaces.
2. Stage construction is possible for asphalt pavement to reduce the initial cost and
3. For foundation conditions in which soft clay and high ground-water prevail, pumping may take place beneath concrete pavement.

Our preliminary designs of both type, concrete and asphalt, are shown in Fig. 17 (c) and (d), respectively. These are based on preliminary traffic volumes and soils data and are considered for comparison of the construction cost of each type.

The minimum thickness of the subgrade, i. e., a compacted sand embankment is tentatively indicated to be on the order of 100 cm and will have to be finalized when more soil data of the weathered crust at shallow depths are obtained.

A subbase of soil aggregate will consist of laterite mixed with sand and a base course of crushed stone will be of limestone or andesite crushed to the specified grading.

The preliminary cost estimate of a pavement section above subgrade would be roughly as follows:

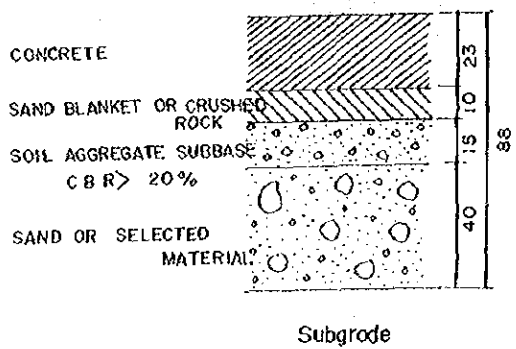
Concrete Pavement

| | <u>Thickness</u> (m) | | <u>Unit Cost</u> (Baht/m ³) | = | <u>Cost</u> (Baht/m ²) |
|----------------|-------------------------|---|--------------------------------------------|---|---------------------------------------|
| Surface Course | 0.23 | x | 900 | = | 207 |
| Base Course | 0.15 | x | 240 | = | 36 |
| Subbase Course | 0.15 | x | 144 | = | <u>22</u> |
| Total | 0.53 | | | | 265 |

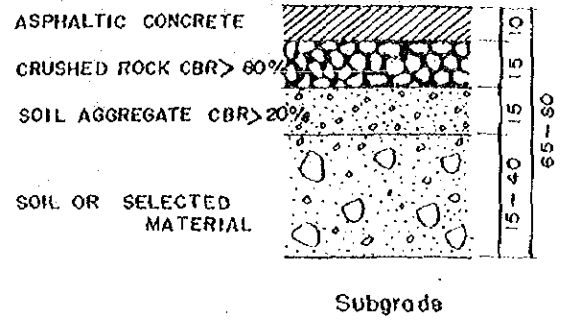
Asphalt Pavement

| | <u>Thickness</u> (m) | | <u>Unit Cost</u> (Baht/m ³) | = | <u>Cost</u> (Baht/m ²) |
|----------------|-------------------------|---|--------------------------------------------|---|---------------------------------------|
| Surface Course | 0.10 | x | 850 | = | 85 |
| Base Course | 0.15 | x | 650 | = | 97 |
| Subbase Course | 0.25 | x | 144 | = | 36 |
| Total | 0.50 | | | | <hr/> 218 |

As shown above, this preliminary comparison indicates that the concrete pavement is 22% more expensive than the asphalt in terms of the cost per square meter. As more accurate traffic data firm up, stage construction may become a possibility in which, for instance, a 6 cm surface is placed initially and another 4 cm layer laid several years after the opening of the Ring Road, which would be an additional economic merit. At this preliminary stage, however, no further elaboration of cost estimate is justified because of the lack of essential data.

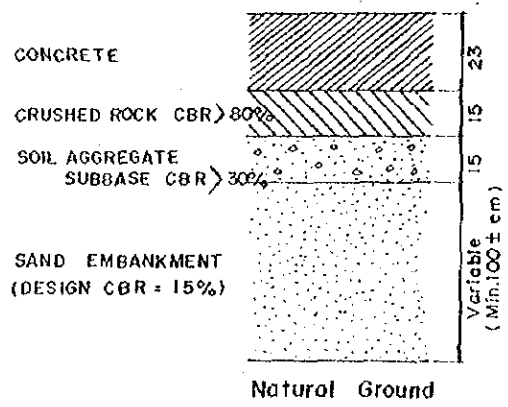


Subgrade
(a) Rigid Pavement

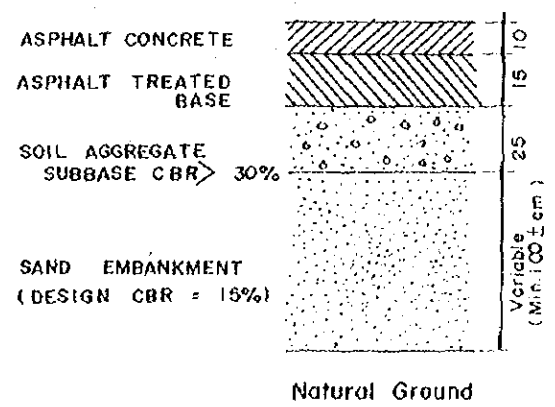


Subgrade
(b) Flexible Pavement

(A) TYPICAL PAVEMENT STRUCTURES FOR PRIMARY HIGHWAYS
(Department of Highway)



(c) Concrete Pavement



(d) Asphalt Pavement

(B) PROPOSED PAVEMENT DESIGN

(Unit : Cm.)

FIG. 5-17 PAVEMENT SECTIONS

5-7 Summary and Conclusions

1. The project area is situated in the vast flat alluvial plain of Chao Phraya River and the subsurface condition appears to be fairly uniform. The area is mantled with a deposit of soft Bangkok clay, typically 12 to 16 m in thickness. The top few meters are in general slightly stiffer, forming a weathered crust. Beneath this deposit exists a layer of stiff to hard clay, several meters thick, and at a depth of 21 to 25 m, is underlain by dense sand and gravel strata with some sandy clay extending to undetermined depths. Groundwater level is generally encountered close to the ground surface.
2. The Bangkok clay is a normally consolidated, sensitive clay with a consistency of "very soft to soft" and with a high compressibility and a high rate of secondary consolidation. This clay appears to be slightly overconsolidated throughout the depth and the "critical load" due to this effect is of great engineering significance in terms of stability and settlement problems.
3. Because of its extremely low strength and high compressibility, it has been found almost impossible to evaluate reliably the stability and settlement of an embankment founded on the Bangkok clay by means of the conventional design analyses alone.
4. It is urgently recommended, therefore, that at least one prototype trial embankment be constructed at a representative site at an earliest practicable time. Settlement plates, piezometers and surface stakes should be installed beneath and around the embankment, and field measurements be taken regularly until construction starts.

Only with such field data would it be possible to predict with reasonable accuracy the performance of the embankment on a similar subsurface condition and to make an economical design of the highway embankment in the project area.

It should be kept in mind that more trial embankments may be called for should the detailed exploration reveal different subsurface conditions.

5. It is recommended that embankment be built by imported river sand rather than utilizing the in-situ surface clay along the proposed route. The embankment, in most part, will have to be compacted so as to form a competent subgrade for pavement section. Approach embankments for structures will have to be pile-supported, or to consist of lightweight fill material such as lime-stabilized rice husk ash after sufficient preloading. Or else, the foundation soil has to be treated by some special methods prior to construction of approach embankments to minimize the settlements.
6. It is recommended that embankment be constructed in stages and field measurements for stability and settlement be maintained at close intervals of distance during construction. Maintenance of such measurements will have to be included in the special provisions for the construction specification.
7. It is our preliminary feeling that the embankment height should be limited to 2 to 3 m to lessen post-construction settlements and also to minimize the possibility of failure which might result in property damages in developed areas.
8. An adequate supply of satisfactory embankment materials and aggregates in the form of crushed stone is available within a reasonable distance from the project area.

9. No particular difficulty is anticipated for deep foundations; large-diameter, cast-in-place, reinforced concrete piers for the major river crossing, and precast reinforced concrete piles or prestressed concrete piles for other structures are recommended.

10. The asphalt concrete pavement, properly designed and constructed, will require no major resurfacing for a design life of at least 20 years. Because of its economy and flexibility the asphalt concrete surface appears more attractive than concrete pavements.

CHAPTER 6. HYDROLOGICAL INVESTIGATIONS

6-1 General Description

6-1-1 Topography

The proposed Bangkok - Thonburi Ring Road traverses an extremely flat plain located in the delta formed by the Chao Phraya River. The project area is situated about 50 river kilometers north of the Gulf of Thailand.

The topography is characterized by a low-level ground formation interlaced with many waterways. The natural ground elevations in the project area are in the order of 0.5 meters to 1.5 meters with a maximum of about 2 meters above local mean sea level. (Mean sea level at Bangkok Bar is set at 35.0 meters.)

In some places along the Chao Phraya River, natural levees can be observed with low-lying marshy areas behind them. In addition to these topographic characteristics, there are many artificial ponds which are the remains of past borrow pits.

The rainfall runoff water in the project area is drained through natural streams or khlongs to the Chao Phraya River, however, water accumulates and forms marshes in some low-lying areas.

The proposed alignment of the Ring Road Part II, surrounds the northern outskirts of the Bangkok - Thonburi metropolitan area. On the west side of the Chao Phraya River, the alignment passes a forest of palm trees alongside the existing Charan Sanitwong Road.

The alignment on this portion is set on comparatively high elevations, and therefore, no major drainage problems are anticipated. On the east side of the Chao Phraya River, the alignment traverses a newly developed residential area, paddy fields and marshes. Some special drainage considerations will be needed in designing this section of the road.

6-1-2 Climate

The climate of Thailand is governed mainly by monsoons. Air streams flowing from the south-west from mid-May to September and from the north-east during the months of November to February are the typical monsoons of Thailand.

Three well-defined seasons arise as a consequence of these prevailing winds: The "Rainy Season" extending from May to September; The "Dry Season" which includes November, January and February, and "Transition Seasons", covering October, March and April.

The rainfall during the rainy season in Bangkok presents some regularities. For example, it generally starts raining in the late afternoon or in the evening with strong showers usually coupled with thunderstorms. The rainfall is characterized by high intensity, short duration storms. About 80% of the total yearly rainfall is recorded during the rainy season.

Whenever typhoon rains overlap with monsoons during the rainy season, large floods, caused by the Chao Phraya River, usually occur on the delta area.

Table 6-1 summarizes the climatic conditions in Bangkok, including temperature, relative humidity, evaporation, days of rain and rainfall, and wind velocities.

Table 6-1 Climatic conditions in Bangkok

| Months | Jan. | Feb. | Mar. | Apr. | May | June | July | Aug. | Sept. | Oct. | Nov. | Dec. |
|---------------------------------------|------|------|------|------|------|------|------|------|-------|------|------|------|
| Temperature, °C | | | | | | | | | | | | |
| High | 33.3 | 33.9 | 35.0 | 36.1 | 35.0 | 33.9 | 33.3 | 33.3 | 32.8 | 32.8 | 31.7 | 31.7 |
| Low | 18.9 | 21.1 | 22.8 | 24.4 | 24.4 | 24.4 | 24.4 | 24.4 | 23.9 | 23.9 | 21.6 | 19.4 |
| Avg. | 26.2 | 28.0 | 29.3 | 30.1 | 29.7 | 29.0 | 28.5 | 28.4 | 28.1 | 27.7 | 26.9 | 25.6 |
| Temperature, °F | | | | | | | | | | | | |
| High | 92 | 93 | 95 | 97 | 95 | 93 | 92 | 92 | 91 | 91 | 89 | 89 |
| Low | 66 | 70 | 73 | 76 | 76 | 76 | 76 | 76 | 75 | 75 | 71 | 87 |
| Avg. | 79 | 82 | 85 | 86 | 85 | 84 | 83 | 83 | 82 | 82 | 80 | 78 |
| Relative Humidity percent | 71.4 | 74.1 | 73.6 | 74.3 | 78.6 | 79.4 | 79.4 | 80.1 | 82.1 | 82.7 | 79.3 | 73.5 |
| Evaporation, millimeter inches | 185 | 169 | 195 | 170 | 140 | 141 | 130 | 119 | 100 | 125 | 153 | 182 |
| | 7.3 | 6.7 | 7.7 | 6.7 | 5.5 | 5.6 | 5.1 | 4.7 | 3.9 | 4.9 | 6.0 | 7.2 |
| Days of Rain, per month | 1 | 3 | 4 | 6 | 17 | 18 | 19 | 19 | 21 | 17 | 7 | 3 |
| Average Rainfall,* millimeters inches | 8 | 31 | 35 | 84 | 179 | 163 | 176 | 187 | 313 | 245 | 53 | 6 |
| | 0.3 | 1.2 | 1.4 | 3.3 | 7.1 | 6.4 | 6.9 | 7.4 | 12.3 | 9.7 | 2.1 | 0.3 |
| Wind Velocities, Max., Km/hr | 47 | 61 | 58 | 97 | 122 | 76 | 76 | 72 | 83 | 72 | 54 | 50 |
| Avg., Km/hr | 7 | 13 | 13 | 11.5 | 8.5 | 7 | 6.5 | 7 | 7 | 6.5 | 6 | 3.5 |

*Mean annual rainfall = 1,482 millimeters (58.4 inches). Period of record = 2480 to 2509 (1937 to 1966).

6-1-3 Soil and Ground Water

Sub-surface soil conditions in the project area are relatively uniform. A soft clay layer, called Bangkok dark heavy clay, is widely found in the ground top stratum. This layer is from two to three meters thick and beneath it, a soft gray clay and brown stiff clay layers are observed. Below 20 meters, the clay is increasingly mixed with sand and gravel.

