

# APPENDIX B-8

Cost and BOQ as of 23 April, 2017

B-8 Cost and BOQ as of 23 April, 2017

**Integrated Water Management System**

As of 23 April, 2017

No.	Item	Grade	Unit	Quantity	Rate			Amount		
					Foreign		Local	Foreign		Local
					JPY	US\$	L.E.	JPY	US\$	L.E.
	WM-1. Gauging station (Type-A (4 nos))									
WM-1-1	WATER LEVEL SENSOR	including Cables	set	4		1,900			7,600	
WM-1-2	SOLAR CELLS PANEL	including Mount Bracket, Cables	set	4		500			2,000	
WM-1-3	GROUNDING RODS	including Cables	set	40		200			8,000	
WM-1-4	ENCLOSURE	including Mount Bracket	set	4		1,400			5,600	
WM-1-5	SURGE PROTECTIVE DEVICE	including Cables for water level sensor	set	4		400			1,600	
WM-1-6	SURGE PROTECTIVE DEVICE	including Cables for solar cells panel	set	4		600			2,400	
WM-1-7	GPRS MODEM	including Antenna, Mount Bracket, Cables	set	4		1,300			5,200	
WM-1-8	RTU	including Mount Bracket, Cables	set	4		2,400			9,600	
WM-1-9	DATA LOGGER	including Mount Bracket, Cables	set	4		2,500			10,000	
WM-1-10	POWER CHARGE CONTROLLER	including Mount Bracket, Cables	set	4		400			1,600	
WM-1-11	STORAGE BATTERY	including Mount Bracket, Cables	set	4		500			2,000	
WM-1-12	GROUNDING TERMINAL	including Mount Bracket	set	4		100			400	
WM-1-13	Program software for RTU		set	4		2,200			8,800	
Total									64,800	

No.	Item	Grade	Unit	Quantity	Rate			Amount		
					Foreign		Local	Foreign		Local
					JPY	US\$	L.E.	JPY	US\$	L.E.
	WM-2. Gauging station (Type-B (34 nos))									
WM-2-1	WATER LEVEL SENSOR	including Cables	set	68		1,900			129,200	
WM-2-2	SOLAR CELLS PANEL	including Mount Bracket, Cables	set	34		500			17,000	
WM-2-3	GROUNDING RODS	including Cables	set	340		200			68,000	
WM-2-4	ENCLOSURE	including Mount Bracket	set	34		1,400			47,600	
WM-2-5	SURGE PROTECTIVE DEVICE	including Cables for water level sensor	set	68		400			27,200	
WM-2-6	SURGE PROTECTIVE DEVICE	including Cables for solar cells panel	set	34		600			20,400	
WM-2-7	GPRS MODEM	including Antenna, Mount Bracket, Cables	set	34		1,300			44,200	
WM-2-8	RTU	including Mount Bracket, Cables	set	34		2,400			81,600	
WM-2-9	DATA LOGGER	including Mount Bracket, Cables	set	34		2,500			85,000	
WM-2-10	POWER CHARGE CONTROLLER	including Mount Bracket, Cables	set	34		400			13,600	
WM-2-11	STORAGE BATTERY	including Mount Bracket, Cables	set	34		500			17,000	
WM-2-12	GROUNDING TERMINAL	including Mount Bracket	set	34		100			3,400	
WM-2-13	Program software for RTU		set	34		2,200			74,800	
Total									629,000	

No.	Item	Grade	Unit	Quantity	Rate			Amount		
					Foreign		Local	Foreign		Local
					JPY	US\$	L.E.	JPY	US\$	L.E.
	WM-3. Master station									
WM-3 1	LCD MONITOR	including Wall mount bracket	set	1		4,600			4,600	
WM-3 2	LASER PRINTER	including Table, Cables	set	1		2,500			2,500	
WM-3 3	SERVER	including Monitor, Keyboard, Mouse, Cables	sets	3		11,400			34,200	
WM-3 4	COMPUTER RACK	including Power duct, Power Unit, Blank panel	set	1		2,900			2,900	
WM-3 5	GPRS MODEM	including Antenna, Rack mount plate, Cables	set	1		1,300			1,300	
WM-3 6	PERSONAL COMPUTER	including Monitor, Keyboard, Mouse, Cables	set	1		2,700			2,700	
WM-3 7	LCD KVM SWITCH	including Rack mount bracket, Cables	set	1		3,200			3,200	
WM-3 8	L2-SW	including Rack mount bracket, Cables	set	1		1,700			1,700	
WM-3 9	ROUTER	including Rack mount plate, Cables	set	2		2,700			5,400	
WM-3 10	UPS	including Rack mount plate, Cables (1KVA)	set	1		3,400			3,400	
WM-3 11	UPS	including Rack mount plate, Cables (3KVA)	set	1		3,400			3,400	
WM-3 12	Software	including Rack mount plate, Cables	set	1		13,500			13,500	
WM-3 13	Application software for Telemetry system		LS	1		121,000			121,000	
	Total								199,800	

No.	Item	Grade	Unit	Quantity	Rate			Amount		
					Foreign		Local	Foreign		Local
					JPY	US\$	L.E.	JPY	US\$	L.E.
	WM-4. Monitoring station									
WM-4 1	LCD MONITOR	including WALL MOUNT EXTENDING OPTION	set	1		4,600			4,600	
WM-4 2	PERSONAL COMPUTER	including 22" Monitor, Keyboard, Mouse, Cables	set	1		2,700			2,700	
WM-4 3	ROUTER	including Cables	set	1		2,700			2,700	
WM-4 4	UPS	including Rack mount plate, Cables (1KVA)	set	1		3,400			3,400	
WM-4 5	Application software for Telemetry system		LS	1		55,000			55,000	
	Total								68,400	

No.	Item	Grade	Unit	Quantity	Rate			Amount		
					Foreign		Local	Foreign		Local
					JPY	US\$	L.E.	JPY	US\$	L.E.
	WM-5. Installation Cost									
WM-5- 1	Gauging station (38 stations)	including site acquisition, cost of fence and pole with foundation works, fixing equipments, adjustment and others	LS	1		602,000			602,000	
WM-5- 2	Master and Monitoring station	including cables, installation and adjustment	LS	1		24,000			24,000	
	Total								626,000	

No.	Item	Grade	Unit	Quantity	Rate			Amount		
					Foreign		Local	Foreign		Local
					JPY	US\$	L.E.	JPY	US\$	L.E.
	WM-6. Spare parts and training									
WM-6- 1	Supply of Spare parts as per requirements in Particular		LS	1		138,000			138,000	
WM-6- 2	On-Site Training		LS	1		16,500			16,500	
	Total								154,500	

WM-1, Gauging station (Type-A (4 nos))	64,800	
WM-2, Gauging station (Type-B (34 nos))	629,000	
WM-3, Master station	199,800	
WM-4, Monitoring station	88,400	
	<hr/>	962,000 USD
WM-5, Installation Cost		
Gauging station (38 stations)	602,000	
Master and Monitoring station	24,000	
	<hr/>	626,000 USD
WM-6, Spare parts and training		
Supply of Spare parts as per requirements in Particular Technical Specification	138,000	
On- Site Training	16,500	
	<hr/>	154,500 USD
<b>Total</b>	<hr/>	<b>1,742,500 USD</b>

## Quotation of equipment for gauging station

(Unit : US\$)

Item	Quotation as of March and April 2017			Average
	A	B	C	
WATER LEVEL SENSOR	1,500	1,300	2,845	1,900
SOLAR CELLS PANEL	300	500	635	500
ENCLOSURE	1,100	1,250	1,729	1,400
SURGE PROTECTIVE DEVICE	100	500	500	400
SURGE PROTECTIVE DEVICE	100	500	1,100	600
GPRS MODEM	1,300	2,050	450	1,300
RTU	1,600	2,700	2,970	2,400
Program software for RTU	1,400	3,050	2,113	2,200
DATA LOGGER	3,800	2,250	1,500	2,500
POWER CHARGE CONTROLLER	400	250	585	400
STORAGE BATTERY	300	750	375	500
GROUNDING TERMINAL	100	150	100	100
GROUNDING RODS	100	100	420	200
Total	12,100	15,350	15,320	14,400

## Quotation of equipment for Master station

(Unit : US\$)

Item	Quotation as of March and April 2017			Average
	A	B	C	
LCD MONITOR	5,500	4,200	4,000	4,600
LASER PRINTER	2,200	3,250	1,900	2,500
SERVER	11,600	9,400	13,200	11,400
COMPUTER RACK	2,100	4,580	2,000	2,900
GPRS MODEM	1,300	2,050	450	1,300
PERSONAL COMPUTER	5,100	1,850	1,200	2,700
LCD KVM SWITCH	2,400	4,200	2,950	3,200
L2-SW	1,200	2,300	1,650	1,700
ROUTER	3,100	3,000	1,950	2,700
UPS	1,100	7,925	1,200	3,400
Software	12,200	12,000	16,250	13,500
Total	47,800	54,755	46,750	49,900

## Quotation of equipment for Monitoring station

(Unit : US\$)

Item	Quotation as of March and April 2017			Average
	A	B	C	
LCD MONITOR	5,500	4,200	4,000	4,600
PERSONAL COMPUTER	5,100	1,850	1,200	2,700
ROUTER	3,100	3,000	1,950	2,700
UPS	1,100	7,925	1,200	3,400
Total	14,800	16,975	8,350	13,400

DETAILED DESIGN STUDY ON THE PROJECT FOR CONSTRUCTION OF  
THE NEW DIROUT GROUP OF REGULATORS  
WATER MANEGEMENT SYSTEM  
QUANTITY CALCULATION SHEET

( As of 23 April, 2017)

### TOTAL AMOUNT FOR EQUIPMENT

FACILITY	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION (PTS-ME)	ACCESSARY	QTY	UNIT	REMARKS	
WATER MANAGEMENT	MASTER STATION	LCD MONITOR	Cp.3 Sec.3.7	Wall mount bracket	1	set		
		LASER PRINTER	Cp.3 Sec.3.10	Table, Cables	1	set		
		SERVER	Cp.3 Sec.3.5	Monitor, Keyboard, Mouse, Cables	3	set		
		COMPUTER RACK	Cp.3 Sec.3.12	Power duct, Power Unit, Brank panel	1	set		
		GPRS MODEM	Cp.3 Sec.3.1	Antenna, Rack mount plate, Cables	1	set		
		PERSONAL COMPUTER	Cp.3 Sec.3.6	Monitor, Keyboard, Mouse, Cables	1	set		
		LCD KVM SWITCH	Cp.3 Sec.3.13	Rack mount bracket, Cables	1	set		
		L2-SW	Cp.3 Sec.3.8	Rack mount bracket, Cables	1	set		
		ROUTER	Cp.3 Sec.3.9	Rack mount plate, Cables	2	set		
		UPS	Cp.3 Sec.5.6	Rack mount plate, Cables	1	set		
	MONITORING STATION	LCD MONITOR	Cp.3 Sec.3.7	WALL MOUNT XETTING OPTION	1	set		
		PERSONAL COMPUTER	Cp.3 Sec.3.6	22"Monitor, Keyboard, Mouse, Cables	1	set		
		ROUTER	Cp.3 Sec.3.9	Cables	1	set		
	GAUGING STATION	WATER LEVEL SENSOR	Cp.3 Sec.3.4	Cables	72	set		
		SOLAR CELLS PANEL	Cp.3 Sec.5.3	Mount Bracket, Cables	38	set		
		GROUNDING RODS	Cp.3 Sec.5.8	Cables	380	set		
		ENCLOSURE	Cp.3 Sec.3.11	Mount Bracket	38	set		
		SURGE PROTECTIVE DEVICE	Cp.3 Sec.5.9	Cables	72	set		
		SURGE PROTECTIVE DEVICE	Cp.3 Sec.5.9	Cables	38	set		
		GPRS MODEM	Cp.3 Sec.3.1	Antenna, Mount Bracket, Cables	38	set		
		RTU	Cp.3 Sec.3.2	Mount Bracket, Cables	38	set		
		DATA LOGGER	Cp.3 Sec.3.3	Mount Bracket, Cables	38	set		
		POWER CHARGE CONTROLLER	Cp.3 Sec.5.4	Mount Bracket, Cables	38	set		
		STORAGE BATTERY	Cp.3 Sec.5.5	Mount Bracket, Cables	38	set		
		GROUNDING TERMINAL	Cp.3 Sec.5.8	Mount Bracket	38	set		

### SUMMARY OF QUANTITY FOR EQUIPMENT

FACILITY	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS	
WATER MANAGEMENT	MASTER STATION	LCD MONITOR	Cp.3 Sec.3.7	Wall mount bracket	1	set		
		LASER PRINTER	Cp.3 Sec.3.10	Table, Cables	1	set		
		SERVER	Cp.3 Sec.3.5	Monitor, Keyboard, Mouse, Cables	3	set		
		COMPUTER RACK	Cp.3 Sec.3.12	Power duct, Power Unit, Brank panel	1	set		
		GPRS MODEM	Cp.3 Sec.3.1	Antenna, Rack mount plate, Cables	1	set		
		PERSONAL COMPUTER	Cp.3 Sec.3.6	Monitor, Keyboard, Mouse, Cables	1	set		
		LCD KVM SWITCH	Cp.3 Sec.3.13	Rack mount bracket, Cables	1	set		
		L2-SW	Cp.3 Sec.3.8	Rack mount bracket, Cables	1	set		
		ROUTER	Cp.3 Sec.3.9	Rack mount plate, Cables	2	set		
		UPS	Cp.3 Sec.5,6	Rack mount plate, Cables	1	set		
	MONITORING STATION	LCD MONITOR	Cp.3 Sec.3,7	WALL MOUNT XETTING OPTION	1	set		
		PERSONAL COMPUTER	Cp.3 Sec.3,6	22"Monitor, Keyboard, Mouse, Cables	1	set		
		ROUTER	Cp.3 Sec.3.9	Cables	1	set		



### SUMMARY OF QUANTITY FOR EQUIPMENT

FACILITY	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS	
WATER MANAGEMENT	GAUGING STATION TYPE-A (WATER LEVEL:1)	WATER LEVEL SENSOR	Cp.3 Sec.3.4	Cables	4	set	Pressure type	
		SOLAR CELLS PANEL	Cp.3 Sec.5.3	Mount Bracket, Cables	4	set		
		GROUNDING RODS	Cp.3 Sec.5.8	Cables	40	set		
		ENCLOSURE	Cp.3 Sec.3.11	Mount Bracket	4	set	IP65, KEY LOCK, Duct	
		SURGE PROTECTIVE DEVICE	Cp.3 Sec.5.9	Cables	4	set	FOR WATER LEVEL SENSOR	
		SURGE PROTECTIVE DEVICE	Cp.3 Sec.5.9	Cables	4	set	FOR SOLAR CELLS PANEL	
		GPRS MODEM	Cp.3 Sec.3.1	Antenna, Mount Bracket, Cables	4	set		
		RTU	Cp.3 Sec.3.2	Mount Bracket, Cables	4	set		
		DATA LOGGER	Cp.3 Sec.3.3	Mount Bracket, Cables	4	set	LCD Monitor, Memory card	
		POWER CHARGE CONTROLLER	Cp.3 Sec.5.4	Mount Bracket, Cables	4	set		
		STORAGE BATTERY	Cp.3 Sec.5.5	Mount Bracket, Cables	4	set		
	GROUNDING TERMINAL	Cp.3 Sec.5.8	Mount Bracket	4	set	8 terminals		
	GAUGING STATION TYPE-B (WATER LEVEL:2)	WATER LEVEL SENSOR	Cp.3 Sec.3.4	Cables	68	set	Pressure type	
		SOLAR CELLS PANEL	Cp.3 Sec.5.3	Mount Bracket, Cables	34	set		
		GROUNDING RODS	Cp.3 Sec.5.8	Cables	340	set		
		ENCLOSURE	Cp.3 Sec.3.11	Mount Bracket	34	set	IP65, KEY LOCK, Duct	
		SURGE PROTECTIVE DEVICE	Cp.3 Sec.5.9	Cables	68	set	FOR WATER LEVEL SENSOR	
		SURGE PROTECTIVE DEVICE	Cp.3 Sec.5.9	Cables	34	set	FOR SOLAR CELLS PANEL	
		GPRS MODEM	Cp.3 Sec.3.1	Antenna, Mount Bracket, Cables	34	set		
		RTU	Cp.3 Sec.3.2	Mount Bracket, Cables	34	set		
		DATA LOGGER	Cp.3 Sec.3.3	Mount Bracket, Cables	34	set	LCD Monitor, Memory card	
		POWER CHARGE CONTROLLER	Cp.3 Sec.5.4	Mount Bracket, Cables	34	set		
		STORAGE BATTERY	Cp.3 Sec.5.5	Mount Bracket, Cables	34	set		
		GROUNDING TERMINAL	Cp.3 Sec.5.8	Mount Bracket	34	set	8 terminals	

B-59

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS	
WATER MANAGEMENT	DIROUT	MASTER STATION	LCD MONITOR	Cp,3 Sec.3.7	Wall mount bracket	1	set		
			LASER PRINTER	Cp,3 Sec.3.10	Table, Cables	1	set		
			SERVER	Cp,3 Sec.3.5	Monitor, Keyboard, Mouse, Cables	3	set		
			COMPUTER RACK	Cp,3 Sec.3.12	Power duct, Power Unit, Brank panel	1	set		
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Rack mount plate, Cables	1	set		
			PERSONAL COMPUTER	Cp,3 Sec.3.6	Monitor, Keyboard, Mouse, Cables	1	set		
			LCD KVM SWITCH	Cp,3 Sec.3.13	Rack mount bracket, Cables	1	set		
			L2-SW	Cp,3 Sec.3.8	Rack mount bracket, Cables	1	set		
			ROUTER	Cp,3 Sec.3.9	Rack mount plate, Cables	2	set		
	UPS	Cp,3 Sec.5.6	Rack mount plate, Cables	1	set				
	MINIA	MONITORING STATION	LCD MONITOR	Cp,3 Sec.3.7	WALL MOUNT XETTING OPTION	1	set		
			PERSONAL COMPUTER	Cp,3 Sec.3.6	22" Monitor, Keyboard, Mouse, Cables	1	set		
			ROUTER	Cp,3 Sec.3.9	Cables	1	set		

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Manshat El Dahab Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	El Hareka Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	El Sabaa Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Qufan Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Wesh El Bab Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Giza Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Hassan Wasef Weir	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	1	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
			STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set	
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Irad El Maharak Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
			STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set	
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	El Kosia Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY			Cp,3 Sec.5.5	Mount Bracket, Cables	1	set		
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	East Hafze Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	West Hafze Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Adkak Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Gendia Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Abo Shosha Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	EI Soultany Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Tansa Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
			STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set	
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	EI Mansour Intake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
			STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set	
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Serry Weir	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	1	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY			Cp,3 Sec.5.5	Mount Bracket, Cables	1	set		
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			



### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Maghagha Weir	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	1	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Ibrahimia Head Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Dahab Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Sakoula Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Mazoura Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Lahoun Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Abo EL Shekok Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	km39 Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	New Hafze Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Matay Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Maghagha Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Sharahna Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	El Gandy Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Ashmont Regulator	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Quarun Lake	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	1	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	New Kamdeer PS	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	New Terfa PS	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Old Terfa PS	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
GPRS MODEM			Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set		
RTU			Cp,3 Sec.3.2	Mount Bracket, Cables	1	set		
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

### COUNT QUANTITY OF EQUIPMENT

FACILITY	SITE NAME	CATEGORY	NAME OF EQUIPMENT	SPECIFICATION	ACCESSARY	QTY	UNIT	REMARKS
WATER MANAGEMENT	Old Sakola PS	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
			DATA LOGGER	Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card
			POWER CHARGE CONTROLLER	Cp,3 Sec.5.4	Mount Bracket, Cables	1	set	
	STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set			
	GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals		
	Mazoura PS	GAUGING STATION	WATER LEVEL SENSOR	Cp,3 Sec.3.4	Cables	2	set	Pressure type
			SOLAR CELLS PANEL	Cp,3 Sec.5.3	Mount Bracket, Cables	1	set	
			GROUNDING RODS	Cp,3 Sec.5.8	Cables	10	set	
			ENCLOSURE	Cp,3 Sec.3.11	Mount Bracket	1	set	IP65, KEY LOCK, Duct
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	2	set	FOR WATER LEVEL SENSOR
			SURGE PROTECTIVE DEVICE	Cp,3 Sec.5.9	Cables	1	set	FOR SOLAR CELLS PANEL
			GPRS MODEM	Cp,3 Sec.3.1	Antenna, Mount Bracket, Cables	1	set	
			RTU	Cp,3 Sec.3.2	Mount Bracket, Cables	1	set	
DATA LOGGER			Cp,3 Sec.3.3	Mount Bracket, Cables	1	set	LCD Monitor, Memory card	
POWER CHARGE CONTROLLER			Cp,3 Sec.5.4	Mount Bracket, Cables	1	set		
STORAGE BATTERY	Cp,3 Sec.5.5	Mount Bracket, Cables	1	set				
GROUNDING TERMINAL	Cp,3 Sec.5.8	Mount Bracket	1	set	8 terminals			

B-73

# APPENDIX C

## Quantity Calculation Sheet

C-1. Quantity Calculation Sheet of Civil Works .....	C- 1
C-2. Quantity Calculation Sheet of Building Works .....	C-287
C-3. Quantity Calculation Sheet of Gate Facility.....	C-323
C-4. Quantity Calculation Sheet of Water Management.....	C-562
C-5. Quantity Calculation Sheet of Temporary Works .....	C-585



# APPENDIX C-1

## Quantity Calculation Sheet of Civil Works

C-1-1. Quantity summary table of civil works .....	C- 2
C-1-2. Quantity calculation of earth works .....	C- 5
C-1-3. Quantity calculation of foundation works .....	C-12
C-1-4. Quantity calculation of cutoff sheet pile works .....	C-23
C-1-5. Quantity calculation of weir works .....	C-25
C-1-6. Quantity calculation of ancillary bridge works .....	C-215
C-1-7. Quantity calculation of bank protection works .....	C-225
C-1-8. Quantity calculation of bed protection works .....	C-265
C-1-9. Quantity calculation of hydraulic pipe pit works .....	C-268
C-1-10. Quantity calculation of ancillary road works .....	C-272

C-1-1. Quantity summary table of civil works

(1/3)

Works	Description	Dimension	Unit	Bahr Yusef	Ibahlmia	Badraman		Abo Gabal	Sahelyia	Total		
						Badraman	Drouthiah					
Earth works	Excavation	C1 : cohesive soil	m <sup>3</sup>	1,500	0	1,000	1,100	1,200	800	5,600		
		C1 : sandy soil	m <sup>3</sup>	3,600	13,700	0	0	290	200	17,790		
		C2 : cohesive soil	m <sup>3</sup>	30	0	30	40	20	10	130		
		C2 : sandy soil	m <sup>3</sup>	60	140	0	0	0	0	200		
	Backfilling	R : sandy soil	m <sup>3</sup>	1,400	3,700	710	600	550	510	7,470		
		B0 : sandy soil	m <sup>3</sup>	1,100	0	0	0	0	0	1,100		
	Embankment	BL : sandy soil	m <sup>3</sup>	38,000	1,200	160	100	840	490	40,790		
		BR : sandy soil	m <sup>3</sup>	7,200	25,400	180	230	1,200	390	34,600		
Earth finishing foundation	L1	m <sup>2</sup>	1,810	1,810	210	290	370	150	4,640			
	Rough finishing	L2	m <sup>2</sup>	250	170	420	370	210	190	1,610		
Foundation works	Cast-in place RC pile	Φ1,000*13.50m	nos	72						72		
		Φ1,000*7.50m	nos	32						32		
		Φ1,000*15.00m	nos		56						56	
		Φ1,000*7.00m	nos		32						32	
Cutoff sheet pileworks	Sheet pile (S270GP)	PU12	ton	89.632	91.152	4.759	7.271	4.429	3.305	200.548		
		PU18	ton	5.229	2.576	1.538					9.343	
		Cutting	m	12.0	12.0						24.0	
		Welding	m	12.0	12.0						24.0	
		Painting on steel sheet pile	m <sup>2</sup>	42.5	39.7	43.2	40.8	12.5	15.0		193.7	
Weir body works	Demolition works	Concrete demolition	PC structure	m <sup>3</sup>					3.6	2.1	5.8	
		RC structure	m <sup>3</sup>						123.6	71.8	195.4	
	Main body	Concrete	Leveling con. σck=20kN/mm <sup>2</sup>	m <sup>3</sup>	153.3	154.6	17.5	24.3	37.9	15.6	403.1	
			Reinforced con. σck=25kN/mm <sup>2</sup>	m <sup>3</sup>	6,503.8	6,460.9	770.9	987.6	1,441.4	752.4	16,917.0	
		Formwork	Second stage con. σck=25kN/mm <sup>2</sup>	m <sup>3</sup>	143.2	142.7	10.3	14.5	23.7	13.4	347.7	
			Leveling con.	m <sup>2</sup>	15.4	15.4	4.9	4.8	6.7	4.8	51.9	
		Circular formwork	Reinforced con.	m <sup>2</sup>	4,586.1	4,498.1	1,146.5	1,350.1	1,660.8	1,060.3	14,302.0	
			Second stage con.	m <sup>2</sup>	468.2	464.6	51.7	70.9	121.3	72.0	1,248.8	
		RC structure Φ2,500-1/2 Circle	RC structure	m <sup>2</sup>	176.7	173.2						349.9
			RC structure Φ1,000-1/2 Circle	m <sup>2</sup>			11.2	20.4	29.9	9.4		70.8
		RC structure Φ1,000-1/4 Circle	RC structure	m <sup>2</sup>	17.3	16.8	11.2	10.2	5.3	9.4		70.1
			Rebar	Steel 360/520 D12	ton	21.103	20.947	2.069	2.686	3.968	1.831	52.604
	Steel 360/520 D16	Steel 360/520 D16	ton	143.639	142.678	31.552	40.722	53.777	31.097		443.465	
		Steel 360/520 D18	ton	26.059	26.059						52.118	
	Steel 360/520 D22	Steel 360/520 D22	ton	16.528	16.490	2.400	2.424				37.842	
		Concrete curing	PC structure	m <sup>3</sup>	153.3	154.6	17.5	24.3	37.9	15.6	403.1	
	RC structure		m <sup>3</sup>	6,647.0	6,603.5	781.2	1,002.1	1,465.1	765.8		17,264.7	
	Scaffolding	Prefabricated scaffold H≤30m	m <sup>2</sup>	3,665.3	3,625.5	848.6	933.3	975.4	707.0		10,755.1	
		Pipe support falsework	m <sup>3</sup>			45.8	61.5	60.6			167.9	
	Wedge linked falsework	Wedge linked falsework	m <sup>3</sup>	1,680.7	1,666.3	56.9	77.0	417.1	294.8		4,192.8	
		Expansion joint	Elastic filler, t=20mm	m <sup>2</sup>	160.1	159.2			96.4	73.1	488.7	
	Water stop	B=300mm	m	95.5	94.9			34.0	26.6		251.0	
		Water swelling water stop	Paste type , 9mm*30mm	m					36.7	29.4	66.1	
	Resin capsule anchor	D20*200 , Drilling dia. Φ25	nos					121	96		217	
		D20*1.00m , VPΦ25*0.50m	nos	321	319			111	86		837	
	Slip bar	D20*0.70m	nos					121	96		217	
		Other miscellaneous	Joint bar	D16*300	nos	1,472	1,472					2,944
	D16*200		nos	1,152	1,120	464	464	1,120	636		4,956	
	Grating	5,200*2,400	set	4	4						8	
		6,000*1,000	set	8	8						16	
	Crane facility	2,000*1,000	set			4	6	4	2		16	
		Rail (15kg/m)	m	35.5	35.5						71.0	
	Manual chain block (5t)	Manual chain block (5t)	set	1	1						2	
		Steps	B=300mm, Resin coated	pcs	212	208	54	75	136	80		765
	Staff gauge	H=7.50m	pcs	1	1						2	
		H=7.00m	pcs	1							1	
	H=6.70m	H=6.70m	pcs		1						1	
		H=5.95m	pcs							1	1	
	H=4.75m	H=4.75m	pcs					2			2	
		H=4.50m	pcs						1		1	
	H=3.50m	H=3.50m	pcs						1		1	
		H=3.00m	pcs			1					1	
	H=3.30m	H=3.30m	pcs					1			1	
		H=3.00m	pcs			1					1	
	H=2.70m	H=2.70m	pcs					1			1	
		Safety facilities work	Guard fence	H=1.10m	m	201.6	201.6	41.2	59.2	29.8	29.8	563.2
	Approach cushion slab		Concrete	Reinforced con. σck=25kN/mm <sup>2</sup>	m <sup>3</sup>	40.0	40.0	40.0	40.0			160.0
Formwork		m <sup>2</sup>		24.0	24.0	24.0	24.0			96.0		
Rebar		Steel 360/520 D12	ton	0.150	0.150	0.150	0.150			0.600		
		Steel 360/520 D16	ton	2.370	2.370	2.370	2.370			9.480		
Steel 360/520 D18		Steel 360/520 D18	ton	1.040	1.040	1.040	1.040			4.160		
		Concrete curing	RC structure	m <sup>3</sup>	40.0	40.0	40.0	40.0			160.0	
Anchor cap		SGPΦ40*180	nos	50	50	50	50			200		
Anchor bar		D22*500 (including D10 spiral reinforcement)	nos	50	50	50	50			200		
Anchor bar		D22*500 (including D10 spiral reinforcement)	nos	50	50	50	50			200		
Anchor bar		D22*500 (including D10 spiral reinforcement)	nos	50	50	50	50			200		
Anchor bar	D22*500 (including D10 spiral reinforcement)	nos	50	50	50	50			200			
Anchor bar	D22*500 (including D10 spiral reinforcement)	nos	50	50	50	50			200			
Rubber shoe	t=30mm	m <sup>2</sup>	6.0	6.0	6.0	6.0				24.0		
	Expansion joint	Elastic filler, t=20mm	m <sup>2</sup>	8.6	8.6	8.6	8.6			34.4		

Works	Description	Dimension	Unit	Bahr Yusef	Ibrahimia	Badrman		Abo Gabal	Sahelyia	Total	
						Badrman	Drouitah				
Ancillary bridge works	Concrete works	Concrete	Reinforced con. $\sigma_{ck}=25kN/mm^2$	m <sup>3</sup>	231.8	231.8	34.1	51.2		548.9	
		Formwork	RC structure	m <sup>2</sup>	759.4	759.4	71.4	106.0		1,696.2	
		Styrofoam	t=150	m <sup>2</sup>	19.2	19.2				38.4	
			t=100	m <sup>2</sup>	11.6	11.6				23.2	
			t=50	m <sup>2</sup>	36.8	36.8	11.4	11.4		96.4	
			t=20cm	m <sup>2</sup>			22.0	33.0		55.0	
		Concrete curing	RC structure	m <sup>3</sup>	231.8	231.8	34.1	51.2		548.9	
		Rebar	Steel 360/520	kg	46,332	46,332	5,003	7,420		105,087	
		Falsework		m <sup>3</sup>	2,700	2,700	298	424		6,122	
		Shoe	Rubber bearing	330×150×40	nos	10	10				20
				330×300×40	nos	20	20				40
				150×10,000×20	nos			1	1		2
				300×10,000×20	nos			2	3		5
			ST formula corrosion proof anchor	F25D×710	set	4	4				8
				M25D×710	set	8	8				16
				F25D×580	set			2	2		4
				M25D×580	set			2	2		4
			PD-Packing	150×65×65	nos	4	4				8
				150×65×165	nos	8	8				16
				20×65×65	nos			2	2		4
				20×65×165	nos			2	2		4
		Expansion joint	Prof joint	Type NI -20 (Carriage way)	m	20.2	20.2	20.2	20.2		80.8
				Type NI -20 (Side way)	m	4.0	4.0	4.0	4.0		16.0
				Type NI -35 (Carriage way)	m	10.1	10.1				20.2
				Type NI -35 (Side way)	m	2.0	2.0				4.0
			Anchor bar		kg	205	205	102	102		614
		Asphalt pavement	Wearing course		m <sup>3</sup>	30.4	30.4	5.4	8.1		74.3
		Bulstade	Bulstade		m	67.6	67.6	12.0	18.0		165.2
			Anchor bar		kg	625	625	108	159		1,517
	Bank protection works	Steel sheet pile protection	Steel sheet pile (S270GP)	TYPE A	m	66.6					66.6
				TYPE BL-1	m	6.6					6.6
				TYPE BL-2	m	6.6					6.6
				TYPE BR-1	m	4.8					4.8
				TYPE BR-2	m	9.0					9.0
				TYPE C-1	m	3.6					3.6
				TYPE C-2	m	9.0					9.0
				TYPE D-1	m		3.6				3.6
				TYPE D-2	m		5.4				5.4
				TYPE E-1	m		3.0				3.0
				TYPE E-2	m		1.8				1.8
				TYPE F	m		72.6				72.6
				TYPE G-1	m		6.6				6.6
			TYPE G-2	m		6.0				6.0	
			TYPE H	m					19.2	19.2	
			TYPE I	m				6.6		6.6	
			TYPE J	m				4.2		4.2	
			TYPE K	m			6.6			6.6	
			TYPE L	m			3.6			3.6	
			TYPE M	m					45.0	45.0	
		Wet stone pitching protection	Wet stone pitching	1:2.0 t=35cm	m <sup>2</sup>	1,202.0					1,202.0
				1:1.5 t=35cm	m <sup>2</sup>		806.8				806.8
			Backfilling concrete	t=20cm, $\sigma_{ck}=20kN/mm^2$	m <sup>3</sup>		161.4				161.4
			Crown works	Type A	m	47.3					47.3
				Type B	m		54.7				54.7
			Foundation works	Type A	m	55.9					55.9
				Type B	m		50.6				50.6
	Gabion protection	Gabion	0.5 <sup>H</sup> ×1.0 <sup>B</sup> ×2.0 <sup>L</sup>	nos	598	586				1,184	
		Stone	1.0 <sup>H</sup> ×1.0 <sup>B</sup> ×2.0 <sup>L</sup>	nos	162	508				670	
		Geotextiles sheet		m <sup>2</sup>	882.7	1,587.0				2,469.7	
				m <sup>2</sup>	1,441.3	2,157.9				3,599.2	
Bed protection works	Riprap	Stone	D <sub>n</sub> =40cm	m <sup>3</sup>	850.5	850.5				1,701.0	
			D <sub>n</sub> =20cm	m <sup>3</sup>	438.0	440.5	30.0	48.0		956.5	
		Gravel	crushed stone	m <sup>3</sup>	94.5	94.5	7.5	12.0		208.5	
		Geotextiles sheet		m <sup>2</sup>	945.0	945.0	75.0	120.0		2,085.0	
Hydraulic pipe pit works	Concrete works	Concrete	Leveling con. $\sigma_{ck}=20kN/mm^2$	m <sup>3</sup>	3.0	7.6				10.5	
			Reinforced con. $\sigma_{ck}=25kN/mm^2$	m <sup>3</sup>	8.9	76.9				85.8	
		Formwork	Leveling con.	m <sup>2</sup>	3.0	6.1				9.1	
			Reinforced con.	m <sup>2</sup>	53.3	321.4				374.7	
		Rebar	Steel 360/520 D12	kg	448	2,923				3,371	
			Steel 360/520 D16	kg		4,820				4,820	
			Steel 360/520 D25	kg		328				328	
		Concrete curing	PC structure	m <sup>3</sup>	3.0	7.6				10.5	
			RC structure	m <sup>3</sup>	8.9	76.9				85.8	
		Scaffolding		m <sup>2</sup>		160.1				160.1	
		Falsework		m <sup>3</sup>		92.0				92.0	
		Other miscellaneous	Expansion joint	Elastic filler, t=20mm	m <sup>2</sup>		5.1				5.1
		Construction joint	Oil paint	m <sup>2</sup>	0.6					0.6	

Works	Description	Dimension	Unit	Bahr Yusef	Ibrahimia	Badrman		Abo Gabal	Sahelyia	Total	
						Badrman	Drouitah				
Hydraulic pipe pit works	Other miscellaneous	Water stop	B=300mm	m		16.8				16.8	
		Slip bar	D $\varnothing$ ×1.00m , VPΦ25×0.50m	pcs		32.0				32.0	
		Checker plate	t=3.2mm	m <sup>2</sup>		1.0				1.0	
			t=4.5mm	m <sup>2</sup>	18.8					18.8	
		Anchor bar	D16 , L=0.20m	pcs		16				16	
Ancillary road works	Earth works	Ebankment	sandy soil	m <sup>3</sup>						10,225	
	Asphalt pavement	Roadwalk		m <sup>2</sup>						4,075.1	
		Side walk		m <sup>2</sup>						623.7	
	Curb concrete	Type-1		m						643.5	
		Type-2		m						629.8	
	Bulstade	Bulstade		m						629.8	
	Slope protection	Wet stone pitching	1:1.5 t=35cm	m <sup>2</sup>						1,987.0	
			1:1.0 t=35cm	m <sup>2</sup>						158.4	
		Backfilling concrete	t=10cm , σ <sub>ck</sub> =20kN/mm <sup>2</sup>	m <sup>3</sup>						15.8	
	Crown works	Type A		m						421.0	
		Type B		m						57.6	
	Foundation works			m						536.8	
	Steps	Type-A	H=3.57m 1:1.5	pieces							2
		Type-B	H=2.57m 1:1.5	pieces							4
		Type-C	H=3.67m 1:1.0	pieces							2
		Type-D	H=2.57m 1:1.0	pieces							2
		Type-E	H=3.87m 1:1.5	pieces							2