The ground water level in Bangkok is very close to the surface during the rainy season. Even during extreme dry seasons, ground water is rarely more than a meter below the surface. The average elevation of ground water level varies approximately from 35.5 to 36.0 meters while the natural ground elevations range from 35.5 to 36.5 meters.

Under these conditions, there exist many waterlogged areas throughout the year.

According to existing records at Bangkok Bar, the mean sea level is 35.03 meters. The maximum and minimum sea levels have been recorded at 36.86 and 33.37 meter respectively.

These water heights closely influence the ground water height in the project area.

6-2 Floods in the Project Area

6-2-1 General

There are two different types of floods in the project area, internal floods and external floods.

Internal floods are observed everywhere in the metropolitan area after heavy rainfalls during the rainy season. They are caused mainly by the fact that the existing drainage system cannot accommodate the rapidly accumulated runoff resulting from high-intensity storms.

External floods are caused by high flood waters from the Chao Phraya River. During the rainy season, high flood waters of the Chao Phraya River are sometimes swelled by high tides and overflow the river banks.

The external floods are more extensive than the internal floods.

6-2-2 Internal floods

The existing drainage facilities in the metropolitan area are not very effective. Because of the extreme flatness and low elevation of the topography as well as the high water level in the khlongs and rivers that receive the discharge runoff from the storm sewer networks, it is quite difficult to maintain the necessary hydraulic gradient to accommodate the expected discharge under the gravity flow drainage system now employed.

Shortly after a high intensity storm, excess surface runoff water slowly moves to low points and sumps where it accumulates and stagnates for several days. Roads constructed in these areas are generally overtopped by this type of flood.

Due to the lack of available data, the design water height for internal floods could not be exactly determined. However, from local information collected during site surveys, a flood height of 36.2 meters can be expected for the Part I portion of the Ring Road alignment. Taking the general slope of the Chao Phraya delta as 1 : 25, 000 in the project area, this flood height of 36.2 meters can be projected and estimated at 36.3 and 36.5 meters for the southern portion and northern portion of the Part II alignment respectively.

Considering that the average ground level is about 36.0 meters and that the total monthly rainfall in the rain season is in the order of 300 millimeters, the internal flood heights thus obtained are considered to be reliable and adoptable.

Although internal flood heights are generally lower than those of external floods, they do occur more frequently during the year and for comparatively longer durations.

The Bangkok municipality is now starting to implement a program for the improvement of the drainage system in the Bangkok - Thonburi metropolitan area. After this scheme is completed, internal floods will be drastically eliminated in the developed portion of the project area.

Under these circumstances, the proposed Ring Road, if designed to cope with external floods, will be quite adequate to handle internal floods.

6-2-3 External floods

There are two major factors which decide the extent of floods in the lower reaches of the Chao Phraya River. The first is the accumulated quantity of water collected by its many upstream tributaries. The second is represented by the tidal influence in the Gulf of Thailand, as the average ground

elevation in the project area is only about one meter above mean sea level. In the past, external floods were observed when high concentrations of sea water appeared in the river discharge.

Geographically, the river basin occupies most of the northern part of the country and also a large area of the central part, totalling 162,000 Km² which constitutes about 31% of the whole area of the Kingdom of Thailand.

In the northern part of the country, mountain ranges form four principal basins which are drained off by the Ping, Wang, Yom and Nam Rivers.

These rivers come into concurrence at their lower reaches forming the Chao Phraya River.

The general river slope near Pak Nam Poh, where the four north tributaries join, is about 1:7,000, with ground level at Pak Nam Poh being about +23.50 meters above mean sea level.

It gradually flattens to 1:10,000 at Ayuttaya, and then further still until it reaches 1:25,000 at Bangkok with practically no slope at all at its intersection with the sea coast.

As Thailand has seasonal rains, the rise and fall of its rivers usually correspond to them. In general, most rivers have their first peak in May or June. During this part of the year, their waters rise about 2 - 3 meters above minimum levels for a period lasting 7 - 21 days, receding thereafter by 1 - 2 meters during July or thereabouts. The second peak follows and builds up gradually reaching its maximum at the end of August or beginning of September, usually overflowing the banks for a short period, then gradually subsiding to the normal minimum level by the end of November or beginning of December.

Fig. 6-1 shows the discharge hydrograph of the Chao Phraya River.

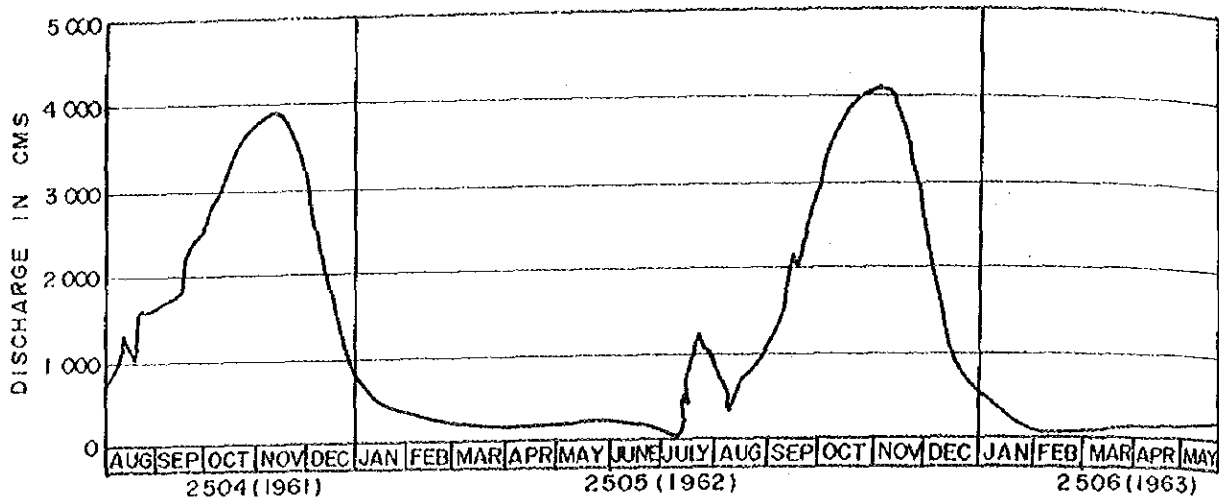


Fig. 6-1 Discharge hydrograph of Chao Phraya River at mouth

The hydraulic grade line in the lower reaches of the Chao Phraya River is very much influenced by tides. In the wet monsoon season, the location where the river flow begins to be noticeably affected by tides is in the vicinity of 75 kilometers upstream from its mouth. In the dry season, this location shifts further up river to about 160 kilometers near Pamok. Fig. 6-2 shows water-level gradients for the Chao Phraya River in its lower reaches.

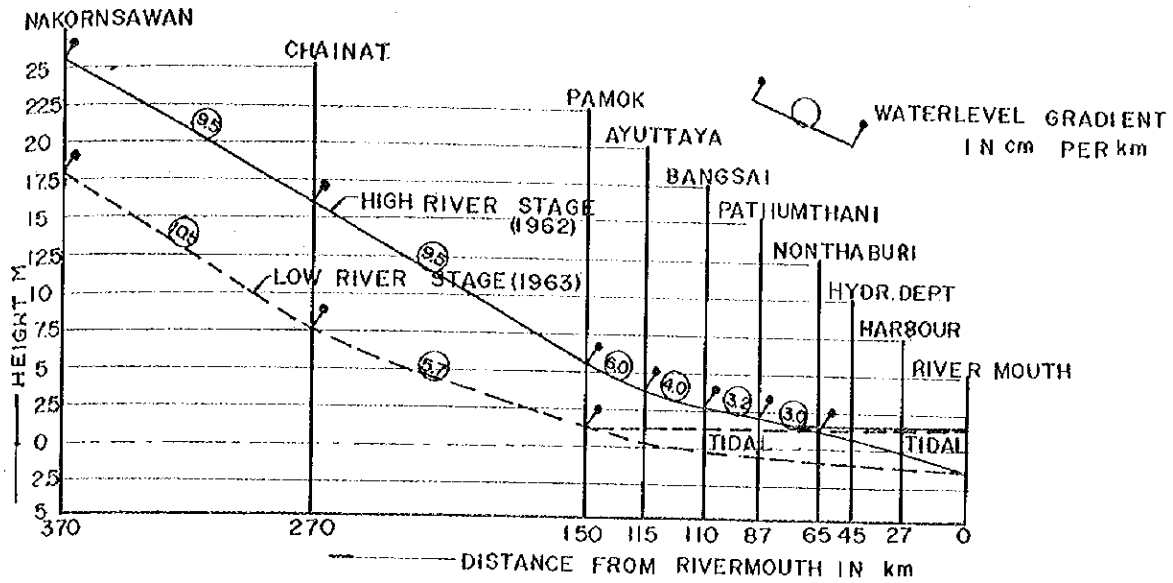


Fig. 6-2 Water-level gradients of Chao Phraya River

During the rainy season, the Chao Phraya's main channel is insufficient to accommodate the total discharge coming from its upstream tributaries. As a result, the excess discharge overflows its embankments downstream from the Chao Phraya Dam.

The flood waters spread over the Chao Phraya delta and find their way south and finally flow into the Gulf of Thailand. In Fig. 6-3, the Pamok water-levels are plotted against the Nakorn Sawan discharge. The curve of this figure clearly shows the limited discharge capacity of the main channel. Flooding starts when the discharge at Nakorn Sawan has attained a value of about 1400 m³/sec.

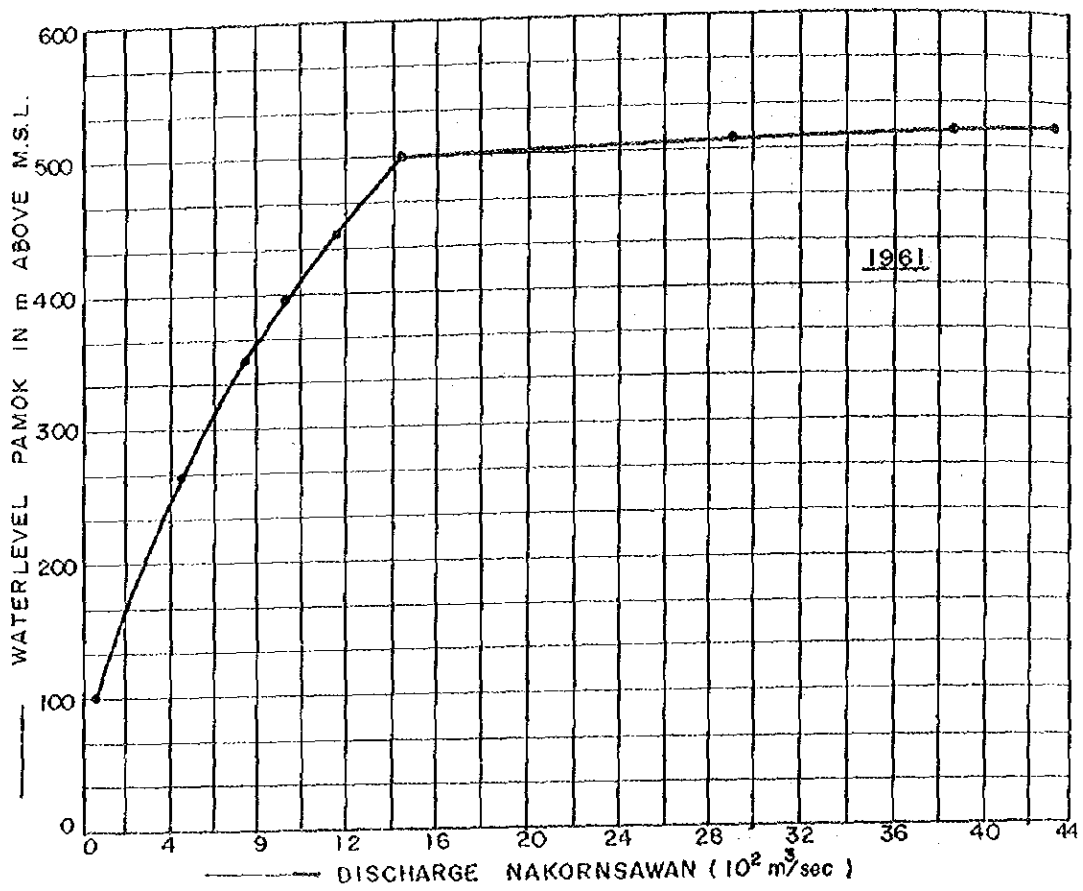


Fig. 6-3 Water-levels at Pamok vs. discharges at Nakorn Sawan

Before the Chao Phraya Dam was completed in 1957, there were many serious floods in the Bangkok-Thonburi metropolitan area. At present, an irrigation scheme, "The Greater Chao Phraya Project" has been completed, and Bumipol Dam on the Ping River, the most important tributary, has been opened. Another dam, the Sirikit Dam was newly completed on the Nam River, the second largest tributary.

Therefore, flooding is no longer a serious problem in the project area, however, the danger still exists in the event of extremely large flows.

The Royal Irrigation Department conducted extensive studies on flood heights and calculated the future expected flows and their recurrences at Bangkok. These values are shown in Table 6-2.