## (2) Bahr Yusef Reg.

Station	Distance (m)	Excavation (C1)			Excavation (C2)			Backfilling (R)			Embankment (B0)		
		Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )
252.2		0.23						0.00					
249.6	2.6	0.73	0.48	1.25				0.00	0.00	0.00			
245.2	4.4	3.21	1.97	8.67				0.00	0.00	0.00			
209.8	35.4	40.09	21.85	766.41				1.98	0.99	35.05			
(+) 204.8	5.0	93.80	66.95	334.75				1.58	1.78	8.90			
(-) 204.8	0.0	104.82	99.31	0.00				1.58	1.58	0.00	0.00		
(+) 179.8	25.0	36.09	70.46	1,761.50				1.58	1.58	39.50	7.50	3.75	93.75
(-) 179.8	0.0	36.09	36.09	0.00				1.58	1.58	0.00	7.50	7.50	0.00
(+) 174.8	5.0	26.37	31.23	156.15	-			1.49	1.54	7.70	15.60	11.55	57.75
(-) 174.8	0.0	22.99	24.68	0.00	12.96	12.96	0.00	18.47	9.98	0.00	2.74	9.17	0.00
173.8	1.0	22.99	22.99	22.99	12.96	12.96	12.96	18.47	18.47	18.47	2.74	2.74	2.74
172.8	1.0	23.18	23.09	23.09	0.00	6.48	6.48	18.47	18.47	18.47	13.87	8.31	8.31
154.7	18.1	0.86	12.02	217.56				19.41	18.94	342.81	49.79	31.83	576.12
(+) 151.8	2.9	0.38	0.62	1.80	-			4.64	12.03	34.89	40.76	45.28	131.31
(-) 151.8	0.0	3.84	2.11	0.00	12.12	12.96	0.00	8.10	6.37	0.00	10.12	25.44	0.00
150.8	1.0	3.84	3.84	3.84	12.12	12.12	12.12	7.63	7.87	7.87	10.12	10.12	10.12
149.8	1.0	2.54	3.19	3.19	0.00	6.06	6.06	7.26	7.45	7.45	31.75	20.94	20.94
(+) 140.0	9.8	62.64	32.59	319.38				10.62	8.94	87.61	0.00	15.88	155.62
(-) 140.0	0.0	62.64	62.64	0.00				10.62	10.62	0.00			
125.4	14.6	78.66	70.65	1,031.49	0.00	0.00	0.00	30.29	20.46	298.72			
124.4	1.0	78.66	78.66	78.66	33.66	16.83	16.83	31.23	30.76	30.76			
(+) 123.4	1.0	78.66	78.66	78.66	33.66	33.66	33.66	30.49	30.86	30.86			
(-) 123.4	0.0	4.28	41.47	0.00	-			11.54	21.02	0.00			
121.2	2.2	5.39	4.84	10.65				11.53	11.54	25.39			
97.7	23.5	9.78	7.59	178.37				6.29	8.91	209.39			
82.3	15.4	4.71	7.25	111.65				10.27	8.28	127.51			
68.8	13.5	0.00	2.36	31.86				0.00	5.14	69.39			
				5,141.92			88.11			1,400.74			1,056.66

Station	Distance (m)	Embankment (BL)			Embankment (BR)			Earth Finishing Foundation (L1)			Rough Finishing (L2)		
		Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Line (m)	Ave. (m)	Area (m <sup>2</sup> )	Line (m)	Ave. (m)	Area (m <sup>2</sup> )
252.2	0.0	0.00											
249.6	2.6	11.46	5.73	14.90									
245.2	4.4	21.03	16.25	71.50									
209.8	35.4	127.55	74.29	2,629.87									
(+) 204.8	5.0	133.93	130.74	653.70									
(-) 204.8	0.0	133.93	133.93	0.00									
(+) 179.8	25.0	184.00	158.97	3,974.25									
(-) 179.8	0.0	184.00	184.00	0.00	0.00								
(+) 174.8	5.0	217.88	200.94	1,004.70	39.61	19.81	99.05	-			-		
(-) 174.8	0.0	277.71	247.80	0.00	76.41	58.01	0.00	34.50			6.99		
173.8	1.0	277.71	277.71	277.71	76.13	76.27	76.27	34.50	34.50	34.50	6.99	6.99	6.99
172.8	1.0	277.71	277.71	277.71	76.58	76.36	76.36	34.70	34.60	34.60	6.91	6.95	6.95
154.7	18.1	308.16	292.94	5,302.21	115.25	95.92	1,736.15	34.70	34.70	628.07	1.37	4.14	74.93
(+) 151.8	2.9	327.86	318.01	922.23	124.50	119.88	347.65	34.70	34.70	100.63	1.10	1.24	3.80
(-) 151.8	0.0	323.30	325.58	0.00	124.50	124.50	0.00	35.50	35.10	0.00	4.97	3.04	0.00
150.8	1.0	323.30	323.30	323.30	118.09	121.30	121.30	35.50	35.50	35.50	4.97	4.97	4.97
149.8	1.0	322.81	323.06	323.06	118.93	118.51	118.51	35.70	35.60	35.60	2.72	3.85	3.85
(+) 140.0	9.8	308.83	315.82	3,095.04	113.29	116.11	1,137.88	35.70	35.70	349.86	5.58	4.15	40.67
(-) 140.0	0.0	373.19	341.01	0.00	140.58	126.94	0.00	35.70	35.70	0.00	5.58	5.58	0.00
125.4	14.6	367.45	370.32	5,406.67	119.11	129.85	1,895.81	35.70	35.70	521.22	6.56	6.07	88.62
124.4	1.0	367.45	367.45	367.45	118.90	119.01	119.01	35.50	35.60	35.60	9.11	7.84	7.84
(+) 123.4	1.0	367.45	367.45	367.45	118.57	118.74	118.74	35.50	35.50	35.50	9.11	9.11	9.11
(-) 123.4	0.0	378.04	372.75	0.00	60.56	89.57	0.00	-			-		
121.2	2.2	376.08	377.06	829.53	56.63	58.60	128.92						
97.7	23.5	294.85	335.47	7,883.55	18.42	37.53	881.96						
82.3	15.4	135.84	215.35	3,316.39	14.41	16.42	252.87						
68.8	13.5	8.67	72.26	975.51	0.00	7.21	97.34						
				38,016.73			7,207.82			1,811.08			247.53

(3) Ibrahimia Reg.

Station	Distance (m)	Excavation (C1)			Excavation (C2)			Backfilling (R)			Embankment (E0)		
		Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )
255.7		0.00						0.00					
250.7	5.0	0.65	0.33	1.65				0.00	0.00	0.00			
245.7	5.0	5.09	2.87	14.35				0.19	0.10	0.50			
209.8	35.9	31.55	18.32	657.69				10.54	5.37	192.78			
(+) 204.8	5.0	90.25	60.90	304.50				10.09	10.32	51.60			
(-) 204.8	0.0	110.51	100.38	0.00				10.88	10.49	0.00			
(+) 179.8	25.0	95.58	103.05	2,576.25				14.69	12.79	319.75			
(-) 179.8	0.0	95.58	95.58	0.00				14.69	14.69	0.00			
(+) 174.8	5.0	90.92	93.25	466.25	-			15.85	15.27	76.35			
(-) 174.8	0.0	107.56	99.24	0.00	24.09	24.09	0.00	37.11	26.48	0.00			
173.8	1.0	107.71	107.84	107.64	24.09	24.09	24.09	37.48	37.30	37.30			
172.8	1.0	109.13	108.42	108.42	0.00	12.05	12.05	37.78	37.63	37.63			
154.7	18.1	119.84	114.49	2,072.27				46.85	42.32	765.99			
(+) 151.8	2.9	116.31	118.08	342.43	-			48.33	47.59	138.01			
(-) 151.8	0.0	126.20	121.26	0.00	33.06	33.06	0.00	49.02	48.68	0.00			
150.8	1.0	126.69	126.45	126.45	33.66	33.66	33.66	49.52	49.27	49.27			
149.8	1.0	126.89	126.79	126.79	0.00	16.83	16.83	48.27	48.90	48.90			
(+) 140.0	9.8	114.83	120.86	1,184.43				44.48	46.38	454.52			
(-) 140.0	0.0	114.83	114.83	0.00				49.74	47.11	0.00			
125.4	14.6	106.20	110.52	1,613.59	0.00	0.00	0.00	54.81	52.28	763.29			
124.4	1.0	106.20	106.20	106.20	33.66	16.83	16.83	56.01	55.41	55.41			
(+) 123.4	1.0	106.64	106.42	106.42	33.66	33.66	33.66	57.18	56.60	56.60			
(-) 123.4	0.0	25.13	65.89	0.00	-			16.68	36.93	0.00			
121.2	2.2	24.75	24.94	54.87				17.00	16.84	37.05			
97.7	23.5	89.42	57.09	1,341.62				20.14	18.57	436.40			
84.7	13.0	125.78	107.60	1,398.80				0.00	10.07	130.91			
69.5	15.2	2.76	64.27	976.90									
62.4	7.1	0.00	1.38	9.80									
				13,697.32			136.82			3,652.26			0.00

Station	Distance (m)	Embankment (BL)			Embankment (BR)			Earth Finishing Foundation (L1)			Rough Finishing (L2)		
		Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )	Line (m)	Ave. (m)	Area (m <sup>2</sup> )	Line (m)	Ave. (m)	Area (m <sup>2</sup> )
255.7					0.00								
250.7	5.0				7.35	3.68	18.40						
245.7	5.0	-			10.01	8.68	43.40						
209.8	35.9	10.77			95.00	52.51	1,885.11						
(+) 204.8	5.0	0.47	5.82	28.10	101.64	98.32	491.60						
(-) 204.8	0.0	0.47	0.47	0.00	101.64	101.64	0.00						
(+) 179.8	25.0	1.54	1.01	25.25	110.76	106.20	2,655.00						
(-) 179.8	0.0	1.54	1.54	0.00	110.76	110.76	0.00						
(+) 174.8	5.0	1.54	1.54	7.70	111.57	111.17	555.85	-			-		
(-) 174.8	0.0	24.06	12.80	0.00	166.33	138.95	0.00	34.50			2.84		
173.8	1.0	24.06	24.06	24.06	166.33	166.33	166.33	34.50	34.50	34.50	2.90	2.87	2.87
172.8	1.0	24.06	24.06	24.06	166.33	166.33	166.33	34.70	34.60	34.60	2.53	2.72	2.72
154.7	18.1	21.78	22.92	414.85	170.60	168.47	3,049.31	34.70	34.70	628.07	3.28	2.90	52.49
(+) 151.8	2.9	21.77	21.78	63.16	169.77	170.19	493.55	34.70	34.70	100.63	3.89	3.58	10.38
(-) 151.8	0.0	18.73	20.25	0.00	165.05	167.41	0.00	35.50	35.10	0.00	5.02	4.46	0.00
150.8	1.0	18.74	18.74	18.74	165.06	165.06	165.06	35.50	35.50	35.50	5.09	5.06	5.06
149.8	1.0	18.74	18.74	18.74	165.06	165.06	165.06	35.70	35.60	35.60	2.60	3.85	3.85
(+) 140.0	9.8	13.44	16.09	157.68	174.56	169.81	1,664.14	35.70	35.70	349.86	3.22	2.91	28.52
(-) 140.0	0.0	24.15	18.80	0.00	224.72	199.64	0.00	35.70	35.70	0.00	3.22	3.22	0.00
125.4	14.6	22.01	23.08	336.97	235.13	229.93	3,356.98	35.70	35.70	521.22	4.17	3.70	54.02
124.4	1.0	22.01	22.01	22.01	235.13	235.13	235.13	35.50	35.60	35.60	6.79	5.48	5.48
(+) 123.4	1.0	22.01	22.01	22.01	235.13	235.13	235.13	35.50	35.50	35.50	6.85	6.82	6.82
(-) 123.4	0.0	-			246.15	240.64	0.00	-			-		
121.2	2.2				252.98	249.57	549.05						
97.7	23.5				232.78	242.88	5,707.68						
84.7	13.0				134.33	183.56	2,386.28						
69.5	15.2				33.47	83.90	1,275.28						
62.4	7.1				8.01	20.74	147.25						
				1,163.33			25,411.92			1,811.08			172.21











### C-1-3. Quantity calculation of foundation works

#### (1) Quantity summary table of foundation

Discription	Dimension	Unit	Bahr Yusef Reg.		Ibrahima Reg.		Total
			Main body	L type wall	Main body	L type wall	
Cast in place RC pile	Φ 1,000× <sup>L</sup> 13.50m	nos	72				72
	Φ 1,000× <sup>L</sup> 7.50m	"		32			32
	Φ 1,000× <sup>L</sup> 15.00m	"			56		56
	Φ 1,000× <sup>L</sup> 7.00m	"				32	32

● Unit quantity								
① Reinforceing bar	Dimension	Unit	D12	D16	D22	D25	D32	Total
(Steel 360/520)	Φ 1,000× <sup>L</sup> 13.50m	ton/nos	0.013	0.523	0.040	0.922		1.498
	Φ 1,000× <sup>L</sup> 7.50m	"	0.010	0.314	0.029		1.481	1.834
	Φ 1,000× <sup>L</sup> 15.00m	"	0.014	0.553	0.050	0.790		1.407
	Φ 1,000× <sup>L</sup> 7.00m	"	0.010	0.296	0.029		1.405	1.740
② Reinforced concrete	Dimension	Unit						Total
(σ <sub>ck</sub> =25N/mm <sup>2</sup> (cube))	Φ 1,000× <sup>L</sup> 13.50m	m <sup>3</sup> /nos						10.60
	Φ 1,000× <sup>L</sup> 7.50m	"						5.89
	Φ 1,000× <sup>L</sup> 15.00m	"						11.78
	Φ 1,000× <sup>L</sup> 7.00m	"						5.50

## (1) Bahr Yusef reg. (Main body)

Cast-in place RC pile Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
Cast-in place RC pile					
Cast-in place RC pile					
Cast-in place RC pile					
Cast-in place RC pile					
Reinforced con.	$\sigma$ ck-25N/mm <sup>2</sup> (cube)				
		$\pi /4 \times 1.00^2 \times 13.50$	= 10.60	10.60	m <sup>3</sup> /nos
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12	= 0.013	0.013	t nos
		D16	= 0.523	0.523	t nos
		D22	= 0.040	0.040	t nos
		D25	= 0.922	0.922	t/nos
<b>【Total quantity】</b>					
Nos of pile		9 × 8	= 72.00	72	nos
Reinforced con.	$\sigma$ ck-25N/mm <sup>2</sup> (cube)				
		10.60 × 72	= 763.20	763.20	m <sup>3</sup>
Rebar	Steel 360/520	D12 0.013 × 72	= 0.936	0.936	t
		D16 0.523 × 72	= 37.656	37.656	t
		D22 0.040 × 72	= 2.880	2.880	t

## (2) Bahr Yusef reg. (Ltype wall)

Cast-in place RC pile Quantity calculation						
Description	Classification	Calculation	Quantity	Unit	Remarks	
Cast-in place RC pile						
Cast-in place RC pile						
Cast-in place RC pile						
Cast-in place RC pile						
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$ (cube)					
		$\pi/4 \times 1.00^2 \times 7.50$	= 5.89	5.89	m <sup>3</sup> /nos	
Rebar	Steel 360/520	refer to rebar weight calculation sheet				
		D12	= 0.010	0.010	t nos	
		D16	= 0.314	0.314	t nos	
		D22	= 0.029	0.029	t nos	
		D32	= 1.481	1.481	t/nos	
<b>【Total quantity】</b>						
Nos of pile		$8 \times 2 \times 2$	= 32.00	32	nos	
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$ (cube)					
		$5.89 \times 32$	= 188.48	188.48	m <sup>3</sup>	
Rebar	Steel 360/520	D12	$0.010 \times 32$	= 0.320	0.320	t
		D16	$0.314 \times 32$	= 10.048	10.048	t
		D22	$0.029 \times 32$	= 0.928	0.928	t
		D32	$1.481 \times 32$	= 47.392	47.392	t

Bahr Yusef Reg. (Main body)

Rebar weight [1/1]

per a pile

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
P1	D 25	A		15.175 <sup>×1</sup>						1.375 <sup>×1</sup>	16.55	14	231.70	
P2	D 22	C		2.051 <sup>×1</sup>	1.210 <sup>×1</sup>					φ 653	3.27	4	13.08	
P3	D 16	D		2.328 <sup>×1</sup>	1.164 <sup>×1</sup>	0.113 <sup>×2</sup>	0.160 <sup>×2</sup>			φ 741	4.04	83	335.32	
P4	D 12	E		0.558 <sup>×1</sup> ~0.697	0.180 <sup>×2</sup>						0.99	8	7.92	
P5	D 12	F		0.188 <sup>×1</sup>	0.050 <sup>×2</sup>					φ 120	0.29	16	4.64	

D12	12.56 × 0.995	=	12.50 kg	=	0.013 t
D16	335.32 × 1.56	=	523.10 kg	=	0.523 t
D22	13.08 × 3.04	=	39.76 kg	=	0.040 t
D25	231.70 × 3.98	=	922.17 kg	=	0.922 t

Type	A	B	C	D	E	F	Dia.	kg m	R	L	J
							D12	0.995	130	204	1000
							D14	1.56	150	236	1000
							D16	1.56	170	267	1000
							D18	2.25	190	298	1000
							D20	2.25	210	330	1100
							D22	3.04	240	377	1210
							D25	3.98	270	424	1375
							D28	5.04	300	471	1540
							D32	6.23	340	534	1760

C-15

Bahr Yusuf Rog. (L type wall)

Rebar weight [1/1]

per a pile

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
P1	D 32	B		8.750 <sup>×1</sup>	0.550 <sup>×1</sup>	0.480 <sup>×1</sup>					9.78	24	234.72	
P2	D 22	C		2.029 <sup>×1</sup>	1.210 <sup>×1</sup>				φ 646		3.24	3	9.72	
P3	D 16	D		2.350 <sup>×1</sup>	1.077 <sup>×1</sup>	0.113 <sup>×2</sup>	0.160 <sup>×2</sup>			φ 748	3.98	50	199.00	
P4	D 12	E		0.562 <sup>×1</sup> ~0.704	0.180 <sup>×2</sup>						1.00	8	8.00	
P5	D 12	F		0.188 <sup>×1</sup>	0.050 <sup>×2</sup>					φ 120	0.29	12	3.48	

D12      11.48 × 0.888      =      10.19 kg      =      0.010 t

D16      199.00 × 1.58      =      314.42 kg      =      0.314 t

D22      9.72 × 2.98      =      28.97 kg      =      0.029 t

D32      234.72 × 6.31      =      1481.08 kg      =      1.481 t

C-16

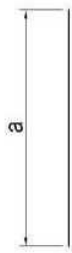
タイプ	A	B	C	D	E	F		Dia.	kg m	R	L	J
								D12	0.888	130	204	1000
								D14	1.21	150	236	1000
								D16	1.58	170	267	1000
								D18	2.00	190	298	1000
								D20	2.47	210	330	1100
								D22	2.98	240	377	1210
								D25	3.85	270	424	1375
								D28	4.83	300	471	1540
								D32	6.31	340	534	1760



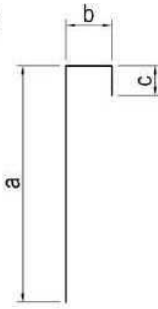
# Schematic design of rebar

(Bahr Yusef reg.)

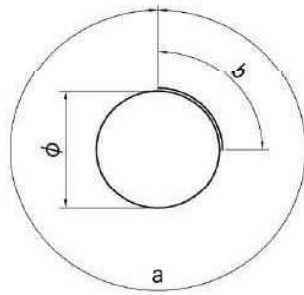
A



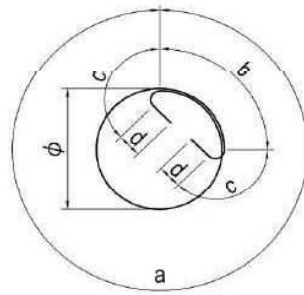
B



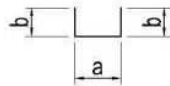
C



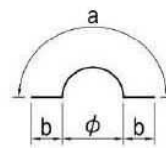
D



E



F



## (3) Ibrahimia reg. (Main body)

Cast-in place RC pile Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
Cast-in place RC pile					
Cast-in place RC pile					
Cast-in place RC pile					
Cast-in place RC pile					
Reinforced con.	$\sigma_{ck}$ -21N/mm <sup>2</sup> (cube)				
		$\pi /4 \times 1.00^2 \times 15.00$	= 11.78	11.78	m <sup>3</sup> /nos
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12	= 0.014	0.014	t nos
		D16	= 0.553	0.553	t nos
		D22	= 0.050	0.050	t nos
		D25	= 0.790	0.790	t/nos
<b>【Total quantity】</b>					
Nos of pile		8 × 7	= 56.00	56	nos
Reinforced con.	$\sigma_{ck}$ -21N/mm <sup>2</sup> (cube)				
		11.78 × 56	= 659.68	659.68	m <sup>3</sup>
Rebar	Steel 360/520	D12 0.014 × 56	= 0.784	0.784	t
		D16 0.553 × 56	= 30.968	30.968	t
		D22 0.050 × 56	= 2.800	2.800	t

## (4) Ibrahimia reg. (Itype wall)

Cast-in place RC pile Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
Cast-in place RC pile					
Cast-in place RC pile					
Cast-in place RC pile					
Cast-in place RC pile					
Reinforced con.	$\sigma_{ck}$ -21N/mm <sup>2</sup> (cube)				
		$\pi /4 \times 1.00^2 \times 7.00$	= 5.50	5.50	m <sup>3</sup> /nos
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12	= 0.010	0.010	t nos
		D16	= 0.296	0.296	t nos
		D22	= 0.029	0.029	t nos
		D32	= 1.405	1.405	t/nos
<b>【Total quantity】</b>					
Nos of pile		8 × 2 × 2	= 32.00	32	nos
Reinforced con.	$\sigma_{ck}$ -21N/mm <sup>2</sup> (cube)				
		5.50 × 32	= 176.00	176.00	m <sup>3</sup>
Rebar	Steel 360/520	D12 0.010 × 32	= 0.320	0.320	t
		D16 0.296 × 32	= 9.472	9.472	t
		D22 0.029 × 32	= 0.928	0.928	t
		D32 1.405 × 32	= 44.960	44.960	t

Ibrahimia Reg. (Main body) Rebar weight [1/1]

per a pile

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
P1	D 25	A		16.675 <sup>x1</sup>						1.375 <sup>x1</sup>	18.05	11	198.55	
P2	D 22	C		2.051 <sup>x1</sup>	1.210 <sup>x1</sup>					φ 653	3.27	5	16.35	
P3	D 16	D		2.328 <sup>x1</sup>	1.058 <sup>x1</sup>	0.113 <sup>x2</sup>	0.160 <sup>x2</sup>			φ 741	3.94	90	354.60	
P4	D 12	E		0.558 <sup>x1</sup> ~0.697	0.180 <sup>x2</sup>						0.99	8	7.92	
P5	D 12	F		0.188 <sup>x1</sup>	0.050 <sup>x2</sup>					φ 120	0.29	20	5.80	

D12	13.72 × 0.995	=	13.65 kg	=	0.014 t
D16	354.60 × 1.56	=	553.18 kg	=	0.553 t
D22	16.35 × 3.04	=	49.70 kg	=	0.050 t
D25	198.55 × 3.98	=	790.23 kg	=	0.790 t

C-20

Type	A	B	C	D	E	F	Dia.	kg m	R	L	J
							D12	0.995	130	204	1000
							D14	1.56	150	236	1000
							D16	1.56	170	267	1000
							D18	2.25	190	298	1000
							D20	2.25	210	330	1100
							D22	3.04	240	377	1210
							D25	3.98	270	424	1375
							D28	5.04	300	471	1540
							D32	6.23	340	534	1760

Ibrahimia Reg. (L type wall)

Rebar weight [1/1]

per a pile

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
P1	D 32	B		8.250 <sup>x1</sup>	0.550 <sup>x1</sup>	0.480 <sup>x1</sup>					9.28	24	222.72	
P2	D 22	C		2.029 <sup>x1</sup>	1.210 <sup>x1</sup>				φ 648		3.24	3	9.72	
P3	D 16	D		2.350 <sup>x1</sup>	1.077 <sup>x1</sup>	0.113 <sup>x2</sup>	0.160 <sup>x2</sup>			φ 748	3.98	47	187.06	
P4	D 12	E		0.562 <sup>x1</sup> ~0.704	0.180 <sup>x2</sup>						1.00	8	8.00	
P5	D 12	F		0.188 <sup>x1</sup>	0.050 <sup>x2</sup>				φ 120		0.29	12	3.48	

D12	11.48 × 0.888	=	10.19 kg	=	0.010 t
D16	187.06 × 1.58	=	295.55 kg	=	0.296 t
D22	9.72 × 2.98	=	28.97 kg	=	0.029 t
D32	222.72 × 6.31	=	1405.36 kg	=	1.405 t

C-21

Type	A	B	C	D	E	F	Dia.	kg m	R	L	J
							D12	0.888	130	204	1000
							D14	1.21	150	236	1000
							D16	1.58	170	267	1000
							D18	2.00	190	298	1000
							D20	2.47	210	330	1100
							D22	2.98	240	377	1210
							D25	3.85	270	424	1375
							D28	4.83	300	471	1540
							D32	6.31	340	534	1760

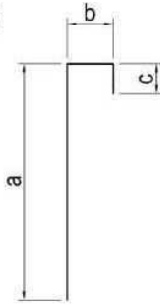
# Schematic design of rebar

(Ibrahimia reg.)

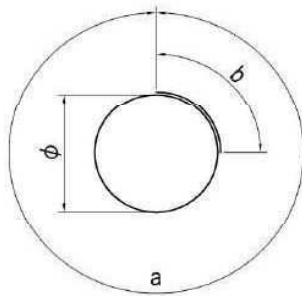
A



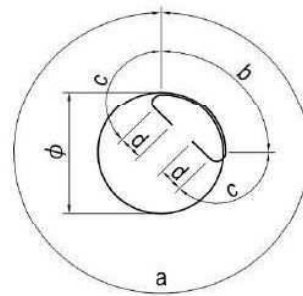
B



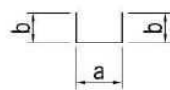
C



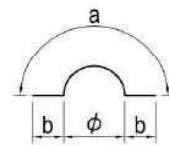
D



E



F



C-1-4 Quantity calculation cutoff sheet pile works

Description	Dimension		Unit	Bahr Yusef			Bahr Yusef			Badhaman (Badraman)		Badraman (Diroutiah)		Sahe-lyia	Abo Gabal	Total
				Up	Mid	Down	Up	Mid	Down	Up	Down	Up	Down	Up	Up	
Sheet pile (S270GP)	PU12	L=12.0 m	piece	60			60									120
		L=6.0 m	"		59			59								118
		L=4.5 m	"			60		60								120
		L=2.0 m	"						14	14	19	19	14	24		104
	PU12	L=13.5 m	L-1	"				1								1
		L=11.0 m	11	"									2			2
		L=9.5 m	G,M	"				1						2		3
		L=9.0 m	1	"							2					2
		L=8.0 m	J,L	"						2		2				4
	PU18	L=21.5 m	A	"	1											1
		L=19.0 m	C-1	"	1											1
		L=17.5 m	I	"			1									1
		L=16.5 m	BR-1	"			1									1
		L=16.0 m	D-1	"			1									1
		L=11.0 m	BL	"			1									1
		L=10.0 m	K	"						2						2
	PU12		m	732.0	354.0	270.0	732.0	354.0	293.0	28.0	44.0	56.0	54.0	50.0	67.0	
		Σ	m	1,356.0			1,379.0			72.0		110.0		50.0	67.0	3,034.0
		Σ	t	89.632			91.152			4.759		7.271		3.305	4.429	200.548
	PU18		m	40.5	0.0	27.5	33.5	0.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	
		Σ	m	68.0			33.5			20.0		0.0		0.0	0.0	121.5
		Σ	t	5.229			2.576			1.538		0.000		0.000	0.000	9.343
Cutting	PU12		m	12.0			12.0									24.0
Welding	PU12		m	12.0			12.0									24.0
Painting on steel sheet pile			m <sup>2</sup>	19.9		22.6	19.9		19.8	24.0	19.2	21.6	19.2	15.0	12.5	
		Σ	m <sup>2</sup>	42.5			39.7			43.2		40.8		15.0	12.5	193.7

Description	Dimension		Penetration depth	Painting on steel sheet pile
Sheet pile (S270GP)	PU12	L=12.0 m	11.50 m/piece	0.00 m <sup>2</sup> /piece
		L=6.0 m	5.50 "	0.00 "
		L=4.5 m	4.00 "	0.00 "
		L=2.0 m	1.50 "	0.00 "
	PU12	L=13.5 m	E-1	5.50 "
		L=11.0 m	II	3.25 "
		L=9.5 m	G	1.10 "
		L=9.5 m	M	2.95 "
		L=9.0 m	I	3.00 "
		L=8.0 m	J	3.10 "
		L=8.0 m	L	2.80 "
	PU18	L=21.5 m	A	11.20 "
		L=19.0 m	C-1	8.30 "
		L=17.5 m	F	7.20 "
		L=16.5 m	BR-1	7.70 "
		L=16.0 m	D-1	5.70 "
		L=11.0 m	BL	2.30 "
		L=10.0 m	K	3.70 "



C-1-5. Quantity calculation of weir works

Description	Dimension		Unit	Bahr Yusuf	Ibrahimia	Badraman		Abo Gabal	Sahelyia	Total
						Badraman	Droutiah			
Demolition works	Concrete demolition	PC structure	m <sup>3</sup>					3.6	2.1	5.8
		RC structure	m <sup>3</sup>					123.6	71.8	195.4
Main body	Concrete	Leveling con. σ <sub>ck</sub> =20kN/mm <sup>2</sup>	m <sup>3</sup>	153.3	154.6	17.5	24.3	37.9	15.6	403.1
		Reinforced con. σ <sub>ck</sub> =25kN/mm <sup>2</sup>	m <sup>3</sup>	6,503.8	6,460.9	770.9	987.6	1,441.4	752.4	16,917.0
		Second stage con. σ <sub>ck</sub> =25kN/mm <sup>2</sup>	m <sup>3</sup>	143.2	142.7	10.3	14.5	23.7	13.4	347.7
	Formwork	Leveling con.	m <sup>2</sup>	15.4	15.4	4.9	4.8	6.7	4.8	51.9
		Reinforced con.	m <sup>2</sup>	4,586.1	4,498.1	1,146.5	1,350.1	1,660.8	1,060.3	14,302.0
		Second stage con.	m <sup>2</sup>	468.2	464.6	51.7	70.9	121.3	72.0	1,248.8
	Circular formwork	RC structure Φ2,500-1/2 Circle	m <sup>2</sup>	176.7	173.2					349.9
		RC structure Φ1,000-1/2 Circle	m <sup>2</sup>			11.2	20.4	29.9	9.4	70.8
		RC structure Φ1,000-1/4 Circle	m <sup>2</sup>	17.3	16.8	11.2	10.2	5.3	9.4	70.1
	Rebar	Steel 360/520 D12	ton	21.103	20.947	2.069	2.686	3.968	1.831	52.604
		Steel 360/520 D16	ton	143.639	142.678	31.552	40.722	53.777	31.097	443.465
		Steel 360/520 D18	ton	26.059	26.059					52.118
		Steel 360/520 D22	ton	16.528	16.490	2.400	2.424			37.842
	Concrete curing	PC structure	m <sup>3</sup>	153.3	154.6	17.5	24.3	37.9	15.6	403.1
		RC structure	m <sup>3</sup>	6,647.0	6,603.5	781.2	1,002.1	1,465.1	765.8	17,264.7
Scaffolding	Prefabricated scaffold H≤30m	m <sup>2</sup>	3,665.3	3,625.5	848.6	933.3	975.4	707.0	10,755.1	
Falsework	Pipe support falsework	m <sup>3</sup>			45.8	61.5	60.6		167.9	
	Wedge linked falsework	m <sup>3</sup>	1,680.7	1,666.3	56.9	77.0	417.1	294.8	4,192.8	
Expansion joint	Elastic filler , t=20mm	m <sup>2</sup>	160.1	159.2			96.4	73.1	488.7	
Water stop	B=300mm	m	95.5	94.9			34.0	26.6	251.0	
Water swelling water stop	Paste type , 9mm×30mm	m					36.7	29.4	66.1	
Resin capsule anchor	D20×200 , Drilling dia. Φ25	nos					121	96	217	
Slip bar	D20×1.00m , VPΦ25×0.50m	nos	321	319			111	86	837	
	D20×0.70m	nos					121	96	217	
Other miscellaneous	Joint bar	D16×300	nos	1,472	1,472					2,944
		D16×200	nos	1,152	1,120	464	464	1,120	636	4,956
	Grating	5,200×2,400	set	4	4					8
		6,000×1,000	set	8	8					16
		2,000×1,000	set			4	6	4	2	16
	Crane facility	Rail (15kg/m)	m	35.5	35.5					71.0
		Manual chain block (50)	set	1	1					2
	Steps	B=300mm, Resin coated	pcs	212	208	54	75	136	80	765
	Staff gauge	H=7.50m	pcs	1	1					2
		H=7.00m	pcs	1						1
H=6.70m		pcs		1					1	
H=5.95m		pcs						1	1	
H=4.75m		pcs					2		2	
H=4.50m		pcs						1	1	
H=3.90m		pcs					1		1	
H=3.60m		pcs			1				1	
Safty facilities work	Guard fense	H=3.30m	pcs				1			1
		H=3.00m	pcs			1				1
		H=2.70m	pcs				1			1
		H=1.10m	m	201.6	201.6	41.2	59.2	29.8	29.8	563.2
Approach cushion slab	Concrete	Reinforced con σ <sub>ck</sub> =25kN/mm <sup>2</sup>	m <sup>3</sup>	40.0	40.0	40.0	40.0			160.0
		Formwork	m <sup>2</sup>	24.0	24.0	24.0	24.0			96.0
	Rebar	Steel 360/520 D12	ton	0.150	0.150	0.150	0.150			0.600
		Steel 360/520 D16	ton	2.370	2.370	2.370	2.370			9.480
	Concrete curing	Steel 360/520 D18	ton	1.040	1.040	1.040	1.040			4.160
		RC structure	m <sup>3</sup>	40.0	40.0	40.0	40.0			160.0
	Anchor cap	SGPΦ40×180	nos	50	50	50	50			200
	Anchor bar	D22×500 (including D10 spiral reinforcement)	nos	50	50	50	50			200
	Rubber shoe	t=30mm	m <sup>2</sup>	6.0	6.0	6.0	6.0			24.0
	Expansion joint	Elastic filler , t=20mm	m <sup>2</sup>	8.6	8.6	8.6	8.6			34.4

## (1) Bahr Yusef regulator

Bahr Yusef Weir body Quantity Summary Sheet							
Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Up stream	Down stream
Weir body	Main body	Leveling con.	$\sigma_{ck}=20N/mm^2$	$m^3$	153.30	81.82	71.48
		Formwork	Leveling con.	$m^2$	15.40	4.90	10.50
		Reinforced con.	$\sigma_{ck}=25N/mm^2$	$m^3$	6,503.79	4,810.04	1,693.75
		Formwork	RC structure	$m^2$	4,586.12	3,686.25	899.87
		Circular formwork	RC structure $\phi 2500 \cdot 1/2$ Circle	$m^2$	176.71	176.71	—
		"	RC structure $\phi 1000 \cdot 1/4$ Circle	$m^2$	17.28	6.28	11.00
		Second stage con.	$\sigma_{ck}=25N/mm^2$	$m^3$	143.19	143.19	—
		Formwork	RC structure	$m^2$	468.24	468.24	—
		Rebar (Steel 360/520)	D12	t	21.103	13.637	7.466
		"	D16	t	143.639	103.030	40.609
		"	D18	t	26.059	26.059	—
		"	D22	t	16.528	3.230	13.298
		Concrete curing	PC structure	$m^3$	153.30	81.82	71.48
		"	RC structure	$m^3$	6,646.98	4,953.23	1,693.75
		Scaffolding	Prefabricated scaffold $H \leq 30m$	$m^2$	3,665.33	2,812.56	852.77
		Falsework	Wedge linked falsework, $H \leq 30m$ , Support bearing force $40KN/m^2$ and less	$m^3$	1,680.69	1,680.69	—
		Expansion joint	Elastic filler; $t=20mm$	$m^2$	160.06	—	160.06
		Water stop	$B=300mm$	m	95.50	48.50	47.00
		Slip bar	$D20 \times 1.00m$ , $VR \phi 25 \times 0.50m$	nos	321	165	156
	Other miscellaneous	Joint bar	$D16 \times 300$	nos	1,472	1,472	—
		"	$D16 \times 200$	nos	1,152	1,152	—
		Grating	$5200 \times 2400$	Set	4	4	—
		"	$6000 \times 1000$	Set	8	8	—
	Safety facilities work	Guard fence	$H=1.10m$	m	201.60	201.60	—