Table 6-2 Estimated present flood expectancy at Bangkok

| Expected Recurrence (Frequency in Years) | Flows at Chainat, (Cubic Meters per Second) | Expected Flood Level at Bangkok | |
|------------------------------------------|---------------------------------------------|---------------------------------|-----------------------------------|
| | | Meters Above Mean Sea Level | Elevation, City Datum (MSL=35.03) |
| 100 | 4,700 | 1.80 | 36.83 |
| 50 | 4,500 | 1.77 | 36.80 |
| 25 | 4,200 | 1.75 | 36.78 |
| 10 | 3,700 | 1.72 | 36.75 |
| 5 | 3,300 | 1.70 | 36.73 |

These flood heights are expected at the Memorial Bridge site in the southern portion of the Ring Road, Part II.

At the Rama VI Bridge site in the northern portion of the proposed highway, flows 0.20 meters above the listed heights are expected based on data from past observations.

6-3 Considerations and Recommendations

6-3-1 General

As stated in the previous sections, the project area is under the influence of internal and external floods.

The proposed Ring Road has to be designed properly so as to meet all-weather road specifications. In order to attain the above purpose, we must consider the proposed profile grade of the Ring Road, necessary river crossings and surface drainage systems within the right of way.

6-3-2 Profile grade

Requirements for an all-weather road call for the pavement surface to be always above the expected high flood level. Moreover, for long flood durations, the adverse effects that prolonged water contact will have upon the supporting strength of the subgrade or pavement materials must be carefully considered.

In determining the proposed vertical alignment, the following information must be taken into consideration.

- (a) High water levels for a 50-year external flood frequency are 36.80 at the Memorial Bridge site and 37.00 at the Rama VI Bridge site.

These heights are hereby defined as high flood levels.

- (b) The peak water level for the expected internal flood is in the order of 36.3 m to 36.5 m.

The height will be defined as average water level, as it stands for a comparatively long duration.

- (c) Due to the very soft and thick clay layers of the subsurface soil in the project area, severe settlement of the highway

embankment is expected. During recent observations, 40 cm - 70 cm of settlement in a highway section between Thonburi and Paktho and 20 cm - 25 cm in the Don Muang - Saraburi Highway, were recorded during their construction stages.

Under such soil conditions, low highway embankments are desirable in order to avoid excess settlement and decrease the quantity of borrow material for economical construction.

- (d) The total thickness of pavement material is estimated at approximately 50 cm. (Please refer to the pavement section, Chapter 5.)
- (e) Newly constructed highways are generally graded to elevations of 37.0 meters - 38.0 meters in the project areas. They are working properly as all-weather roads.

After careful examination, the recommended relationships between the high flood level, average water level and the roadway were determined as shown in Fig. 6-4.

The height at the edge of the outside traffic lane has a freeboard of 50 cm above the high flood level. A freeboard of 50 cm above the outside edge of the travelled way is considered to be adequate for the specialized conditions existing in the delta area.

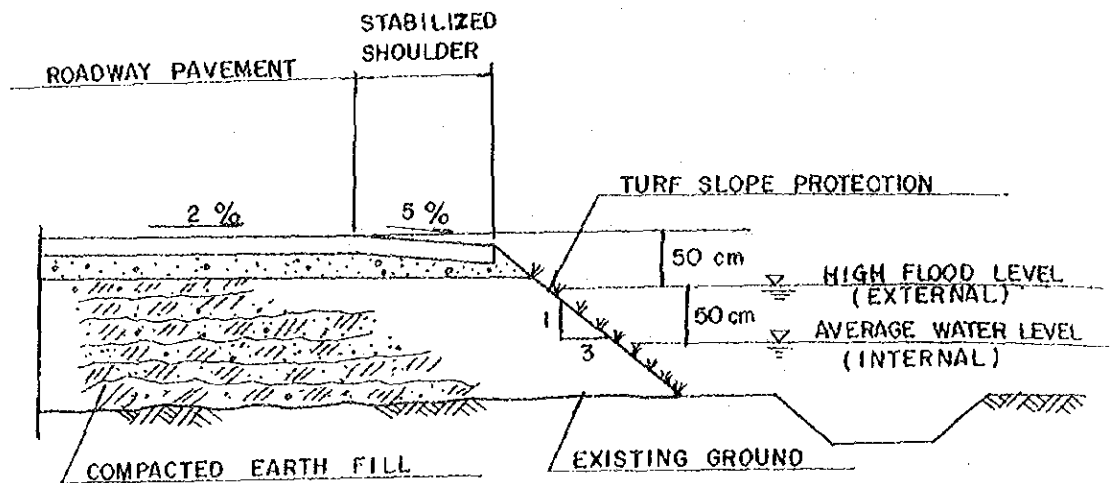


Fig. 6-4 Relation between flood levels and roadway

Because of its extreme flatness, the delta area has a high capability for storing flood waters.

For this reason, the high level of a 100-year recurrence frequency flood, is only 3 cm above the 50-year high flood level previously determined.

Therefore, if properly and periodically maintained, the proposed freeboard should be sufficient to cope with anticipated floods. If the road formation level is set at the center of the traffic lane, the proposed formation elevations thus obtained will range from 37.5 to 37.7 meters.

6-3-3 River crossings and surface drainage system

After careful alignment studies, a bridge site for the Chao Phraya River crossing was proposed at about 1 kilometer upstream from the existing Rama VI Bridge. However, as the crossing site is on the meandering portion of the river, the effect of scour on the proposed bridge foundations should be further studied during subsequent and more detailed investigation.

After considering the facts that no river training works are provided and that the existing Rama VI bridge is working properly, the total length of the proposed bridge may be determined based on the existing river width and the total length of the existing Rama VI bridge.

The streams which will be crossed by the Ring Road are mostly khlongs. According to information obtained from the Harbour Department, a vertical clearance of 3.5 m above high water level and a horizontal clearance of 30 m are required in crossing the khlongs used for combined irrigation-navigation purposes. For the smaller khlongs, the span length and clearance will be decided according to detailed surveys on their present condition.

As the proposed Ring Road alignment will be set so as not to interfere with flood flows, no special structures to equalize the flood levels on each side of the highway will be provided. An appropriate surface drainage system within the right of way shall be provided. In designing the roadway and roadside drainage system, a 5 years frequency storm in conjunction with the rational formula will be suitably employed in calculating run-off. The rainfall intensity-duration-frequency curves, in Fig. 6-5 which were developed for the Bangkok metropolitan drainage study, will be utilized for the above purpose. The close cooperation with the Bangkok metropolitan drainage scheme shall be considered essential in the detailed design stage.

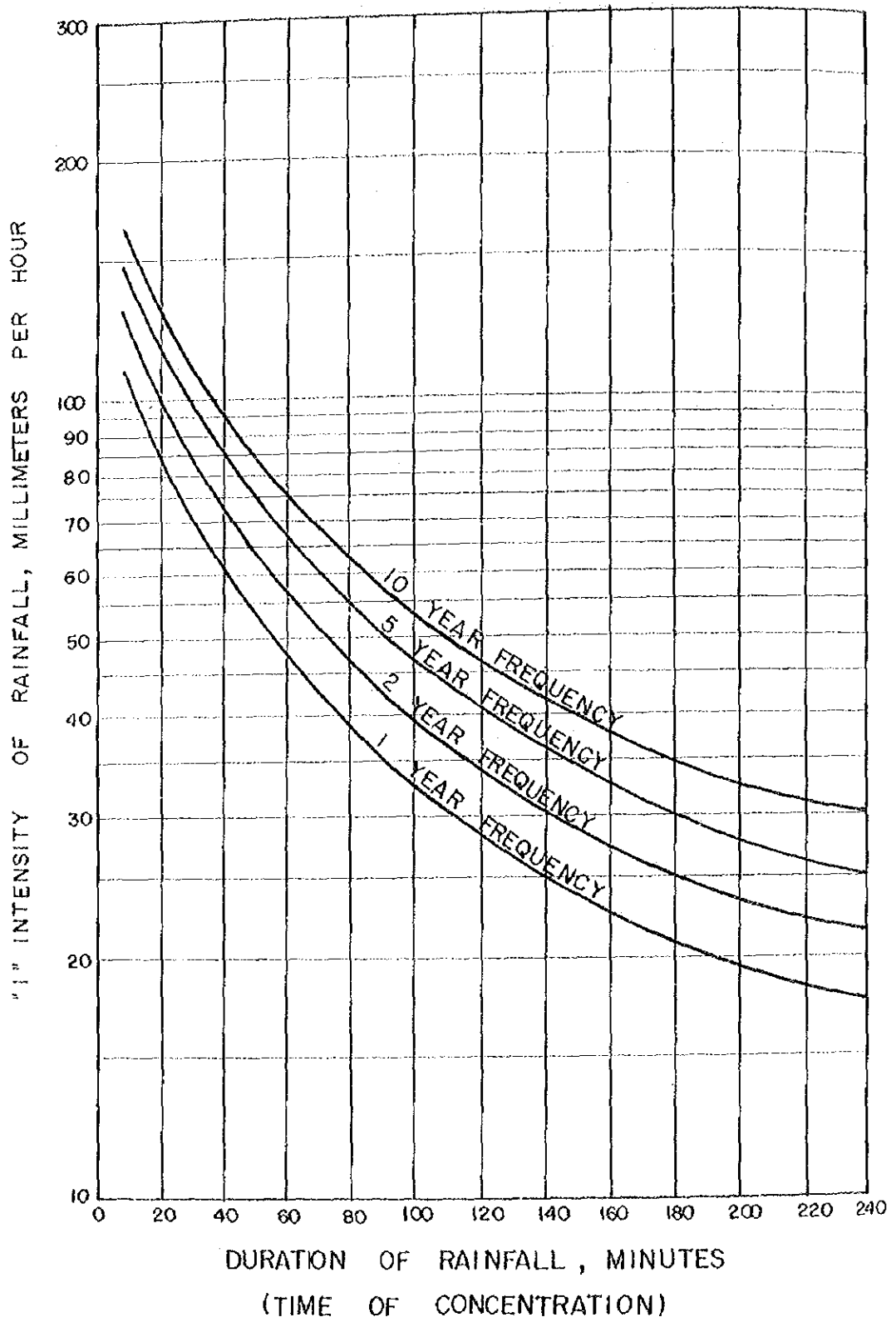


Fig. 6-5 Rainfall intensity-durations curves

CHAPTER 7. HIGHWAY ENGINEERING

7-1 Introduction

Bangkok, the capital of the Royal Kingdom of Thailand, is a fast expanding city, which, together with Thonburi, has a present population of well over three million. In twenty years time, it is anticipated that the population will grow to over six million.

The Ring Road as planned by the Department of Highways, is a ring running at a distance of from seven to ten kilometers from the city centre, and has a total length of about 45 kilometers.

The role of the Ring Road may briefly be summarized as follows.

- (a) To serve as a collector and distributor road for the urban traffic.

The commercial and business activities in Bangkok are scattered over a large area. The Ring Road will serve as a collector and distributor for various subcentres, thus reducing unnecessary through passing of traffic across the undestined area.

- (b) To serve as a bypass for through traffic

The existing highway system of Bangkok is such that the major part of the through traffic in all directions will have to transverse the city centre of Bangkok, adding to the congestion of the city road network.

While the percentage of through traffic in a big city is generally small, the absolute number of vehicles will be considerable, and the construction of a bypass warranted.

(c) To serve as a control for unplanned sprawl of city growth.

Without a properly planned guidance, a city tends to sprawl in all directions in its growth. This phenomenon is also seen in Bangkok, especially in the eastern and north-eastern direction, where growth of residents along any available existing roads is at a rapid rate. A Ring Road will serve as a check and control for such sprawling.

In this study, full consideration for the roll of the Ring Road is made in the process of selection of the route so that the final choice will be able to dispense its function most efficiently.

7-2 Base Data

a) Aerial photographs

The aerial photographs, prepared in 1971, in the scale of 1 : 6,000, supplied by the Department of Highways, were used in the selection of the route. Any new building constructions or developments were confirmed during field investigation trips, and the aerial photographs duly modified.

b) Unit price of land

The unit price of land as officially evaluated by the Land Department of the Municipalities of Bangkok and Thonburi were adopted in the calculation of cost of right-of-way.

However, as the unit price was not obtained for the whole route, some portions were evaluated through deduction by comparison with other known portions of similar condition.

In the case of cost of demolition, it can be anticipated that the actual cost will vary case by case, and in the calculation in this study, a standard cost is assumed and applied to all buildings. In this case, the buildings are classified into either concrete structure or wooden structures, and the standard demolition cost including compensation per m² applied.

7-3 Design Standard

7-3-1 General

The Ring Road was planned as full controlled access with access permitted only at designated intersections.

The design criteria for Ring Road is shown as follows, where desirable standards and minimum requirement are indicated based on the AASHO standard, and the Japanese Standard for Geometric Design of Highway used as reference.

7-3-2 Design criteria for Ring Road

| | <u>Main roadway</u> | <u>Service road</u> |
|---------------------------------------|---------------------|---------------------|
| 1. Design speed | 100 km/hr | 60 |
| 2. Minimum Radius of Horizontal Curve | | |
| Through roadway (desirable) | 700 m | 200 |
| Through roadway (absolute min.) | 400 m | 120 |
| Ramps (desirable) | | |
| 1-lane | 50 m | |
| 2-lane | 60 m | |
| Ramps (absolute minimum) | | |
| 1-lane | 40 m | |
| 2-lane | 50 m | |
| 3. Maximum Superelevation | 8 % | |
| 4. Maximum profile grade | | |
| Through roadway | 4 % | 5% |
| Ramps | 6 % | |
| 5. Minimum Vertical clearance | | |
| Ring Road over highway | 5.0 m | |
| Ring Road over railway | 5.1 m | |
| Minimum free-board over major khlong | 3.5 m | |
| " minor khlong (Chapter 6) | varies | |

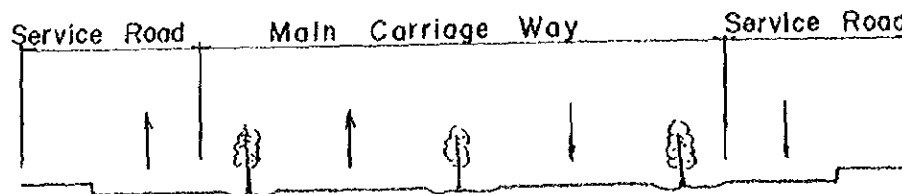
| | | | |
|---------------------------------------|---------------|--|-------|
| 6. Sight Distance | | | |
| Through roadway | 160 m | | 75 |
| Ramps | 50 m | | |
| 7. Vertical curve radius | | | |
| Convex: Desirable | 9,000 m | | 2,000 |
| Minimum | 6,500 m | | 1,400 |
| Concave: Desirable | 4,500 m | | 1,500 |
| Minimum | 3,000 m | | 1,000 |
| 8. Cross Section Elements for Roadway | | | |
| Traffic lane | 3.60 m | | 3.25 |
| Left shoulder | | | |
| Roadway | 2.50 m | | |
| Viaduct | 1.50 m | | |
| Right shoulder | | | |
| Roadway | 1.00 m | | |
| Viaduct | 1.00 m | | |
| 9. Median | | | |
| Except on viaduct | 6.00 - 10.0 m | | |
| on viaduct | varies | | |
| 10. Drainage | | | |
| a. Design flood frequency | 50 years | | |
| b. Design storm frequency | | | |
| Pipe culvert | 25 years | | |
| Box culvert | 25 years | | |
| Short span bridge | 50 years | | |
| Long span bridge | 50 years | | |

7-3-3 Additional explanations on design criteria

1) Cross section composition

The estimated traffic volumes for the Ring Road are as shown in Fig. 3-25 - Fig. 3-35. Besides this volume, an additional 30% of intrazonal traffic is also expected when the area along the highway is fully developed.

The cross section composition of the Ring Road should be such that the road will offer adequate service to the traffic, and will be able to play the roles as described previously. For this purpose, the composition of the cross-section is decided where the main roadway is provided in the centre to cater for high speed through traffic, and service or frontage roads provided on both sides with stopping lanes and sidewalks, to serve the users along the road, and also to serve as distributor and collector connecting roads for the main roadway.



2) Design speed

a) Main roadway

The speed at which vehicles can travel safely and comfortably on a road is variable according to conditions of the climate, the traffic volume as well as the highway structure.

In the case of the proposed Ring Road, the alignment is across flat terrain, and sufficiently great curvature is

planned for the curves sections. However, due to the many elevated sections across major roads, klongs or railway line at an average interval of about 2 kilometers, the profile is rather undulating. It is undeniable that for this reason, there is some lack of harmony between the profile and alignment. As a remedy, endeavours are made to secure as much of lateral allowance along the traffic lane as possible, and to maintain sufficient sight distance at all points. With these considerations, the main roadway is planned at a design speed of 100 km per hour.

b) Service road

From the point of structures, service roads have a smaller number of elevated sections, and the harmony between alignment and profile is maintained.

With the purpose of serving the area along the highway, the service roads, intersecting with other roads at grade, provided with stopping & parking facilities, and being uncontrolled for the access, cater for traffic of varied characteristics. The service road also serve as access ramps to the main roadway. For this purpose, the design speed for the service road is planned at a normal urban city-road speed of 60 km/h.

3) Width of shoulder

The left shoulder of the roadway is planned at 2.5 m wide, to permit emergency stopping of all vehicles.

However, at bridge or elevated roads of more than 50 m in length, the width of the shoulder is reduced to 1.5 m, which will only permit stopping for light vehicles such as passenger sedans.

For the bridge of section 7b across the Chao Phraya River, (the substitute for Rama VI Bridge), the shoulder is planned at 2.5 m, in order that dual direction traffic may be served even if only one half of the bridge is constructed during stage construction.

The right shoulder is planned at 1.0 m to secure sufficient lateral allowance for flowing traffic.

4) Median

The width of the median should be sufficiently provided to allow free and safe flow of high-speed traffic travelling in opposite direction. A minimum of 3 m will be sufficient for this purpose, although in this case the separator will have to be of mounted-up type with kerb stone.

A wide median will reduce psychological resistance to vehicle drivers and thus increase capacity of the inner traffic lanes. Taking the characteristics of the Metropolitan into consideration, the median for the Ring Road is planned at 10 metres.

7-4 Required Number of Traffic Lanes for Ring Road

The design traffic capacity per traffic lane may be calculated from the following formula.

$$C_D = C_b \times R \times B$$

where C_D = Design traffic capacity (vehicle per hour per lane)

C_b = Basic traffic capacity (vehicle per hour per lane)
= 2,000

R = Total modifying coefficient = $R_e \times R_c \times R_t \times R_i$

R_e = Modifying coefficient for width of traffic lane.

R_c = " " for lateral clearance

R_t = " " for heavy vehicle composition.

R_i = " " for roadside obstruction.

In the case of the Ring Road, the width of a traffic lane is planned at 3.6 m, sufficient lateral clearance as well as amply wide median is provided, and the conditions along the route are planned such that no obstruction to traffic flow will exist.

The modifying coefficient are thus adopted as follows.

$$R_e = R_c = R_i = 1.0$$

The modifying coefficient for heavy vehicle composition is calculated as follows.

$$R_t = \frac{100}{100 - P_t + E_t \cdot P_t}$$

where P_t = Composition of heavy vehicle (%)

E_t = Equivalent standard vehicle unit per heavy vehicle.

In this study, the composition of heavy vehicles is estimated to be no more than 15%. At an equivalent factor of 2.0, R_i will be 0.87.

B = Modifying coefficient for planned service level.

The modifying coefficient for planned service level differs according to the relation between the possible traffic capacity and the planned peak-hour volume of the target year.

From the existing traffic situation on the present road network, it is difficult to assume a high level of service for the proposed Ring Road. For calculation of the maximum capacity, the coefficient is assumed at 1.0.

With the above-mentioned figures, the design traffic capacity per traffic lane can be calculated as follows.

$$C_D = 2,000 \times 0.87 \times 1.0 = 1,740 \text{ veh/hour per lane.}$$

This hourly traffic volume is converted into daily traffic volume with the following calculation:

$$ADT = \frac{D, H, V.}{K \times D} = \frac{C_b \times N}{K \times D}$$

Where ADT = Design average daily traffic (veh/day)

D, H, V. = Design hourly traffic volume in one direction.

K = Ratio of 30th hour traffic volume in both directions to ADT = 0.09

(In the city area of Bangkok, the peak hour ratio of traffic volume to daily traffic volume is rather low as can be seen from the results of cross section traffic counts. However, considering that the major part Ring Road traverses through the suburban district, thus resulting in possible concentration of traffic during commuting hours, the k value is set at a comparatively high 0.09)

D = Ratio of major directional traffic flow to total flow at 30th hour = 0.60

N = Number of traffic lane in one direction.

The design average daily traffic capacity for a six-lane highway can thus be calculated as follows:

$$A. D. T = \frac{1,740 \times 3}{0.09 \times 0.60} = 98,000 \text{ vehicles/day}$$

The same calculation for a four lane highway will result in a design capacity of 76,000 veh/day. From the estimated traffic volume for target year of 1990, it is evident that a six-lane main roadway is required of all sections of the Ring Road.

As for the service roads which cater mainly for localized intrazonal traffic, it is seen that some sections may suffice with 2 traffic lanes. However, as stopping and parking will be allowable on the service roads, it is decided that 2 lanes in each direction for a total of 4 traffic lanes should be planned for the service roads through the whole section.

The results of traffic assignment shows the that by 1990, some small sections of the Ring Road have traffic demand slightly exceeding the capacity. However, with the anticipation that some portion of the traffic volume may simultaneously utilize the service road, a six-lane main roadway will be adequate, and expansion of the roadway to 8 lanes cannot be easily justified.

7-5 Route Selection for the Ring Road Part II.

7-5-1 Outline

The route selection was made mainly from the aerial photographs supplied by the Department of Highways and supplemented with extensive field investigation trips.

The investigations in this study alone is not sufficient in the decision of the best routes for certain minor portions, and it is suggested that in the preliminary design, comparison of alternatives for some localized section should further be made in detail.

The alternative studies made for various sections of the Ring Road Part II are described in details in the subsequent paragraphs.

7-5-2 Section 6, 7a(1)

(Section 6 : Phetchaburi Road - Pahol Yothin Road - Mittraphap Road)

Section 7a(1) : Mittraphap Road - Pracharat Road)

(1) Status quo

- (a) Along the vicinity of the proposed route for Section 6 of the Ring Road Part II, the Railway Department had in the past secured a strip of land of 80 m in width with the plan of connecting the Northern and the Northeastern Railway Line in the future. Along this strip of railway right-of-way, building up have been in progress. For the region between Phetchaburi Road and Pahol Yothin Road, some paddy fields are still in existence on the Phetchaburi side. Along Inthrama Road and Lat Phrao Road, which will meet the proposed route about perpendicularly, the building up is prominent in a ribbon shape, showing conspicuous sprawl phenomenon. Also, in the district of Ban Tambon Lat Yao,

large scale housing estate development is at present underway and building up in progress although the district is not conveniently accessible from any major roads.

- (b) Along the section from Pahol Yothin Road to Krung Thep-Nonthaburi Road are found the Mittraphap Road which connects the town centre of Bangkok with Don Muang International Airport and the Northern regions of the Kingdom, and also the Northern and Northeastern Railway Line which runs generally parallel to the Mittraphap Road. In the district lying between the Mittraphap Road and Prachachun Road, construction of concrete structure housing units is going on at a rapid rate centering around Chon Niwet Road. In this new residential district, schools and other public facilities are also well provided in a well planned pattern.
- (2) Basic concept in the selection of alternative routes.

The Ban Tambon Lat Yao and Chon Niwet Districts are undergoing development throughout an extensive area. In order to maintain harmony between the Ring Road and the newly developed district, the proposed route has to be selected at the vicinity of the boundary between newly developed and old housing estates in order that the new high-class residential area will not be disrupted by any new road construction.

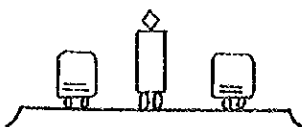
For this section, with the relation with the railway right-of-way taken into due consideration, 4 alternative routes are selected for comparative study, two of which propose making use of the railway right-of-way and the remaining two are unrelated to the railway right-of-way.

The conclusion reached from the comparative study is that alternative 2A, a route unrelated to the railway right-of-way and running about 0.6 - 1.0 km to the east of the right-of-way is the best route.

(3) The route of alternative 1 (case 1)

(a) Location and form

In this alternative, the Ring Road will utilize the railway right-of-way, leaving only 20 m of the 80 m in width for future railway use in the centre, and having the Ring Road on both sides of the railway. This will locate the Ring Road at about 1.5 km to the east of the Mittraphap Road and about 2.0 km to the west of Khlong Lat Phrao. However, considering the existing state of building-up of the environment, a road further to the east is considered more desirable.



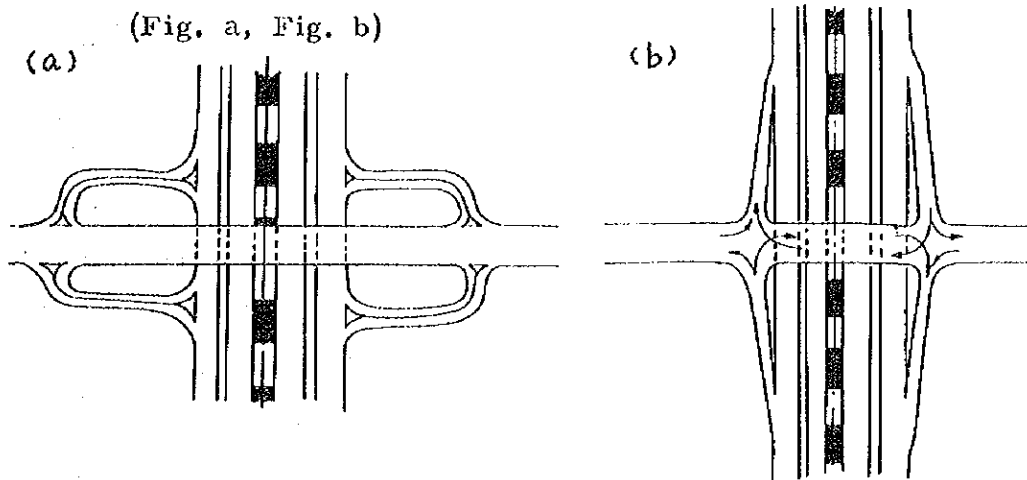
(b) Relation with future railway line

i) Traffic management

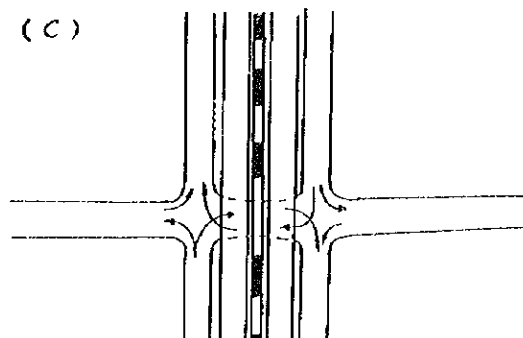
Many problems on traffic management are expected due to the future existence of a railway line in the middle of the highway. To eliminate most of these problems, the elevation of the whole railway line is considered necessary. The reasons are as follows: If the railway line were to be constructed at ground level, then the high-speed main roadway of the Ring Road will also have to be at ground level at intersections with other roads.