## Bahr Yusef Weir body      Quantity calculation

**[Upstream]**

Description	Classification	Calculation	Quantity	Unit	Remarks
Mainbody Leveling con.	$\sigma_{ck}=20\text{N/mm}^2$				
		$35.70 \times (24.40 + 24.60) / 2 \times 0.10$	87.47		
		$-\pi/4 \times 1.00^2 \times 0.10 \times 72$	-5.65		(-)Piles part
		Total	81.82	81.82	m <sup>3</sup>
Formwork	Leveling con.				
		$(24.40 + 24.60) / 2 \times 0.10 \times 2$	4.90		
		Total	4.90	4.90	m <sup>2</sup>
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 36.50$	54.75		Upstream
	"	$1/2 \times (1.00 + 2.00) \times 1.00 \times 8.00 \times 2$	24.00		Downstream
	"	$0.50 \times 0.50 \times 8.00 \times 2$	4.00		"
	"	$1/2 \times (0.50 + 1.50) \times 1.00 \times 19.50$	19.50		"
	"	$1/2 \times (1.00 + 1.023) \times 0.50 \times 19.50$	9.86		"
	Invert	$1.00 \times 3.00 \times 1.00$	3.00		Upstream left
	"	$2.00 \times 2.00 \times 1.00$	4.00		Upstream right
	"	$27.9 \times 2.00 \times 35.50$	1980.90		
	"	$1/2 \times (9.00 + 11.00) \times 0.50 \times 6.00 \times 4$	120.00		
	"	$-2.40 \times 1.05 \times 0.40 \times 8$	-8.06		(-)Blockout part
	"	$-0.60 \times 5.60 \times 0.40 \times 4$	-5.38		"
	"	$-1.00 \times 6.70 \times 0.30 \times 4 \times 2$	-16.08		"
	"	$-\pi/4 \times 1.00^2 \times 0.10 \times 72$	-5.65		(-)Piles part
	Wall A	$1.00 \times 8.00 \times 1.00$	8.00		
	"	$2.00 \times 8.00 \times 11.15$	178.40		
	"	$2.00 \times 7.00 \times 16.75$	234.50		
	"	$-(1.00 \times 0.35 - 2.40 \times 0.85) \times 7.50$	-17.93		(-)Blockout part
	"	$-1.00 \times 0.35 \times 7.00$	-2.45		"

Description	Classification	Calculation	Quantity	Unit	Remarks
	Wall B	$2.50 \times 8.00 \times 10.65 \times 3 = 639.00$			
	"	$2.50 \times 7.00 \times 16.25 \times 3 = 853.13$			
	"	$-(1 - \pi/4) \times 1.25^2 \times 2 \times (8.00 - 7.00) \times 3 = -30.18$			
	"	$-(1.00 \times 0.35 - 2.40 \times 0.85) \times 7.50 \times 2 \times 3 = -107.55$			(-)Blockout part
	"	$-1.00 \times 0.35 \times 7.00 \times 2 \times 3 = -14.70$			"
	Wall C	$1.00 \times 8.00 \times 1.00 = 8.00$			
	"	$2.00 \times 8.00 \times 11.15 = 178.40$			
	"	$2.00 \times 7.00 \times 16.75 = 234.50$			
	"	$-(1 - \pi/4) \times 1.00^2 \times 8.00 = -1.72$			
	"	$-(1.00 \times 0.35 - 2.40 \times 0.85) \times 7.50 = -17.93$			(-)Blockout part
	"	$-1.00 \times 0.35 \times 7.00 = -2.45$			"
	Corridor	$1.00 \times 0.50 \times 6.00 \times 4 \times 4 = 48.00$			
	"	$0.15 \times 0.15 \times 6.00 \times 4 \times 4 = 2.16$			(-)Second stage con.
	Bridge seat	$11.00 \times 2.07 \times 2.00 \times 2 = 91.08$			Abut pier
	"	$11.00 \times 2.07 \times (2.50 + 1.00 \times 2) = 102.47$			Middle pier
	Earth pressure wall	$0.75 \times 33.83 \times 2 = 50.75$			
	Pedestal	$1/2 \times (0.30 + 0.70) \times 0.40 \times 10.00 \times 2 = 4.00$			
	Gate pier	$(2.00 \times 4.40 - 2.40 \times 0.85) \times 4.50 \times 2 = 60.84$			
	"	$0.85 \times 1.00 \times 4.00 \times 4 \times 3 = 40.80$			
	"	$2.50 \times 1.00 \times 0.50 \times 2 \times 3 = 7.50$			
	Top slab	$6.40 \times 0.50 \times 35.50 = 113.60$			
	"	$1/2 \times 1.00 \times 0.50 \times (2.00 \times 2 - 2.50 \times 3) = 2.88$			
	"	$-2.40 \times 0.50 \times 7.70 \times 4 = -36.96$			(-)Opening
	"	$-0.15 \times 0.15 \times 5.20 \times 2 \times 4 = -0.94$			(-)Second stage con.
		Total	4810.04	4810.04	m <sup>3</sup>
Formwork	RC structure				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 2 \times 2 = 6.00$			Side
	"	$1.00 \times 35.50 = 35.50$			Upstream end
	"	$1.50 \times 8.00 \times 2 + 1.00 \times 19.50 = 43.50$			Downstream end

Description	Classification	Calculation	Quantity	Unit	Remarks
	Invert	$1.00 \times 3.00 \times 3$	= 9.00		Upstream left
	"	$1.00 \times 2.00$	= 2.00		Upstream right
	"	$35.50 \times 2.00$	= 71.00		Upstream end
	"	$27.90 \times 2.00 \times 2$	= 111.60		Side
	"	$1.50 \times 8.00 \times 2 + 2.00 \times 19.50$	= 63.00		Downstream end
	"	$0.50 \times 0.50 \times 2$	= 0.50		"
	"	$1.118 \times 6.00 \times 2 \times 4$	= 53.66		Raising part
	"	$(2.40 - 7.70) \times 2 \times 0.40 \times 4$	= 32.32		Blockout part
	"	$(0.812 + 0.988) \times 0.40 \times 2 \times 4$	= 5.76		"
	"	$(1.00 - 6.70) \times 2 \times 0.30 \times 4 \times 2$	= 36.96		"
	Wall A	$1.00 \times 8.00 \times 3$	= 24.00		
	"	$8.00 \times (1.00 + 11.15 \times 2)$	= 186.40		
	"	$7.00 \times (2.00 + 16.75 \times 2)$	= 248.50		
	"	$7.50 \times (0.35 + 0.85) \times 2$	= 18.00		Blockout part
	"	$7.00 \times 0.35 \times 2$	= 4.90		"
	"	$-1/2 \times (9.00 + 11.00) \times 0.50$	= -5.00		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	= -1.91		(-)Corridor
	Wall B	$8.00 \times 9.40 \times 2 \times 3$	= 451.20		
	"	$2.50 \times 1.00 \times 2$	= 5.00		
	"	$7.00 \times 15.00 \times 2 \times 3$	= 630.00		
	"	$7.50 \times (0.35 + 0.85) \times 4 \times 3$	= 108.00		Blockout part
	"	$7.00 \times 0.35 \times 4 \times 3$	= 29.40		"
	"	$-1/2 \times (9.00 + 11.00) \times 0.50 \times 2 \times 3$	= -30.00		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4 \times 2 \times 3$	= -11.46		(-)Corridor
	Wall C	$8.00 \times (2.00 + 1.00 + 11.15 + 10.15)$	= 194.40		
	"	$7.00 \times (2.00 + 16.75 \times 2)$	= 248.50		
	"	$7.50 \times (0.35 + 0.85) \times 2$	= 18.00		Blockout part
	"	$7.00 \times 0.35 \times 2$	= 4.90		"
	"	$-1/2 \times (9.00 + 11.00) \times 0.50$	= -5.00		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	= -1.91		(-)Corridor

Description	Classification	Calculation	Quantity	Unit	Remarks
	Comidor	$(1.00 - 0.50 \times 2) \times 6.00 \times 4 \times 4 = 192.00$			
	Bridge seat	$(11.00 \times 2 + 1.25) \times 2.07 \times 2 = 96.26$			Abut pier
	"	$1.25 \times 1.07 \times 2 = 2.68$			"
	"	$(11.00 \times 2 + 2.50) \times 2.07 = 50.72$			Middle pier
	"	$2.50 \times 1.07 = 2.68$			"
	"	$(11.00 \times 2 + 1.00) \times 2.07 \times 2 = 95.22$			"
	"	$1.00 \times 1.07 \times 2 = 2.14$			"
	Earth pressure wall	$33.83 \times 2 \times 2 = 135.32$			
	"	$(0.601 + 0.20 \times 4 + 0.15 \times 2 + 4.633 - 1.00) \times 0.75 \times 2 = 11.00$			
	"	$-0.70 \times 10.00 \times 2 = -14.00$			(-)Pedestal
	Gate pier	$(2.00 - 4.40 + 0.85) \times 2 \times 4.00 \times 2 = 116.00$			Abut pier
	"	$\{2.00 - (4.40 + 0.85) \times 2\} \times 0.50 \times 2 = 12.50$			"
	"	$-0.75 \times 2.237 \times 2 = -3.36$			(-)Earth pressure wall
	"	$(0.85 - 1.00) \times 2 \times 4.00 \times 4 \times 3 = 177.60$			Middle pier
	"	$(1.00 \times 4 + 2.50 \times 3) \times 0.50 \times 3 = 17.25$			"
	Top slab	$(6.40 - 35.50) \times 2 \times 0.50 = 41.90$			Side
	"	$(2.40 - 7.70) \times 2 \times 0.50 \times 4 = 40.40$			Opening
	"	$0.15 \times 0.15 \times 4 \times 4 = 0.36$			Second stage con
	"	$1.00 \times 35.50 = 35.50$			Underside
	"	$(2.00 - 1.00) \times 6.00 \times 4 = 72.00$			"
	"	$1/2 \times 1.00 \times 0.50 \times 2 \times 5 = 2.50$			Haunch
	"	$1.118 \times (2.00 \times 2 - 2.50 \times 3) = 12.86$			"
		Total = 3686.25	3686.25	m <sup>2</sup>	
Circular formwork	RC structure				
	φ2500-1/2Circle	$\pi/2 \times 2.50 \times (8.00 + 7.00) \times 3 = 176.71$	176.71	m <sup>2</sup>	
	φ1000-1/4Circle	$\pi/4 \times 1.00 \times 8.00 = 6.28$	6.28	m <sup>2</sup>	

Description	Classification	Calculation	Quantity	Unit	Remarks
Second stage con.	$\sigma_{ck}=25\text{N/mm}^2$				
		$2.40 \times 1.05 \times 0.40 \times 8 = 8.06$			Invert part
		$0.60 \times 5.60 \times 0.40 \times 4 = 5.38$			"
		$1.00 \times 6.70 \times 0.30 \times 4 \times 2 = 16.08$			"
		$2.40 \times 0.85 \times 7.50 \times 8 = 122.40$			Gate part
		$-0.54 \times (0.705 + 0.455) \times 7.50 \times 8 = -37.58$			"
		$1.00 \times 0.35 \times (7.50 + 7.00) \times 8 = 40.60$			Stop log part
		$-0.50 \times 0.25 \times (7.50 + 7.00) \times 8 = -14.50$			"
		$(0.15^2 - 0.05^2) \times 6.00 \times 4 \times 4 = 1.92$			Corridor part
		$(0.15^2 - 0.05^2) \times 5.20 \times 2 \times 4 = 0.83$			Top slab part
		Total = 143.19	143.19	$\text{m}^3$	
Formwork	RC structure				
		$(2.40 - 0.54 \times 4) \times 7.50 \times 8 = 273.60$			Gate part
		$(1.00 - 0.25 \times 2) \times (7.50 + 7.00) \times 8 = 174.00$			Stop log part
		$0.15 \times 6.00 \times 4 \times 4 = 14.40$			Corridor part
		$0.15 \times 5.20 \times 2 \times 4 = 6.24$			Top slab part
		Total = 468.24	468.24	$\text{m}^2$	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 = 13.637	13.637	t	
		D16 = 103.030	103.030	t	
		D18 = 26.059	26.059	t	
		D22 = 3.230	3.230	t	
Concrete curing	FC structure	Leveling concrete = 81.82	81.82	$\text{m}^3$	
	RC structure	4810.04 + 143.19 = 4953.23	4953.23	$\text{m}^3$	



Description	Classification	Calculation	Quantity	Unit	Remarks
Scaffolding	Prefabricated scaffold, H≤ 30m				
	Outer perimeter	$10.00 \times (38.90 + 1.00 + 13.20 + 12.20 + 1.20 + 0.40 \times 2)$			
		= 673.00			
	"	$12.57 \times 16.80 \times 2$			
		= 422.35			
	Inside	$8.00 \times 2.00 \times 8$			
	"	$7.50 \times 1.70 \times 8$			
		= 102.00			
	"	$9.07 \times 11.80 \times 8$			
		= 856.21			
	"	$7.00 \times (1.00 + 2.00) \times 8$			
		= 168.00			
	Gate pier part	$5.00 \times (8.60 + 37.70) \times 2$			
		= 463.00			
		Total = 2812.56	2812.56	m <sup>2</sup>	
Falsework	Wedge linked falsework, H≤ 30m Support bearing force 40KN/m <sup>2</sup> and less				
	①	$1.00 \times (7.325 + 7.00) / 2 \times 6.00 \times 4$	171.90		Corridor part
	②	$1.00 \times 7.00 \times 6.00 \times 4$	168.00		"
	③	$1.00 \times 6.50 \times 6.00 \times 2 \times 4$	312.00		"
	④	$2.00 \times 4.50 \times 6.00 \times 4$	216.00		Gate pier part
	⑤	$1.00 \times 12.00 \times 6.00 \times 2 \times 4$	576.00		"
	⑥	$1.00 \times 4.50 \times (2.00 \times 2 + 2.50 \times 3)$	51.75		"
	⑦	$1.00 \times (4.50 + 4.00) / 2 \times (2.00 \times 2 + 2.50 \times 3)$	48.88		"
	⑧	$1.00 \times 4.00 \times 0.80 \times 3 \times 2$	19.20		"
	⑨	$2.40 \times 4.50 \times 0.80 \times 3$	25.92		"
	⑩	$1/2 \times (11.58 + 11.18) \times 0.40 \times 10.00 \times 2$	91.04		Pedestal part
		Total = 1680.69	1680.69	m <sup>3</sup>	
Water stop	B=300mm	$33.50 + 7.50 \times 2$	48.50	48.50	m <sup>2</sup>
Slip bar	D20×1.00m, VPφ25×0.50m		165.0	165	nos

Description	Classification	Calculation	Quantity	Unit	Remarks
Other miscellaneous					
Joint bar	D16×300	$368 \times 4 = 1472.0$	1472	nos	Gate part
	D16×200	$(148 + 140) \times 4 = 1152.0$	1152	nos	Stop log part
Grating	5200×2400	$= 4.0$	4	Set	
	6000×1000	$= 8.0$	8	Set	
Safety facilities work					
Guard fence	H=1.10m	$35.20 \times 2 - 6.10 - 5.10 = 81.60$			Top slab part
		$7.50 \times 4 \times 2 \times 2 = 120.00$			Pedestal part
		Total = 201.60	201.60	m	
Approach cushion slab					
Reinforced con.	$\sigma_{ck} = 25 \text{ N/mm}^2$				
		$10.00 \times 5.00 \times 0.40 \times 2 = 40.00$	40.00	m <sup>3</sup>	
Formwork	RC structure				
		$(10.00 + 5.00) \times 0.40 \times 2 \times 2 = 24.00$	24.00	m <sup>2</sup>	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 $0.075 \times 2 = 0.150$	0.150	t	
		D16 $1.185 \times 2 = 2.370$	2.370	t	
		D18 $0.520 \times 2 = 1.040$	1.040	t	
Concrete curing	RC structure	Reinforced con. = 40.00	40.00	m <sup>3</sup>	







Bahr Yusef Reg. (Upstream) Rebar weight [1/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
F1	D 18	A		34.920 <sup>x1</sup>	2.660 <sup>x1</sup>	4.110 <sup>x1</sup>	0.298 <sup>x2</sup>		φ170	1.000 <sup>x1</sup>	46.29	140	6480.60		
F2	D 18	B		35.300 <sup>x1</sup>						1.000 <sup>x3</sup>	38.30	141	5400.30		
F3	D 18	B		3.100 <sup>x1</sup>							3.10	14	43.40		
F4	D 18	C		5.400 <sup>x1</sup>	1.300 <sup>x1</sup>	1.300 <sup>x1</sup>					8.00	36	288.00		
F5	D 18	B		5.900 <sup>x1</sup>							5.90	21	123.90		
F6	D 16	B		27.700 <sup>x1</sup>						1.000 <sup>x2</sup>	29.70	179	5316.30		
F7	D 16	D		23.683 <sup>x1</sup>	1.183 <sup>x1</sup>					1.000 <sup>x2</sup>	26.87	8	214.96		
F8	D 16	B		2.550 <sup>x1</sup>							2.55	157	400.35		
F9	D 16	E		1.856 <sup>x1</sup>	0.205 <sup>x1</sup>	1.183 <sup>x1</sup>					3.25	120	390.00		
F10	D 16	B		3.000 <sup>x1</sup>							3.00	152	456.00		
F11	D 16	C		1.766 <sup>x1</sup>	1.183 <sup>x1</sup>	1.283 <sup>x1</sup>					4.24	8	33.92		
F12	D 16	C		2.578 <sup>x1</sup>	1.183 <sup>x1</sup>	1.283 <sup>x1</sup>					5.05	112	565.60		
F13	D 16	B		4.400 <sup>x1</sup>							4.40	64	281.60		
F14	D 16	B		2.600 <sup>x1</sup>							2.60	112	291.20		
F15	D 16	E		1.856 <sup>x1</sup>	3.105 <sup>x1</sup>	1.283 <sup>x1</sup>					6.25	8	50.00		
F16	D 16	E		1.856 <sup>x1</sup>	4.093 <sup>x1</sup>	1.283 <sup>x1</sup>					7.24	112	810.88		
F17	D 16	D		12.133 <sup>x1</sup>	1.183 <sup>x1</sup>					1.000 <sup>x1</sup>	14.32	60	859.20		
F18	D 16	D		12.133 <sup>x1</sup>	1.183 <sup>x1</sup>					1.000 <sup>x1</sup>	14.32	60	859.20		
Type	A	B	C	D	E	F	G	H	I	J					
											Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
										D28	4.83	300	471	1540	
										D32	6.31	340	534	1760	

Bahr Yusef Reg. (Upstream) Rebar weight [2/4]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
F19	D 16	B		3.000 <sup>x1</sup>							3.00	152	456.00		
F20	D 16	D		2.783 <sup>x1</sup>	1.183 <sup>1</sup>						3.97	161	639.17		
F21	D 16	C		1.716 <sup>x1</sup>	0.240 <sup>2</sup>						2.20	323	710.60		
F22	D 18	C		35.266 <sup>x1</sup>	0.270 <sup>x1</sup>	0.270 <sup>x1</sup>				1.000 <sup>x3</sup>	38.81	16	620.96		
F23	D 16	B		27.664 <sup>x1</sup>						1.000 <sup>2</sup>	29.67	16	474.72		
F24	D 18	B		1.900 <sup>x1</sup>							1.90	30	57.00		
F25	D 18	C		1.750 <sup>x1</sup>	0.270 <sup>x1</sup>	0.270 <sup>x1</sup>					2.29	5	11.45		
F26	D 16	B		1.800 <sup>x1</sup>							1.80	6	10.80		
F27	D 16	B		1.000 <sup>x1</sup>							1.00	4	4.00		
F28	D 16	C		1.716 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					2.20	4	8.80		
F29	D 16	B		1.900 <sup>x1</sup>							1.90	189	359.10		
F30	D 16	B		2.273 <sup>x1</sup>							2.28	184	419.52		
F31	D 16	F		1.000 <sup>x1</sup>	1.331 <sup>x1</sup>						2.34	5	11.70		
F32	D 16	B		36.300 <sup>x1</sup>						1.000 <sup>x3</sup>	39.30	12	471.60		
F33	D 16	G		1.100 <sup>x1</sup>	0.500 <sup>1</sup>	1.300 <sup>x1</sup>					2.90	80	232.00		
F34	D 16	G		1.100 <sup>x1</sup>	0.500 <sup>x1</sup>	0.800 <sup>x1</sup>					2.40	99	237.60		
F35	D 16	B		2.273 <sup>x1</sup>							2.28	179	408.12		
F36	D 16	B		35.300 <sup>x1</sup>						1.000 <sup>3</sup>	38.30	11	421.30		
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

C-39

Bahr Yusef Reg. (Upstream) Rebar weight [3/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
F37	D 16	B	—	21.500 <sup>x1</sup>						1.000 <sup>x1</sup>	22.50	3	67.50		
F38	D 16	B	—	7.800 <sup>x1</sup>							7.80	12	93.60		
F39	D 12	H	□	0.430 <sup>x1</sup>	1.780 <sup>1</sup>	1.780 <sup>x1</sup>	0.100 <sup>2</sup>				4.19	1084	4541.96		
F40	D 12	H	□	0.430 <sup>x1</sup>	1.788 <sup>x1</sup> ~2.063	2.003 <sup>x1</sup> ~2.278	0.100 <sup>x2</sup>				4.70	120	564.00		
F41	D 12	II	□	0.430 <sup>x1</sup>	1.980 <sup>1</sup>	1.980 <sup>x1</sup>	0.100 <sup>2</sup>				4.59	88	403.92		
F42	D 12	H	□	0.430 <sup>x1</sup>	2.280 <sup>-1</sup>	2.280 <sup>x1</sup>	0.100 <sup>-2</sup>				5.19	412	2138.28		
F43	D 12	H	□	0.430 <sup>x1</sup>	1.880 <sup>-1</sup>	1.880 <sup>x1</sup>	0.100 <sup>-2</sup>				4.39	68	298.52		
F44	D 12	II	□	0.430 <sup>x1</sup>	1.480 <sup>1</sup>	1.480 <sup>x1</sup>	0.100 <sup>2</sup>				3.59	88	315.92		
F45	D 18	C	┌	0.800 <sup>x1</sup>	0.270 <sup>1</sup>	0.270 <sup>x1</sup>					1.34	3	4.02		
W 1	D 16	B		7.900 <sup>x1</sup>							7.90	56	442.40		
W 2	D 16	B		6.450 <sup>x1</sup>							6.45	56	361.20		
W 3	D 16	B		8.970 <sup>x1</sup>							8.97	55	493.35		
W 4	D 16	B		7.520 <sup>x1</sup>							7.52	55	413.60		
W 5	D 16	B		6.900 <sup>x1</sup>							6.90	29	200.10		
W 6	D 16	B		5.450 <sup>x1</sup>							5.45	29	158.05		
W 7	D 16	D	└	9.750 <sup>x1</sup>	0.240 <sup>-1</sup>						9.99	582	5814.18		
W 8	D 16	D	└	10.820 <sup>x1</sup>	0.240 <sup>-1</sup>						11.06	220	2433.20		
W 9	D 16	D	└	8.750 <sup>x1</sup>	0.240 <sup>1</sup>						8.99	506	4548.94		
Type	A	B	C	D	E	F	G	H	I	J					
											Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
										D32	6.31	340	534	1760	

C-40



Bahr Yusef Reg. (Upstream) Rebar weight [4/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W10	D 16	B		10.800 <sup>x1</sup>							10.80	10	108.00			
W11	D 16	B		1.970 <sup>x1</sup>							1.97	29	57.13			
W12	D 16	B		2.970 <sup>x1</sup>							2.97	243	721.71			
W13	D 16	D		9.733 <sup>x1</sup>	0.240 <sup>x1</sup>						9.98	10	99.80			
W14	D 16	C		1.800 <sup>x1</sup>	1.000 <sup>1</sup>	1.000 <sup>&gt;1</sup>					3.80	202	767.60			
W15	D 16	C		1.450 <sup>x1</sup>	1.000 <sup>-1</sup>	1.000 <sup>x1</sup>					3.45	20	69.00			
W16	D 16	C		0.800 <sup>x1</sup>	0.240 <sup>-1</sup>	0.240 <sup>x1</sup>					1.28	10	12.80			
W17	D 16	C		1.794 <sup>x1</sup> ~1.466	1.000 <sup>1</sup>	1.000 <sup>&gt;1</sup>					3.63	4	14.52			
W18	D 16	C		2.300 <sup>x1</sup>	1.000 <sup>1</sup>	1.000 <sup>&gt;1</sup>					4.30	279	1199.70			
W19	D 16	C		1.600 <sup>x1</sup>	1.000 <sup>-1</sup>	1.000 <sup>x1</sup>					3.60	30	108.00			
W20	D 16	C		0.600 <sup>x1</sup>	1.000 <sup>-1</sup>	1.000 <sup>x1</sup>					2.60	36	93.60			
W21	D 16	C		2.265 <sup>x1</sup> ~1.136	1.000 <sup>1</sup>	1.000 <sup>&gt;1</sup>					3.71	30	111.30			
W22	D 16	C		0.800 <sup>x1</sup>	1.000 <sup>-1</sup>	1.000 <sup>x1</sup>					2.80	106	296.80			
W23	D 16	B		27.700 <sup>x1</sup>						1.000 <sup>-2</sup>	29.70	72	2138.40			
W24	D 16	C		2.768 <sup>x1</sup>	1.768 <sup>1</sup>	1.234 <sup>&gt;1</sup>					5.77	42	242.34			
W25	D 16	B		3.000 <sup>x1</sup>							3.00	84	252.00			
W26	D 16	C		1.768 <sup>x1</sup>	1.234 <sup>-1</sup>	1.768 <sup>&gt;1</sup>					4.77	84	400.68			
W27	D 16	B		4.400 <sup>x1</sup>							4.40	84	369.60			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
											D22	2.98	240	377	1210	
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

C-41

Bahr Yusef Reg. (Upstream) Rebar weight [5/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
W28	D 16	C		15.268 <sup>x1</sup>	1.768 <sup>v1</sup>	1.768 <sup>v1</sup>				1.000 <sup>v1</sup>	19.81	72	1426.32		
W29	D 16	B		3.000 <sup>x1</sup>							3.00	72	216.00		
W30	D 16	C		2.768 <sup>x1</sup>	1.234 <sup>1</sup>	1.768 <sup>v1</sup>					5.77	72	415.44		
W31	D 16	B		10.950 <sup>x1</sup>							10.95	12	131.40		
W32	D 16	C		2.518 <sup>x1</sup>	1.768 <sup>1</sup>	1.768 <sup>v1</sup>					6.06	12	72.72		
W33	D 16	B		10.800 <sup>x1</sup>							10.80	56	604.80		
W34	D 16	C		1.768 <sup>x1</sup>	0.240 <sup>v1</sup>	0.240 <sup>v1</sup>					2.25	44	99.00		
W35	D 16	I		0.884 <sup>x1</sup>	1.388 <sup>1</sup>	1.884 <sup>v1</sup>	1.234 <sup>1</sup>		φ884		5.39	42	226.38		
W36	D 16	B		1.200 <sup>x1</sup>							1.20	6	7.20		
W37	D 16	B		1.900 <sup>x1</sup>							1.90	194	368.60		
W38	D 16	J		3.562 <sup>x1</sup>	1.150 <sup>v2</sup>				φ1.134		5.87	126	739.62		
W39	D 16	B		3.000 <sup>x1</sup>							3.00	252	756.00		
W40	D 16	B		1.800 <sup>x1</sup>							1.80	252	453.60		
W41	D 16	B		3.400 <sup>x1</sup>							3.40	252	856.80		
W42	D 16	B		15.300 <sup>x1</sup>						1.000 <sup>1</sup>	16.30	216	3520.80		
W43	D 16	B		3.000 <sup>x1</sup>							3.00	216	648.00		
W44	D 16	J		3.562 <sup>x1</sup>	1.150 <sup>v2</sup>				φ1.134		5.87	216	1267.92		
W45	D 16	B		2.550 <sup>x1</sup>							2.55	36	91.80		
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

C-42

Bahr Yusef Reg. (Upstream) Rebar weight [6/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
W46	D 16	B	—	10.800 <sup>x1</sup>							10.80	83	896.40		
W47	D 16	C	┌	2.268 <sup>x1</sup>	0.240 <sup>1</sup>	0.240 <sup>1</sup>					2.75	760	2090.00		
W48	D 16	C	┌	0.768 <sup>x1</sup>	0.240 <sup>1</sup>	0.240 <sup>1</sup>					1.25	44	55.00		
W49	D 16	B	┆	1.900 <sup>x1</sup>							1.90	12	22.80		
W50	D 16	B	—	5.800 <sup>x1</sup>							5.80	11	63.80		
W51	D 16	B	—	2.800 <sup>x1</sup>							2.80	1	2.80		
W52	D 16	B	—	1.800 <sup>x1</sup>							1.80	2	3.60		
W53	D 16	B	—	3.650 <sup>x1</sup>							3.65	12	43.80		
W54	D 16	B	—	6.650 <sup>x1</sup>							6.65	10	66.50		
W55	D 16	B	—	2.650 <sup>x1</sup>							2.65	2	5.30		
W56	D 16	B	—	2.800 <sup>x1</sup>							2.80	2	5.60		
W57	D 16	B	—	5.794 <sup>x1</sup> ~5.648							5.73	3	17.19		
W58	D 16	B	—	2.466 <sup>x1</sup>							2.47	1	2.47		
W59	D 16	B	—	11.550 <sup>x1</sup> ~11.532							11.55	9	103.95		
W60	D 16	B	—	5.228 <sup>x1</sup> ~5.131							5.18	12	62.16		
W61	D 16	B	—	1.976 <sup>x1</sup> ~1.718							1.85	12	22.20		
W62	D 16	B	—	1.800 <sup>x1</sup>							1.80	12	21.60		
W63	D 16	B	—	2.250 <sup>x1</sup>							2.25	8	18.00		
Type	A	B	C	D	E	F	G	H	I	J					
											Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
										D32	6.31	340	534	1760	

C-43

Bahr Yusef Reg. (Upstream) Rebar weight [7/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
W64	D 16	B	—	3.650 <sup>x1</sup>							3.65	12	43.80		
W65	D 16	B	—	13.650 <sup>x1</sup>						1.000 <sup>1</sup>	14.65	8	117.20		
W66	D 16	B	—	16.981 <sup>x1</sup>						1.000 <sup>1</sup>	17.99	4	71.96		
W67	D 16	B	—	6.150 <sup>x1</sup> ~5.961							6.06	17	103.02		
W68	D 16	B	—	1.976 <sup>x1</sup> ~1.718							1.85	12	22.20		
W69	D 16	B	—	2.650 <sup>x1</sup>							2.65	4	10.60		
W70	D 12	C	┌	1.829 <sup>x1</sup>	0.100 <sup>v1</sup>	0.100 <sup>x1</sup>					2.03	982	1993.46		
W71	D 12	C	┌	1.479 <sup>x1</sup>	0.100 <sup>1</sup>	0.100 <sup>v1</sup>					1.68	110	184.80		
W72	D 12	C	┌	0.979 <sup>x1</sup>	0.100 <sup>1</sup>	0.100 <sup>v1</sup>					1.18	120	141.60		
W73	D 12	C	┌	0.828 <sup>x1</sup>	0.100 <sup>v1</sup>	0.100 <sup>x1</sup>					1.03	53	54.59		
W74	D 12	C	┌	2.328 <sup>x1</sup>	0.100 <sup>v1</sup>	0.100 <sup>x1</sup>					2.53	1351	3418.03		
W75	D 12	C	┌	1.628 <sup>x1</sup>	0.100 <sup>1</sup>	0.100 <sup>v1</sup>					1.83	165	301.95		
W76	D 12	C	┌	0.628 <sup>x1</sup>	0.100 <sup>v1</sup>	0.100 <sup>x1</sup>					0.83	180	149.40		
W77	D 12	C	┌	0.828 <sup>x1</sup>	0.100 <sup>v1</sup>	0.100 <sup>x1</sup>					1.03	130	133.90		
P 1	D 22	B		2.460 <sup>x1</sup>							2.46	204	501.84		
P 2	D 22	B		2.610 <sup>x1</sup>							2.61	16	41.76		
P 3	D 22	B		3.680 <sup>x1</sup>							3.68	24	88.32		
P 4	D 22	B		3.660 <sup>x1</sup> ~3.393							3.53	12	42.36		
Type	A	B	C	D	E	F	G	H	I	J					
											Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
										D28	4.83	300	471	1540	
										D32	6.31	340	534	1760	

Bahr Yusef Reg. (Upstream) Rebar weight [8/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
P 5	D 22	B		4.680 <sup>x1</sup>							4.68	24	112.32		
P 6	D 22	B		4.660 <sup>x1</sup> ~2.126							3.40	80	272.00		
P 7	D 22	B		2.110 <sup>x1</sup>							2.11	12	25.32		
P 8	D 12	C		0.550 <sup>x1</sup>	0.180 <sup>x1</sup>	0.180 <sup>x1</sup>					0.91	186	169.26		
P 9	D 16	B		15.820 <sup>x1</sup> ~13.890						1.000 <sup>1</sup>	15.86	28	444.08		
P 10	D 16	B		2.650 <sup>x1</sup>							2.65	24	63.60		
P 11	D 16	B		5.775 <sup>x1</sup> ~4.470							5.13	24	123.12		
P 12	D 16	B		6.650 <sup>x1</sup>							6.65	20	133.00		
P 13	D 16	E		0.517 <sup>x1</sup>	1.620 <sup>1</sup>	1.050 <sup>x1</sup>					3.19	4	12.76		
P 14	D 16	E		5.325 <sup>x1</sup>	1.620 <sup>x1</sup>	1.050 <sup>x1</sup>					8.00	4	32.00		
P 15	D 16	C		0.512 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.00	30	30.00		
P 16	D 12	C		0.584 <sup>x1</sup>	0.100 <sup>1</sup>	0.100 <sup>x1</sup>					0.79	186	146.94		
S 1	D 16	B		8.000 <sup>x1</sup>							8.00	176	1408.00		
S 2	D 16	B		0.800 <sup>x1</sup>							0.80	960	768.00		
S 3	D 16	C		0.618 <sup>x1</sup>	0.268 <sup>1</sup>	0.268 <sup>x1</sup>					1.16	480	556.80		
C 1	D 16	K		0.950 <sup>x1</sup>	0.159 <sup>x1</sup>	1.344 <sup>x1</sup>	0.240 <sup>x1</sup>				2.70	102	275.40		
C 2	D 16	B		9.800 <sup>x1</sup>							9.80	8	78.40		
G 1	D 16	A		34.960 <sup>x1</sup>	5.730 <sup>1</sup>	5.730 <sup>x1</sup>	0.267 <sup>2</sup>		φ170	1.000 <sup>6</sup>	52.96	8	423.68		
Type	A	B	C	D	E	F	G	H	I	J					
											Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
										D32	6.31	340	534	1760	

C-45

Bahr Yusef Reg. (Upstream) Rebar weight [9/10]