At these intersections, grade separation is necessary to maintain the function of the Ring Road, by way of elevating the intersecting roads. In this case, connection between the Ring Road and the intersecting roads will have to be effected with a full clover leaf type interchange or an elevated diamond interchange.

This results in increase in cost of construction or land acquisition. Also, traffic regulation will have to be carried out at the crest of elevated part of the elevated intersecting road or at the terminal part of the slope, both being undesirable for safe traffic operation.



If only the high-speed main roadway of the Ring Road is elevated at intersection, leaving the railway line at ground level, then traffic may be regulated with a ground level diamond interchange. In this case, however, undesirable interruption of smooth flow of traffic on the intersecting road by the at grade railway crossing is anticipated. (Fig. c)



ii) Interchange structure of Ring Road

The starting point of section 6 of the Ring Road Part II is decided by the terminating point of the Ring Road Part I presently in the design stage. The railway line will confluent with the Ring Road at the southern part of section 6, coming in from the southeast.

For the profile, the eastern half of the main roadway will therefore overcross the railway line at the confluence point. For the alignment, to reduce the crossing angle at the confluence point, the Ring Road will be swayed into a gradual S shape. This results in increase in land acquisition and is also undesirable for smooth traffic flow.

Again, at the vicinity of the Mittraphap Road, the proposed railway line will swing out to merge with the existing Northern and the Northeastern Railway line, and the inner lanes of the Ring Road will again have to cross with the railway line with grade separation.

At this point, the Ring Road is to be connected to the Mittraphap Road with a partial clover leaf type interchange, and the service road, which serves as the collecting and distributing road, will overpass the Mittraphap Road.

As a result, in order that the railway line will pass the inner roadway of the Ring Road at a two storey level, the overpassing of the railway line has to be completed before reaching the interchange.

A structure as this, where the angle of crossing is small, and where railway crossing of the loops of the interchange is unavoidable, is with very complicated positioning of the abutments. In this case, it is not

possible to limit the railway line to within the right-of-way already procured, and some new acquisition of land for the railway is necessary.

iii) Relation with the width of existing right-of-way

With 20 m of the existing 80 m of railway right-of-way reserved for the future railway line, the Ring Road will be able to purchase the remaining 60 m from the Railway Department. This width will be insufficient to fulfil the planned cross-section for the Ring Road and an additional 15 m in width has to be acquired.

From the existing land use along the right-of-way, it is planned that for the section from Phetchaburi Road to Lat Phrao Road, the expansion will be to the east side of the existing railway right-of-way and for the section from Lat Phrao Road to Mittraphap Road the expansion will be to the west (or south) side of the existing right-of-way.

If it is decided that the Ring Road is to be planned within the available 60 m in width without any new acquisition, then it is necessary to reduce the width of separators and the sidewalks, resulting in certain lowering in efficiency of the Ring Road.

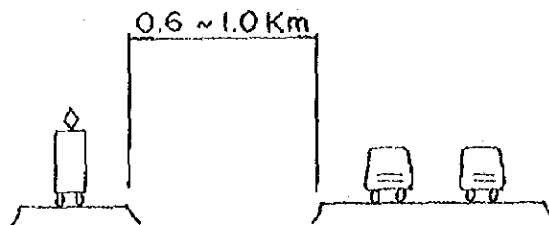
(c) The geometric structure of the Ring Road

This alternative is the shortest route of all the alternatives under comparison. However, as mentioned in (ii) above, there are some problem in alignment and profile. Also, as continuous elevation will be necessary for the section from Mittraphap Road to Khlong Prem Prachakon, it becomes an expensive alternative despite being the shortest route.

(4) The route of alternative 2A (case 2A).

(a) Location and form

In this alternative, the route is proposed at 0,6 - 1,0 km to the east (or north) of the existing railway right-of-way, with a width of 80 m. From the existing situation of the building up of the environing district, the alternative is considered most desirable, as it will serve also as the main trunk road for the newly developed residential estate in Ban Tambon Lat Yao.



(b) Relation with future railway line.

The Ring Road will be connected to the Mittraphap Road with an interchange. At the intersection point between the Mittraphap Road and the future railway line, it will therefore not be possible to elevated the Mittraphap Road to cross the railway, but the railway will have to overpass the Mittraphap Road. From the point of maximum gradient allowable to the railway, it will be necessary for the railway to be elevated continuously across both the Pahal Yothin Road and the Mittraphap Road.

(c) The geometric structure of the Ring Road

This alternative is the longest of all the alternative routes under comparison. Large curvatures are

planned for the curve sections, and, as the distance between intersecting existing road is longer than other alternatives, the distance between points of change in profile is consequently long enough for efficient traffic operation.

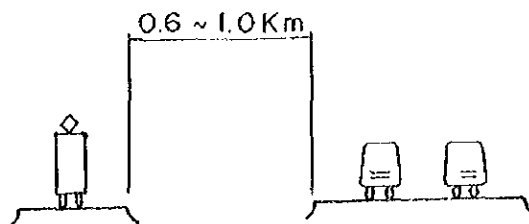
- (d) Position of intersecting point between Mittraphap Road and the Ring Road

After field reconnaissance, the intersection is planned at the open space between existing buildings such as Mazda Automobile factory, Pepsi Cola factory, District Police Department and Press Office. A tower for high tension wire stands on the extension of this route in the Chon Niwet District and further detail investigations will be necessary to decide the final route.

- (5) The route of alternative 2B (case 2B)

- (a) Location and form

For the section from Phetchaburi Road to Lat Phrao Road, the route for this alternative is the same as that for alternative 2A. Beyond Lat Phrao Road, the Ring Road intersects with the future railway line and runs to the south of it.



- (b) Relation with the future Railway Line

- i) Intersection between the railway and Mittraphap Road
As the Ring Road will be connected to the Mittraphap Road with an interchange, and future railway line will

have to overpass the Mittraphap Road for the same reason as in case 2A.

- ii) Intersection between the railway and the service road of the Ring Road.

The Ring Road will intersect with the future railway line at the vicinity of Sta. No. 85 and Sta. No. 115. At these intersections, the service roads will intersect with the railway at grade, resulting in reduction in efficiency as compared with other alternatives. Especially at the vicinity of Sta. No. 115, the service roads will intersect with both the future railway line and the existing northern railway line, and the role of the service roads as the distributor and collectors will thus be greatly impeded.

- (c) The geometric structure of the Ring Road

Large curvature is planned for the alignment of the route. However, for the section between Lat Phrao Road and the existing Northern Railway Line, there are four grade-separated intersections for a distance of about 4.5 kilometer, resulting in undulation in the profile. The distance between Pahol Yothin Road and Mittraphap Road is only 300 m, being the shortest of all the alternative routes, and weaving within such a short distance between the service roads and the high-speed lanes of the Ring Road is not particularly desirable from the traffic point of view.

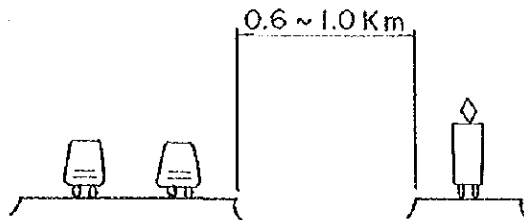
Also, two consecutive elevated parts will have to be planned for the crossing of the Mittraphap Road and the Northern Railway Line. The improvement of the profile by combining these two elevated parts into one

single structure will result in lengthening of the bridge and consequent increase in construction cost.

(6) The route of alternative 3 (case 3)

(a) Location and form

In this alternative it is proposed that the whole 80 m of the railway right-of-way be used by the Ring Road and a new right-of-way of also 80 m in width acquired and compensated to the Railway Department at 0.6 - 1.0 km to the east (or north) of the existing right-of-way.



(b) Relation with the future railway line

i) Intersection with the Mittraphap Road

Here again, the railway line will have to overpass the Mittraphap Road, as the later will be connected to the Ring Road with an interchange. From the maximum grade allowable to the railway, the position of the intersection will have to be 1.0 km north of the point of intersection between the Ring Road and the Northern Railway Line. For this reason the whole curve section of the railway has to be moved to about 1.0 km north of the existing railway right of way.

ii) Intersection with the service road of the Ring Road

With the moving of the future railway line to the east, the intersection at the vicinity of Phetchaburi Road is

eliminated.

On the other hand, at the intersection point between the Ring Road and the Northern Railway Line, the Ring Road will also have to cross the future railway line at the same time, resulting in more interruption of vehicle traffic at the railway crossing than Alternative 2A.

(c) Geometric structure of the Ring Road

Due to the removal of the railway line, the number of elevated sections of the Ring Road is reduced to the same as that for Alternative 2A. Moreover, the length of the elevated structure is short and large curvature can easily be planned for the alignment.

7-5-3 Section 7a(2), 7(b)

(Pracharat Road - Tachang Bridge Extension)

(1) Status quo

On the Bangkok side of this section, along the route of the proposed Ring Road, schools and wats are dotted sporadically over the region. Especially at the vicinity of Phibun Songkham Road, besides schools and wats, small-scale factories and residential houses are also widely scattered. The Thonburi side is mainly orchards and agriculture land.

(2) The location of the new bridge across Chao Phraya River

The existing bridge across the Chao Phraya River, the Rama VI Bridge, is of only two lanes in width, and the geometric structure of the access road is of low standard. The existing Rama VI Bridge will therefore not be sufficient to play the role as part of the Ring Road.

A new location for a bridge is chosen with due considerations that the construction will not cause disruption to the schools, wats or factories along the Phibun Songkham Road and that the alignment will avoid running over the thermal power station on the Thonburi side. The final proposed site of the bridge is chosen at some 900 m up-stream of the existing Rama VI Bridge.

- (3) The position of intersecting point between the Ring Road and the Southern Railway Line

The point where the Ring Road intersects with the Southern Railway Line is chosen in the orchard region so that the angle of intersection will not be too acute. For the section from the intersection of Tachang extension, considering the existence of wats and residential areas within this region, the proposed route is chosen as far to the west as possible, avoiding any high concrete buildings in the vicinity.

- (4) The stage construction of the new cross-river bridge and traffic handling.

The new bridge across the Chao Phraya River will be of about 300 m in span, and, together with the approach on both sides, will form a bridge of about 1.0 km in length. The construction cost for the bridge will be high and the time required for the construction will be long. It is therefore planned that service roads will not cross the river, but will end at the river banks, with provision for U-turn under the bridge.

As for the width of the roadway across the river, it is planned that the width of one half of the roadway (3 lanes) should be sufficient to accommodate traffic for 4 lanes with some sacrifice in width of each traffic lane, and that only the

outer half of the roadway will be constructed first, leaving the inner half to be constructed at a later stage. The part completed first will thus be able to accommodate traffic of 2 lanes in each direction. The second half will be constructed as a second independent bridge. To avoid adverse effects to the abutments of the first bridge, and to provide sufficient clearance for upper structure construction work, it is planned that the two bridge structures be constructed 15 m apart.

7-5-4 Section 7a(3)

(Tachang Extension - Phetkasem Road)

(1) Status Quo

The Tachang Bridge at present under construction will be extended west to intersect with the existing Charan Sanitwong Road and the proposed Ring Road and eventually extended further to the west.

The Ring Road is planned to meet the Tachang extension with an interchange. The proposed route for the Ring Road is chosen at about 300 - 500 west of the Charan Sanitwong Road. This will come to about 300 - 500 m to the east of Khlong Bangkok Yai. Around this region are sporadic existence of wats and schools.

(2) Selection of route

In the vicinity of the Khlong Bangkok Noi, the Tachang Extension and the Southern Railway Line, along the general region of the proposed route, are found such wats as Wat Bankunom, Wat Pen etc., and the section from Khlong Mon to Phetkasem Road is dotted sporadically with schools and wats. With these wat and schools as control points, the route is finally chosen some 300 - 500

to the west of Charan Sanitwong Road,

The Part I of the Ring Road will initially end at the Tha Phra Junction. However, it is planned that on completion of the whole ring, the Ring Road will intersect with Phetkasem Road at some 350 m west of the Tha Phra Junction.

7-6 Route Selection for Section 5

7-6-1 Status Quo

At the vicinity of the junction between Rama IV Road and the Ring Road Part I, the Rama IV Road branches into Sunthon Kosa Road which leads to the Port area and the through road which meets the Sukhumvit Road before turning south to the Pattaya direction. At this branching point, the Rama IV, with a 8 lane roadway on the west, abruptly reduces to 4 traffic lanes on the east. Moreover, the alignment takes a sharp S shape at this point, and the maintenance of adequate sight distance is difficult.

The Rama IV Road meets the Shukhumvit Road at the vicinity of Soi 69, near which, the Soi 71 (Phrakhanong-Khlong Tan) is the major link connecting the Sukhumvit Road to the Phetchaburi Road and to Lat Phrao Road to the north.

7-6-2 Basic concept in the selection of alternative routes

(1) Type of alternative routes

An estimated 200 thousand vehicles per day is forecast for the year 1990 for the Rama IV Road and the Section 5 in total. Four alternative routes were chosen for comparative study and alternative 2, which proposes the overall utilization of the Khlong Toei, is considered the best alternative. The four alternatives are:

alternative 1 : Improvement and expansion of the existing Rama IV Road

alternative 2 : Overall utilization of the Khlong Toei

alternative 3 : Utilization of the northern bank of the Khlong Toei.

alternative 4 : A route south of the Khlong Toei running through the port area.