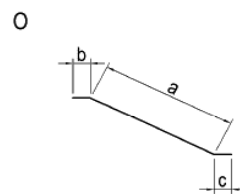
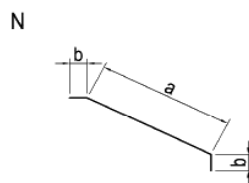
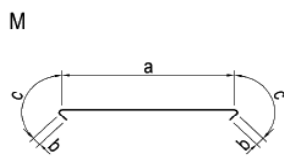
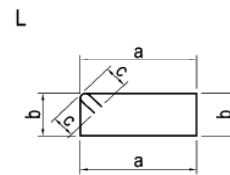
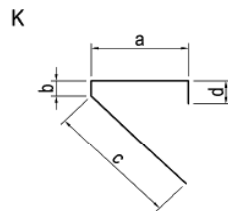
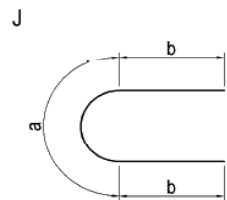
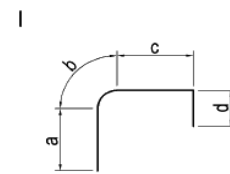
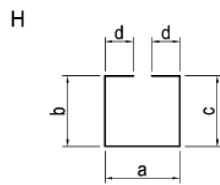
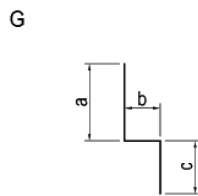
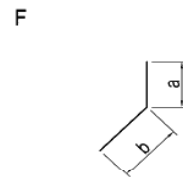
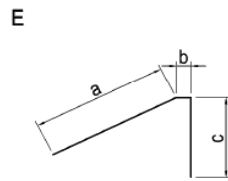
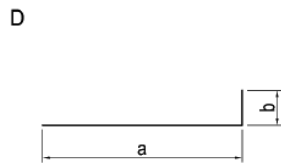
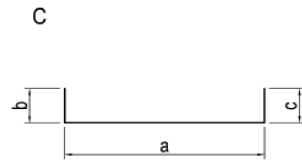
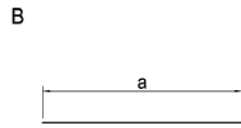
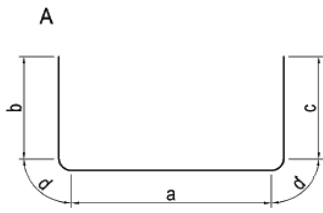
No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
G 2	D 16	I		5.730 <sup>x1</sup>	0.267 <sup>x1</sup>	2.030 <sup>x1</sup>	0.300 <sup>x1</sup>		φ170		8.33	4	33.32		
G 3	D 16	I		5.730 <sup>x1</sup>	0.267 <sup>x1</sup>	0.950 <sup>x1</sup>	0.300 <sup>x1</sup>		φ170		7.25	24	174.00		
G 4	D 16	B		5.900 <sup>x1</sup>							5.90	292	1722.80		
G 5	D 16	L		1.832 <sup>x2</sup>	0.832 <sup>x2</sup>	0.225 <sup>x2</sup>					5.78	92	531.76		
G 6	D 16	L		0.982 <sup>x2</sup>	4.232 <sup>x2</sup>	0.225 <sup>x2</sup>					10.88	46	500.48		
G 7	D 16	L		0.682 <sup>x2</sup>	0.832 <sup>x2</sup>	0.225 <sup>x2</sup>					3.48	276	960.48		
G 8	D 16	M		2.236 <sup>x1</sup>	0.160 <sup>x2</sup>	0.113 <sup>x2</sup>			φ48		2.79	18	50.22		
G 9	D 12	C		0.978 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>					1.18	72	84.96		
G10	D 12	C		0.678 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>					0.88	132	116.16		
T 1	D 16	C		35.300 <sup>x1</sup>	0.300 <sup>x1</sup>	0.300 <sup>x1</sup>				1.000 <sup>x3</sup>	38.90	10	389.00		
T 2	D 16	B		7.200 <sup>x1</sup>							7.20	8	57.60		
T 3	D 16	C		3.100 <sup>x1</sup>	0.300 <sup>x1</sup>	0.300 <sup>x1</sup>					3.70	6	22.20		
T 4	D 16	C		0.600 <sup>x1</sup>	0.300 <sup>x1</sup>	0.300 <sup>x1</sup>					1.20	36	43.20		
T 5	D 16	C		35.300 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>				1.000 <sup>x3</sup>	38.78	19	736.82		
T 6	D 16	C		0.950 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.43	24	34.32		
T 7	D 16	C		0.600 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.08	36	38.88		
T 8	D 16	C		6.232 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					6.90	24	165.60		
T 9	D 16	C		2.832 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					3.50	56	196.00		
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

C-46

Bahr Yusef Reg. (Upstream) Rebar weight [10/10]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
T10	D 16	C		0.832 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					1.50	56	84.00		
T11	D 16	C		2.682 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					3.35	104	348.40		
T12	D 16	D		0.832 <sup>x1</sup>	0.182 <sup>x1</sup>						1.02	208	212.16		
T13	D 16	C		0.682 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					1.35	104	140.40		
T14	D 16	C		6.232 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					6.72	24	161.28		
T15	D 16	C		2.832 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					3.32	56	185.92		
T16	D 16	C		0.832 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.32	56	73.92		
T17	D 16	C		2.832 <sup>x1</sup>	0.240 <sup>x1</sup>	0.182 <sup>x1</sup>					3.26	104	339.04		
T18	D 16	C		0.832 <sup>x1</sup>	0.182 <sup>x1</sup>	0.240 <sup>x1</sup>					1.26	104	131.04		
T19	D 16	N		2.804 <sup>x1</sup>	0.100 <sup>x2</sup>						3.01	34	102.34		
T20	D 16	O		1.860 <sup>x1</sup>	0.100 <sup>x1</sup>	0.240 <sup>x1</sup>					2.20	30	66.00		
T21	D 16	C		0.832 <sup>x1</sup>	0.832 <sup>x1</sup>	0.832 <sup>x1</sup>					2.50	24	60.00		
T22	D 16	B		2.300 <sup>x1</sup>							2.30	18	41.40		
T23	D 12	C		0.328 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>					0.53	377	199.81		
D12				15357.46×0.888		=	13637.42 kg	=	13.637 t						
D16				65209.11×1.58		=	103030.39 kg	=	103.030 t						
D18				13029.63×2.00		=	26059.26 kg	=	26.059 t						
D22				1083.92×2.98		=	3230.08 kg	=	3.230 t						
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

Schematic design of rebar  
(Upstream)







Schematic design of rebar  
(Approach cushion slab)



Bahr Yusef Weir body      Quantity calculation						
[Downstream]						
Description	Classification	Calculation		Quantity	Unit	Remarks
Main body Leveling con.	σck=20N/mm2					
		$7.60 \times (21.50 + 21.60) / 2 \times 0.10 \times 2$	—	32.76		L type wall part
		$0.50 \times 1.50 \times 0.10 \times 2$	—	0.15		"
		$1/2 \times 0.50 \times 0.50 \times 0.10 \times 2$	—	0.03		"
		$-\pi/4 \times 1.00^2 \times 0.10 \times 16 \times 2$	—	-2.51		(-) Piles part
		$19.50 \times (21.00 + 21.10) / 2 \times 0.10$	—	41.05		Apron part
		Total	—	71.48	71.48	m <sup>3</sup>
Fomwork	Leveling con.					
		$(21.50 + 21.60) / 2 \times 0.10 \times 2 \times 2$	=	8.62		L type wall part
		$-0.50 \times 0.10 \times 2$	=	-0.10		"
		$0.50 \times \sqrt{2} \times 0.10 \times 2$	=	0.14		"
		$(8.10 + 10.25) / 2 \times 0.10 \times 2$	=	1.84		Apron part
		Total	=	10.50	10.50	m <sup>2</sup>
Reinforced con.	σck=25N/mm2					
	Cut-off	$1/2 \times (1.00 + 1.50) \times 0.50 \times 8.50 \times 2$	—	10.63		L type wall part
	"	$1/2 \times (1.00 + 2.00) \times 1.00 \times 19.50$	=	29.25		Apron part
	Invert	$8.00 \times 1.50 \times 2.00 \times 2$	=	48.00		L type wall part
	"	$1/2 \times (8.00 + 7.50) \times 1.50 \times 0.50 \times 2$	=	11.63		"
	"	$7.50 \times 1.50 \times 19.50 \times 2$	—	438.75		"
	"	$8.50 \times 1.50 \times 1.50 \times 2$	—	38.25		"
	"	$-\pi/4 \times 1.00^2 \times 0.10 \times 16 \times 2$	—	-2.51		(-) Piles part
	"	$1/2 \times (2.00 + 1.00) \times 21.50 \times 19.50$	=	628.88		Apron part
	"	$1.00 \times 2.00 \times 19.50$	=	39.00		"
	Wall A	$2.00 \times 7.00 \times 2.00$	=	28.00		
	"	$1/2 \times (2.00 + 1.00) \times 7.00 \times 1.00$	=	10.50		
	"	$1.00 \times 7.00 \times (20.50 + 1.50)$	=	154.00		

Description	Classification	Calculation	Quantity	Unit	Remarks
	Wall A	$1/2 \times 0.50 \times 7.00 \times (19.50 + 20.00)/2$	= 34.56		
	"	$-(1 - \pi/4) \times 1.00^2 \times 7.00$	= -1.50		
	Wall C	$2.00 \times 7.00 \times 2.00$	= 28.00		
	"	$1/2 \times (2.00 + 1.00) \times 7.00 \times 1.00$	= 10.50		
	"	$1.00 \times 7.00 \times (20.50 + 1.50)$	= 154.00		
	"	$1/2 \times 0.50 \times 7.00 \times (19.50 + 20.00)/2$	= 34.56		
	"	$-(1 - \pi/4) \times 1.00^2 \times 7.00$	= -1.50		
	Earth pressure wall	$1.00 \times 0.75 \times 1.00$	= 0.75		
		Total =	1693.75	1693.75	m <sup>3</sup>
Fomwork	RC structure				
	Cut-off	$1/2 \times (1.00 + 1.50) \times 0.50 \times 2 \times 2$	= 2.50		L type wall part
	"	$0.50 \times 8.50 \times 2$	= 8.50		"
	"	$1.00 \times 19.50$	= 19.50		Apron part
	Invert	$1.50 \times (23.50 - 0.50 + 1.00) \times 2$	= 72.00		L type wall part
	"	$1.50 \times 0.50 \times \sqrt{2} \times 2$	= 2.12		"
	"	$1.50 \times (23.50 + 8.50) \times 2$	= 96.00		"
	"	$1/2 \times 8.60 \times 0.40 \times 2$	= 3.44		Apron part
	"	$1.00 \times 19.50$	= 19.50		"
	Wall A	$7.00 \times (22.50 + 2.00)$	= 171.50		L type wall part
	"	$7.00 \times (0.50 + 1.00)/2 \times \sqrt{2}$	= 7.42		"
	"	$7.018 \times (19.50 + 20.00)/2$	= 138.61		"
	"	$7.00 \times \{1.50 - 1/2 \times (1.00 - 1.50)\}$	= 19.25		"
	Wall C	$7.00 \times (22.50 + 2.00)$	= 171.50		L type wall part
	"	$7.00 \times (0.50 + 1.00)/2 \times \sqrt{2}$	= 7.42		"
	"	$7.018 \times (19.50 + 20.00)/2$	= 138.61		"
	"	$7.00 \times \{1.50 - 1/2 \times (1.00 - 1.50)\}$	= 19.25		"
	Earth pressure wall	$(1.00 \times 2 + 0.75) \times 1.00$	= 2.75		
		Total =	899.87	899.87	m <sup>2</sup>

Description	Classification	Calculation	Quantity	Unit	Remarks
Circular formwork	RC structure				
	φ1000-1/4Circle	$\pi/4 \times 1.00 \times 7.00 \times 2 = 11.00$	11.00	m <sup>2</sup>	
Rebar		refer to rebar weight calculation sheet			
		Ltype wall part      Apron part			
		D12 $1.794 \times 2 + 3.878 = 7.466$	7.466	t	
		D16 $11.185 \times 2 + 18.239 = 40.609$	40.609	t	
		D22 $6.649 \times 2 = 13.298$	13.298	t	
Concrete curing	PC structure	Leveling con.      = 71.48	71.48	m <sup>3</sup>	
	RC structure	Reinforced con.      = 1693.75	1693.75	m <sup>3</sup>	
Scaffolding	Prefabricated scaffold, H≤30m				
	Outer perimeter	$8.50 \times (2.46 + 0.71 + 17.94 + 1.70 - 3.70 - 4.30) \times 2$			
			523.77		
	Inside	$7.00 \times 23.50 \times 2$	329.00		
		Total =	852.77	m <sup>2</sup>	
Expansion joint	Elastic filler; t=20mm				
		$2.00 \times (19.50 + 7.00 \times 2) + (1.50 \times 8.00 - 0.50 \times 1.00) \times 2$			
			92.00		
		$1/2 \times (2.00 + 1.00) \times 21.50 \times 2$	64.50		
		$2.00 \times 1.00 \times 2$	4.00		
		$1/2 \times (1.00 + 2.00) \times 1.00 \times 2$	3.00		
		$-1/2 \times 8.60 \times 0.40 \times 2$	-3.44		
		Total =	160.06	m <sup>2</sup>	
Water stop	B=300mm	$23.50 \times 2$	47.00	m <sup>2</sup>	
Slip bar	D20×1.00m, VPφ25×0.50m	$78 \times 2$	156.0	nos	

Bahr Yusef Reg. (Downstream-L type wall) Rebar weight [1/4]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks				
				a	b	c	d	e	φ									
F1	D 22	B		7.800 <sup>x1</sup>	1.250 <sup>x2</sup>						10.30	10	103.00					
F2	D 22	B		7.459 <sup>x1</sup> ~7.659	1.250 <sup>x2</sup>						10.06	2	20.12					
F3	D 22	B		7.300 <sup>x1</sup>	1.250 <sup>x2</sup>						9.80	98	960.40					
F4	D 22	B		8.300 <sup>x1</sup>	1.250 <sup>x2</sup>						10.80	8	86.40					
F5	D 16	B		7.800 <sup>x1</sup>	0.240 <sup>x2</sup>						8.28	10	82.80					
F6	D 16	B		7.459 <sup>x1</sup> ~7.659	0.240 <sup>x2</sup>						8.04	2	16.08					
F7	D 16	B		7.300 <sup>x1</sup>	0.240 <sup>x2</sup>						7.78	98	762.44					
F8	D 16	B		8.300 <sup>x1</sup>	0.240 <sup>x2</sup>						8.78	8	70.24					
F9	D 16	A		23.300 <sup>x1</sup>						1.000 <sup>x2</sup>	25.30	70	1771.00					
F10	D 16	D		1.851 <sup>x1</sup>	2.519 <sup>x1</sup>						4.37	2	8.74					
F11	D 16	A		2.009 <sup>x1</sup> ~2.159							2.09	2	4.18					
F12	D 16	B		1.300 <sup>x1</sup>	1.250 <sup>x2</sup>						3.80	5	19.00					
F13	D 16	B		1.300 <sup>x1</sup>	0.240 <sup>x2</sup>						1.78	5	8.90					
F14	D 16	B		7.765 <sup>x1</sup>	0.240 <sup>x2</sup>						8.25	6	49.50					
F15	D 16	C		8.284 <sup>x1</sup>	0.240 <sup>x1</sup>						8.53	6	51.18					
F16	D 16	A		8.300 <sup>x1</sup>							8.30	6	49.80					
F17	D 16	A		23.284 <sup>x1</sup>						1.000 <sup>x2</sup>	25.29	12	303.48					
F18	D 16	D		1.851 <sup>x1</sup>	2.519 <sup>x1</sup>						4.37	6	26.22					
Type	A	B	C	D	E	F	G	H						Dia.	kg m	R	L	J
														D12	0.888	130	204	1000
										D14	1.21	150	236	1000				
										D16	1.58	170	267	1000				
										D18	2.00	190	298	1000				
										D20	2.47	210	330	1100				
										D22	2.98	240	377	1210				
										D25	3.85	270	424	1375				
										D28	4.83	300	471	1540				
									D32	6.31	340	534	1760					

C-54

Bahr Yusef Reg. (Downstream-L type wall) Rebar weight [2/4]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks
				a	b	c	d	e	φ					
F19	D 16	B		1.218 <sup>x1</sup>	0.240 <sup>x2</sup>						1.70	29	49.30	
F20	D 16	A		1.800 <sup>x1</sup>							1.80	43	77.40	
F21	D 16	A		1.566 <sup>x1</sup>							1.57	35	54.95	
F22	D 16	E		1.000 <sup>x1</sup>	0.554 <sup>x1</sup>						1.56	8	12.48	
F23	D 16	A		8.300 <sup>x1</sup>							8.30	6	49.80	
F24	D 12	H		0.434 <sup>x1</sup>	1.281 <sup>x2</sup>	0.100 <sup>x2</sup>					3.20	420	1344.00	
W1	D 22	C		8.250 <sup>x1</sup>	0.330 <sup>x1</sup>						8.58	13	111.54	
W2	D 22	F		6.904 <sup>x1</sup>	1.349 <sup>x1</sup>	0.330 <sup>x1</sup>					8.59	1	8.59	
W3	D 22	F		6.914 <sup>x1</sup>	1.347 <sup>x1</sup>	0.330 <sup>x1</sup>					8.60	1	8.60	
W4	D 22	F		6.918 <sup>x1</sup>	1.350 <sup>x1</sup>	0.330 <sup>x1</sup>					8.60	98	842.80	
W5	D 22	C		8.250 <sup>x1</sup>	0.330 <sup>x1</sup>						8.58	8	68.64	
W6	D 16	C		8.250 <sup>x1</sup>	0.240 <sup>x1</sup>						8.49	135	1146.15	
W7	D 16	A		23.300 <sup>x1</sup>						1.000 <sup>x2</sup>	25.30	35	885.50	
W8	D 16	D		1.851 <sup>x1</sup>	2.519 <sup>x1</sup>						4.37	35	152.95	
W9	D 16	A		2.292 <sup>x1</sup>							2.30	35	80.50	
W10	D 16	A		22.400 <sup>x1</sup>						1.000 <sup>x2</sup>	24.40	35	854.00	
W11	D 16	G		1.400 <sup>x1</sup>	1.389 <sup>x1</sup>	1.000 <sup>x1</sup>			φ884		3.79	35	132.65	
W12	D 16	A		22.966 <sup>x1</sup> ~23.249						1.000 <sup>x2</sup>	25.11	3	75.33	
Type	A	B	C	D	E	F	G	H						
									Dia.	kg m	R	L	J	
									D12	0.888	130	204	1000	
									D14	1.21	150	236	1000	
									D16	1.58	170	267	1000	
									D18	2.00	190	298	1000	
									D20	2.47	210	330	1100	
									D22	2.98	240	377	1210	
									D25	3.85	270	424	1375	
								D28	4.83	300	471	1540		
								D32	6.31	340	534	1760		

C-55

Bahr Yusef Reg. (Downstream-L type wall) Rebar weight [3/4]









No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks
				a	b	c	d	e	φ					
W13	D 16	A	—	2.009 <sup>x1</sup> ~2.659							2.34	5	11.70	
W14	D 16	A	—	1.966 <sup>x1</sup> ~2.249							2.11	3	6.33	
W15	D 22	A	—	2.110 <sup>x1</sup>							2.11	10	21.10	
W16	D 16	A	—	0.800 <sup>x1</sup>							0.80	11	8.80	
W17	D 16	B	┌	1.800 <sup>x1</sup>	0.240 <sup>2</sup>						2.28	10	22.80	
W18	D 16	B	┌	0.859 <sup>x1</sup> ~1.659	0.240 <sup>-2</sup>						1.74	5	8.70	
W19	D 16	B	┌	0.807 <sup>x1</sup>	0.240 <sup>-2</sup>						1.29	98	126.42	
W20	D 16	B	┌	0.800 <sup>x1</sup>	0.240 <sup>2</sup>						1.28	8	10.24	
W21	D 16	B	┌	1.765 <sup>x1</sup>	0.240 <sup>2</sup>						2.25	34	76.50	
W22	D 16	B	┌	0.550 <sup>x1</sup>	0.240 <sup>-2</sup>						1.03	5	5.15	
W23	D 16	B	┌	0.512 <sup>x1</sup>	0.240 <sup>-2</sup>						1.00	8	8.00	
W24	D 12	B	┌	1.831 <sup>x1</sup>	0.100 <sup>2</sup>						2.04	43	87.72	
W25	D 12	B	┌	1.145 <sup>x1</sup> ~1.328	0.100 <sup>-2</sup>						1.44	17	24.48	
W26	D 12	B	┌	0.867 <sup>x1</sup> ~1.324	0.100 <sup>-2</sup>						1.30	416	540.80	
W27	D 12	B	┌	0.831 <sup>x1</sup>	0.100 <sup>2</sup>						1.04	21	21.84	
W28	D 12	B	┌	0.584 <sup>x1</sup>	0.100 <sup>-2</sup>						0.79	2	1.58	

Type	A	B	C	D	E	F	G	H	Dia.	kg m	R	L	J
	—	┌	┌	┌	┌	┌	┌	┌	D12	0.888	130	204	1000
									D14	1.21	150	236	1000
									D16	1.58	170	267	1000
									D18	2.00	190	298	1000
									D20	2.47	210	330	1100
									D22	2.98	240	377	1210
									D25	3.85	270	424	1375
									D28	4.83	300	471	1540
								D32	6.31	340	534	1760	



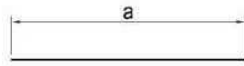
Bahr Yusef Reg. (Downstream-L type wall) Rebar weight [4/4]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks	
				a	b	c	d	c	φ						
	D12			2020.42×0.888		=	1794.13 kg	=	1.794 t						
	D16			7079.21×1.58		=	11185.15 kg	=	11.185 t						
	D22			2231.19×2.98		=	6648.95 kg	=	6.649 t						
Type	A	B	C	D	E	F	G	H							
															
									Dia.	kg m	R	L	J		
									D12	0.888	130	204	1000		
									D14	1.21	150	236	1000		
									D16	1.58	170	267	1000		
									D18	2.00	190	298	1000		
									D20	2.47	210	330	1100		
									D22	2.98	240	377	1210		
									D25	3.85	270	424	1375		
								D28	4.83	300	471	1540			
								D32	6.31	340	534	1760			

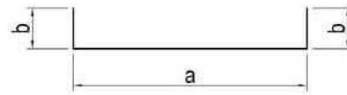
C-57

Schematic design of rebar  
(Downstream-L type wall)

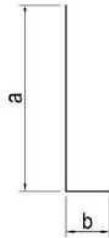
A



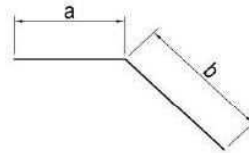
B



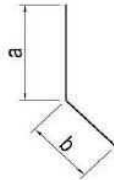
C



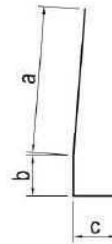
D



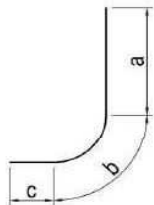
E



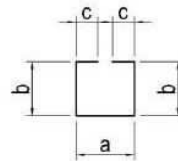
F



G



H

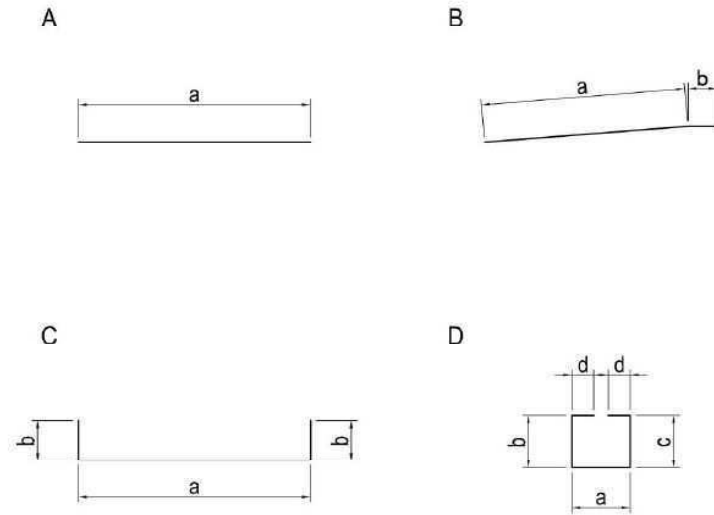


Bahr Yusef Reg. (Downstream-Apron) Rebar weight [1/1]

No.	Dia.	Type	Shape	L						Joint	length nos	nos	total length	Remarks			
				a	b	c	d	e	φ								
A1	D 16	A	—	19.300 <sup>x1</sup>						1.000 <sup>x1</sup>	20.30	236	4790.80				
A2	D 16	B	—	21.420 <sup>x1</sup>	1.903 <sup>1</sup>					1.000 <sup>2</sup>	25.33	50	1266.50				
A3	D 16	B	—	21.420 <sup>x1</sup>	1.903 <sup>1</sup>					1.000 <sup>2</sup>	25.33	49	1241.17				
A4	D 16	A	—	23.300 <sup>x1</sup>						1.000 <sup>x2</sup>	25.30	99	2504.70				
A5	D 16	A	—	23.300 <sup>x1</sup>						1.000 <sup>2</sup>	25.30	6	151.80				
A6	D 16	A	—	12.798 <sup>x1</sup> ~21.398						1.000 <sup>x1</sup>	18.10	6	108.60				
A7	D 16	A	—	4.198 <sup>x1</sup> ~8.498							6.35	4	25.40				
A8	D 16	C	┌—┐	19.268 <sup>x1</sup>	0.240 <sup>2</sup>					1.000 <sup>1</sup>	20.75	11	228.25				
A9	D 16	C	┌—┐	1.763 <sup>x1</sup>	0.240 <sup>2</sup>						2.25	99	222.75				
A10	D 16	C	┌—┐	0.770 <sup>x1</sup> ~1.756	0.240 <sup>x2</sup>						1.75	214	374.50				
A11	D 16	C	┌—┐	0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	18	22.50				
A12	D 16	A	┆	1.800 <sup>x1</sup>							1.80	99	178.20				
A13	D 16	A	—	2.273 <sup>x1</sup>							2.28	99	225.72				
A14	D 16	A	—	19.300 <sup>x1</sup>						1.000 <sup>x1</sup>	20.30	10	203.00				
A15	D 12	D	□	0.428 <sup>x1</sup>	0.840 <sup>1</sup> ~1.807	0.828 <sup>x1</sup> ~1.788	0.100 <sup>2</sup>				3.26	1272	4146.72				
A16	D 12	D	□	0.428 <sup>x1</sup>	0.828 <sup>x1</sup>	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>				2.29	96	219.84				
D12				4366.56×0.888		=	3877.51 kg	=	3.878 t								
D16				11543.89×1.58		=	18239.35 kg	=	18.239 t								
Type	A	B	C	D									Dia.	kg m	R	L	J
	—	—	┌—┐	□									D12	0.888	130	204	1000
													D14	1.21	150	236	1000
													D16	1.58	170	267	1000
													D18	2.00	190	298	1000
													D20	2.47	210	330	1100
													D22	2.98	240	377	1210
													D25	3.85	270	424	1375
													D28	4.83	300	471	1540
												D32	6.31	340	534	1760	

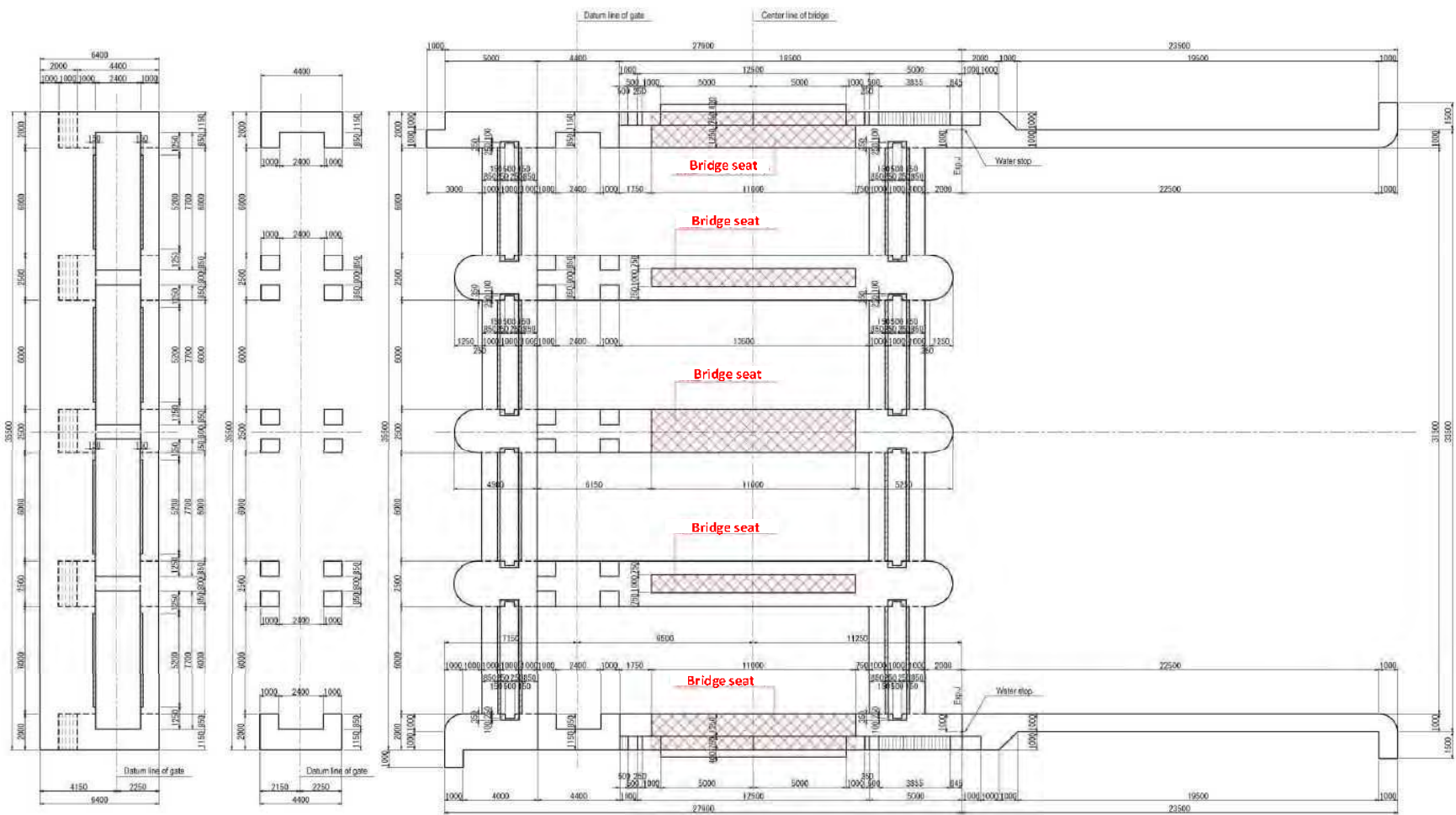
C-59

Schematic design of rebar  
(Downstream-Apron)

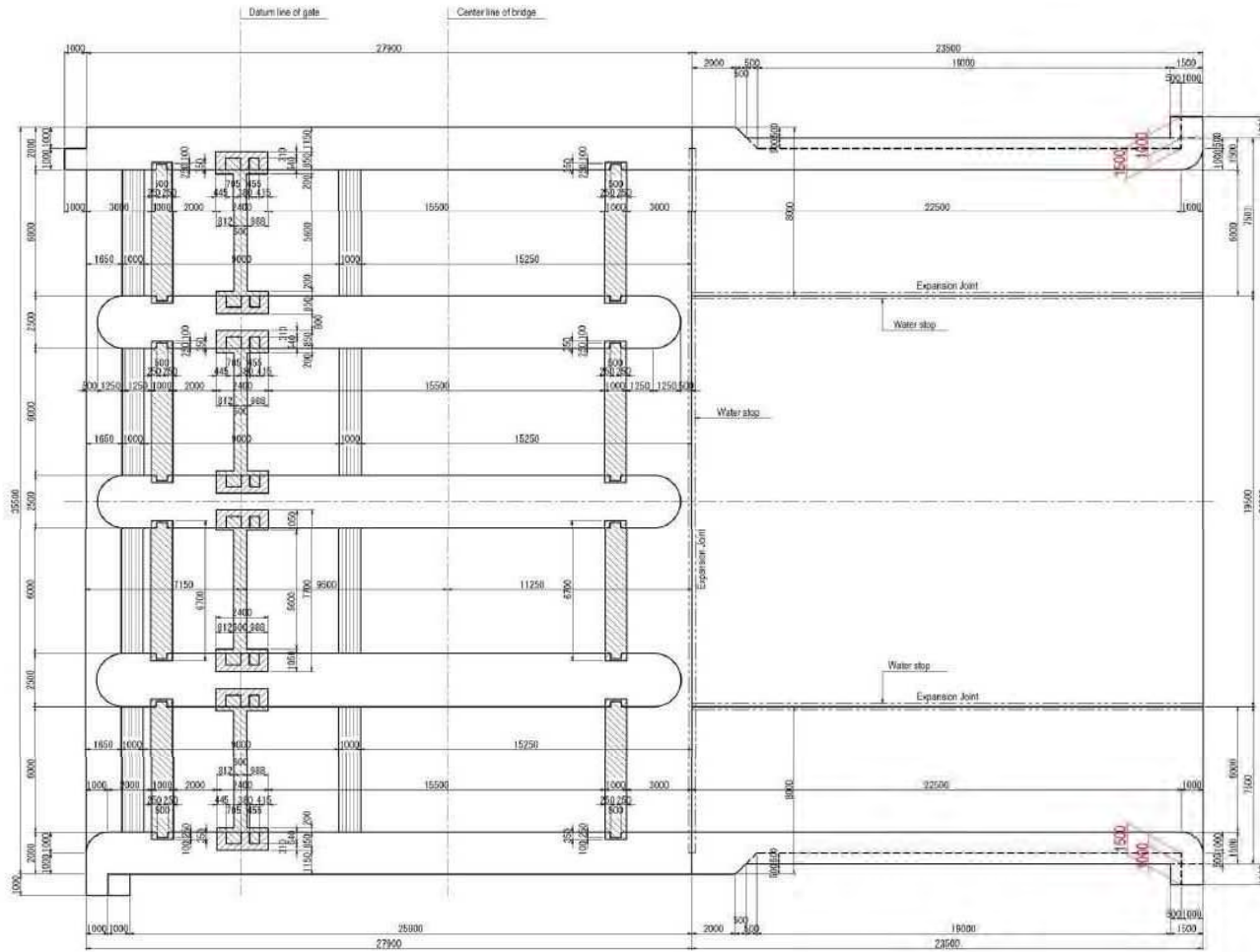


### Concrete / Formwork (1/5)

C-61

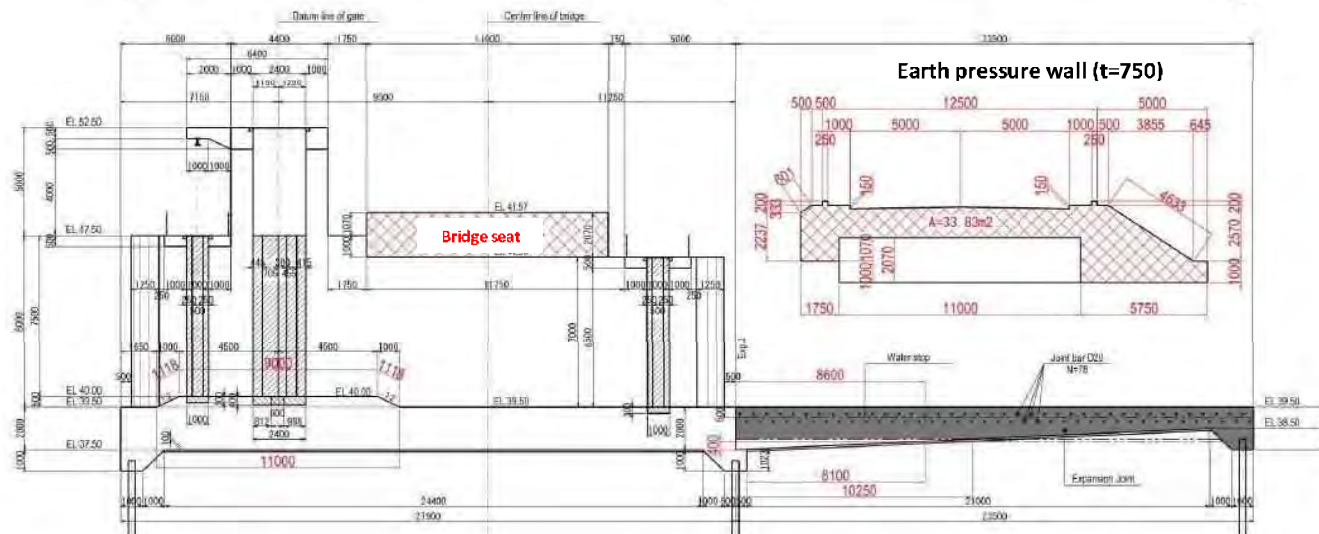
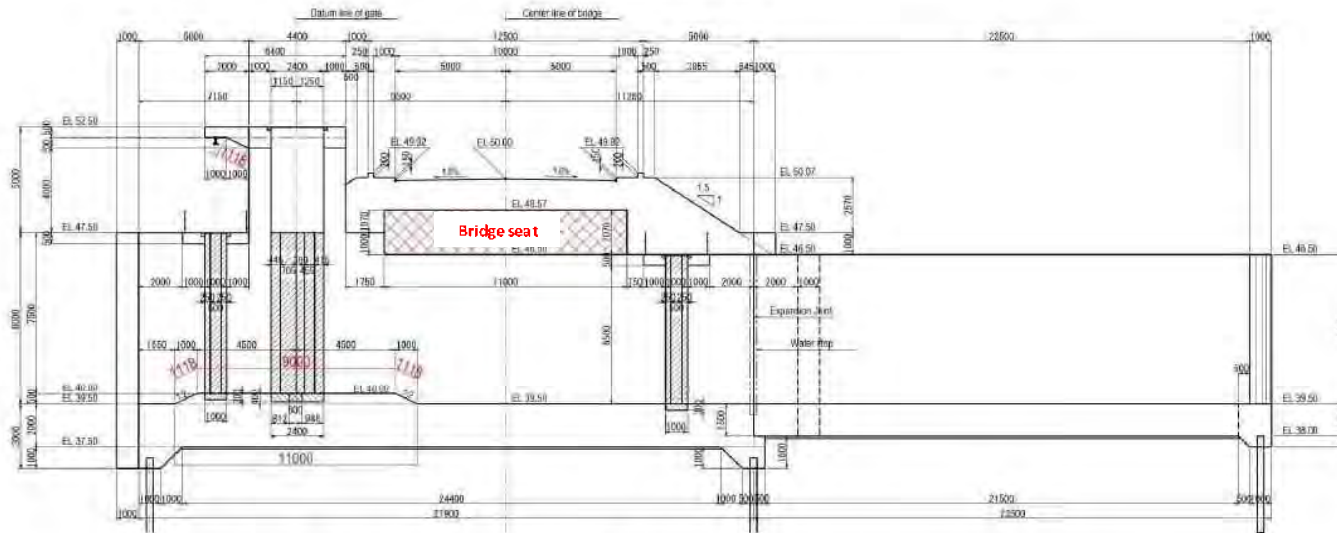