(2) Cross section composition

The cross section composition of the Section 5 is proposed to be the same as the Ring Road with high-speed roadway in the centre and service roads on both sides, because the results of traffic analysis shows that some 65 % of the traffic on Section 5 will directly flow into the Ring Road.

(3) Type of connection with the Ring Road

As the majority of the traffic on Section 5 will use the Ring Road, and the traffic volume at the point of connection will be heavy, purely from the point of traffic treatment, it is desirable to have the Section 5 connected directly to the Ring Road with some sort of full interchange. However, the connecting point is within heavily built-up area, where cost of land and compensation for building demolition will be high. The construction of a full interchange (such as a 3 level Y-type junction or a trumpet-type interchange) from the initial stage will be very costly, and it is more desirable that stage construction be considered, and the full structure carried out at a later stage when traffic situation necessitates. In this case one of the methods that may be adopted is the reconstruction or improvement of the access roads in the vicinity and the control and regulation of traffic (e. g. enforcement of one way traffic etc.) so that these access roads may serve the purpose of a clover-leaf type interchange.

In the case that a full interchange will eventually be constructed, the types that may be adopted are the 3-level Y-type junction and the single-trumpet-type interchange. Of the two types, the Section 4 of Ring Road Part I being to be elevated, considering the ease in stage construction

and the low cost of construction in the initial stage, it can be said that the 3-level Y-type junction is more preferable.

- (4) Connection with national highway route No. 34 at the Eastern End.

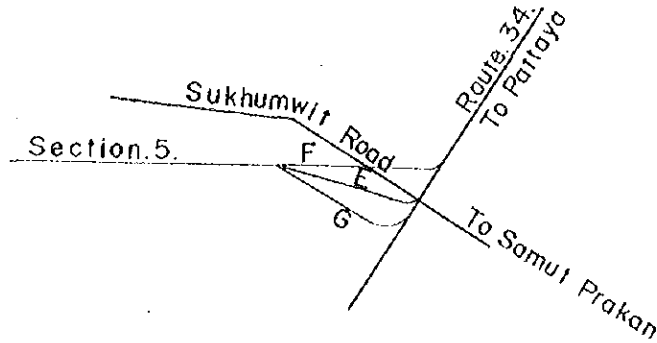
As previously mentioned, 4 alternative routes are considered for the western half of Section 5 from Sta. No. 0 to Sta. No. 35. From Sta. No. 35 to Sta. No. 50, due to the existence of schools and the Bangkok Oil Refinery, only one route is proposed in between Chao Phraya River and Sukhumvit Road. For the eastern terminal of the Section 5 at around Sta. No. 70, three alternatives as shown in the following sketch are studied.

The first route (route F) is that the route chosen for Sta. No. 35 - Sta. No. 50 be extended in a straight line to fly over Sukhumvit Road and then connected directly to National Highway Route No. 34.

The second route (route G) connects the Section 5 to the minor road which leads to the River. In this case, a flyover at the intersection with the Sukhumvit Road will be necessary for smooth traffic flow.

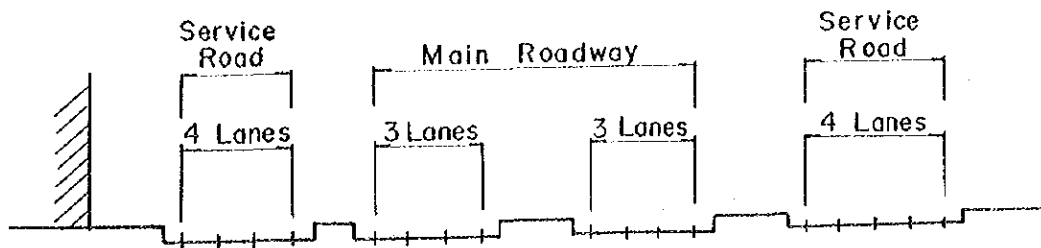
The third route (route E) puts section 5 directly into the existing intersection. In this case, improvement in channelization of the existing intersection will be absolutely necessary. Also, the traffic lanes joining Route 34 and the Section 5 will have to be elevated. However, as the elevated lanes will be a curve, there is physical limit in securing the length of accelerating and decelerating lane. For this reason, the design speed of the Section 5 will have to be sacrificed to 40 - 60 km/hr.

Traffic analysis shows that about 65% of the traffic entering the Section 5 at this point will be from Route 34. To maintain the function of Section 5 as a high-speed highway, it can be said that route F is most preferable. However, a decision on this will have to be made together with consideration on the role to be assignment to Section 5, and further study in this respect is most necessary.



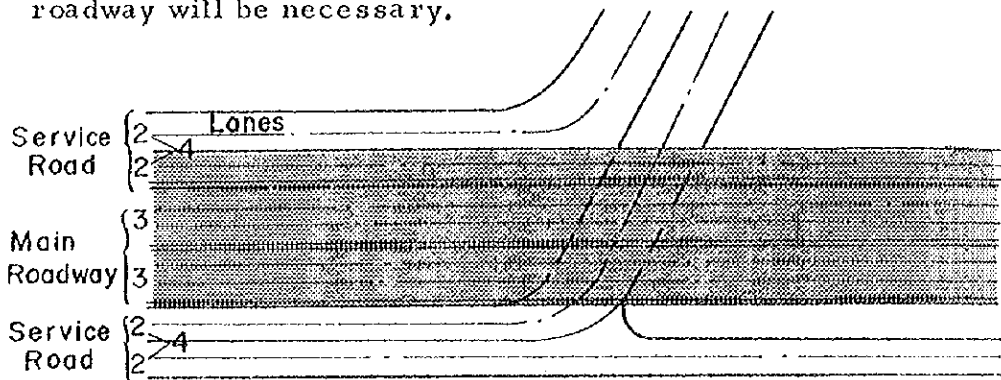
7-6-3 Alternative 1. (The Improvement and Expansion of the Existing Rama IV Road)

This alternative calls for the expansion of a section of the existing Rama IV Road which is, at this section, with 4 traffic lanes. As the number of lanes required for the Ring Road with service road is 10 traffic lanes, the expansion will result in a 14 lane highway. (Figure below).



The width of the roadway will thus come to about 100 m. However, due to the reasons described below, such a plan is not practical from the points of traffic operation and construction cost.

- (i) The ground level of the existing Rama IV Road does not reach the level planned for the cross-section of the Ring Road. Due to the existence of building on both side of the existing road, it will not be possible to raise the ground level.
- (ii) During construction, traffic on the existing road will suffer great inconvenience. Also, heavy burden will be put on the Sukhumwit Road which will be the detour route.
- (iii) At Station No. 15 (in the vicinity of Posteri Anuson School), the Ring Road will diverge from the Rama IV Road. For this reason, the elevation of the 6-lane high-speed roadway will be necessary.



- (iv) At present it is possible for vehicle and pedestrians to cross the Rama IV Road at places with traffic light control. However, as the high-speed lanes will be separated with separators, crossing of vehicles will not be possible. As for pedestrians, pedestrian-crossing bridges of over 100 meters will have to be provided, and this will be greatly inconvenient.
- (v) The expansion of one side of the existing road will be adversary aesthetically.
- (vi) Both sides of the existing road are densely lined with concrete-structure buildings of over 3 storeys. The cost

of land here will be around 3,000 baht/m² or about 6 times that along Khlong Toei, which is at around 500 baht/m². The cost of demolition will also be more than 5 times costlier. Together with the construction of the bridge at Station No. 51, the total construction cost will be extremely high.

The alignment from Station No. 0 to No. 5 will be of a S-shape curve, and will not be desirable from the point of safety in operation.

- (viii) The future connection with the Ring Road Section 4 will necessarily be of the trumpet-type, as the Y-type direct junction will not be possible.

7-6-4 Alternative 2 (The full use of the Khlong Toei)

In this alternative, Khlong Toei will be reclaimed and used as the roadway, and residents on both sides of the Khlong will be removed.

The Khlong Toei is highly polluted, and is serving no other purpose than as an outlet for the sewerage of the vicinity, and as an unloading point for construction materials such as sand and gravel. It is possible therefore, to make use of the full width of 70 m which includes the width of the existing roads on both sides of the khlong. A waterway of 7 m in width will be left in the middle for drainage purpose. In this alternative, large quantity of fill material will be required for the reclamation of the khlong. The cost of land acquisition and demolition will be much lower than that of other alternatives. The alignment of the route will be extended to the city center through Rama IV Road in a straight line. It is also superior in the connection of the existing built up area to the port area. This alternative is most desirable from every aspect.

7-6-5 Alternative 3 (Route on the northern bank of Khlong Toei)

In this alternative, the route will run over schools, Military Pharmaceutical building as well as Leather Tanning Organization, and the acquisition and reparation of these facilities will remain a big problem.

Moreover, as the route will be flanked on the southern side by Khlong Toei, development effect cannot be too greatly expected, resulting in a delay in the improvement of Khlong Toei and the clearing of the low standard residents along the Khlong.

The route is about 150 m away from Rama IV Road and the cost of land will be about 1500 baht/m², or 3 times that for the Khlong Toei route.

7-6-6 Alternative 4 (Route through the port area)

This alternative calls for the expansion of At Narong Road and extending the route east, running parallel to Khlong Toei at about 300 m away. The route will branch out from Rama IV Road at an angle of about 60°, run in a straight line for about 300 m, and again turn at 60° angle to return to a parallel course with the Khlong Toei.

Along this route are lined many 3 storey shophouse-cum-residential buildings, and the demolition of these buildings will be necessary in road expansion work. When the route is eventually connected to the Ring Road through an interchange, the elevated section will have to be sufficiently long to allow clearance with the Rama IV Road, resulting in increase in construction cost.

Also, the Rama IV Road will be connected to the Ring Road with a split diamond type interchange. As heavy traffic volume

is anticipated at this junction, traffic control will remain a problem.

The route from Station 10 to Station 30 will greatly reduce the available land for the port, and at this section, the rerouting of the railway line is necessary. This will result in the necessity for a new railway bridge across Khlong Phra. The cost for such rerouting will be high, and the problem of the effective use of the land in relation to the trunk road remains to be solved.

The alignment of the route will be a continuation of curves to avoid factories, Wat Saphra and Bangkok Oil Refinery, on both sides of Khlong Phra.

7-7 Interchange

7-7-1 The location of interchanges

For the connection between the Ring Road or Section 5 with other existing or future roads, full interchanges are planned for interchanges with the future East-West Highway, the Mittraphap Road (Super Highway) and the Tachang Extension, Y-shaped direction junction planned for junction between Section 5 and Section 4, and direct junction for the terminal of Section 5 at National Highway Route No. 34. Diamond interchanges are planned for all other connections from the point of economy.

The full list of interchanges are as follows:

(1) Ring Road Part II

| | | |
|-----|---------------------------------|--------------------------------------------|
| 1) | Phetchaburi Road | diamond interchange |
| 2) | Future Din Daeng Road extension | diamond interchange (left turn ramps only) |
| 3) | Future East-West Highway | full clover leaf - type interchange |
| 4) | Future town planning Road | diamond interchange |
| 5) | Lat Phrao Road | " |
| 6) | Pahol Yothin Road | " |
| 7) | Mittraphap Road (Super Highway) | partial clover leaf type interchange |
| 8) | Prachachun Road | diamond interchange |
| 9) | Pracharat Road | " |
| 10) | Phibun Songkhram Road | half diamond interchange |
| 11) | Tachang Extension | full clover leaf type interchange |

- | | | |
|-----|---------------------------------|---------------------|
| 12) | Bangkok Noi Taling Chan Road | diamond interchange |
| 13) | Phet Kasem Road | " |

(2) Section 5

- | | | |
|----|------------------------------------|--------------------------|
| 1) | Rama IV Road and Section 4 | Y-shape direct junction |
| 2) | Soi Paknan (Khlung Phrakhanong) | diamond interchange |
| 3) | Soi 54 | " |
| 4) | Sukhumwit Road | half diamond interchange |
| 5) | Route 34 | direction junction |

The average distance between interchange is about 2 kilometers.

Consideration has to be provided that there is a limit to the capability of service to right-turning traffic in the case of a diamond interchange. Also, to enable efficient functioning of the Ring Road, it should be ensured that traffic on other roads should have easy access to the service road (frontage road) of the Ring Road.

Half-clover leaf type interchanges may be considered in place of diamond interchanges. However, in this case, right turning will still occur for traffic on the low standard roads. The full clover-type interchange provides separation for the major traffic stream direction. In this case, large area of land will be necessary and the cost of land acquisition and construction will be high. The adoption of the full clover leaf type will therefore have to be made with consideration for the future land use of the vicinity.

In this study, the main roadway of the Ring Road is planned to intersect all other roads with grade separation, and the role of collection and distribution left to the service road. In this case, it is necessary that proper control of access from the service road to the main roadway or vice versa be made, to provide separation for through traffic and in or out going traffic.

7-7-2 Interchange with future East-West Highway

(1) Location

The Ring Road is planned to be connected to the future East-West Highway with a full interchange.

The position of the interchange has to be selected so that sufficient length of speed-change lane can be secure after the Ring Road, having crossed the Phetchaburi Road and the future railway line, reaches the ground level.

Again, near this point the future Din Daeng Road is expected to run parallel to the Khlong Samsen, and provision for the access of this road into the Ring Road has to be made.