**Concrete / Formwork (2/5)**

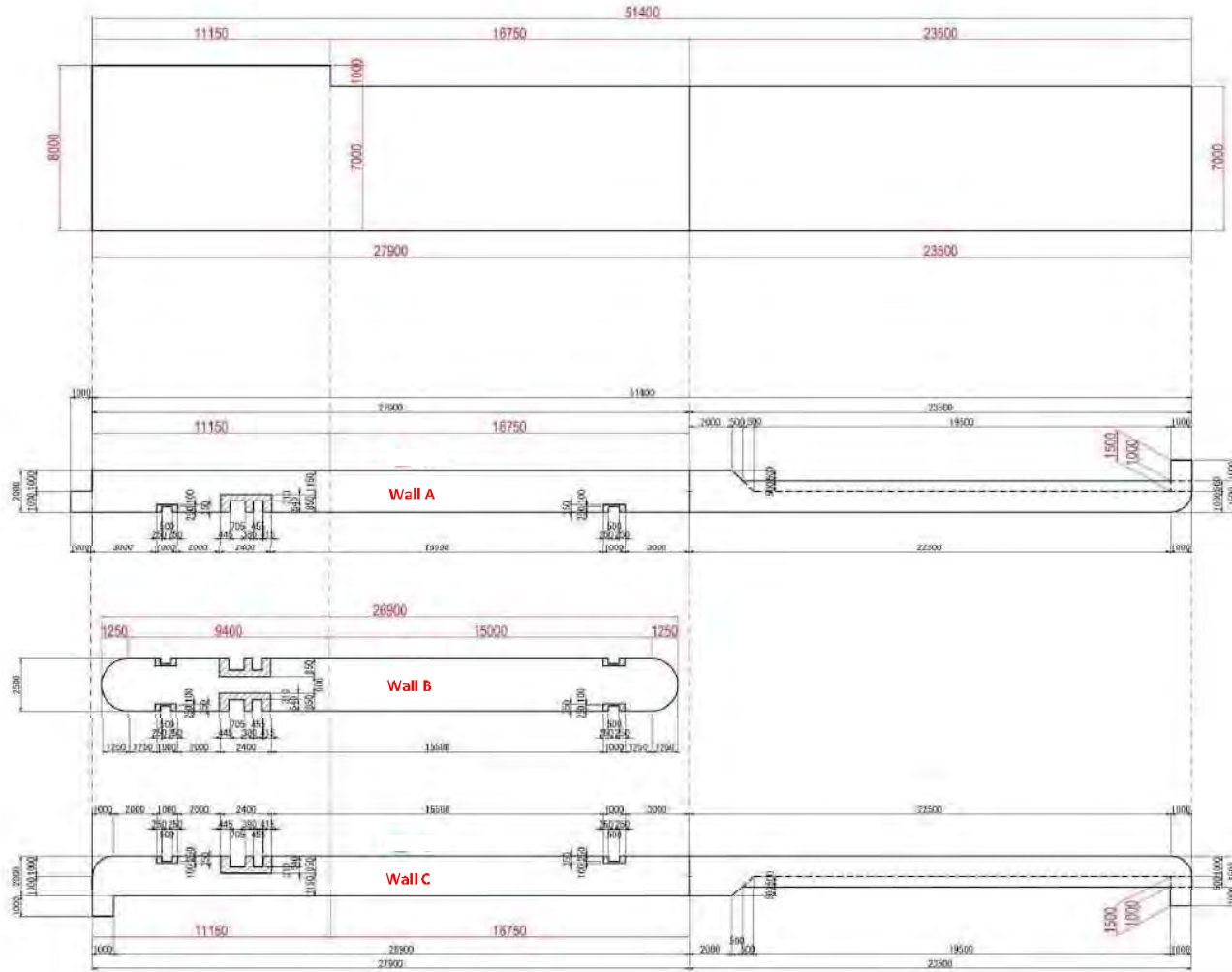


C-62

### Concrete / Formwork (3/5)

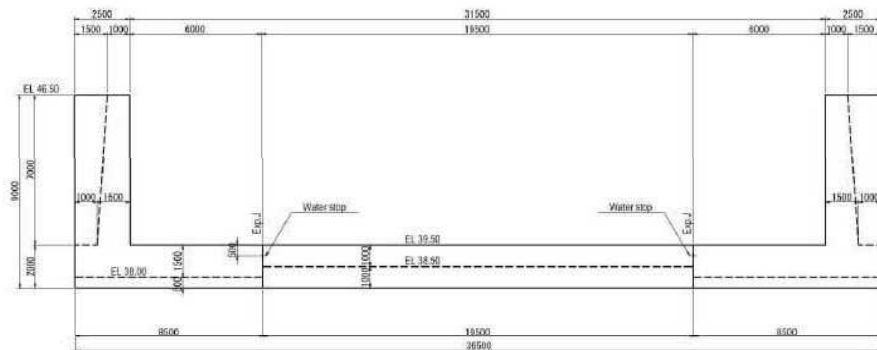
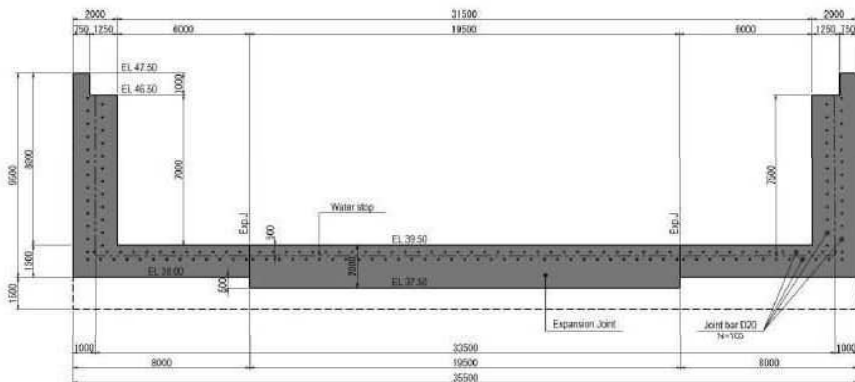
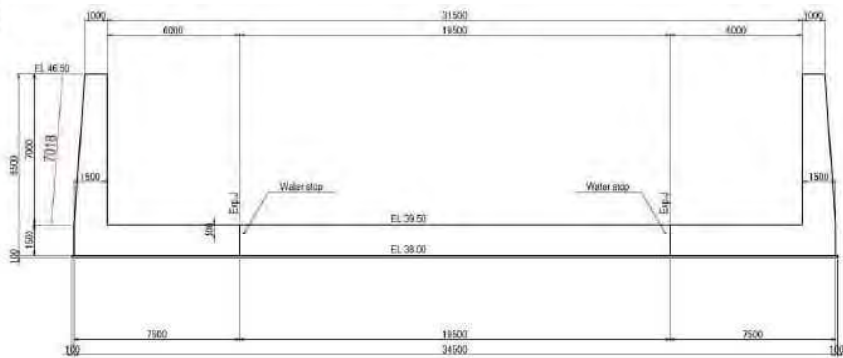
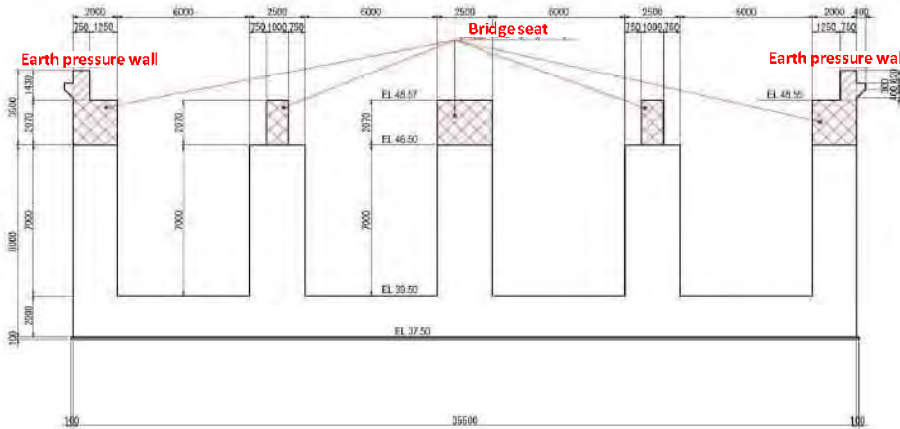


### Concrete / Formwork (4/5)



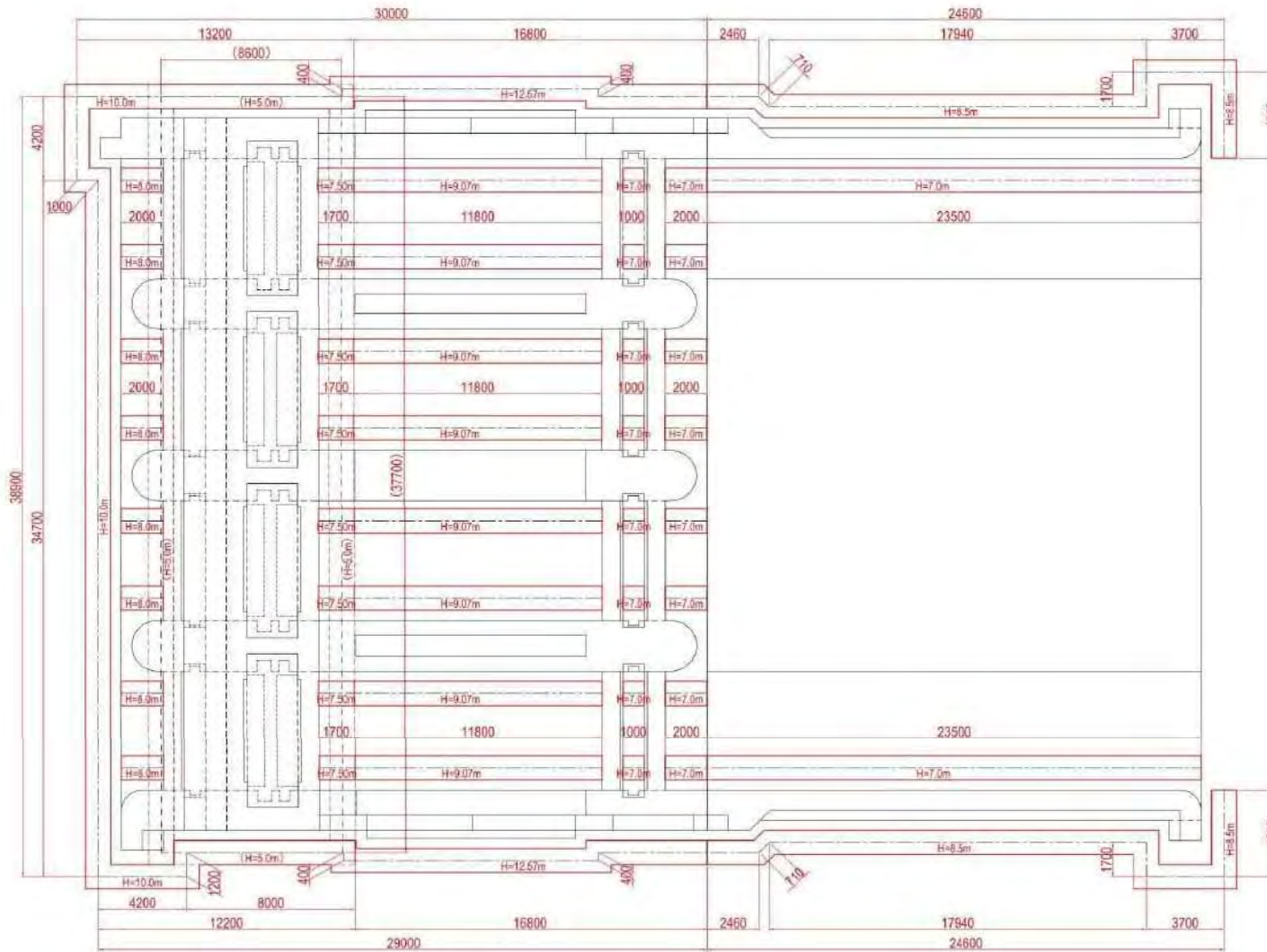


**Concrete / Formwork (5/5)**



C-65

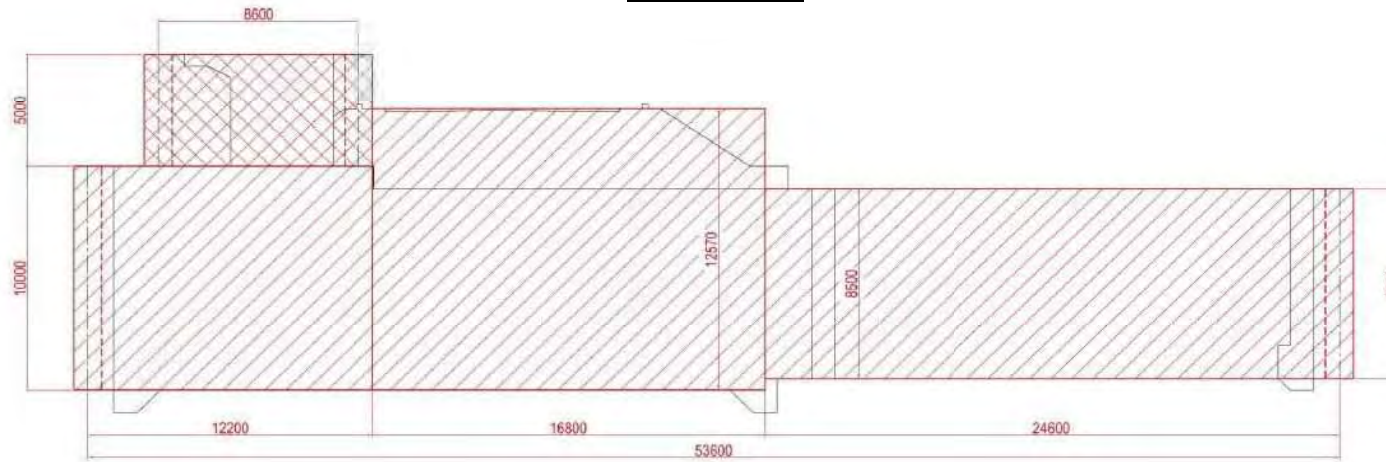
### Scaffolding (1/2)



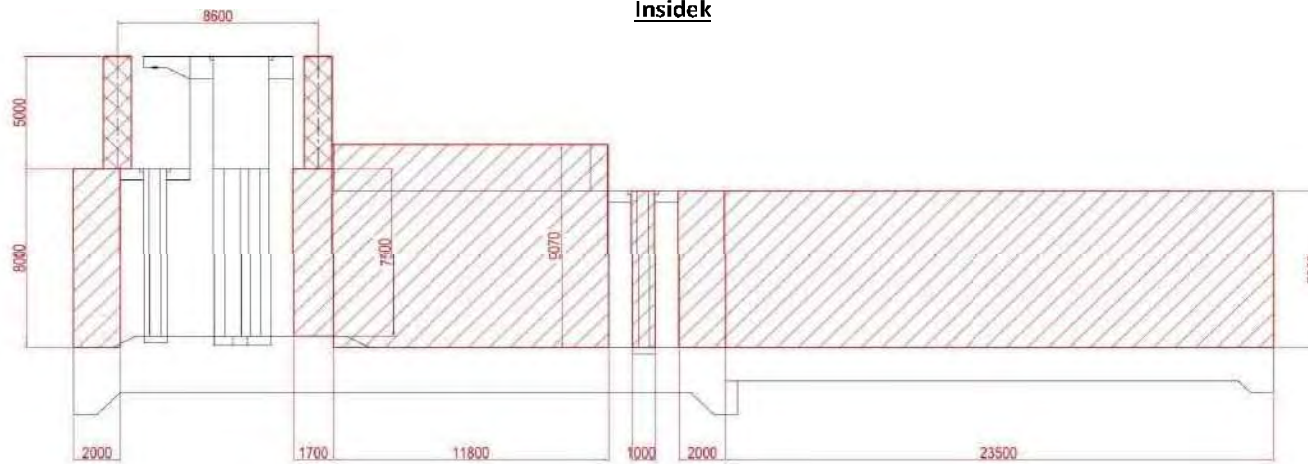
C-66

**Scaffolding (2/2)**

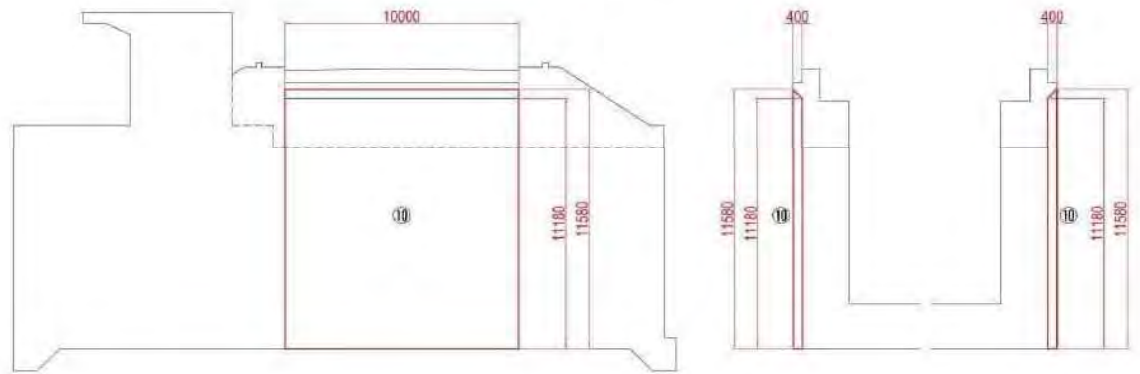
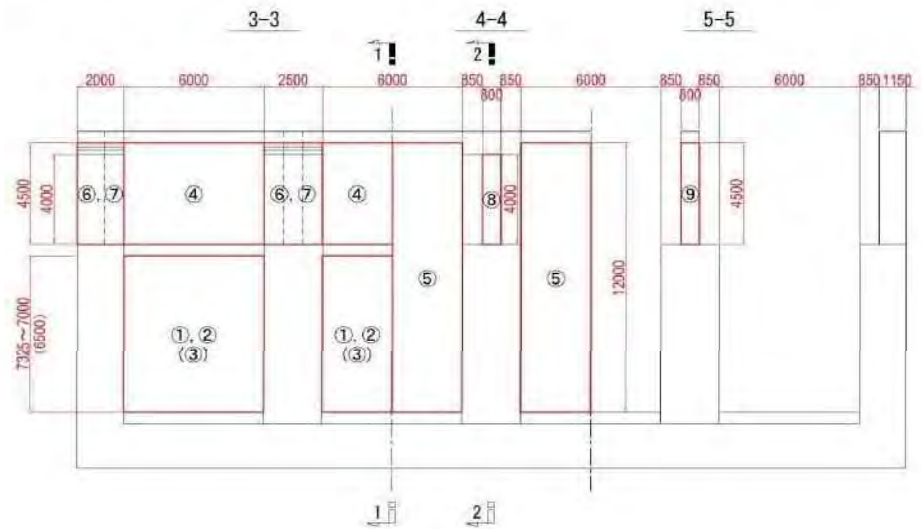
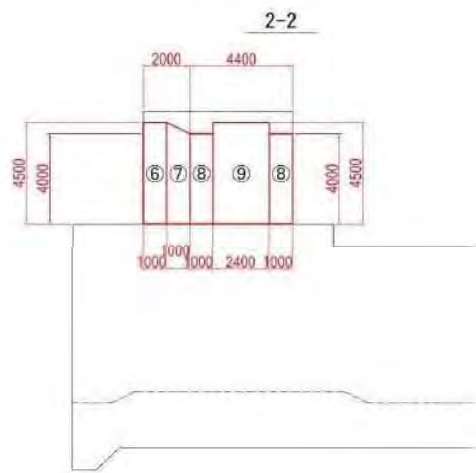
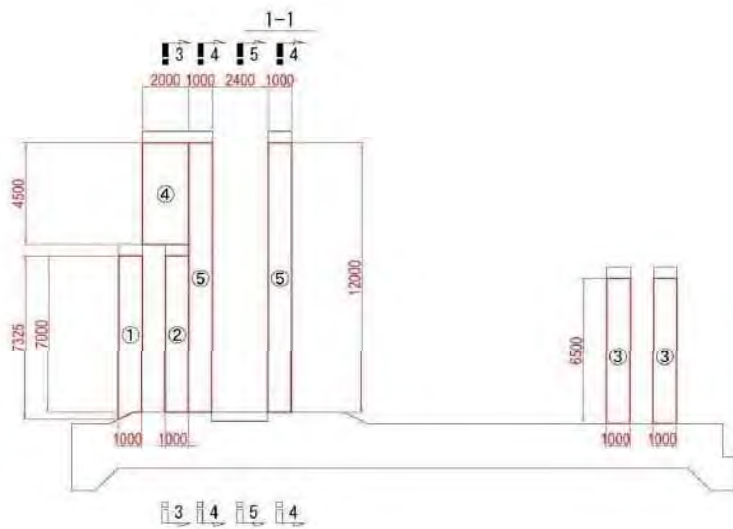
**Outer perimeter**



**Insidek**



### Falsework



## (2) Ibrahimia regulator

Ibrahimia Weir body Quantity Summary Sheet							
Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Up stream	Down stream
Weir body	Main body	leveling con.	$\sigma_{ck}=20\text{N/mm}^2$	$\text{m}^3$	154.55	83.07	71.48
		Formwork	Leveling con.	$\text{m}^2$	15.40	4.90	10.50
		Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$	$\text{m}^3$	6,460.87	4,786.70	1,674.17
		Formwork	RC structure	$\text{m}^2$	4,498.09	3,625.98	872.11
		Circular formwork	RC structure $\phi 2500-1/2\text{Circle}$	$\text{m}^2$	173.18	173.18	—
		"	RC structure $\phi 1000-1/4\text{Circle}$	$\text{m}^2$	16.80	6.28	10.52
		Second stage con.	$\sigma_{ck}=25\text{N/mm}^2$	$\text{m}^3$	142.65	142.65	—
		Formwork	RC structure	$\text{m}^2$	464.64	464.64	—
		Rebar (Steel 360/520)	D12	t	20.947	13.555	7.392
		"	D16	t	142.678	102.351	40.327
		"	D18	t	26.059	26.059	—
		"	D22	t	16.490	3.334	13.156
		Concrete curing	PC structure	$\text{m}^3$	154.55	83.07	71.48
		"	RC structure	$\text{m}^3$	6,603.52	4,929.35	1,674.17
		Scaffolding	Prefabricated scaffold $H \leq 30\text{m}$	$\text{m}^2$	3,625.54	2,805.36	820.18
		Falsework	Wedge linked falsework, $H < 30\text{m}$ Support bearing force $40\text{KN/m}^2$ and less	$\text{m}^3$	1,666.29	1,666.29	—
		Expansion joint	Elastic filler, $t=20\text{mm}$	$\text{m}^2$	159.16	—	159.16
		Water stop	$B=300\text{mm}$	m	94.90	47.90	47.00
		Slip bar	$D20 \times 1.00\text{m}$ , $VP \phi 25 \times 0.50\text{m}$	nos	319	163	156
	Other miscellaneous	Joint bar	$D16 \times 300$	Set	1,472	1,472	—
		"	$D16 \times 200$	Set	1,120	1,120	—
		Grating	$5200 \times 2400$	Set	4	4	—
		"	$6000 \times 1000$	Set	8	8	—
	Safety facilities work	Guard fence	$H=1.10\text{m}$	m	201.60	201.60	—



## Ibrahimia Weir body

## Quantity calculation

【Upstream】

Discription	Classification	Calculation	Quantity	Unit	Remarks
Main body Leveling con.	$\sigma$ ck-20\N/mm <sup>2</sup>				
		$35.70 \times (24.40 + 24.60) / 2 \times 0.10 = 87.47$			
		$-\pi / 4 \times 1.00^2 \times 0.10 \times 56 = -4.10$			(-)Piles part
		Total = 83.07	83.07	m <sup>3</sup>	
Formwork					
		$(24.40 + 24.60) / 2 \times 0.10 \times 2 = 4.90$			
		Total = 4.90	4.90	m <sup>2</sup>	
Formwork					
Reinforced con.	$\sigma$ ck-25\N/mm <sup>2</sup>				
	Cut-off	$1 \times 2 \times (1.00 + 2.00) \times 1.00 \times 36.50 = 51.75$			Upstream
	"	$1 \times 2 \times (1.00 + 2.00) \times 1.00 \times 8.00 \times 2 = 24.00$			Downstream
	"	$0.50 \times 0.50 \times 8.00 \times 2 = 4.00$			"
	"	$1/2 \times (0.50 + 1.50) \times 1.00 \times 19.50 = 19.50$			"
	"	$1/2 \times (1.00 + 1.023) \times 0.50 \times 19.50 = 9.86$			"
	Invert	$1.00 \times 3.00 \times 1.00 = 3.00$			Upstream-right
	"	$2.00 \times 2.00 \times 1.00 = 4.00$			Upstream-left
	"	$27.9 \times 2.00 \times 35.50 = 1980.90$			
	"	$1/2 \times (9.00 + 11.00) \times 0.50 \times (6.00 \times 4) = 120.00$			
	"	$-2.40 \times 1.05 \times 0.10 \times 8 = -8.06$			(-)Blockout part
	"	$-0.60 \times 5.60 \times 0.10 \times 1 = -5.38$			"
	"	$-1.00 \times 6.70 \times 0.30 \times 1 \times 2 = -16.08$			"
	"	$-\pi \times 4 \times 1.00^2 \times 0.10 \times 56 = -4.40$			(-)Piles part
	Wall C	$1.00 \times 8.00 \times 1.00 = 8.00$			
	"	$2.00 \times 8.00 \times 11.15 = 178.10$			
	"	$2.00 \times 6.70 \times 16.75 = 224.15$			
	"	$-(1.00 \times 0.35 + 2.40 \times 0.85) \times 7.50 = -17.93$			(-)Blockout part
	"	$-1.00 \times 0.35 \times 6.70 = -2.35$			"

Discription	Classification	Calculation	Quantity	Unit	Remarks
	Wall B	$2.50 \times 8.00 \times 10.65 \times 3$	— 639.00		
	"	$2.50 \times 6.70 \times 16.25 \times 3$	= 816.56		
	"	$-(1 - \pi/4) \times 1.25^2 \times 2 \times (8.00 + 6.70) \times 3$	= -29.57		
	"	$-(1.00 \times 0.35 + 2.40 \times 0.85) \times 7.50 \times 2 \times 3$	— -107.55		(-)Blockout part
	"	$-1.00 \times 0.35 \times 6.70 \times 2 \times 3$	= -14.07		"
	Wall A	$1.00 \times 8.00 \times 1.00$	= 8.00		
	"	$2.00 \times 8.00 \times 11.15$	= 178.10		
	"	$2.00 \times 6.70 \times 16.75$	= 224.45		
	"	$-(1 - \pi/4) \times 1.00^2 \times 8.00$	— -1.72		
	"	$-(1.00 \times 0.35 + 2.40 \times 0.85) \times 7.50$	— -17.93		(-)Blockout part
	"	$-1.00 \times 0.35 \times 6.70$	= -2.35		"
	Corridor	$1.00 \times 0.50 \times 6.00 \times 4 \times 4$	— 48.00		
	"	$0.15 \times 0.15 \times 6.00 \times 4 \times 4$	= 2.16		(-)Second stage con.
	Bridge seal	$11.00 \times 2.37 \times 2.00 \times 2$	= 104.28		Abut pier
	"	$11.00 \times 2.37 \times (2.50 + 1.00 \times 2)$	— 117.32		Middle pier
	Earth pressure wall	$0.75 \times 35.56 \times 2$	= 53.34		
	Pedestal	$1/2 \times (0.30 + 0.70) \times 0.40 \times 10.00 \times 2$	— 4.00		
	Gate pier	$(2.00 \times 1.40 - 2.10 \times 0.85) \times 4.50 \times 2$	— 60.84		
	"	$0.85 \times 1.00 \times 1.00 \times 4 \times 3$	= 10.80		
	"	$2.50 \times 1.00 \times 0.50 \times 2 \times 3$	= 7.50		
	Top slab	$6.40 \times 0.50 \times 35.50$	— 113.60		
	"	$1/2 \times (1.00 + 0.50) \times (2.00 \times 2 + 2.50 \times 3)$	= 2.88		
	"	$-2.40 \times 0.50 \times 7.70 \times 4$	= -36.96		(-)Opening
	"	$-0.15 \times 0.15 \times 5.20 \times 2 \times 1$	— -0.94		(-)Second stage con.
			Total = 4786.70	4786.70	m <sup>3</sup>
Formwork	RC structure				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 2 \times 2$	= 6.00		Side
	"	$1.00 \times 35.50$	= 35.50		Upstream end
	"	$1.50 \times 8.00 \times 2 + 1.00 \times 19.50$	— 13.50		Downstream end
	Invert	$1.00 \times 3.00 \times 3$	= 9.00		Upstream-right



Discription	Classification	Calculation	Quantity	Unit	Remarks
	Invert	$1.00 \times 2.00$	—	2.00	Upstream-left
	"	$35.50 \times 2.00$	=	71.00	Upstream end
	"	$27.90 \times 2.00 \times 2$	=	111.60	Side
	"	$1.50 \times 8.00 \times 2 + 2.00 \times 19.50$	—	63.00	Downstream end
	"	$0.50 \times 0.50 \times 2$	=	0.50	"
	"	$1.118 \times 6.00 \times 2 \times 4$	=	53.66	Raising part
	"	$(2.40 + 7.70) \times 2 \times 0.40 \times 4$	=	32.32	Blockout part
	"	$(0.812 - 0.988) \times 0.40 \times 2 \times 4$	=	5.76	"
	"	$(1.00 + 6.70) \times 2 \times 0.30 \times 4 \times 2$	—	36.96	"
	Wal. C	$1.00 \times 8.00 \times 3$	—	24.00	
	"	$8.00 \times (1.00 + 11.15 \times 2)$	=	186.40	
	"	$6.70 \times (2.00 + 16.75 \times 2)$	—	237.85	
	"	$7.50 \times (0.35 + 0.85) \times 2$	=	18.00	Blockout part
	"	$6.70 \times 0.35 \times 2$	=	4.69	"
	"	$-1/2 \times (9.00 - 11.00) \times 0.50$	—	-5.00	(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	=	-1.91	(-)Corridor
	Wall B	$8.00 \times 9.40 \times 2 \times 3$	—	451.20	
	"	$2.50 \times 1.35 \times 2$	—	6.75	
	"	$6.70 \times 15.00 \times 2 \times 3$	=	603.00	
	"	$7.50 \times (0.35 + 0.85) \times 4 \times 3$	=	108.00	Blockout part
	"	$6.70 \times 0.35 \times 4 \times 3$	—	28.14	"
	"	$-1/2 \times (9.00 - 11.00) \times 0.50 \times 2 \times 3$	=	-30.00	(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4 \times 2 \times 3$	=	-11.46	(-)Corridor
	Wall A	$8.00 \times (2.00 + 1.00 + 11.15 - 10.15)$	—	194.10	
	"	$6.70 \times (2.00 + 16.75 \times 2)$	=	237.85	
	"	$7.50 \times (0.35 + 0.85) \times 2$	—	18.00	Blockout part
	"	$6.70 \times 0.35 \times 2$	=	4.69	"
	"	$-1/2 \times (9.00 - 11.00) \times 0.50$	=	-5.00	(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	—	-1.91	(-)Corridor
	Corridor	$(1.00 + 0.50 \times 2) \times 6.00 \times 4 \times 4$	=	192.00	

Discription	Classification	Calculation	Quantity	Unit	Remarks
	Bridge seat	$(11.00 \times 2 - 1.25) \times 2.37 \times 2$	= 110.21		Abut pier
	"	$1.25 \times 1.07 \times 2$	= 2.68		"
	"	$(11.00 \times 2 - 2.50) \times 2.37$	= 58.07		Middle pier
	"	$2.50 \times 1.07$	= 2.68		"
	"	$(11.00 \times 2 - 1.00) \times 2.37$	= 54.51		"
	"	$1.00 \times 1.07 \times 2$	= 2.14		"
	Earth pressure wall	$35.56 \times 2 \times 2$	= 142.24		
	"	$(0.601 - 0.20 \times 4 + 0.15 \times 2 - 4.633 + 1.30) \times 0.75 \times 2$	= 11.45		
	"	$-0.70 \times 10.00 \times 2$	= -14.00		(-) Pedestal
	Gate pier	$(2.00 + 4.40 + 0.85) \times 2 \times 4.00 \times 2$	= 116.00		Abut pier
	"	$\{2.00 + (4.10 - 0.85) \times 2\} \times 0.50 \times 2$	= 12.50		"
	"	$-0.75 \times 2.237 \times 2$	= -3.36		(-) Earth pressure wall
	"	$(0.85 + 1.00) \times 2 \times 1.00 \times 4 \times 3$	= 177.60		Middle pier
	"	$(1.00 \times 4 + 2.50 \times 3) \times 0.50 \times 3$	= 17.25		"
	Top slab	$(6.40 + 35.50) \times 2 \times 0.50$	= 41.90		Side
	"	$(2.40 + 7.70) \times 2 \times 0.50 \times 4$	= 40.40		Opening
	"	$0.15 \times 0.15 \times 1 \times 4$	= 0.36		Second stage con.
	"	$1.00 \times 35.50$	= 35.50		Underside
	"	$(2.00 + 1.00) \times 6.00 \times 4$	= 72.00		"
	"	$1.2 \times 1.00 \times 0.50 \times 2 \times 5$	= 2.50		Launch
	"	$1.118 \times (2.00 > 2 + 2.50 < 3)$	= 12.86		"
		Total =	3625.98	3625.98	m <sup>2</sup>
Circular formwork	RC structure				
	φ 2500-1/2 Circle	$\pi/2 \times 2.50 \times (8.00 + 6.70) \times 3$	= 173.18	173.18	m <sup>2</sup>
	φ 1000-1/4 Circle	$\pi/4 \times 1.00 \times 8.00$	= 6.28	6.28	m <sup>2</sup>
Second stage con.	σ ck-25 N/mm <sup>2</sup>				
		$2.40 \times 1.05 \times 0.40 \times 8$	= 8.06		Invert part
		$0.60 \times 5.60 \times 0.40 \times 4$	= 5.38		"
		$1.00 \times 6.70 \times 0.30 \times 4 \times 2$	= 16.08		"

Discription	Classification	Calculation	Quantity	Unit	Remarks
		$2.40 \times 0.85 \times 7.50 \times 8$	- 122.10		Gate part
		$-0.51 \times (0.705 + 0.155) \times 7.50 \times 8$	= -37.58		"
		$1.00 \times 0.35 \times (7.50 + 6.70) \times 8$	= 39.76		Stop log part
		$-0.50 \times 0.25 \times (7.50 + 6.70) \times 8$	- -14.20		"
		$(0.15^2 - 0.05^2) \times 6.00 \times 4 \times 4$	= 1.92		Corridor part
		$(0.15^2 - 0.05^2) \times 5.20 \times 2 \times 4$	= 0.83		Top slab part
		Total =	112.65	142.65	m <sup>3</sup>
Formwork	RC structure				
		$(2.40 + 0.54 \times 4) \times 7.50 \times 8$	- 273.60		Gate part
		$(1.00 + 0.25 \times 2) \times (7.50 + 6.70) \times 8$	= 170.40		Stop log part
		$0.15 \times 6.00 \times 1 \times 4$	- 14.10		Corridor part
		$0.15 \times 5.20 \times 2 \times 4$	= 6.24		Top slab part
		Total =	464.64	164.64	m <sup>2</sup>
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12	- 13.555	13.555	t
		D16	- 102.351	102.351	t
		D18	= 26.059	26.059	t
		D22	= 3.334	3.334	t
Concrete curing	PC structure	Leveling con.	= 83.07	83.07	m <sup>3</sup>
	RC structure	4786.70 + 142.65	= 4929.35	4929.35	m <sup>3</sup>
Scaffolding	Prefabricated scaffold, H≤30m				
	Outer perimeter	$10.00 \times (38.90 + 1.00 - 13.20 + 12.20 + 1.20 + 0.40 \times 2)$	- 673.00		
	"	$12.57 \times 16.80 \times 2$	= 422.35		
	Inside	$8.00 \times 2.00 \times 8$	= 128.00		
	"	$7.50 \times 1.70 \times 8$	- 102.00		
	"	$9.07 \times 11.80 \times 8$	= 856.21		