With all these points taken into consideration, it is proposed that the full interchange should be planned at 0.8 - 1.0 km north of the Phetchaburi Road.

(2) Type

The future railway line will cross the Ring Road near the interchange, and the future East-West Highway will cross the Ring Road as an elevated section. Under such conditions, full service to all directions should be provided with a double trumpet type or a full clover leaf type interchange.

A double trumpet type interchange will require the crossing of the Ring Road with a bridge, and will also require bigger area. For this reason, it is considered that a full-clover-

leaf type is suitable. In this case, the service roads will fully serve the purpose of collecting and distributing traffic to and from the high-speed traffic lanes.

7-7-3 Interchange with the Mittraphap Road (Super Highway)

(1) Location

The Super Highway is the main trunk road of Bangkok connecting the town centre of Bangkok with the Don Muang Airport and further north with the north and northeastern regions of the Kingdom.

The interchange is planned at about 2 km south of the intersection between Super Highway and Ngan Wongwan Road. At the later intersection, grade separation is being planned to replace the existing at grade junction.

At about 2 km south of the proposed interchange, the Mittraphap Road crosses with the Pahol Yothin Road at grade.

Grade separation construction work is at present underway.

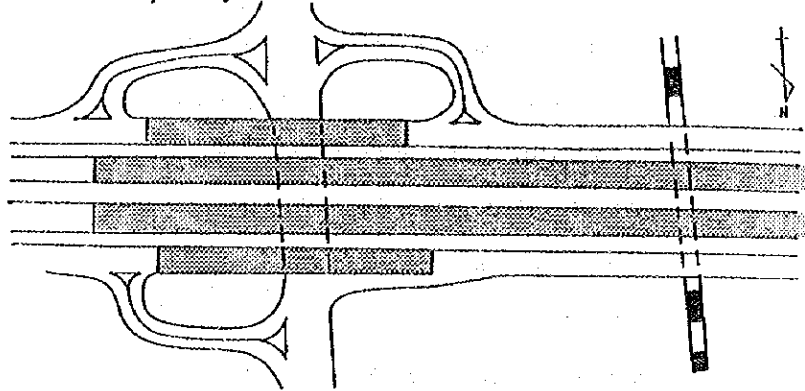
(2) Type

The Super Highway is under improvement to become a controlled access highway eventually rid of traffic lights. The interchange with the Super Highway will have therefore to be such that proper channelization between the two highways is possible. It is proposed that a clover-leaf type interchange, whereby all main traffic streams be guided into loops, be adopted. In this case, two basic types of in and out flow are considered, and the type (a) where the service roads serve as the collector and distributor roads is considered more desirable.

(a) Collector and distributor road type

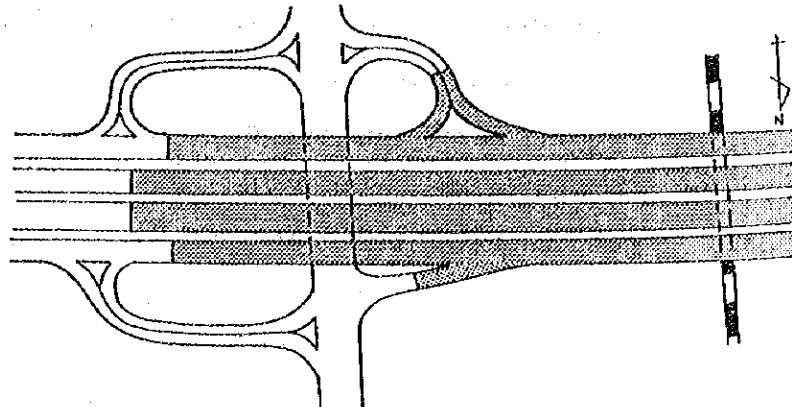
- (i) Intersection of the service road with the railway line at grade

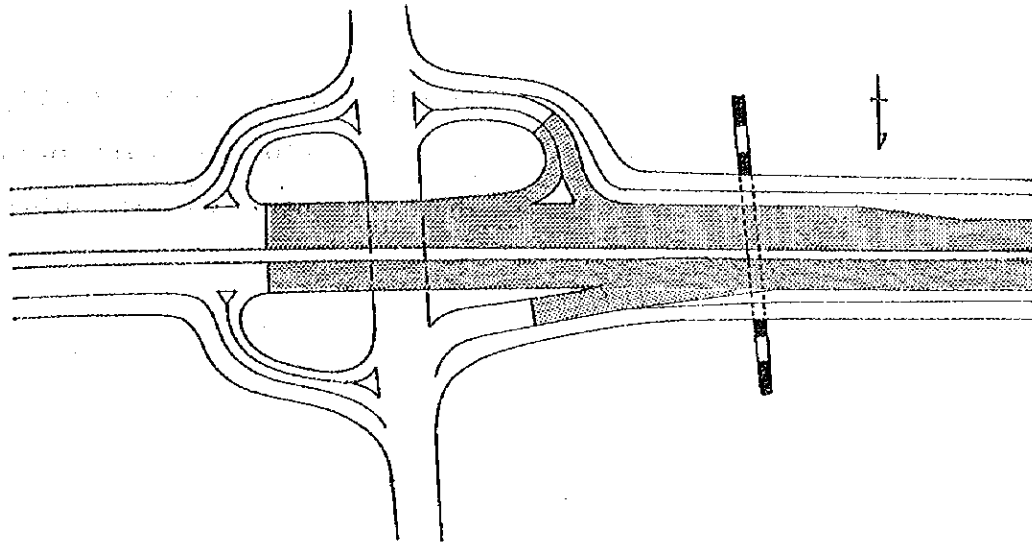
In this type the service roads play the role of collector and distributor roads for the high speed traffic lanes but will cross the Northern and Northeastern Railway at grade. During stage construction, work will start with the service road and weaving occurs on the elevated section of the service road. However, construction cost will be very low, and the whole interchange can be constructed with only earth work without any major structures.



- (ii) Intersection of service road and railway with grade separation

Basically this is the same type as (i), with the difference that the service roads also cross the railway lines with grade separation. On this type, some ramps of the interchange are bridge structures, and the construction cost is higher.





(b) Direct access type

The in and out flow traffic will make access to the service road directly from the high speed traffic lanes in this type. When the traffic volume is heavy, this type will considerably lower the capacity of the highway. Also, as the service roads are discontinued at the interchange, the construction of the main roadway will have to be carried out together with the interchange during stage construction.

7-7-4 Interchange with Tachang Extension

(1) Location

The extension of the Tachang is at present planned to stop at the Charan Sanitwong Road, but will eventually be extended further west. The interchange with the Ring Road will be planned at the orchard district with the Ring Road running at ground level.

(2) Type

The interchange adopted here will be a full clover-leaf type with the service road serving as collector and distributor road, the same as that for the intersection with the future East-West Highway.

7-8 Structures

7-8-1 Criteria for design of structures

(i) Design load H20-S16-44 (AASHO)

(ii) Allowable stress

Substructures:

Factory made piles

| | |
|----------------------|--------------------------|
| concrete | 84 kg/cm ² |
| reinforcing bar | 1,400 kg/cm ² |
| prestressed concrete | 350 kg/cm ² |

Others: in accordance to Thailand Industrial Standard

(iii) Clearance limit

Railway : 5,100 m (from top of rail)

Highway : 5,000 m

Khlong : Navigation clearance : H. W. L. + 3,500m
Navigation channel : 30,0 m

Major rivers: Navigation clearance: H. W. L. + 5,500m
Navigation channel : 60,0 m

(The figures for khlongs will be a guiding standard.

In principal, the clearance for existing bridges will be adopted).

(iv) Specification : 1) The Standard of the Americal Association of State Highway Officials (ASSHO)
2) Thailand Industrial Standard
3) The Standard of the Thai Department of Highways.

7-8-2 The decision on type of bridge

(1) Superstructure

(a) New bridge across Chao Phraya River (New Rama VI Bridge)

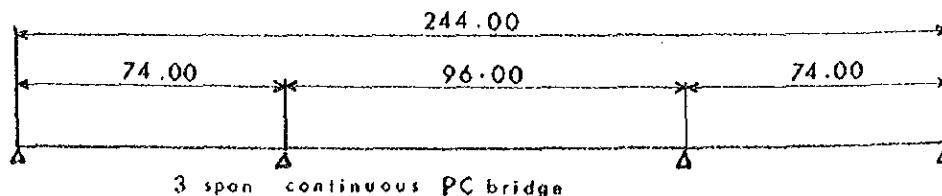
The decision on the type of superstructure for the new bridge across Chao Phraya River (the New Rama VI Bridge), is made basing on findings on the span, clearance, and width of navigation channel for the existing Rama VI Bridge, and a conclusion is reached that a centre span of at least 100 m, and a clearance of MSL + 11.500 m will be sufficient.

The location chosen for the new bridge is highly sensitive to tide movement, and the difference in water level between high and low tide is great. The velocity of water flow is estimated at a maximum of 2.5 m/sec. All these were taken into consideration in the study.

Two different types were chosen as alternatives for study, the one being of prestressed concrete (D & W construction method) bridge, and the other being continuous box girder metal bridge.

(i) Three-span PC bridge (D & W construction method)

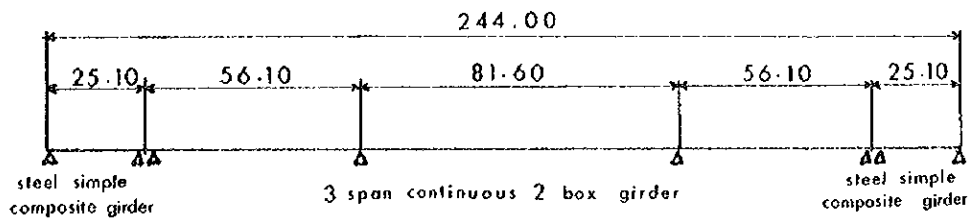
The span division is as below.



In the D & W construction method, construction work will be carried out in cantilever fashion and will not cause any impediment to traffic on the river during construction. Also, maintenance work after completion will be easier and cheaper than that for a metal bridge. The construction work will be a repetition of simple routine steps and the training for skilled workers will be easier than that for a metal bridge. However, there is a demerit that the construction period will be longer than that for a metal bridge.

(ii) Three-span continuous two-box-girder metal bridge

The span division will be as follows:



In the case of the three-span continuous two-box-girder metal bridge, there is an advantage that the level of profile will be lower by 3.4 m (due to difference in height of the wagon mould during construction and in height of supporting point) and the length of the bridge will be shorter than a PC bridge. The construction period is shorter, and there are less technical problems in construction. Construction work will be carried out by the floating crane method.

(iii) Choice of type

From the comparative study of the above alternatives, it is considered that P. C. bridge is more advantageous from points of engineering and construction cost, and further works are carried out with the P. C. bridge.

(iv) Approach to the bridge

From the points of economy, ease in erection and speed in construction, the PC composite I-girder is adopted for the superstructure of the approach section. Also, to avoid excess settlement of the embankment for the approach, the bridge of the approach is extended until it reaches a height of 1.0 - 1.5 m above ground level, where it is continued with earth work.

(b) Superstructure for elevated section and for bridges over khlongs

For the superstructure of elevated sections of the roadway and for bridge over khlongs, the PC pre-tension girder type is adopted where the span is from 10.0 to 21.0 m, and reinforced concrete slab bridge adopted where the span is below 10.0 m.

For span of over 21.0 m, the post-tension composite I-girder is adopted. The dimensions are as below.

(i) Reinforced concrete slab girder = 5.0m - 10.0m

| | | | | | | |
|-----------------------|-----|-----|-----|-----|-----|------|
| length of girder (m) | 5.0 | 6.0 | 7.0 | 8.0 | 9.0 | 10.0 |
| height of girder (cm) | 30 | 34 | 36 | 39 | 42 | 45 |

(ii) Prestressed concrete box girder = 10.0m - 21.0m

| | | | | | | |
|-----------------------|------|------|------|------|------|------|
| length of girder (m) | 12.0 | 14.0 | 16.0 | 18.0 | 20.0 | 21.0 |
| height of girder (cm) | 50 | 60 | 70 | 70 | 70 | 75 |

(c) Span division and clearance of major khlongs

A survey of the span division and clearance of existing bridges over major khlongs were carried out.

In this study, in principle, the same span division and clearance is to be adopted for bridges over the same khlongs and situated near the existing bridges.

The following are the results of the survey.