Discription	Classification	Calculation	Quantity	Unit	Remarks
	"	$6.70 \times (1.00 + 2.00) \times 8$	—	160.80	
	Gate pier part	$5.00 \times (8.60 + 37.70) \times 2$	=	463.00	
			Total =	2805.36	2805.36 m <sup>2</sup>
Falsework	Wedge linked falsework, $H \leq 30m$ , Support bearing force $40KN/m^2$ and less				
	①	$1.00 \times (7.325 + 7.00) / 2 \times 6.00 \times 4$	=	171.90	Corr'dor part
	②	$1.00 \times 7.00 \times 6.00 \times 4$	=	168.00	"
	③	$1.00 \times 6.20 \times 6.00 \times 2 \times 4$	=	297.60	"
	④	$2.00 \times 4.50 \times 6.00 \times 4$	=	216.00	Gate pier part
	⑤	$1.00 \times 12.00 \times 6.00 \times 2 \times 4$	—	576.00	"
	⑥	$1.00 \times 4.50 \times (2.00 \times 2 + 2.50 \times 3)$	=	51.75	"
	⑦	$1.00 \times (1.50 + 4.00) / 2 \times (2.00 \times 2 - 2.50 \times 3)$	—	48.88	"
	⑧	$1.00 \times 1.00 \times 0.80 \times 3 \times 2$	—	19.20	"
	⑨	$2.40 \times 1.50 \times 0.80 \times 3$	=	25.92	"
	⑩	$1.2 \times (11.58 - 11.18) \times 0.40 \times 10.00 \times 2$	=	91.04	Pedestal part
			Total =	1666.29	1666.29 m <sup>3</sup>
Water stop	B-300mm	$33.50 + 7.20 \times 2$	=	47.90	47.90 m <sup>2</sup>
Slip bar	D20 $\times$ 1.00m, VP $\phi$ 25 $\times$ 0.50m		—	163.0	163 nos
Other miscellaneous					
Joint bar	D16 $\times$ 300	$368 \times 4$	—	1172.0	1172 Set Gate part
	D16 $\times$ 200	$(148 + 132) \times 1$	—	1120.0	1120 Set Stop log part
Grating	5200 $\times$ 2400		—	4.0	4 Set
	6000 $\times$ 1000		=	8.0	8 Set

Description	Classification	Calculation	Quantity	Unit	Remarks
Safety facilities work					
Guard fence	H=1.10m	$35.20 \times 2 + 6.10 - 5.10 = 81.60$			Top slab
		$7.50 \times 4 \times 2 \times 2 = 120.00$			Corridor
			201.60	m	
Approach cushion slab					
Reinforced con.	$\sigma_{ck} = 25 \text{ N/mm}^2$				
		$10.00 \times 5.00 \times 0.40 \times 2 = 40.00$	40.00	m <sup>3</sup>	
Formwork	RC structure				
		$(10.00 - 5.00) \times 0.40 \times 2 \times 2 = 24.00$	24.00	m <sup>2</sup>	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 $0.075 \times 2 = 0.150$	0.150	t	
		D16 $1.185 \times 2 = 2.370$	2.370	t	
		D18 $0.520 \times 2 = 1.040$	1.040	t	
Concrete curing	RC structure	reinforced con.	40.00	m <sup>3</sup>	
Anchor cap	SGP $\phi 40 \times 180$				
		$25 \times 2 = 50.0$	50	nos	
Anchor bar	D22 $\times 500$ (including D10 spiral reinforcement)				
		$25 \times 2 = 50.0$	50	nos	
Rubber shoe	t=30mm				
		$10.00 \times 0.30 \times 2 = 6.00$	6.00	m <sup>2</sup>	
Expansion joint	Elastic filler, t=20mm				
		$0.43 \times 10.00 \times 2 = 8.60$	8.60	m <sup>2</sup>	





Ibrahimia Reg. (Upstream) Rebar weight [1/10]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F1	D 18	A		34.920 <sup>x1</sup>	2,660 <sup>x1</sup>	4,110 <sup>x1</sup>	0,298 <sup>x2</sup>		φ 190	1,000 <sup>x4</sup>	46.29	140	6480.60			
F2	D 18	B		35.300 <sup>x1</sup>						1,000 <sup>x3</sup>	38.30	141	5400.30			
F3	D 18	B		3.100 <sup>x1</sup>							3.10	14	43.40			
F4	D 18	C		5.400 <sup>x1</sup>	1,300 <sup>x1</sup>	1,300 <sup>x1</sup>					8.00	36	288.00			
F5	D 18	B		5.900 <sup>x1</sup>							5.90	21	123.90			
F6	D 16	B		27.700 <sup>x1</sup>						1,000 <sup>x3</sup>	29.70	179	5316.30			
F7	D 16	D		23.683 <sup>x1</sup>	1.183 <sup>x1</sup>					1,000 <sup>x5</sup>	26.87	8	214.96			
F8	D 16	B		2.550 <sup>x1</sup>							2.55	157	400.35			
F9	D 16	E		1.856 <sup>x1</sup>	0.205 <sup>x1</sup>	1.183 <sup>x1</sup>					3.25	120	390.00			
F10	D 16	B		3.000 <sup>x1</sup>							3.00	152	456.00			
F11	D 16	C		1.766 <sup>x1</sup>	1.183 <sup>x1</sup>	1.283 <sup>x1</sup>					4.21	8	33.92			
F12	D 16	C		2.578 <sup>x1</sup>	1.183 <sup>x1</sup>	1.283 <sup>x1</sup>					5.05	112	565.60			
F13	D 16	B		4.400 <sup>x1</sup>							4.40	64	281.60			
F14	D 16	B		2.600 <sup>x1</sup>							2.60	112	291.20			
F15	D 16	E		1.856 <sup>x1</sup>	3.105 <sup>x1</sup>	1.283 <sup>x1</sup>					6.25	8	50.00			
F16	D 16	E		1.856 <sup>x1</sup>	4.093 <sup>x1</sup>	1.283 <sup>x1</sup>					7.21	112	810.88			
F17	D 16	D		12.133 <sup>x1</sup>	1.183 <sup>x1</sup>					1,000 <sup>x1</sup>	14.32	60	859.20			
F18	D 16	D		12.133 <sup>x1</sup>	1.183 <sup>x1</sup>					1,000 <sup>x1</sup>	14.32	60	859.20			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	



Ibrahimia Reg. (Upstream) Rebar weight [2/10]															
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
F19	D 16	B	—	3.000 <sup>x1</sup>							3.00	152	456.00		
F20	D 16	D	┌	2.783 <sup>x1</sup>	1.183 <sup>^1</sup>						3.97	161	639.17		
F21	D 16	C	┌	1.716 <sup>x1</sup>	0.240 <sup>^2</sup>						2.20	323	710.60		
F22	D 18	C	┌	35.266 <sup>x1</sup>	0.270 <sup>^1</sup>	0.270 <sup>^1</sup>					1.000 <sup>x3</sup>	38.81	620.96		
F23	D 16	B	—	27.664 <sup>x1</sup>							1.000 <sup>x2</sup>	29.67	474.72		
F24	D 18	B	—	1.900 <sup>x1</sup>							1.90	30	57.00		
F25	D 18	C	┌	1.750 <sup>x1</sup>	0.270 <sup>^1</sup>	0.270 <sup>^1</sup>					2.29	5	11.45		
F26	D 16	B	—	1.800 <sup>x1</sup>							1.80	6	10.80		
F27	D 16	B	—	1.000 <sup>x1</sup>							1.00	4	4.00		
F28	D 16	C	┌	1.716 <sup>x1</sup>	0.240 <sup>^1</sup>	0.240 <sup>^1</sup>					2.20	1	8.80		
F29	D 16	B	—	1.900 <sup>x1</sup>							1.90	189	359.10		
F30	D 16	B	—	2.273 <sup>x1</sup>							2.28	184	419.52		
F31	D 16	F	└	1.000 <sup>x1</sup>	1.331 <sup>^1</sup>						2.34	5	11.70		
F32	D 16	B	—	36.300 <sup>x1</sup>							1.000 <sup>x3</sup>	39.30	471.60		
F33	D 16	G	└	1.100 <sup>x1</sup>	0.500 <sup>x1</sup>	1.300 <sup>x1</sup>					2.90	80	232.00		
F34	D 16	G	└	1.100 <sup>x1</sup>	0.500 <sup>x1</sup>	0.800 <sup>x1</sup>					2.40	99	237.60		
F35	D 16	B	—	2.273 <sup>x1</sup>							2.28	179	408.12		
F36	D 16	B	—	35.300 <sup>x1</sup>							1.000 <sup>x3</sup>	38.30	421.30		
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg/m	R	L	J
											D12	0.888	130	201	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

Ibrahimia Reg. (Upstream) Rebar weight [3/10]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F37	D 16	B		21.500 <sup>x1</sup>						1.000 <sup>x1</sup>	22.50	3	67.50			
F38	D 16	B		7.800 <sup>x1</sup>							7.80	12	93.60			
F39	D 12	II		0.430 <sup>x1</sup>	1.780 <sup>^1</sup>	1.780 <sup>^1</sup>	0.100 <sup>^2</sup>				4.19	1084	4541.96			
F40	D 12	II		0.430 <sup>x1</sup>	1.788 <sup>x1</sup> ~2.063	2.003 <sup>x1</sup> ~2.278	0.100 <sup>x2</sup>				4.70	120	564.00			
F41	D 12	H		0.430 <sup>x1</sup>	1.980 <sup>x1</sup>	1.980 <sup>x1</sup>	0.100 <sup>x2</sup>				4.59	88	403.92			
F42	D 12	H		0.430 <sup>x1</sup>	2.280 <sup>x1</sup>	2.280 <sup>x1</sup>	0.100 <sup>x2</sup>				5.19	412	2138.28			
F43	D 12	II		0.430 <sup>x1</sup>	1.880 <sup>^1</sup>	1.880 <sup>^1</sup>	0.100 <sup>^2</sup>				4.39	68	298.52			
F44	D 12	II		0.430 <sup>x1</sup>	1.480 <sup>^1</sup>	1.480 <sup>^1</sup>	0.100 <sup>^2</sup>				3.59	88	315.92			
F45	D 18	C		0.800 <sup>x1</sup>	0.270 <sup>^1</sup>	0.270 <sup>^1</sup>					1.34	3	4.02			
W 1	D 16	B		7.900 <sup>x1</sup>							7.90	56	442.40			
W 2	D 16	B		6.450 <sup>x1</sup>							6.45	56	361.20			
W 3	D 16	B		8.970 <sup>x1</sup>							8.97	55	493.35			
W 4	D 16	B		7.520 <sup>x1</sup>							7.52	55	413.60			
W 5	D 16	B		6.600 <sup>x1</sup>							6.60	29	191.40			
W 6	D 16	B		5.150 <sup>x1</sup>							5.15	29	149.35			
W 7	D 16	D		9.750 <sup>x1</sup>	0.240 <sup>x1</sup>						9.99	582	5814.18			
W 8	D 16	D		10.820 <sup>x1</sup>	0.240 <sup>^1</sup>						11.06	220	2433.20			
W 9	D 16	D		8.450 <sup>x1</sup>	0.240 <sup>^1</sup>						8.69	506	4397.14			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

Ibrahimia Reg. (Upstream) Rebar weight [4/10]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W10	D 16	B		10.800 <sup>x1</sup>							10.80	10	108.00			
W11	D 16	B		1.970 <sup>x1</sup>							1.97	29	57.13			
W12	D 16	B		3.270 <sup>x1</sup>							3.27	243	794.61			
W13	D 16	D		9.733 <sup>x1</sup>	0.240 <sup>x1</sup>						9.98	10	99.80			
W14	D 16	C		1.800 <sup>x1</sup>	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					3.80	202	767.60			
W15	D 16	C		1.450 <sup>x1</sup>	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					3.45	20	69.00			
W16	D 16	C		0.800 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.28	10	12.80			
W17	D 16	C		1.794 <sup>x1</sup> ~1.466	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					3.63	4	14.52			
W18	D 16	C		2.300 <sup>x1</sup>	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					4.30	279	1199.70			
W19	D 16	C		1.600 <sup>x1</sup>	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					3.60	30	108.00			
W20	D 16	C		0.600 <sup>x1</sup>	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					2.60	36	93.60			
W21	D 16	C		2.265 <sup>x1</sup> ~1.136	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					3.71	30	111.30			
W22	D 16	C		0.800 <sup>x1</sup>	1.000 <sup>x1</sup>	1.000 <sup>x1</sup>					2.80	106	296.80			
W23	D 16	B		27.700 <sup>x1</sup>						1.000 <sup>x2</sup>	29.70	68	2019.60			
W24	D 16	C		2.768 <sup>x1</sup>	1.768 <sup>x1</sup>	1.231 <sup>x1</sup>					4.77	41	236.57			
W25	D 16	B		3.000 <sup>x1</sup>							3.00	82	246.00			
W26	D 16	C		1.768 <sup>x1</sup>	1.234 <sup>x1</sup>	1.768 <sup>x1</sup>					4.77	82	391.14			
W27	D 16	B		4.400 <sup>x1</sup>							4.40	82	360.80			
T y p e	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
											D22	2.98	240	377	1210	
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

Ibrahimia Reg. (Upstream) Rebar weight [5/10]															
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
W28	D 16	C		15.268 <sup>x1</sup>	1.768 <sup>x1</sup>	1.768 <sup>x1</sup>				1.000 <sup>x1</sup>	19.81	68	1317.08		
W29	D 16	B		3.000 <sup>x1</sup>							3.00	68	204.00		
W30	D 16	C		2.768 <sup>x1</sup>	1.234 <sup>x1</sup>	1.768 <sup>x1</sup>					5.77	68	392.36		
W31	D 16	B		10.950 <sup>x1</sup>							10.95	14	153.30		
W32	D 16	C		2.518 <sup>x1</sup>	1.768 <sup>x1</sup>	1.768 <sup>x1</sup>					6.06	11	84.84		
W33	D 16	B		10.800 <sup>x1</sup>							10.80	61	691.20		
W34	D 16	C		1.768 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					2.25	52	117.00		
W35	D 16	I		0.884 <sup>x1</sup>	1.388 <sup>x1</sup>	1.884 <sup>x1</sup>	1.234 <sup>x1</sup>		φ 884		5.39	41	220.99		
W36	D 16	B		1.200 <sup>x1</sup>							1.20	6	7.20		
W37	D 16	B		1.900 <sup>x1</sup>							1.90	192	361.80		
W38	D 16	J		3.562 <sup>x1</sup>	1.150 <sup>x2</sup>				φ 1134		5.87	123	722.01		
W39	D 16	B		3.000 <sup>x1</sup>							3.00	246	738.00		
W40	D 16	B		1.800 <sup>x1</sup>							1.80	246	442.80		
W41	D 16	B		3.400 <sup>x1</sup>							3.40	246	836.40		
W42	D 16	B		15.300 <sup>x1</sup>						1.000 <sup>x1</sup>	16.30	201	3325.20		
W43	D 16	B		3.000 <sup>x1</sup>							3.00	201	612.00		
W44	D 16	J		3.562 <sup>x1</sup>	1.150 <sup>x2</sup>				φ 1134		5.87	204	1197.48		
W45	D 16	B		2.550 <sup>x1</sup>							2.55	42	107.10		
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg/m	R	L	J
											D12	0.888	130	201	1000
	K	L	M	N	O						D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

Ibrahimia Reg. (Upstream) Rebar weight [6/10]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W16	D 16	B	—	10.800 <sup>x1</sup>							10.80	95	1026.00			
W47	D 16	C	┌	2.268 <sup>x1</sup>	0.240 <sup>^1</sup>	0.240 <sup>^1</sup>					2.75	743	2043.25			
W48	D 16	C	┌	0.768 <sup>x1</sup>	0.240 <sup>^1</sup>	0.240 <sup>^1</sup>					1.25	52	65.00			
W49	D 16	B	—	2.200 <sup>x1</sup>							2.20	12	26.40			
W50	D 16	B	—	5.800 <sup>x1</sup>							5.80	11	63.80			
W51	D 16	B	—	2.800 <sup>x1</sup>							2.80	1	2.80			
W52	D 16	B	—	1.800 <sup>x1</sup>							1.80	2	3.60			
W53	D 16	B	—	3.650 <sup>x1</sup>							3.65	12	43.80			
W54	D 16	B	—	6.650 <sup>x1</sup>							6.65	10	66.50			
W55	D 16	B	—	2.650 <sup>x1</sup>							2.65	2	5.30			
W56	D 16	B	—	2.800 <sup>x1</sup>							2.80	2	5.60			
W57	D 16	B	—	5.794 <sup>x1</sup> ~5.648							5.73	3	17.19			
W58	D 16	B	—	2.466 <sup>x1</sup>							2.47	1	2.47			
W59	D 16	B	—	11.550 <sup>x1</sup> ~11.532							11.55	9	103.95			
W60	D 16	B	—	5.228 <sup>x1</sup> ~5.131							5.18	12	62.16			
W61	D 16	B	—	1.976 <sup>x1</sup> ~1.718							1.85	12	22.20			
W62	D 16	B	—	1.800 <sup>x1</sup>							1.80	12	21.60			
W63	D 16	B	—	2.250 <sup>x1</sup>							2.25	8	18.00			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

Ibrahimia Reg. (Upstream) Rebar weight [7/10]															
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
W64	D 16	B	—	3.650 <sup>×1</sup>							3.65	12	43.80		
W65	D 16	B	—	13.650 <sup>×1</sup>						1.000 <sup>×1</sup>	14.65	8	117.20		
W66	D 16	B	—	16.981 <sup>×1</sup>						1.000 <sup>×1</sup>	17.99	4	71.96		
W67	D 16	B	—	6.150 <sup>×1</sup> ~5.961							6.06	17	103.02		
W68	D 16	B	—	1.976 <sup>×1</sup> ~1.718							1.85	12	22.20		
W69	D 16	B	—	2.650 <sup>×1</sup>							2.65	4	10.60		
W70	D 12	C	⌈	1.829 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					2.03	942	1912.26		
W71	D 12	C	⌈	1.479 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					1.68	108	181.44		
W72	D 12	C	⌈	0.979 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					1.18	120	141.60		
W73	D 12	C	⌈	0.828 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					1.03	53	54.59		
W74	D 12	C	⌈	2.328 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					2.53	1335	3377.55		
W75	D 12	C	⌈	1.628 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					1.83	162	296.46		
W76	D 12	C	⌈	0.628 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					0.83	180	149.40		
W77	D 12	C	⌈	0.828 <sup>×1</sup>	0.100 <sup>×1</sup>	0.100 <sup>×1</sup>					1.03	156	160.68		
P 1	D 22	B		2.460 <sup>×1</sup>							2.46	201	501.84		
P 2	D 22	B		2.610 <sup>×1</sup>							2.61	16	41.76		
P 3	D 22	B		3.680 <sup>×1</sup>							3.68	24	88.32		
P 4	D 22	B		3.660 <sup>×1</sup> ~3.393							3.53	12	42.36		
T y p e	A	B	C	D	E	F	G	H	I	J	Dia.	kg/m	R	L	J
											D12	0.888	130	201	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

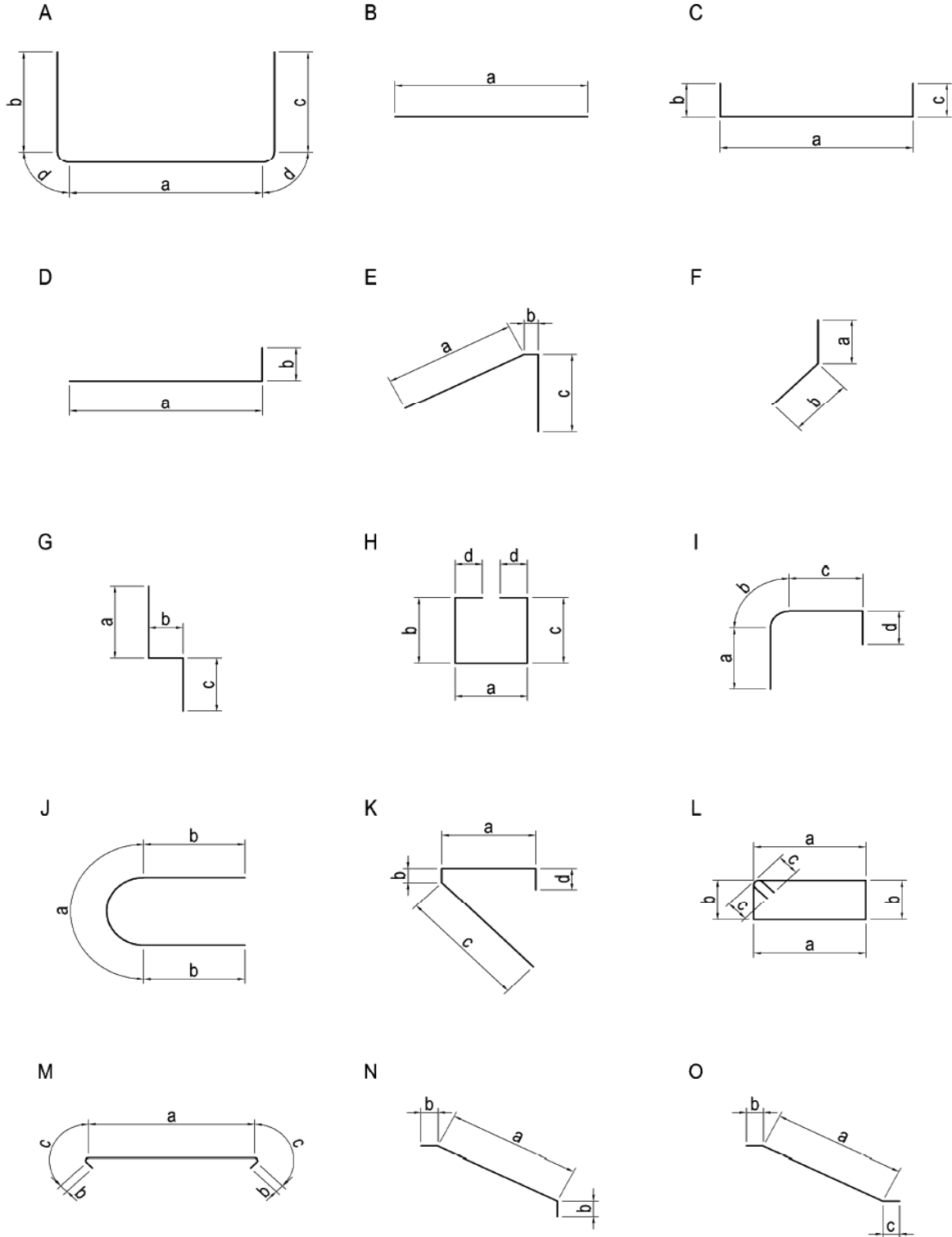
Ibrahimia Reg. (Upstream) Rebar weight [8/10]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
P 5	D 22	B		4.980 <sup>x1</sup>							1.98	21	119.52			
P 6	D 22	B		4.960 <sup>x1</sup> ~2.426							3.70	80	296.00			
P 7	D 22	B		2.410 <sup>x1</sup>							2.41	12	28.92			
P 8	D 12	C		0.550 <sup>x1</sup>	0.180 <sup>x1</sup>	0.180 <sup>x1</sup>					0.91	186	169.26			
P 9	D 16	B		15.820 <sup>x1</sup> ~13.890						1.000 <sup>x1</sup>	15.86	28	444.08			
P 10	D 16	B		2.650 <sup>x1</sup>							2.65	24	63.60			
P 11	D 16	B		5.775 <sup>x1</sup> ~4.470							5.13	24	123.12			
P 12	D 16	B		6.650 <sup>x1</sup>							6.65	28	186.20			
P 13	D 16	E		0.517 <sup>x1</sup>	1.620 <sup>x1</sup>	1.050 <sup>x1</sup>					3.19	4	12.76			
P 14	D 16	E		5.325 <sup>x1</sup>	1.620 <sup>x1</sup>	1.050 <sup>x1</sup>					8.00	1	32.00			
P 15	D 16	C		0.512 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.00	31	31.00			
P 16	D 12	C		0.584 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>					0.79	200	158.00			
S 1	D 16	B		8.000 <sup>x1</sup>							8.00	176	1408.00			
S 2	D 16	B		0.800 <sup>x1</sup>							0.80	960	768.00			
S 3	D 16	C		0.618 <sup>x1</sup>	0.268 <sup>x1</sup>	0.268 <sup>x1</sup>					1.16	480	556.80			
C 1	D 16	K		0.950 <sup>x1</sup>	0.159 <sup>x1</sup>	1.344 <sup>x1</sup>	0.240 <sup>x1</sup>				2.70	102	275.10			
C 2	D 16	B		9.800 <sup>x1</sup>							9.80	8	78.40			
G 1	D 16	A		34.960 <sup>x1</sup>	5.730 <sup>x1</sup>	5.730 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x6</sup>	52.96	8	423.68			
T y p e	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

Ibrahimia Reg. (Upstream) Rebar weight [9/10]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
G 2	D 16	I		5.730 <sup>x1</sup>	0.267 <sup>x1</sup>	2.030 <sup>x1</sup>	0.300 <sup>x1</sup>			φ 170		8.33	1	33.32		
G 3	D 16	I		5.730 <sup>x1</sup>	0.267 <sup>x1</sup>	0.950 <sup>x1</sup>	0.300 <sup>x1</sup>			φ 170		7.25	24	174.00		
G 4	D 16	B		5.900 <sup>x1</sup>								5.90	292	1722.80		
G 5	D 16	L		1.832 <sup>x2</sup>	0.832 <sup>x2</sup>	0.225 <sup>x2</sup>						5.78	92	531.76		
G 6	D 16	L		0.982 <sup>x2</sup>	4.232 <sup>x2</sup>	0.225 <sup>x2</sup>						10.88	46	500.48		
G 7	D 16	L		0.682 <sup>x2</sup>	0.832 <sup>x2</sup>	0.225 <sup>x2</sup>						3.48	276	960.48		
G 8	D 16	M		2.236 <sup>x1</sup>	0.160 <sup>x2</sup>	0.113 <sup>x2</sup>				φ 48		2.79	18	50.22		
G 9	D 12	C		0.978 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>						1.18	72	84.96		
G10	D 12	C		0.678 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>						0.88	132	116.16		
T 1	D 16	C		35.300 <sup>x1</sup>	0.300 <sup>x1</sup>	0.300 <sup>x1</sup>					1.000 <sup>x3</sup>	38.90	10	389.00		
T 2	D 16	B		7.200 <sup>x1</sup>								7.20	8	57.60		
T 3	D 16	C		3.100 <sup>x1</sup>	0.300 <sup>x1</sup>	0.300 <sup>x1</sup>						3.70	6	22.20		
T 4	D 16	C		0.600 <sup>x1</sup>	0.300 <sup>x1</sup>	0.300 <sup>x1</sup>						1.20	36	43.20		
T 5	D 16	C		35.300 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.000 <sup>x3</sup>	38.78	19	736.82		
T 6	D 16	C		0.950 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>						1.43	21	31.32		
T 7	D 16	C		0.600 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>						1.08	36	38.88		
T 8	D 16	C		6.232 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>						6.90	24	165.60		
T 9	D 16	C		2.832 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>						3.50	56	196.00		
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M	N	O											
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	



Ibrahimia Reg. (Upstream) Rebar weight [10/10]															
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
T10	D 16	C		0.832 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					1.50	56	84.00		
T11	D 16	C		2.682 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					3.35	104	348.40		
T12	D 16	D		0.832 <sup>x1</sup>	0.182 <sup>x1</sup>						1.02	208	212.16		
T13	D 16	C		0.682 <sup>x1</sup>	0.332 <sup>x1</sup>	0.332 <sup>x1</sup>					1.35	104	140.40		
T14	D 16	C		6.232 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					6.72	24	161.28		
T15	D 16	C		2.832 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					3.32	56	185.92		
T16	D 16	C		0.832 <sup>x1</sup>	0.240 <sup>x1</sup>	0.240 <sup>x1</sup>					1.32	56	73.92		
T17	D 16	C		2.832 <sup>x1</sup>	0.240 <sup>x1</sup>	0.182 <sup>x1</sup>					3.26	104	339.04		
T18	D 16	C		0.832 <sup>x1</sup>	0.182 <sup>x1</sup>	0.240 <sup>x1</sup>					1.26	104	131.04		
T19	D 16	N		2.804 <sup>x1</sup>	0.100 <sup>x2</sup>						3.01	31	102.34		
T20	D 16	O		1.860 <sup>x1</sup>	0.100 <sup>x1</sup>	0.240 <sup>x1</sup>					2.20	30	66.00		
T21	D 16	C		0.832 <sup>x1</sup>	0.832 <sup>x1</sup>	0.832 <sup>x1</sup>					2.50	24	60.00		
T22	D 16	B		2.300 <sup>x1</sup>							2.30	18	41.40		
T23	D 12	C		0.328 <sup>x1</sup>	0.100 <sup>x1</sup>	0.100 <sup>x1</sup>					0.53	377	199.81		
D12				15264.77 × 0.888		=	13555.12 kg		=	13.555 t					
D16				61778.91 × 1.58		=	102350.68 kg		=	102.351 t					
D18				13029.63 × 2.00		=	26059.26 kg		=	26.059 t					
D22				1118.72 × 2.98		=	3333.79 kg		=	3.334 t					
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg/m	R	L	J
											D12	0.888	130	204	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

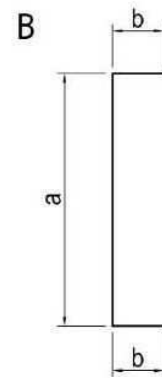
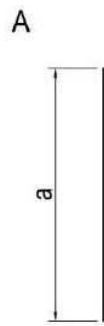
## Schematic design of rebar (Upstream)



C-91

Ibrahimiya Reg. (Approach cushion slab)      Rebar weight [1/1]      per a place														
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
F1	D 18	B	┌	4.800 <sup>x1</sup>	0.200 <sup>x2</sup>						5.20	50	260.00	
F2	D 16	B	┌	4.800 <sup>x1</sup>	0.200 <sup>x2</sup>						5.20	50	260.00	
F3	D 16	A	—	9.800 <sup>x1</sup>							9.80	25	245.00	
F4	D 16	A	—	9.800 <sup>x1</sup>							9.80	25	245.00	
F5	D 12	B	┐	0.229 <sup>x1</sup>	0.180 <sup>x2</sup>						0.59	144	84.96	
D12				84.96 × 0.888	=	75.44 kg	=	0.075 t						
D16				750.00 × 1.58	=	1185.00 kg	=	1.185 t						
D18				260.00 × 2.00	=	520.00 kg	=	0.520 t						
Type	A      B		┌											
			Dia.	kg/m	R	L	J							
			D12	0.888	130	204	1000							
			D14	1.21	150	236	1000							
			D16	1.58	170	267	1000							
			D18	2.00	190	298	1000							
			D20	2.47	210	330	1100							
			D22	2.98	240	377	1210							
			D25	3.85	270	424	1375							
			D28	4.83	300	471	1540							
			D32	6.31	340	534	1760							

Schematic design of rebar  
(Approach cushion slab)



## Ibrahimia Weir body

## Quantity calculation

【Downstream】

Discription	Classification	Calculation	Quantity	Unit	Remarks
Main body Leveling con.	$\sigma_{ck}=20\text{N/mm}^2$				
		$7.60 \times (21.50 + 21.60) \times 2 \times 0.10 \times 2 = 32.76$			L type wall part
		$0.50 \times 1.50 \times 0.10 \times 2 = 0.15$			"
		$1/2 \times 0.50 \times 0.50 \times 0.10 \times 2 = 0.03$			"
		$-\pi \times 4 \times 1.00^2 \times 0.10 \times 16 \times 2 = -2.51$			(-)Piles part
		$19.50 \times (21.60 + 21.10) / 2 \times 0.10 = 41.05$			Apron part
		Total = 71.48	71.48	m <sup>3</sup>	
Formwork	Leveling con.				
		$(21.50 - 21.60) / 2 \times 0.10 \times 2 \times 2 = 8.62$			L type wall part
		$-0.50 \times 0.10 \times 2 = -0.10$			"
		$0.50 \times \sqrt{2} \times 0.10 \times 2 = 0.14$			"
		$(8.10 + 10.25) \times 2 \times 0.10 \times 2 = 1.84$			Apron part
		Total = 10.50	10.50	m <sup>2</sup>	
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$				
	Cut-off	$1 \times 2 \times (1.00 + 1.50) \times 0.50 \times 8.50 \times 2 = 10.63$			L type wall part
	"	$1 \times 2 \times (1.00 + 2.00) \times 1.00 \times 19.50 = 29.25$			Apron part
	Invert	$8.00 \times 1.50 \times 2.00 \times 2 = 48.00$			L type wall part
	"	$1/2 \times (8.00 + 7.50) \times 1.50 \times 0.50 \times 2 = 11.63$			"
	"	$7.50 \times 1.50 \times 19.50 \times 2 = 438.75$			"
	"	$8.50 \times 1.50 \times 1.50 \times 2 = 38.25$			"
	"	$-\pi \times 4 \times 1.00^2 \times 0.10 \times 16 \times 2 = -2.51$			(-)Piles part
	"	$1 \times 2 \times (2.00 + 1.00) \times 21.50 \times 19.50 = 628.88$			Apron part
	"	$1.00 \times 2.00 \times 19.50 = 39.00$			"
	Wall A	$2.00 \times 6.70 \times 2.00 = 26.80$			
	Wall A	$1 \times 2 \times (2.00 + 1.00) \times 6.70 \times 1.00 = 10.05$			
	"	$1.00 \times 6.70 \times (20.50 + 1.50) = 117.10$			

Discription	Classification	Calculation	Quantity	Unit	Remarks
	"	$1/2 \times 0.50 \times 6.70 \times (19.50 + 20.00)/2$	— 33.08		
	"	$-(1 - \pi/4) \times 1.00^2 \times 6.70$	= -1.11		
	Wall B	$2.00 \times 6.70 \times 2.00$	= 26.80		
	"	$1/2 \times (2.00 + 1.00) \times 6.70 \times 1.00$	— 10.05		
	"	$1.00 \times 6.70 \times (20.50 + 1.50)$	= 147.40		
	"	$1/2 \times 0.50 \times 6.70 \times (19.50 + 20.00)/2$	= 33.08		
	"	$-(1 - \pi/4) \times 1.00^2 \times 6.70$	= -1.11		
	Earth pressure wall	$1.30 \times 0.30 \times 1.30$	= 0.51		
			Total — 1674.17	1674.17	m <sup>3</sup>
Formwork	RC structure				
	Cut-off	$1/2 \times (1.00 + 1.50) \times 0.50 \times 2 \times 2$	— 2.50		L type wall part
	"	$0.50 \times 8.50 \times 2$	= 8.50		"
	"	$1.00 \times 19.50$	= 19.50		Apron part
	Invert	$1.50 \times (23.50 - 0.50 + 1.00) \times 2$	— 72.00		L type wall part
	"	$1.50 \times 0.50 \times \sqrt{(2)} \times 2$	= 2.12		"
	"	$1.50 \times (23.50 + 8.50) \times 2$	— 96.00		"
	"	$1/2 \times 8.60 \times 0.40 \times 2$	— 3.44		Apron part
	"	$1.00 \times 19.50$	= 19.50		"
	Wall A	$6.70 \times (22.50 + 2.00)$	= 164.15		L type wall part
	"	$6.70 \times (0.50 + 1.00)/2 \times \sqrt{(2)}$	— 7.11		"
	"	$6.719 \times (19.50 + 20.00)/2$	= 132.70		"
	"	$6.70 \times \{1.50 + 1/2 \times (1.00 + 1.50)\}$	= 18.43		"
	Wall C	$6.70 \times (22.50 + 2.00)$	— 164.15		L type wall part
	"	$6.70 \times (0.50 + 1.00)/2 \times \sqrt{(2)}$	= 7.11		"
	"	$6.719 \times (19.50 + 20.00)/2$	— 132.70		"
	"	$6.70 \times \{1.50 + 1/2 \times (1.00 + 1.50)\}$	= 18.43		"
	Earth pressure wall	$(1.30 \times 2 + 0.30) \times 1.30$	= 3.77		
			Total — 872.11	872.11	m <sup>2</sup>

Discription	Classification	Calculation	Quantity	Unit	Remarks
Circular formwork	RC structure				
	$\phi$ 1000-1/1Circle	$\pi/4 \times 1.00 \times 6.70 \times 2$	= 10.52	10.52	m <sup>2</sup>
Rebar	Steel 360/520	refer to rebar weight calculation sheet.			
		I. type wall part      Apron part			
		D12      1.757 × 2             3.878      =      7.392	7.392	t	
		D16      11.041 × 2      +      18.239      =      40.327	40.327	t	
		D22      6.578 × 2      =      13.156	13.156	t	
Concrete curing	PC structure	Leveling con.	= 71.48	71.48	m <sup>3</sup>
	RC structure	Reinforced con.	= 1674.17	1674.17	m <sup>3</sup>
Scaffolding	Prefabricated scaffold, H≤30m				
	Outer perimeter	$8.20 \times (2.46 + 0.71 + 17.94 - 1.70 + 3.70 + 1.30) \times 2$	= 505.28		
	Inside	$6.70 \times 23.50 \times 2$	= 314.90		
		Total	= 820.18	820.18	m <sup>2</sup>
Expansion joint	Elastic filler, t=20mm				
		$2.00 \times (19.50 + 6.70 \times 2) - (1.50 \times 8.00 - 0.50 \times 1.30) \times 2$	= 91.10		
		$1/2 \times (2.00 + 1.00) \times 21.50 \times 2$	= 64.50		
		$2.00 \times 1.00 \times 2$	= 4.00		
		$1/2 \times (1.00 + 2.00) \times 1.00 \times 2$	= 3.00		
		$-1/2 \times 8.60 \times 0.40 \times 2$	= -3.44		
		Total	= 159.16	159.16	m <sup>2</sup>
Water stop	B=300mm	$23.50 \times 2$	= 47.00	47.00	m <sup>2</sup>
Slip bar	D20×1.00m, VPφ25×0.50m	$78 \times 2$	= 156.0	156	nos









Ibrahimia Reg. (Downstream-L type wall) Rebar weight [1/4]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F1	D 22	B		7.800 <sup>x1</sup>	1.250 <sup>x2</sup>						10.30	10	103.00			
F2	D 22	B		7.459 <sup>x1</sup> ~7.659	1.250 <sup>x2</sup>						10.06	2	20.12			
F3	D 22	B		7.300 <sup>x1</sup>	1.250 <sup>x2</sup>						9.80	98	960.40			
F4	D 22	B		8.300 <sup>x1</sup>	1.250 <sup>x2</sup>						10.80	8	86.40			
F5	D 16	B		7.800 <sup>x1</sup>	0.240 <sup>x2</sup>						8.28	10	82.80			
F6	D 16	B		7.459 <sup>x1</sup> ~7.659	0.240 <sup>x2</sup>						8.01	2	16.08			
F7	D 16	B		7.300 <sup>x1</sup>	0.240 <sup>x2</sup>						7.78	98	762.44			
F8	D 16	B		8.300 <sup>x1</sup>	0.240 <sup>x2</sup>						8.78	8	70.24			
F9	D 16	A		23.300 <sup>x1</sup>						1.000 <sup>x3</sup>	25.30	70	1771.00			
F10	D 16	D		1.851 <sup>x1</sup>	2.519 <sup>x1</sup>						4.37	2	8.74			
F11	D 16	A		2.009 <sup>x1</sup> ~2.159							2.09	2	4.18			
F12	D 16	B		1.300 <sup>x1</sup>	1.250 <sup>x2</sup>						3.80	5	19.00			
F13	D 16	B		1.300 <sup>x1</sup>	0.240 <sup>x2</sup>						1.78	5	8.90			
F14	D 16	B		7.765 <sup>x1</sup>	0.240 <sup>x2</sup>						8.25	6	49.50			
F15	D 16	C		8.284 <sup>x1</sup>	0.240 <sup>x1</sup>						8.53	6	51.18			
F16	D 16	A		8.300 <sup>x1</sup>							8.30	6	49.80			
F17	D 16	A		23.284 <sup>x1</sup>						1.000 <sup>x3</sup>	25.29	12	303.48			
F18	D 16	D		1.851 <sup>x1</sup>	2.519 <sup>x1</sup>						4.37	6	26.22			
Type	A B C D E F G H											Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	



Ibrahimia Reg. (Downstream-L type wall) Rebar weight [2/4]														
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
F19	D 16	B		1.218 <sup>x1</sup>	0.240 <sup>x2</sup>						1.70	29	49.30	
F20	D 16	A		1.800 <sup>x1</sup>							1.80	43	77.40	
F21	D 16	A		1.566 <sup>x1</sup>							1.57	35	54.95	
F22	D 16	E		1.000 <sup>x1</sup>	0.554 <sup>x1</sup>						1.56	8	12.48	
F23	D 16	A		8.300 <sup>x1</sup>							8.30	6	49.80	
F24	D 12	H		0.434 <sup>x1</sup>	1.281 <sup>x2</sup>	0.100 <sup>x2</sup>					3.20	420	1344.00	
W1	D 22	C		7.950 <sup>x1</sup>	0.330 <sup>x1</sup>						8.28	13	107.64	
W2	D 22	F		6.604 <sup>x1</sup>	1.349 <sup>x1</sup>	0.330 <sup>x1</sup>					8.29	1	8.29	
W3	D 22	F		6.615 <sup>x1</sup>	1.347 <sup>x1</sup>	0.330 <sup>x1</sup>					8.30	1	8.30	
W4	D 22	F		6.622 <sup>x1</sup>	1.346 <sup>x1</sup>	0.330 <sup>x1</sup>					8.30	98	813.10	
W5	D 22	C		7.950 <sup>x1</sup>	0.330 <sup>x1</sup>						8.28	8	66.24	
W6	D 16	C		7.950 <sup>x1</sup>	0.240 <sup>x1</sup>						8.19	135	1105.65	
W7	D 16	A		23.300 <sup>x1</sup>						1.000 <sup>x2</sup>	25.30	34	860.20	
W8	D 16	D		1.851 <sup>x1</sup>	2.519 <sup>x1</sup>						1.37	31	148.58	
W9	D 16	A		2.292 <sup>x1</sup>							2.30	31	78.20	
W10	D 16	A		22.100 <sup>x1</sup>						1.000 <sup>x2</sup>	24.40	34	829.60	
W11	D 16	G		1.400 <sup>x1</sup>	1.389 <sup>x1</sup>	1.000 <sup>x1</sup>			φ 884		3.79	34	128.86	
W12	D 16	A		22.966 <sup>x1</sup> ~23.249						1.000 <sup>x2</sup>	25.11	3	75.33	
Type	A	B	C	D	E	F	G	H						
										Dia.	kg/m	R	L	J
										D12	0.888	130	201	1000
										D14	1.21	150	236	1000
										D16	1.58	170	267	1000
										D18	2.00	190	298	1000
										D20	2.47	210	330	1100
										D22	2.98	240	377	1210
									D25	3.85	270	424	1375	
									D28	4.83	300	471	1540	
									D32	6.31	340	534	1760	

C-98

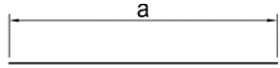
Ibrahimia Reg. (Downstream-L type wall) Rebar weight [3/4]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W13	D 16	A	—	2.009 <sup>x1</sup> ~2.659							2.31	5	11.70			
W14	D 16	A	—	1.966 <sup>x1</sup> ~2.249							2.11	3	6.33			
W15	D 22	A	—	2.410 <sup>x1</sup>							2.41	14	33.74			
W16	D 16	A	—	1.100 <sup>x1</sup>							1.10	15	16.50			
W17	D 16	B	┌	1.800 <sup>x1</sup>	0.240 <sup>x2</sup>						2.28	10	22.80			
W18	D 16	B	┌	0.859 <sup>x1</sup> ~1.659	0.240 <sup>x2</sup>						1.71	5	8.70			
W19	D 16	B	┌	0.807 <sup>x1</sup>	0.240 <sup>x2</sup>						1.29	98	126.42			
W20	D 16	B	┌	0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	8	10.24			
W21	D 16	B	┌	1.765 <sup>x1</sup>	0.240 <sup>x2</sup>						2.25	33	74.25			
W22	D 16	B	┌	0.550 <sup>x1</sup>	0.240 <sup>x2</sup>						1.03	7	7.21			
W23	D 16	B	┌	0.512 <sup>x1</sup>	0.240 <sup>x2</sup>						1.00	12	12.00			
W24	D 12	B	┌	1.831 <sup>x1</sup>	0.100 <sup>x2</sup>						2.04	40	81.60			
W25	D 12	B	┌	1.143 <sup>x1</sup> ~1.322	0.100 <sup>x2</sup>						1.44	16	23.04			
W26	D 12	B	┌	0.861 <sup>x1</sup> ~1.308	0.100 <sup>x2</sup>						1.29	392	505.68			
W27	D 12	B	┌	0.831 <sup>x1</sup>	0.100 <sup>x2</sup>						1.01	20	20.80			
W28	D 12	B	┌	0.584 <sup>x1</sup>	0.100 <sup>x2</sup>						0.79	5	3.95			
Type												Dia.	kg/m	R	L	J
A												D12	0.888	130	201	1000
B												D14	1.21	150	236	1000
C												D16	1.58	170	267	1000
D												D18	2.00	190	298	1000
E												D20	2.47	210	330	1100
F												D22	2.98	240	377	1210
G												D25	3.85	270	424	1375
H												D28	4.83	300	471	1540
I												D32	6.31	340	534	1760

Ibrahimia Reg. (Downstream-L type wall)      Rebar weight [3/4]														
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
			D12	1979.07 × 0.888		=	1757.41 kg		=	1.757 t				
			D16	6990.06 × 1.58		=	11044.29 kg		=	11.044 t				
			D22	2207.53 × 2.98		=	6578.44 kg		=	6.578 t				
Type	A	B	C	D	E	F	G	H						
														

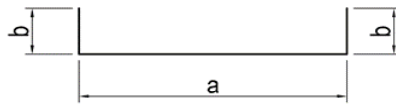
Dia.	kg/m	R	L	J
D12	0.888	130	201	1000
D14	1.21	150	236	1000
D16	1.58	170	267	1000
D18	2.00	190	298	1000
D20	2.47	210	330	1100
D22	2.98	240	377	1210
D25	3.85	270	424	1375
D28	4.83	300	471	1540
D32	6.31	340	534	1760

Schematic design of rebar  
(Downstream-L type wall)

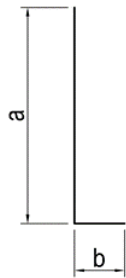
A



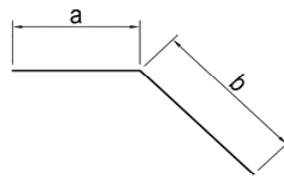
B



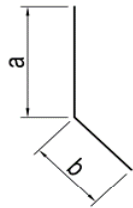
C



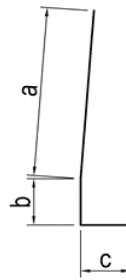
D



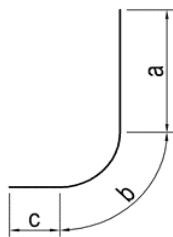
E



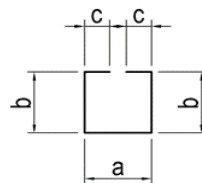
F



G



H



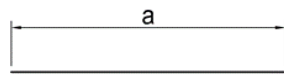
Ibrahimia Reg. (Downstream-Apron) Rebar weight [1/1]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
A1	D 16	A	—	19.300 <sup>x1</sup>						1.000 <sup>x1</sup>	20.30	236	1790.80			
A2	D 16	B	—	21.420 <sup>x1</sup>	1.903 <sup>x1</sup>					1.000 <sup>x2</sup>	25.33	50	1266.50			
A3	D 16	B	—	21.420 <sup>x1</sup>	1.903 <sup>x1</sup>					1.000 <sup>x3</sup>	25.33	49	1241.17			
A4	D 16	A	—	23.300 <sup>x1</sup>						1.000 <sup>x2</sup>	25.30	99	2504.70			
A5	D 16	A	—	23.300 <sup>x1</sup>						1.000 <sup>x2</sup>	25.30	6	151.80			
A6	D 16	A	—	12.798 <sup>x1</sup> ~21.398						1.000 <sup>x1</sup>	18.10	6	108.60			
A7	D 16	A	—	4.98 <sup>x1</sup> ~8.498							6.35	4	25.40			
A8	D 16	C	┌—┐	19.268 <sup>x1</sup>	0.240 <sup>x2</sup>					1.000 <sup>x1</sup>	20.75	11	228.25			
A9	D 16	C	┌	1.763 <sup>x1</sup>	0.240 <sup>x2</sup>						2.25	99	222.75			
A10	D 16	C	┌	0.770 <sup>x1</sup> ~1.756	0.240 <sup>x2</sup>						1.75	211	371.50			
A11	D 16	C	┌	0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	18	22.50			
A12	D 16	A		1.800 <sup>x1</sup>							1.80	99	178.20			
A13	D 16	A	—	2.273 <sup>x1</sup>							2.28	99	225.72			
A14	D 16	A	┌—┐	19.300 <sup>x1</sup>						1.000 <sup>x1</sup>	20.30	10	203.00			
A15	D 12	D	□	0.428 <sup>x1</sup>	0.810 <sup>x1</sup> ~1.807	0.828 <sup>x1</sup> ~1.788	0.100 <sup>x2</sup>				3.26	1272	1146.72			
A16	D 12	D	□	0.428 <sup>x1</sup>	0.828 <sup>x1</sup>	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>				2.29	96	219.84			
D12				4366.56 × 0.888		=	3877.51 kg		=	3.878 t						
D16				11543.89 × 1.58		=	18239.35 kg		=	18.239 t						
Type	A	B	C	D								Dia.	kg/m	R	L	J
	—	—	┌—┐	□								D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	

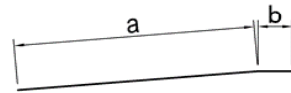
C-101

Schematic design of rebar  
(Downstream-Apron)

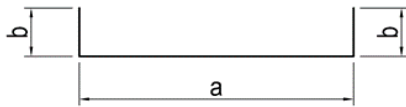
A



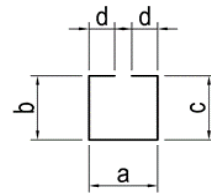
B



C

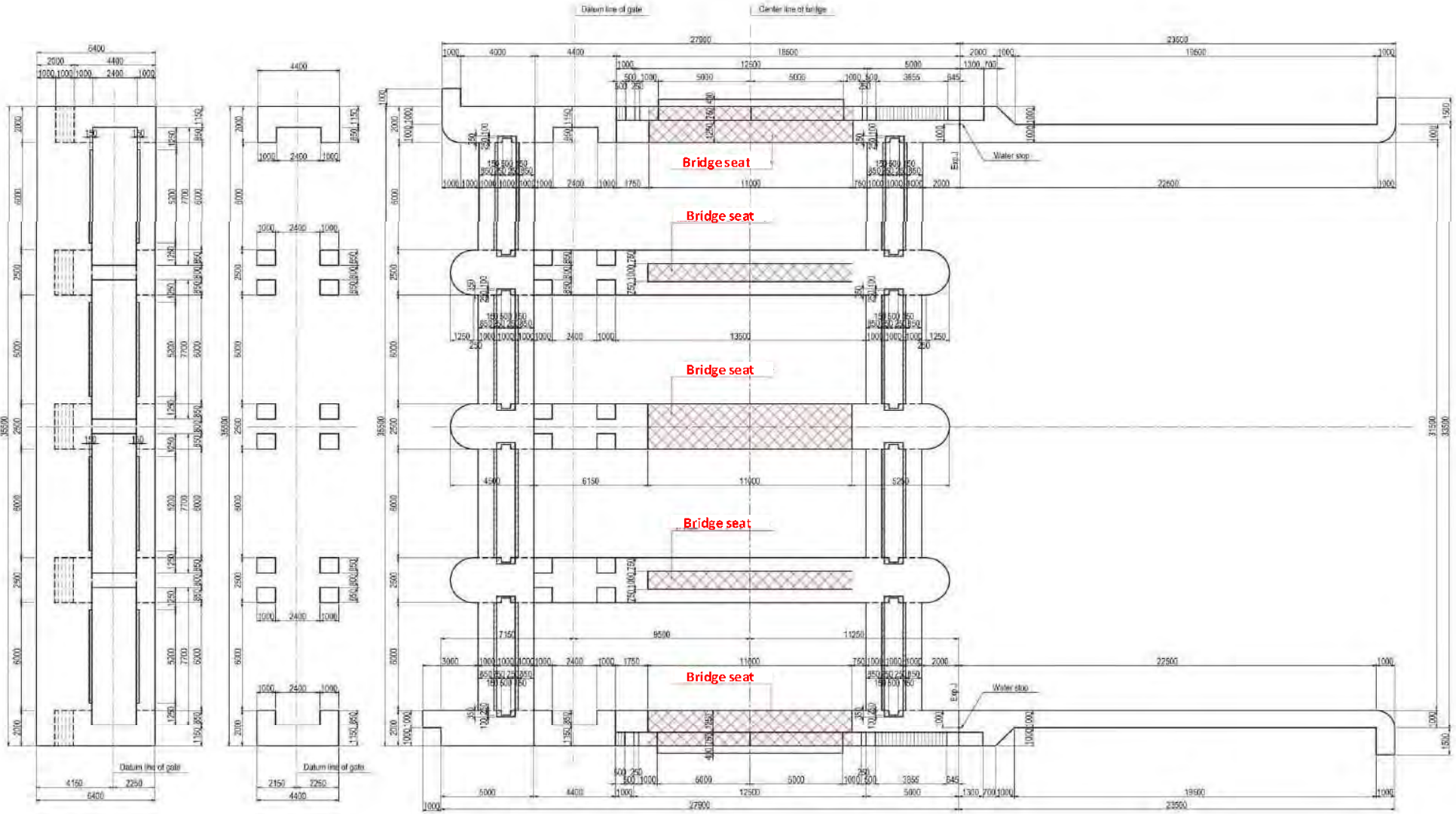


D

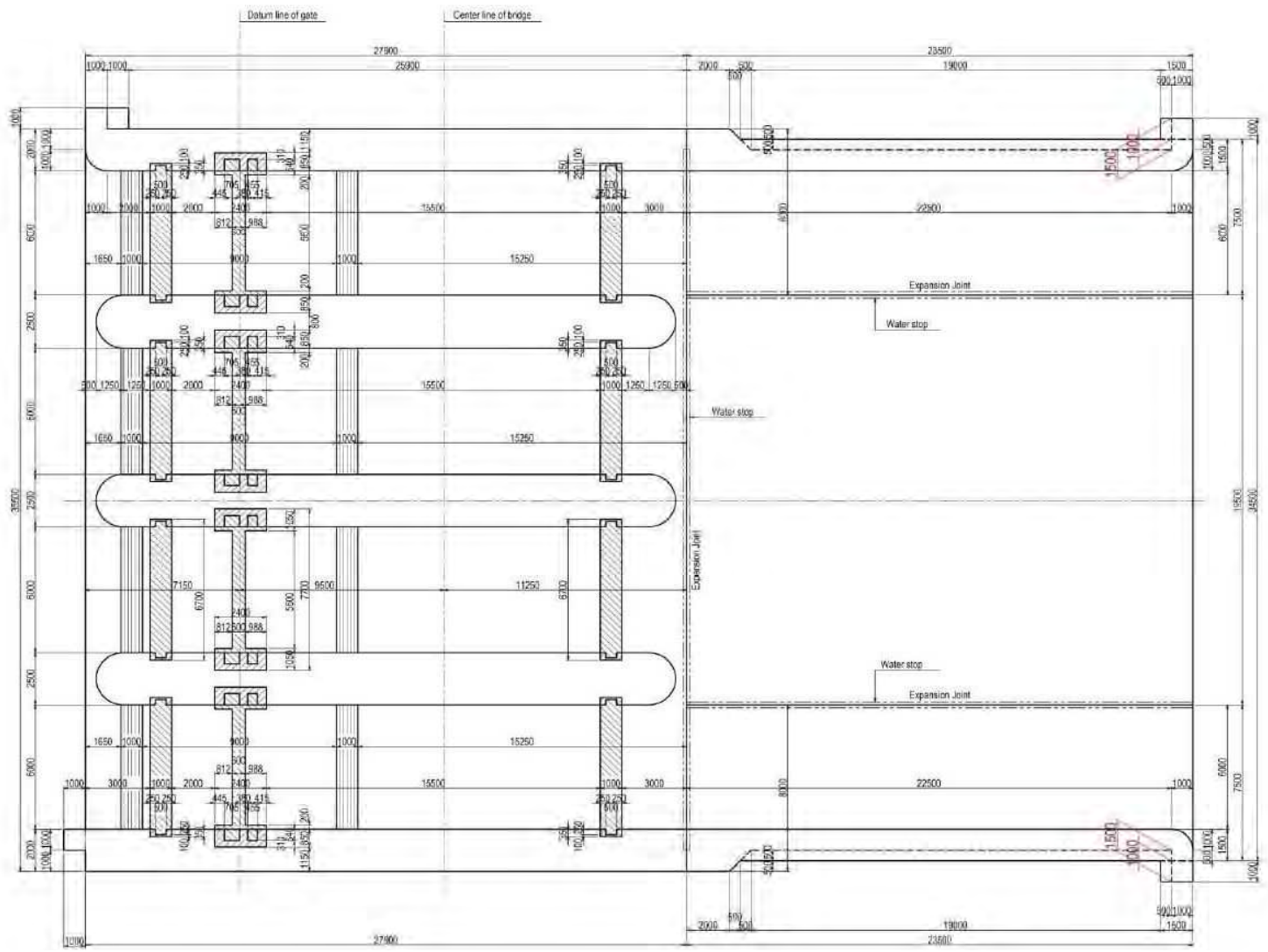


# Concrete / Formwork (1/5)

C-103



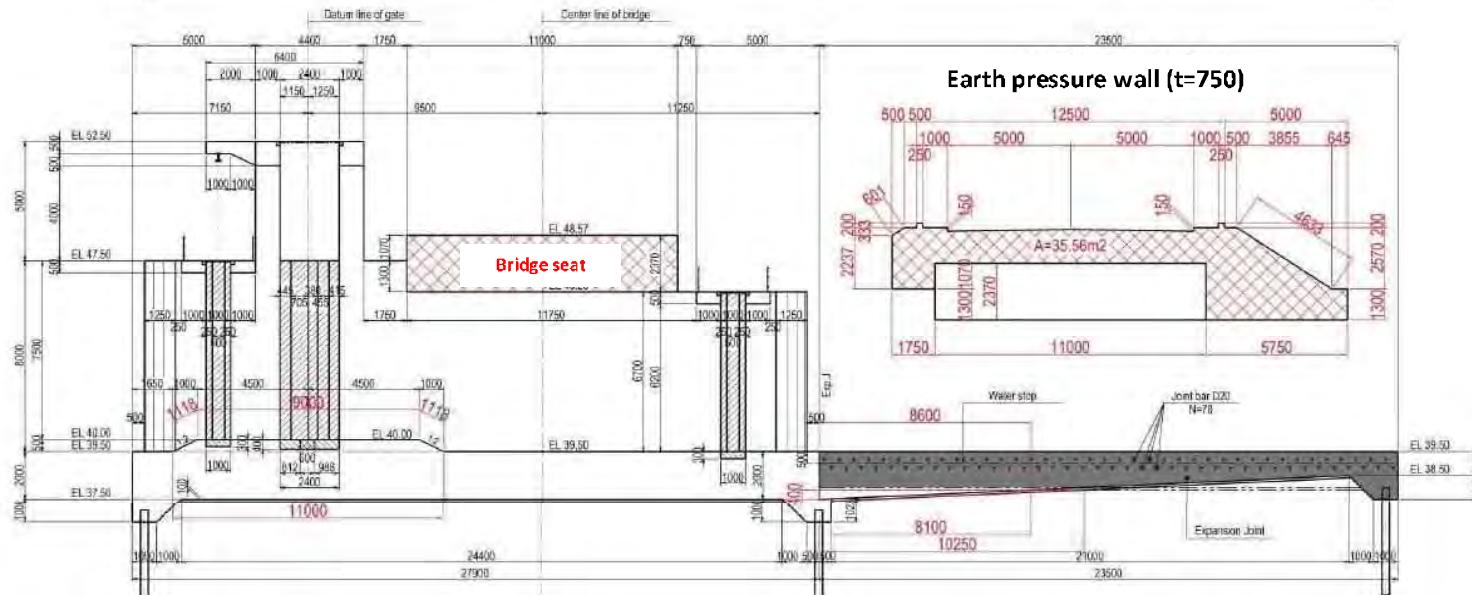
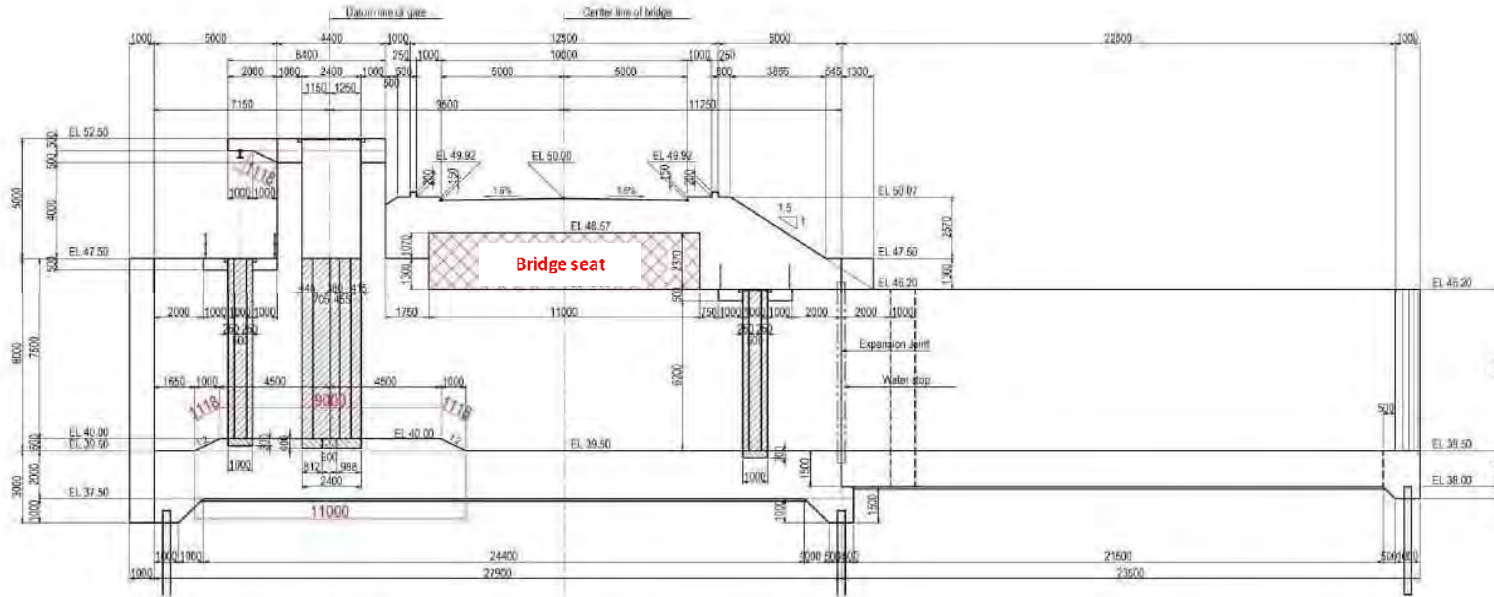
# Concrete / Formwork (2/5)



C-104



### Concrete / Formwork (3/5)

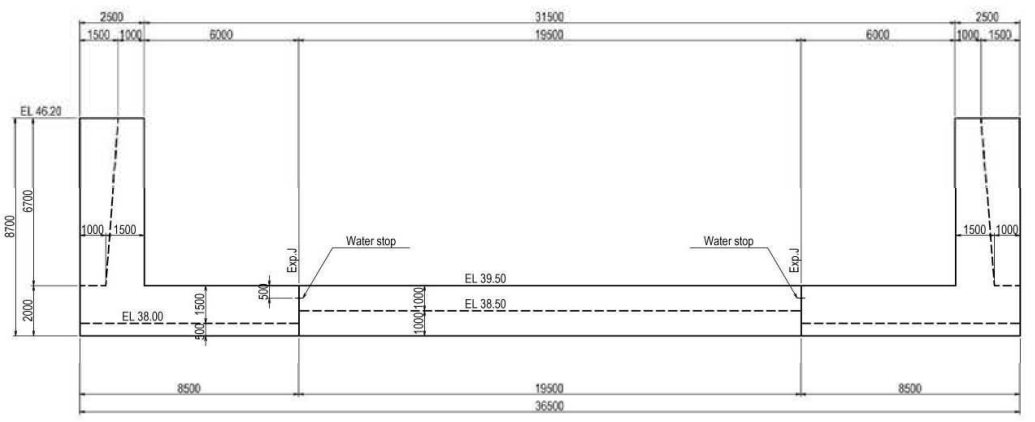
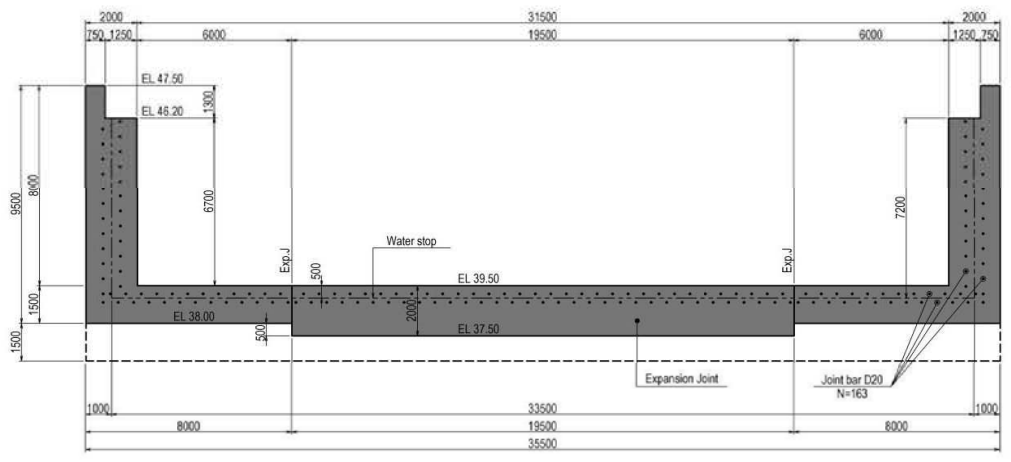
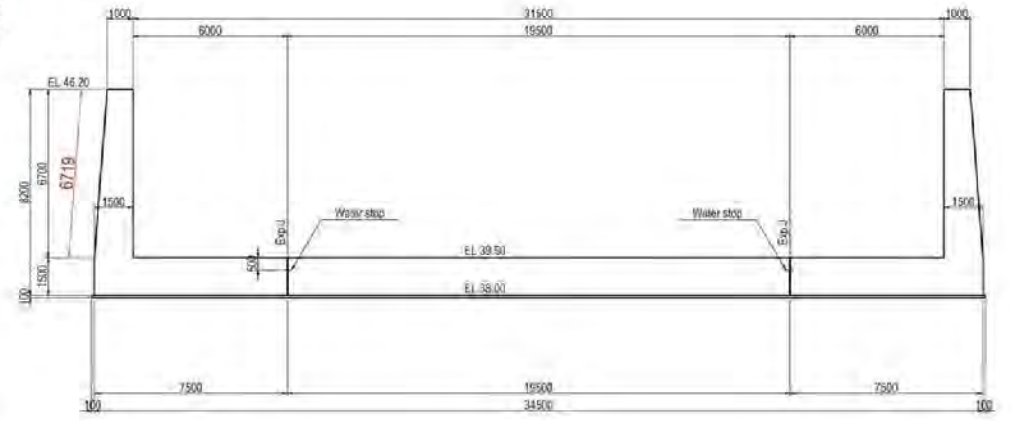
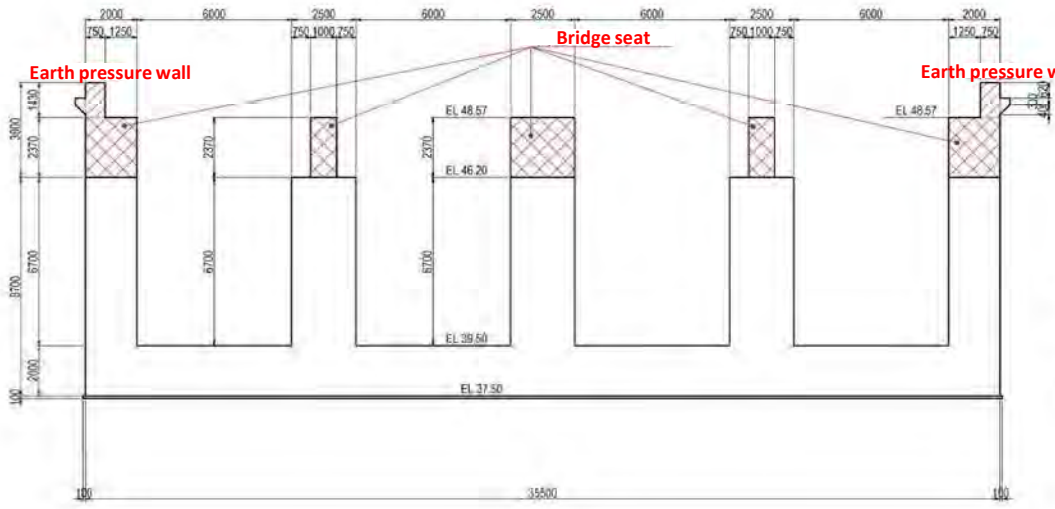


C-105



## Concrete / Formwork (5/5)

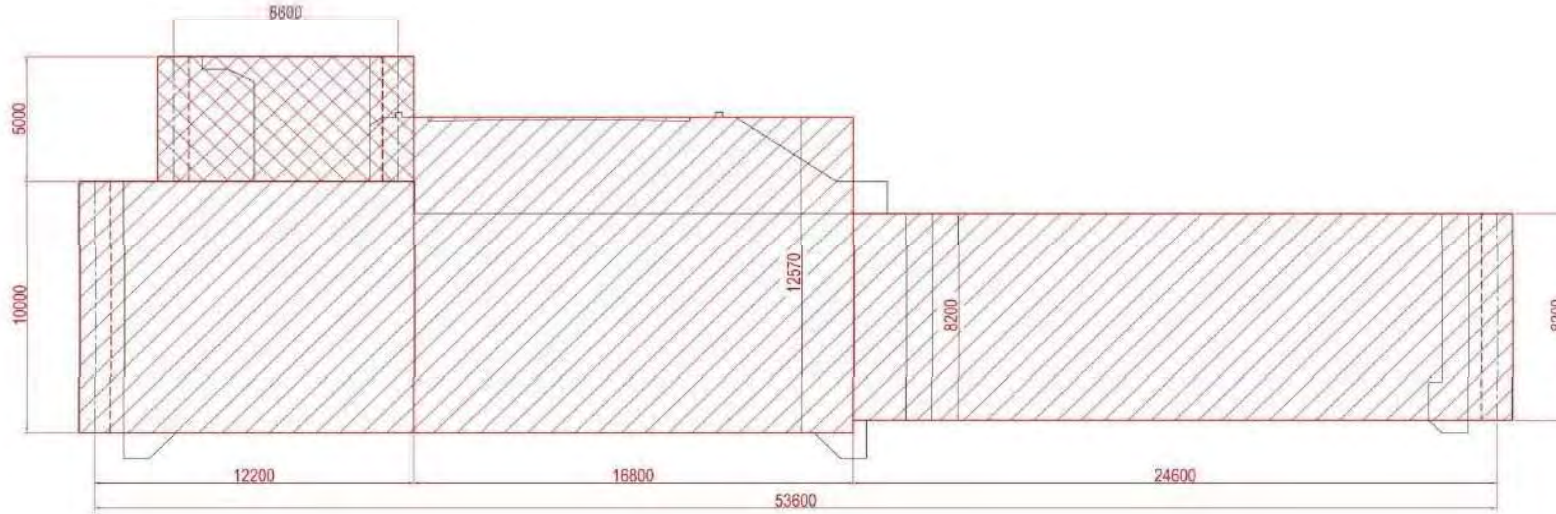
C-107



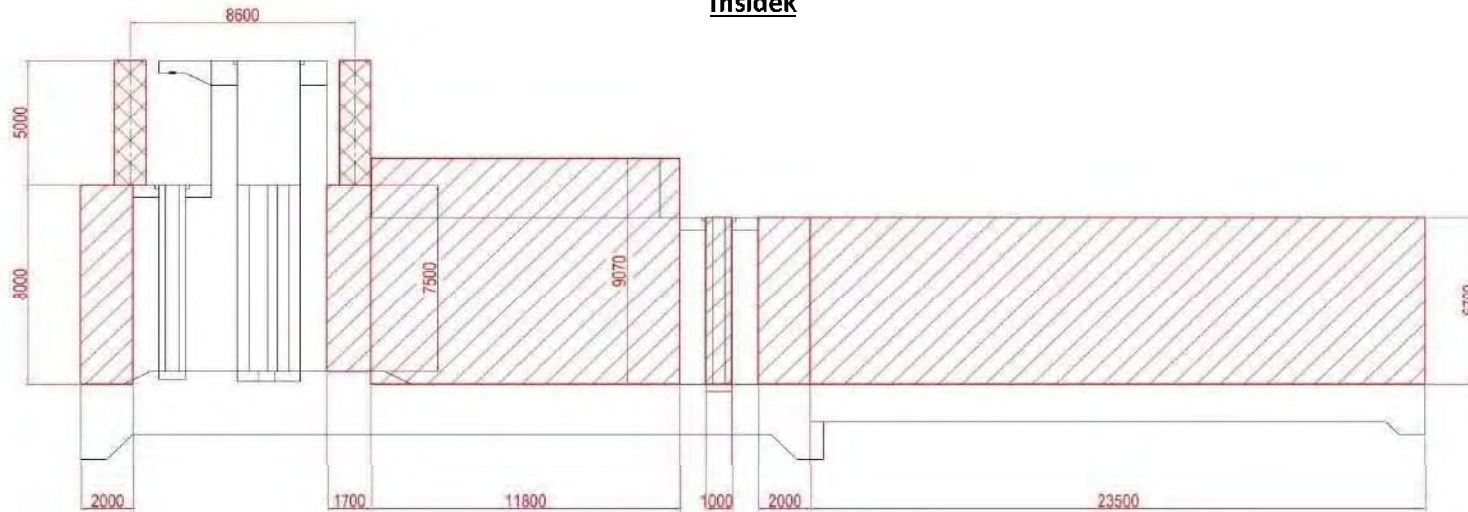


**Scaffolding (2/2)**

**Outer perimeter**



**Insidek**





## (3) Badraman regulator

Badraman Weir body Quantity Summary Sheet						
Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Remarks
Weir body	Main body	Leveling con.	$\sigma_{ck}=20N/mm^2$	$m^3$	17.50	
		formwork	Leveling con.	$m^2$	4.86	
		Reinforced con.	$\sigma_{ck}=25N/mm^2$	$m^3$	770.91	
		Formwork	RC structure	$m^2$	1,146.49	
		Circular formwork	RC structure $\phi 1000-1/2$ Circle	$m^2$	11.15	
		"	RC structure $\phi 1000-1/4$ Circle	$m^2$	11.15	
		Second stage con.	$\sigma_{ck}=21N/mm^2$	$m^3$	10.31	
		Formwork	RC structure	$m^2$	51.74	
		Rebar	D12	t	2.069	
		"	D16	t	31.552	
		"	D22	t	2.400	
		Concrete curing	PC structure	$m^3$	17.50	
		"	RC structure	$m^3$	781.22	
		Scaffolding	Prefabricated scaffold $H \leq 30m$	$m^2$	848.56	
		Falsework	Pipe support falsework. $H < 4.0m$ Support bearing force $40KN/m^2$ and less	$m^3$	45.80	
		"	Wedge linked falsework. $H \leq 30m$ , Support bearing force $40KN/m^2$ and less	$m^3$	56.93	
	Other miscellaneous	Joint bar	D16 $\times$ 200	nos	464	
		Gration	2000 $\times$ 1000	set	4	
	Safety facilities work	Guard fence	H=1.10m	m	41.20	