Dimension of Existing Bridges over khlongs

Unit : m

| Section | Name of Khlong | Span Division | Clear- ance | Type of Superstructure |
|---------|----------------|-----------------------|----------------|---------------------------|
| 6 | Bang Sue | 13.80 | 2.70 | PC |
| 7a | Prem Prachakon | 23.00 | 5.20 | RC cantilever |
| 7a | " | 20.50 | 2.20 | Railway bridge |
| 7a | " | 15.0 + 12.0 + 15.0 | 6.00 | PC |
| 7a | Bangkhen Mai | 6.0 + 8.5 + 6.0 | 4.10 | PC Slab |
| 7a | Bangkok Noi | 20.0 + 20.0 + 20.0 | 6.60 | RC hollow girder |
| 7a | Mon | 12.00 + 16.00 + 12.0 | 4.20 | RC Slab |
| 7a | " | 10.0 + 10.0 + 10.0 | 3.00 | " |
| 5 | Phrakhanong | 12.30 + 12.30 + 12.30 | 7.00 | RC |
| 5 | " | 12.20 + 16.00 + 12.20 | 3.60 | RC |

With reference to the above table, and with due consideration for the conditions of the site of proposed bridge, the dimensions of the proposed bridges for the Ring Road are decided as follows:

Unit : m

| Section | Name of Khlong | Span Division | Clear- ance | Type of Superstructure |
|---------|----------------|-----------------------|----------------|---------------------------|
| 6 | Bang Sue | 20.0 + 20.0 + 20.0 | 3.60 | PC |
| 7a | Prem Prachakon | 16.0 + 16.0 + 16.0 | 5.00 | PC |
| 7a | Prapa | 10.0 + 10.0 + 10.0 | 3.00 | RC |
| 7a | Bangkhen Mai | 10.0 + 10.0 + 10.0 | 3.00 | RC |
| 7a | Bangkok Noi | 20.0 + 20.0 + 20.0 | 5.00 | PC |
| 7a | Mon | 10.0 + 10.0 + 10.0 | 3.00 | RC |
| 5 | Phrakhanong | 20.00 + 20.00 + 20.00 | 3.60 | PC hollow girder |

(2) Type of Substructure

(a) New Rama VI Bridge

As no detailed soil survey was carried out at the proposed site of the New Rama VI Bridge, data of the soil profile was not available, and in this study, a deduction was made from the soil profile of four other existing bridges across the Chao Phraya River.

From these data, it is found that the four existing bridges are supporting on the 'stiff brown-gray clay with laterite stratum' at a depth of 20 - 24 m, and that the soil profiles are similar in all cases. (Fig. 5-15) It can be deduced therefore that the soil profile at New Rama VI Bridge site will also be of similar formation.

However, the report on the Memorial Bridge (T. P. O-Sullivan 1971), states that scouring is found due to meandering of the river, and that a founding level of -29.700 m M. S. L. was indicated. The river shows the same meandering about 500 m upstream of the proposed site, and the possibility of scouring on the right bank cannot be ignored. In this study, we have therefore planned the supporting stratum at the "sandy clay, coarse sand" layer at around -30.0 m M. S. L. Confirmation of this assumption at the design stage through detail soil investigation is of course necessary.

For the study on the substructure of the New Rama VI Bridge, studies are made for the two general types of (i) Caisson foundation and (ii) pile foundation.

(i) Caisson foundation

The four existing bridges across the Chao Phraya River (Krung Thep, Krung Thon, Memorial, and Nonthaburi) are all with caisson foundation at a supporting level of about M. S. L. -22.0 m.

The caisson foundation is widely adopted in construction of bridges with great spans for its high stability and high bearing power. On the other hand, the construction cost is high and the construction period required is longer. In the case of the New Rama VI Bridge, where the depth is around 15 - 20 m and the speed of water flow at 1.0 - 2.5 m/sec., there may be problem in construction whether with build-island method or floating method. Also, with the supporting stratum at below M. S. L. -30 m, there may be difficulties sinking the caisson through the stiff clay layer. (at M. S. L. -22 - -27 m).

(ii) Pile foundation

Pile foundation may be classified into cast-in-place large diameter reinforced concrete pile and large diameter steel piles. The use of steel piles will incur the problem of availability of the material. In this study, consideration is made of only the cast-in-place reinforced concrete pile.

From the condition of the construction site, availability of equipments, construction cost, and length of piles required, it is considered that the reverse circulation drill construction method is most desirable. This method is also adopted in the construction work of the Tachang Bridge with success. The reverse circulation drill method multi-pile type, with diameter of pile of 1.5 m, is therefore recommended for adoption for the New Rama VI Bridge. The allowable bearing power for a 1.5 m diameter pile will be 430 tons/pile.

(b) Substructure for elevated sections and for bridges over khlongs.

Rigid frame pier is adopted as the substructure for the elevated section of Ring Road, using reinforced concrete pile or prestressed concrete pile as foundation pile.

For the bridges over khlongs, the pile-bed type pier is adopted, whereby the substructure is unified with the foundation pile. The pile used will be reinforced concrete pile. The dimension of piles are as follows.

| Type of pile | Dimension | Bearing power (ton/pile) |
|---------------------------|-----------------|--------------------------|
| Reinforced concrete pile | 25 cm x 25 cm | 20 |
| " | 35 cm x 35 cm | 45 |
| Prestressed concrete pile | 35 cm x 35 cm | 60 |
| " | 40 cm octagonal | 70 |

(length of above piles = 20 - 24 m)

7-9 Estimation of Construction Cost

7-9-1 Conditions for the estimation of construction cost.

The estimation of construction is carried out under the following conditions.

- (1) The estimation is made at 1975 value, with an assumption of increase in labour and material cost from the present level at 3.5 % per annum.
- (2) The unit cost also includes the following items.
 - (a) Overhead cost for contractor 20 %
 - (b) Cost of temporary road, lighting, pedestrian bridge, repairs of existing facilities, insurance, etc. 10 %
 - (c) Cost of design, survey and supervision 10 %
 - (d) Contingency 10 %

7-9-2 Unit cost

The unit cost used in estimation are as follows.

- (1) Earth work
 - (a) Ring Road : Service road 20,000 baht/m
Main roadway 18,000 baht/m
 - (b) Section 5 : Service road 19,500 baht/m
Main roadway 48,700 baht/m
- (2) Bridge
 - (a) Reinforced concrete slab bridge (10 m span) 3,500 baht/m
 - (b) Prestressed concrete bridge (20 m span) 5,800 baht/m
 - (c) New Rama VI Bridge 19,600 baht/m

Details of Unit Cost for Earth Works & Road Works

| | Item | Unit | Unit Cost (baht) |
|-----|-------------------------|----------------|---------------------|
| 1. | Clearing & Grubbing | m ² | 3 |
| 2. | Excavation | m ³ | 32 |
| 3. | Embankment | m ³ | 77 |
| 4. | Drainage A (cross pipe) | m | 660 |
| 5. | Drainage B (φ 100) | m | 1,500 |
| 6. | Asphalt Surfacing | m ² | 35 |
| 7. | Asphalt Binder Course | m ² | 50 |
| 8. | Asphalt Stabilized Base | m ² | 97 |
| 9. | Sub-base Course | m ² | 36 |
| 10. | Curb Stone | m | 170 |
| 11. | Guard Rail | m | 400 |
| 12. | Main Strip | m | 400 |
| 13. | Side Strip | m | 350 |
| 14. | Pedestrain Walkway | m | 250 |

Details of Unit Cost for Grade Separation Structures

| | Item | Unit | Unit Cost (baht) |
|-----|-------------------------|----------------|---------------------|
| | SUPEP-STRUCTURE | | |
| 1. | Class A Concrete | m ³ | 810 |
| 2. | Class B Concrete | m ³ | 520 |
| 3. | Form Work | m ² | 200 |
| 4. | Falsework | m ² | 98 |
| 5. | P. C. Cable 12.7 mm | ton | 53,000 |
| 6. | Reinforcing Bar | ton | 6,400 |
| 7. | Bearing | each | 600 |
| 8. | Erection of Girders | ton | 812 |
| 9. | Drainage | each | 150 |
| 10. | Expansion Joint | m | 3,510 |
| 11. | Curb | m | 630 |
| 12. | Pavement | ton | 330 |
| | SUB-STRUCTURE | | |
| 13. | Class C Concrete | m ³ | 930 |
| 14. | Form Work | m ² | 220 |
| 15. | Falsework | each | 50,000 |
| 16. | Reinforcing Bar | ton | 9,000 |
| 17. | P. C. Pile | each | 8,860 |
| 18. | Excavation and Backfill | m ³ | 100 |
| 19. | Load Testpiece Pile | each | 35,000 |

Estimation of Construction Cost

Ring Road Part II, Case 1 L = 26.69 Km Ring Road Part II, Case 2A L=27.59 Km Thousand bahts

| | Section A Petchaburi - Superhighway | Section B Superhighway - Pracharat Road | Section C Pracharat Road - Tachang Ext. Phekasem | Section D Tachang Ext. - Phekasem | Total | Section A Petchaburi - Superhighway | Section B Superhighway - Pracharat Road | Section C Pracharat Road - Tachang Ext. Phekasem | Section D Tachang Ext. - Phekasem | Total |
|----------------------------|-------------------------------------------|--------------------------------------------------|--------------------------------------------------------------|--------------------------------------------|-----------|-------------------------------------------|--------------------------------------------------|--------------------------------------------------------------|--------------------------------------------|-----------|
| Land Acquisition | 217,775 | 136,275 | 125,200 | 77,920 | 557,170 | 271,600 | 129,200 | 125,200 | 77,920 | 603,920 |
| Demolition | 2,400 | 35,250 | 22,400 | 24,000 | 84,050 | 25,000 | 18,000 | 22,400 | 24,000 | 89,400 |
| Viaduct (1) Main Rd. | 166,896 | 300,139 | 440,689 | 270,180 | 1,177,904 | 158,688 | 270,864 | 440,689 | 270,180 | 1,140,421 |
| Viaduct (2) Service Rd. | 17,192 | 59,173 | 8,596 | 29,226 | 114,187 | 17,192 | 59,346 | 8,596 | 29,226 | 114,360 |
| Roadway (1) Main Road | 115,110 | 40,698 | 100,620 | 75,150 | 331,578 | 128,520 | 46,420 | 100,620 | 75,150 | 352,710 |
| Roadway (2) Service Rd. | 170,300 | 79,100 | 146,200 | 113,400 | 509,000 | 182,920 | 84,480 | 146,200 | 113,400 | 527,000 |
| Interchange | - | 30,000 | - | - | 30,000 | - | 30,500 | - | - | 30,500 |
| Sub-total | 689,673 | 680,635 | 843,705 | 589,876 | 2,803,889 | 783,920 | 640,810 | 843,705 | 589,876 | 2,858,311 |
| Cost for Railway | 268,800 | 99,360 | - | - | 368,160 | 57,600 | 86,400 | - | - | 144,000 |
| Total | 958,473 | 779,995 | 843,705 | 589,876 | 3,172,049 | 841,520 | 727,210 | 843,705 | 589,876 | 3,002,311 |

Estimation of Construction Cost

| Ring Road Part II, Case 2B | | L = 26.84 Km | | Ring Road Part II, Case 3 | | L = 26.69 Km | | Thousand bahts | | |
|----------------------------|-------------------------------------------|--------------------------------------------------|--------------------------------------------------|---------------------------------------------|-----------|-----------------------------------|--------------------------------------------------|--------------------------------------------------|---------------------------------------------|-----------|
| | Section A Petchaburi - Superhighway | Section B Superhighway - Pracharat Road | Section C Pracharat Road - Tachang Ext. | Section D Tachang Ext. - Phetkasem | Total | Section A Petchaburi - Road | Section B Superhighway - Pracharat Road | Section C Pracharat Road - Tachang Ext. | Section D Tachang Ext. - Phetkasem | Total |
| Land Acquisition | 266,800 | 119,600 | 125,200 | 77,920 | 589,520 | 259,200 | 129,000 | 125,200 | 77,920 | 591,320 |
| Demolition | 37,000 | 25,800 | 22,400 | 24,000 | 109,200 | 61,400 | 59,250 | 22,400 | 24,000 | 167,050 |
| Viaduct (1) Main Rd. | 240,768 | 300,960 | 440,689 | 270,180 | 1,252,597 | 166,896 | 300,139 | 440,689 | 270,180 | 1,177,904 |
| Viaduct (2) Service Rd. | 17,192 | 59,346 | 8,596 | 29,226 | 114,360 | 17,192 | 59,346 | 8,596 | 29,226 | 114,360 |
| Roadway (1) Main Road | 111,060 | 37,620 | 100,620 | 75,150 | 324,450 | 115,110 | 40,698 | 100,620 | 75,150 | 331,578 |
| Roadway (2) Service Rd. | 176,600 | 75,800 | 146,200 | 113,400 | 512,000 | 170,300 | 79,100 | 146,200 | 113,400 | 509,000 |
| Interchange | - | 31,000 | - | - | 31,000 | - | 30,000 | - | - | 30,000 |
| Sub total | 849,420 | 650,126 | 843,705 | 589,876 | 2,933,127 | 790,098 | 697,533 | 843,705 | 589,876 | 2,921,212 |
| Cost for Railway | 57,600 | 86,400 | - | - | 144,000 | 57,600 | 105,600 | - | - | 163,200 |
| Total | 907,020 | 736,526 | 843,705 | 589,876 | 3,077,127 | 847,698 | 803,133 | 843,705 | 589,876 | 3,084,412 |

Estimation of Construction Cost

Section 5

L = 7.06 Km.

(thousand bahts)

| | <i>Case 1</i> | <i>Case 2</i> | <i>Case 3</i> | <i>Case 4</i> |
|----------------------------------|---------------|---------------|---------------|---------------|
| Land Acquisition | 430,800 | 107,800 | 206,100 | 72,500 |
| Demolition | 398,000 | 70,000 | 365,000 | 122,000 |
| Viaduct (1) Main Rd. | 292,975 | 224,575 | 251,935 | 292,975 |
| Viaduct (2) Service Rd. | 26,525 | 26,525 | 26,525 | 26,525 |
| Roadway (1) Main Road | 81,800 | 161,400 | 87,200 | 90,800 |
| Roadway (2) Service Rd. | 199,500 | 154,200 | 135,500 | 140,500 |
| Railway Line Diversion | -- | -- | -- | 22,500 |
| Sub Total | 1,429,600 | 744,500 | 1,072,260 | 767,800 |
| Interchange (Direct Junction) | 347,500 | 345,000 | 345,000 | 378,000 |
| Total | 1,777,100 | 1,089,500 | 1,417,260 | 1,145,800 |