Badraman Weir body

Quantity calculation

Description	Classification	Calculation	Quantity	Unit	Remarks
Main body					
Leveling con.	$\sigma_{ck}=20\text{N/mm}^2$				
		$7.20 \times (24.20 + 24.40) / 2 \times 0.10$	17.50	$\text{m}^3$	
Fomwork	Leveling con.	$(24.20 + 24.40) / 2 \times 0.10 \times 2$	4.86	$\text{m}^2$	
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 9.00 \times 2$	27.00		
	Invert	$1.00 \times 1.50 \times 2.00 \times 2 \times 2$	12.00		
	"	$7.00 \times 1.50 \times 28.20$	296.10		
	"	$1/2 \times (5.35 + 7.35) \times 0.50 \times 2.00 \times 2$	12.70		
	"	$-0.25 \times 0.70 \times 2.50 \times 3 \times 2$	-2.63		(-)Blockout
	Wall A	$1.00 \times 4.10 \times 1.00$	4.10		
	"	$1.00 \times 4.10 \times 8.45$	34.65		
	"	$1.00 \times 3.00 \times 19.75$	59.25		
	"	$1.00 \times 3.00 \times 1.00$	3.00		
	"	$-(1 - \pi/4) \times 0.50^2 \times (4.10 + 3.00)$	-0.38		
	"	$-0.25 \times 0.70 \times (3.60 \times 2 + 3.00)$	-1.79		(-)Blockout
	Wall B	$1.00 \times 4.10 \times 7.95$	32.60		
	"	$1.00 \times 3.00 \times 15.25$	45.75		
	"	$-(1 - \pi/4) \times 0.50^2 \times 2 \times (4.10 + 3.00)$	-0.76		
	"	$-0.25 \times 0.70 \times (3.60 \times 2 + 3.00) \times 2$	-3.57		(-)Blockout
	Wall C	$1.00 \times 4.10 \times 1.00$	4.10		
	"	$1.00 \times 4.10 \times 8.45$	34.65		
	"	$1.00 \times 3.00 \times 19.75$	59.25		
	"	$1.00 \times 3.00 \times 1.00$	3.00		
	"	$-(1 - \pi/4) \times 0.50^2 \times (4.10 + 3.00)$	-0.38		
	"	$-0.25 \times 0.70 \times (3.60 \times 2 + 3.00)$	-1.79		(-)Blockout

Description	Classification	Calculation	Quantity	Unit	Remarks
	Comidor	$1.00 \times 0.50 \times 2.00 \times 4 \times 2$	8.00		
	"	$-0.15 \times 0.15 \times 2.00 \times 4 \times 2$	-0.36		(-)Second stage con.
	Bridge seat	$1.00 \times 2.95 \times 11.00 \times 3$	97.35		
	Earth pressure wall	$0.45 \times 24.18 \times 2$	21.76		
	Pedestal	$1/2 \times (0.30 + 0.70) \times 0.40 \times 10.00 \times 2$	4.00		
	Gate pier	$1.00 \times 3.15 \times 1.70 \times 3$	16.07		
	"	$-0.25 \times 0.70 \times 3.15 \times 4$	-2.21		(-)Blockout
	Top slab	$2.70 \times 0.50 \times 7.00$	9.45		
		Total	770.91	770.91	m <sup>3</sup>
Fomwork	RC structure				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 2 \times 2$	6.00		Side
	"	$1.00 \times 9.00 \times 2$	18.00		End
	Invert	$(9.00 - 1.00 \times 2) \times 1.50 \times 2$	33.00		End
	"	$28.20 \times 1.50 \times 2$	84.60		Side
	"	$1.118 \times 2.00 \times 2 \times 2$	8.94		Raising part
	"	$(0.70 - 2.50) \times 2 \times 0.25 \times 3 \times 2$	9.60		Blockout part
	Wall A	$1.00 \times 4.10 \times 2$	8.20		
	"	$4.10 \times (0.50 + 8.45 - 7.95)$	69.29		
	"	$3.00 \times (0.50 + 19.75 + 19.25)$	118.50		
	"	$1.00 \times 3.00 \times 3$	9.00		
	"	$3.60 \times 0.25 \times 2 \times 2$	3.60		Blockout part
	"	$3.00 \times 0.25 \times 2$	1.50		"
	"	$-1/2 \times (5.35 + 7.35) \times 0.50$	-3.18		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	-1.91		(-)Comidor
	Wall B	$4.10 \times 7.45 \times 2$	61.09		
	"	$3.00 \times 14.75 \times 2$	88.50		
	"	$3.60 \times 0.25 \times 2 \times 4$	7.20		Blockout part
	"	$3.00 \times 0.25 \times 4$	3.00		"
	"	$-1/2 \times (5.35 + 7.35) \times 0.50 \times 2$	-6.35		(-)Raising part

Description	Classification	Calculation	Quantity	Unit	Remarks
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4 \times 2$	-3.82		(-)Corridor
	Wall C	$1.00 \times 4.10 \times 2$	8.20		
	"	$4.10 \times (0.50 + 8.45 - 7.95)$	69.29		
	"	$3.00 \times (0.50 + 19.75 + 19.25)$	118.50		
	"	$1.00 \times 3.00 \times 3$	9.00		
	"	$3.60 \times 0.25 \times 2 \times 2$	3.60		Blockout part
	"	$3.00 \times 0.25 \times 2$	1.50		"
	"	$-1/2 \times (5.35 + 7.35) \times 0.50$	-3.18		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	-1.91		(-)Corridor
	Comidor	$(1.00 + 0.50 \times 2) \times 2.00 \times 4 \times 2$	32.00		
	Bridge seat	$(11.00 \times 2 + 0.55) \times 2.95 \times 2$	133.05		Abut pier
	"	$0.55 \times 1.85 \times 2$	2.04		"
	"	$-0.70 \times 10.00 \times 2$	-14.00		(-)Pedestal
	"	$(11.00 \times 2 + 1.00) \times 2.95$	67.85		Middle pier
	"	$1.00 \times 1.85$	1.85		"
	Earth pressure wall	$24.18 \times 2 \times 2$	96.72		
	"	$(2.121 + 0.20 \times 4 + 0.15 \times 2 + 5.19) \times 0.45 \times 2$	7.57		
	Pedestal	$(0.30 - 0.566) \times 10.00 \times 2$	17.32		
	"	$1/2 \times (0.30 + 0.70) \times 0.40 \times 2 \times 2$	0.80		
	Gate pier	$(1.70 + 1.00) \times 2 \times 3.15 \times 3$	51.03		
	"	$0.25 \times 3.15 \times 2 \times 4$	6.30		
	Top slab	$(2.70 - 7.00) \times 2 \times 0.50$	9.70		Side
	"	$2.70 \times 7.00$	18.90		Underside
	"	$-(1.00 \times 1.70 - 0.25 \times 0.70) \times 2$	-3.05		(-)Pier
	"	$-(1.00 \times 1.70 - 0.25 \times 0.70 \times 2)$	-1.35		"
		Total	1146.49	m <sup>2</sup>	

Description	Classification	Calculation	Quantity	Unit	Remarks
Circular formwork	RC structure				
	φ1000-1/2Circle				
		$\pi/2 \times 1.00 \times (4.10 + 3.00) = 11.15$	11.15	m <sup>2</sup>	
	φ1000-1/4Circle				
		$\pi/4 \times 1.00 \times (4.10 + 3.00) \times 2 = 11.15$	11.15	m <sup>2</sup>	
Second stage con.	σck=25N/mm <sup>2</sup>				
		$0.70 \times 2.50 \times 0.25 \times 3 \times 2 = 2.63$			Invert part
		$0.70 \times 0.25 \times (3.60 + 3.00) \times 4 = 4.62$			Stop log part
		$-0.22 \times 0.13 \times (3.60 + 3.00) \times 4 = -0.76$			"
		$(0.70 \times 0.25 - 0.185 \times 0.13) \times 3.60 \times 4 = 2.17$			Gate part
		$\{1/2 \times (0.315 + 0.435) \times 0.12 + 0.265 \times 0.25\} \times 2.70 \times 4 = 1.20$			Gate pier part
		$(0.15^2 - 0.05^2) \times 2.00 \times 4 \times 2 = 0.32$			Corridor part
		$0.05/6 \times \{(2 \times 0.81 + 0.71) \times 1.72 + (2 \times 0.71 + 0.81) \times 1.62\} \times 2 = 0.13$			Top slab part
		Total = 10.31	10.31	m <sup>3</sup>	
Fomwork	RC structure				
		$(0.70 - 0.13 \times 2) \times 3.60 \times 2 \times 2 = 13.82$			Stop log part
		$(0.70 - 0.13 \times 2) \times 3.00 \times 2 \times 2 = 11.52$			"
		$(0.70 - 0.13 \times 2) \times 3.60 \times 2 \times 2 = 13.82$			Gate part
		$(0.265 + 0.13 + 0.315 + 0.12 \times \sqrt{(2)}) \times 2.70 \times 4 = 9.50$			Gate pier part
		$0.15 \times 2.00 \times 4 \times 2 = 2.40$			Corridor part
		$1/2 \times (0.71 + 0.81) \times 0.05 \times \sqrt{(2)} \times 2 \times 2 = 0.21$			Top slab part
		$1/2 \times (1.62 + 1.72) \times 0.05 \times \sqrt{(2)} \times 2 \times 2 = 0.47$			"
		Total = 51.74	51.74	m <sup>2</sup>	

Description	Classification	Calculation	Quantity	Unit	Remarks
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 = 2.069	2.069	t	
		D16 = 31.552	31.552	t	
		D22 = 2.400	2.400	t	
Concrete curing	PC structure	Leveling concrete = 17.50	17.50	m <sup>3</sup>	
	RC structure	770.91   10.31 = 781.22	781.22	m <sup>3</sup>	
Scaffolding	Prefabricated scaffold, H ≤ 30m				
	外周	$5.60 \times \{11.20 + (9.00 + 1.00 + 0.40) \times 2\} = 179.20$			
	"	$8.17 \times (16.47 + 0.40) \times 2 = 275.66$			
	"	$4.50 \times (4.93 + 1.00 - 3.10) \times 2 = 81.27$			
	内部	$4.10 \times (1.00 + 1.10) \times 2 = 17.22$			
	"	$3.60 \times (1.00 + 1.15) \times 2 = 15.48$			
	"	$5.95 \times 11.00 \times 2 = 130.90$			
	"	$3.00 \times \{(0.75 + 1.00 + 5.00) \times 2 + 1.80\} = 45.90$			
	門柱部	$3.65 \times (4.90 + 9.20) \times 2 = 102.93$			
		Total = 848.56	848.56	m <sup>2</sup>	
Falsework	Pipe support falsework, H < 4.0m, Support bearing force 40KN/m <sup>2</sup> and less				
	①	$1.00 \times (3.60 + 3.10) / 2 \times 2.00 \times 2 = 13.40$			Corridor part
	②	$1.00 \times 3.10 \times 2.00 \times 2 = 12.40$			"
	③	$1.00 \times 2.50 \times 2.00 \times 2 \times 2 = 20.00$			"
		Total = 45.80	45.80	m <sup>3</sup>	



Description	Classification	Calculation	Quantity	Unit	Remarks
Approach cushion slab					
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$				
		$10.00 \times 5.00 \times 0.40 \times 2 = 40.00$	40.00	$\text{m}^3$	
Fomwork	RC structure				
		$(10.00 + 5.00) \times 0.40 \times 2 \times 2 = 24.00$	24.00	$\text{m}^2$	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 $0.075 \times 2 = 0.150$	0.150	t	
		D16 $1.185 \times 2 = 2.370$	2.370	t	
		D18 $0.520 \times 2 = 1.040$	1.040	t	
Concrete curing	RC structure	Reinforced con. = 40.00	40.00	$\text{m}^3$	
Anchor cap	SGP $\phi 40 \times 180$				
		$25 \times 2 = 50.0$	50	nos	
Anchor bar	D22 $\times 500$ (including D10 spiral reinforcement)				
		$25 \times 2 = 50.0$	50	nos	
Rubber shoe	t=30mm				
		$10.00 \times 0.30 \times 2 = 6.00$	6.00	$\text{m}^2$	
Expansion joint	Elastic filler,t=20mm				
		$0.43 \times 10.00 \times 2 = 8.60$	8.60	$\text{m}^2$	





Badraman Reg. Rebar weight [1/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F1	D 16	A		6.460 <sup>x1</sup>	5.230 <sup>x1</sup>	5.230 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	18.46	43	793.78			
F2	D 16	A		6.460 <sup>x1</sup>	7.080 <sup>x1</sup>	7.080 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	22.16	55	1218.80			
F3	D 16	A		6.460 <sup>x1</sup>	4.130 <sup>x1</sup>	4.130 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	16.26	44	715.44			
F4	D 16	C		6.800 <sup>x1</sup>							6.80	148	1006.40			
F5	D 16	C		28.000 <sup>x1</sup>						1.000 <sup>x2</sup>	30.00	35	1050.00			
F6	D 16	C		21.150 <sup>x1</sup>						1.000 <sup>x1</sup>	22.15	4	88.60			
F7	D 16	C		5.950 <sup>x1</sup>							5.95	7	41.65			
F8	D 16	C		1.900 <sup>x1</sup>							1.90	23	43.70			
F9	D 16	G		1.866 <sup>x1</sup>	1.137 <sup>x1</sup>						3.01	20	60.20			
F10	D 16	D		1.418 <sup>x1</sup>	1.134 <sup>x2</sup>						3.69	20	73.80			
F11	D 16	H		1.134 <sup>x1</sup>	2.007 <sup>x1</sup>	1.859 <sup>x1</sup>					5.00	20	100.00			
F12	D 16	E		13.884 <sup>x1</sup>	1.134 <sup>x1</sup>					1.000 <sup>x1</sup>	16.02	20	320.40			
F13	D 16	E		5.934 <sup>x1</sup>	1.134 <sup>x1</sup>						7.07	20	141.40			
F14	D 16	C		5.050 <sup>x1</sup>							5.05	12	60.60			
F15	D 16	C		4.726 <sup>x1</sup>							4.73	20	94.60			
F16	D 16	C		2.700 <sup>x1</sup>							2.70	32	86.40			
F17	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	58	101.50			
F18	D 16	D		6.800 <sup>x1</sup>	0.240 <sup>x2</sup>						7.28	12	87.36			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-121

Badraman Reg. Rebar weight [2/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F19	D 16	C		28.000 <sup>x1</sup>						1.000 <sup>x2</sup>	30.00	12	360.00			
F20	D 16	C		1.900 <sup>x1</sup>							1.90	104	197.60			
F21	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	16	28.00			
F22	D 16	C		1.800 <sup>x1</sup>							1.80	24	43.20			
F23	D 16	C		1.000 <sup>x1</sup>							1.00	16	16.00			
F24	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	20	35.00			
F25	D 16	C		1.900 <sup>x1</sup>							1.90	90	171.00			
F26	D 16	C		2.273 <sup>x1</sup>							2.28	70	159.60			
F27	D 16	I		1.331 <sup>x1</sup>	1.000 <sup>x1</sup>						2.34	20	46.80			
F28	D 16	C		8.800 <sup>x1</sup>							8.80	20	176.00			
F29	D 12	M		0.428 <sup>x1</sup>	1.328 <sup>x1</sup>	1.328 <sup>x1</sup>	0.100 <sup>x2</sup>				3.29	6	19.74			
F30	D 12	M		0.428 <sup>x1</sup>	1.468 <sup>x1</sup> ~1.575	1.561 <sup>x1</sup> ~1.668	0.100 <sup>x2</sup>				3.77	20	75.40			
F31	D 12	M		0.428 <sup>x1</sup>	1.578 <sup>x1</sup>	1.578 <sup>x1</sup>	0.100 <sup>x2</sup>				3.79	10	37.90			
F32	D 12	M		0.428 <sup>x1</sup>	1.828 <sup>x1</sup>	1.828 <sup>x1</sup>	0.100 <sup>x2</sup>				4.29	30	128.70			
F33	D 12	M		0.428 <sup>x1</sup>	1.328 <sup>x1</sup>	1.328 <sup>x1</sup>	0.100 <sup>x2</sup>				3.29	239	786.31			
F34	D 12	M		0.428 <sup>x1</sup>	1.078 <sup>x1</sup>	1.078 <sup>x1</sup>	0.100 <sup>x2</sup>				2.79	6	16.74			
W1	D 16	C		5.400 <sup>x1</sup>							5.40	108	583.20			
W2	D 16	C		7.250 <sup>x1</sup>							7.25	110	797.50			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-122

Badraman Reg. Rebar weight [3/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W3	D 16	C		4.300 <sup>x1</sup>							1.30	102	438.60			
W4	D 16	E		5.400 <sup>x1</sup>	0.240 <sup>x1</sup>						5.64	100	564.00			
W5	D 16	E		7.250 <sup>x1</sup>	0.240 <sup>x1</sup>						7.49	110	823.90			
W6	D 16	E		4.300 <sup>x1</sup>	0.240 <sup>x1</sup>						4.54	54	245.16			
W7	D 16	C		3.850 <sup>x1</sup>							3.85	18	69.30			
W8	D 16	C		5.400 <sup>x1</sup>							5.40	20	108.00			
W9	D 16	C		4.300 <sup>x1</sup>							4.30	20	86.00			
W10	D 16	D		0.800 <sup>x1</sup>	1.000 <sup>x2</sup>						2.80	220	616.00			
W11	D 16	D		0.550 <sup>x1</sup>	1.000 <sup>x2</sup>						2.55	20	51.00			
W12	D 16	D		0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	20	25.60			
W13	D 16	D		0.747 <sup>x1</sup>	1.000 <sup>x2</sup>						2.75	4	11.00			
W14	D 16	D		0.800 <sup>x1</sup>	1.000 <sup>x2</sup>						2.80	92	257.60			
W15	D 16	D		0.300 <sup>x1</sup>	1.000 <sup>x2</sup>						2.30	10	23.00			
W16	D 16	D		0.693 <sup>x1</sup>	1.000 <sup>x2</sup>						2.70	2	5.40			
W17	D 16	C		28.000 <sup>x1</sup>						1.000 <sup>x2</sup>	30.00	30	900.00			
W18	D 16	C		8.250 <sup>x1</sup>							8.25	12	99.00			
W19	D 16	J		1.400 <sup>x1</sup>	1.534 <sup>x1</sup>	0.768 <sup>x1</sup>	0.603 <sup>x1</sup>			φ 384	4.31	42	181.02			
W20	D 16	D		1.418 <sup>x1</sup>	0.768 <sup>x2</sup>						2.96	42	124.32			
Type	A	B	C	D	E	F	G	H	I	J	K	Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-123

Badraman Reg. Rebar weight [4/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W21	D 16	D		13.600 <sup>x1</sup>	0.768 <sup>x2</sup>					1.000 <sup>x1</sup>	16.11	30	484.20			
W22	D 16	D		3.018 <sup>x1</sup>	0.768 <sup>x2</sup>						4.56	12	54.72			
W23	D 16	J		1.400 <sup>x1</sup>	5.534 <sup>x1</sup>	0.768 <sup>x1</sup>	0.603 <sup>x1</sup>		φ 384		8.31	30	249.30			
W24	D 16	C		5.050 <sup>x1</sup>							5.05	40	202.00			
W25	D 16	C		2.700 <sup>x1</sup>							2.70	34	91.80			
W26	D 16	C		10.800 <sup>x1</sup>							10.80	60	648.00			
W27	D 16	D		0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	60	75.00			
W28	D 16	C		1.800 <sup>x1</sup>							1.80	80	144.00			
W29	D 16	C		1.746 <sup>x1</sup>							1.75	4	7.00			
W30	D 16	K		1.050 <sup>x2</sup>	1.206 <sup>x1</sup>				φ 381		3.31	21	69.51			
W31	D 16	C		1.450 <sup>x1</sup>							1.45	42	60.90			
W32	D 16	C		15.950 <sup>x1</sup>						1.000 <sup>x1</sup>	16.95	30	508.50			
W33	D 16	C		3.050 <sup>x1</sup>							3.05	12	36.60			
W34	D 16	K		1.050 <sup>x2</sup>	1.206 <sup>x1</sup>				φ 381		3.31	15	49.65			
W35	D 16	C		5.050 <sup>x1</sup>							5.05	40	202.00			
W36	D 16	C		2.700 <sup>x1</sup>							2.70	34	91.80			
W37	D 16	C		10.800 <sup>x1</sup>							10.80	30	324.00			
W38	D 16	D		0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	150	187.50			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-124

Badraman Reg. Rebar weight [5/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W39	D 16	C	—	1.896 <sup>x1</sup>							1.90	2	3.80			
W40	D 16	C	—	4.300 <sup>x1</sup>							4.30	4	17.20			
W41	D 16	C	—	1.450 <sup>x1</sup>							1.45	2	2.90			
W42	D 16	C	—	4.150 <sup>x1</sup>							4.15	6	24.90			
W13	D 16	C	—	10.800 <sup>x1</sup>							10.80	6	64.80			
W14	D 16	C	—	2.800 <sup>x1</sup>							2.80	2	5.60			
W45	D 16	C	—	5.896 <sup>x1</sup>							5.90	2	11.80			
W46	D 16	C	—	9.650 <sup>x1</sup>							9.65	4	38.60			
W47	D 16	C	—	1.396 <sup>x1</sup>							1.40	2	2.80			
W18	D 16	C	—	3.800 <sup>x1</sup>							3.80	1	3.80			
W19	D 16	C	—	1.450 <sup>x1</sup>							1.45	2	2.90			
W50	D 16	C	—	4.150 <sup>x1</sup>							4.15	3	12.45			
W51	D 16	C	—	10.800 <sup>x1</sup>							10.80	3	32.40			
W52	D 16	C	—	2.800 <sup>x1</sup>							2.80	2	5.60			
W53	D 16	C	—	5.150 <sup>x1</sup>							5.15	1	5.15			
W54	D 16	C	—	1.396 <sup>x1</sup>							1.40	2	2.80			
W55	D 12	D	└┘	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	652	671.56			
W56	D 12	D	└┘	0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	50	39.00			
Type	A	B	C	D	E	F	G	H	I	J	K	Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-125

Badraman Reg. Rebar weight [6/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W57	D 12	D		0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	31	35.02			
W58	D 12	D		0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	293	301.79			
W59	D 12	D		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	25	13.25			
P1	D 22	C		1.470 <sup>x1</sup>							1.47	204	299.88			
P2	D 22	C		1.620 <sup>x1</sup>							1.62	16	25.92			
P3	D 22	C		2.029 <sup>x1</sup> ~3.429							2.73	32	87.36			
P4	D 22	C		3.470 <sup>x1</sup>							3.47	24	83.28			
P5	D 22	C		4.570 <sup>x1</sup>							4.57	24	109.68			
P6	D 22	C		4.529 <sup>x1</sup> ~1.329							2.93	68	199.24			
P7	D 12	D		0.250 <sup>x1</sup>	0.180 <sup>x2</sup>						0.61	181	112.24			
P8	D 16	C		13.717 <sup>x1</sup> ~14.457						1.000 <sup>x1</sup>	15.09	12	181.08			
P9	D 16	C		2.929 <sup>x1</sup> ~3.479							3.21	16	51.36			
P10	D 16	C		3.650 <sup>x1</sup>							3.65	20	73.00			
P11	D 16	C		2.929 <sup>x1</sup> ~5.679							4.31	60	258.60			
P12	D 16	H		1.050 <sup>x1</sup>	1.609 <sup>x1</sup>	2.038 <sup>x1</sup>					4.70	4	18.80			
P13	D 16	H		1.050 <sup>x1</sup>	1.609 <sup>x1</sup>	6.049 <sup>x1</sup>					8.71	4	34.84			
P14	D 16	D		0.218 <sup>x1</sup>	0.240 <sup>x2</sup>						0.70	10	7.00			
P15	D 12	D		0.278 <sup>x1</sup>	0.100 <sup>x2</sup>						0.48	118	56.64			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	

Badraman Reg. Rebar weight [7/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
S1	D 16	C		6.800 <sup>x1</sup>							6.80	41	299.20			
S2	D 16	C		0.800 <sup>x1</sup>							0.80	160	128.00			
S3	D 16	D		0.618 <sup>x1</sup>	0.268 <sup>x2</sup>						1.16	80	92.80			
C1	D 16	L		0.240 <sup>x1</sup>	0.650 <sup>x1</sup>	0.159 <sup>x1</sup>	0.919 <sup>x1</sup>				1.97	102	200.94			
C2	D 16	C		9.800 <sup>x1</sup>							9.80	8	78.40			
G1	D 16	A		8.460 <sup>x1</sup>	1.230 <sup>x1</sup>	1.630 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170		11.86	8	94.88			
G2	D 16	B		1.024 <sup>x1</sup>	1.230 <sup>x1</sup> ~1.630	0.267 <sup>x1</sup>					2.73	2	5.46			
G3	D 16	B		1.053 <sup>x1</sup>	1.630 <sup>x1</sup> ~1.230	0.267 <sup>x1</sup>					2.75	2	5.50			
G4	D 16	C		0.789 <sup>x1</sup> ~0.845							0.82	4	3.28			
G5	D 16	C		1.589 <sup>x1</sup> ~1.645							1.62	2	3.24			
G6	D 16	C		1.550 <sup>x1</sup>							1.55	82	373.10			
G7	D 16	F		0.332 <sup>x3</sup>	0.832 <sup>x2</sup>	0.225 <sup>x2</sup>					2.78	96	266.88			
G8	D 16	F		0.582 <sup>x2</sup>	1.532 <sup>x2</sup>	0.225 <sup>x2</sup>					4.68	32	149.76			
G9	D 16	F		0.332 <sup>x2</sup>	1.532 <sup>x2</sup>	0.225 <sup>x2</sup>					1.18	16	66.88			
G10	D 12	D		0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	16	12.48			
G11	D 12	D		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	8	4.24			
T1	D 16	D		6.800 <sup>x1</sup>	0.300 <sup>x2</sup>						7.40	6	44.40			
T2	D 16	D		6.800 <sup>x1</sup>	0.240 <sup>x2</sup>						7.28	14	101.92			
Type	<div style="display: flex; justify-content: space-around; text-align: center;"> <span>A</span><span>B</span><span>C</span><span>D</span><span>E</span><span>F</span><span>G</span><span>H</span><span>I</span><span>J</span><span>K</span> </div>											Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	

C-127

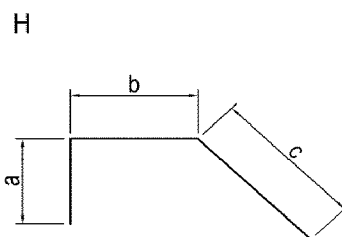
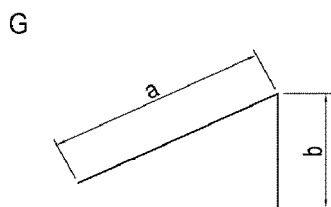
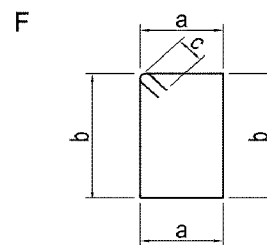
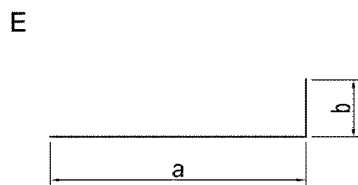
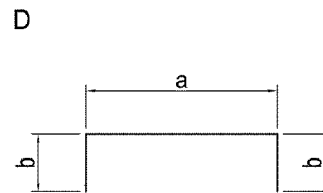
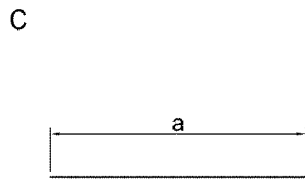
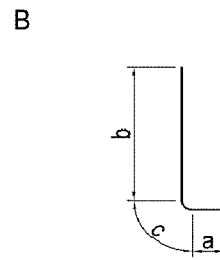
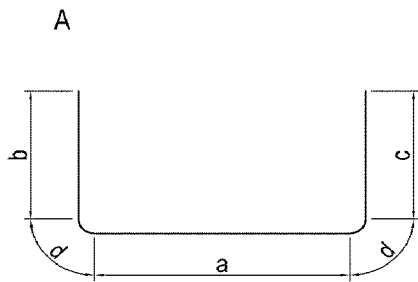
Badraman Reg. Rebar weight [8/8]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
T3	D 16	E		1.94 <sup>x1</sup> ~1.223	0.240 <sup>x1</sup>						1.45	1	5.80			
T4	D 16	C		0.789 <sup>x1</sup> ~0.845							0.82	4	3.28			
T5	D 16	C		1.589 <sup>x1</sup> ~1.645							1.62	2	3.24			
T6	D 16	D		2.532 <sup>x1</sup>	0.332 <sup>x2</sup>						3.20	31	99.20			
T7	D 16	D		2.532 <sup>x1</sup>	0.240 <sup>x2</sup>						3.02	31	93.62			
T8	D 16	E		1.078 <sup>x1</sup> ~1.156	0.332 <sup>x1</sup>						1.45	8	11.60			
T9	D 16	E		1.96 <sup>x1</sup> ~1.118	0.332 <sup>x1</sup>						1.49	8	11.92			
T10	D 16	E		1.078 <sup>x1</sup> ~1.156	0.240 <sup>x1</sup>						1.36	8	10.88			
T11	D 16	E		1.96 <sup>x1</sup> ~1.118	0.240 <sup>x1</sup>						1.40	8	11.20			
T12	D 12	D		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	36	19.08			
T13	D 16	C		6.250 <sup>x1</sup>							6.25	1	25.00			
T14	D 16	C		2.150 <sup>x1</sup>							2.15	48	103.20			
				D12		2330.09 × 0.888		=	2069.12 kg		=	2.069 t				
				D16		19969.67 × 1.58		=	31552.08 kg		=	31.552 t				
				D22		805.36 × 2.98		=	2399.97 kg		=	2.400 t				
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

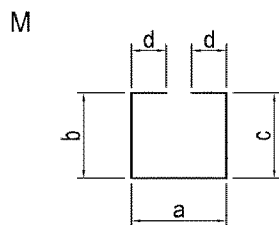
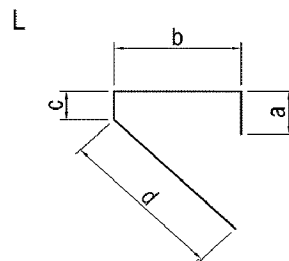
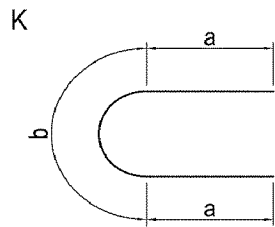
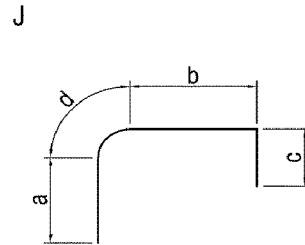
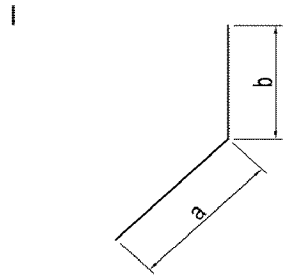
C-128



# Schematic design of rebar (1/2)



## Schematic design of rebar (2/2)



Badraman Reg. (Approach cushion slab)

Rebar weight [1/1]

per a place

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
F1	D 18	B	┌	4.800 <sup>x1</sup>	0.200 <sup>x2</sup>						5.20	50	260.00	
F2	D 16	B	┌	4.800 <sup>x1</sup>	0.200 <sup>x2</sup>						5.20	50	260.00	
F3	D 16	A	—	9.800 <sup>x1</sup>							9.80	25	245.00	
F4	D 16	A	—	9.800 <sup>x1</sup>							9.80	25	245.00	
F5	D 12	B	┐	0.229 <sup>x1</sup>	0.180 <sup>x2</sup>						0.59	144	84.96	
				D12	84.96 × 0.888		=	75.44 kg	=	0.075 t				
				D16	750.00 × 1.58		=	1185.00 kg	=	1.185 t				
				D18	260.00 × 2.00		=	520.00 kg	=	0.520 t				
Type	A          B		┌   ┐											
	Dia.	kg/m		R	L	J								
	D12	0.888		130	204	1000								
	D14	1.21		150	236	1000								
	D16	1.58		170	267	1000								
	D18	2.00		190	298	1000								
	D20	2.47		210	330	1100								
	D22	2.98		240	377	1210								
	D25	3.85		270	424	1375								
	D28	4.83		300	471	1540								
D32	6.31	340	534	1760										

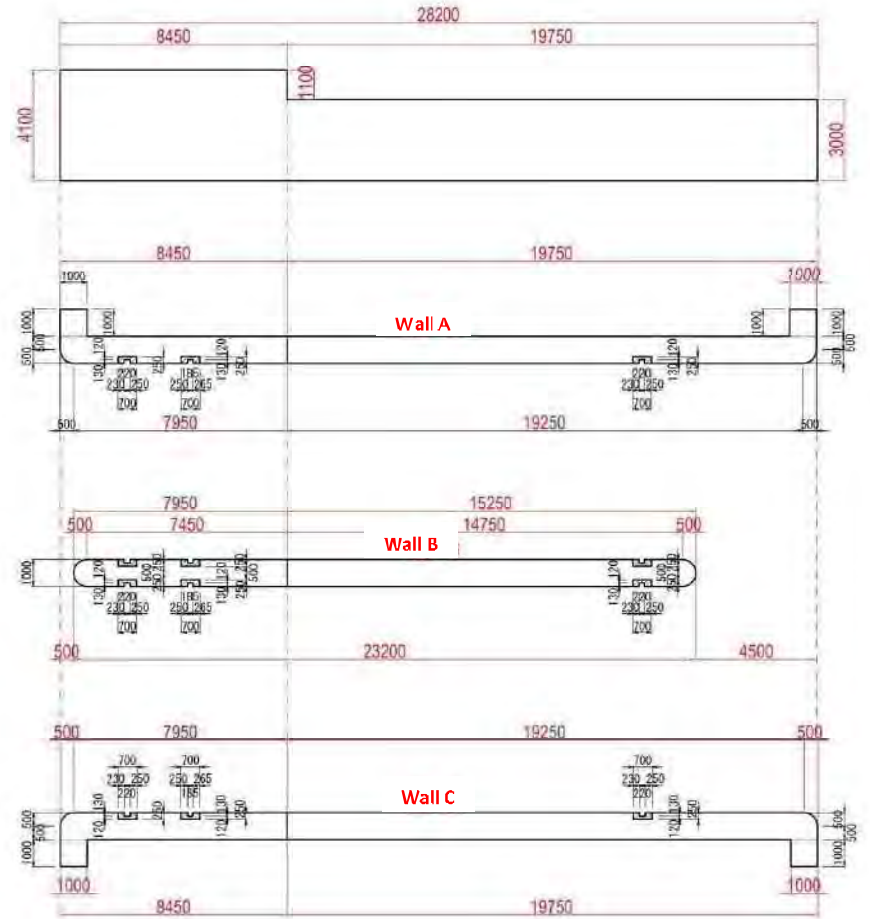
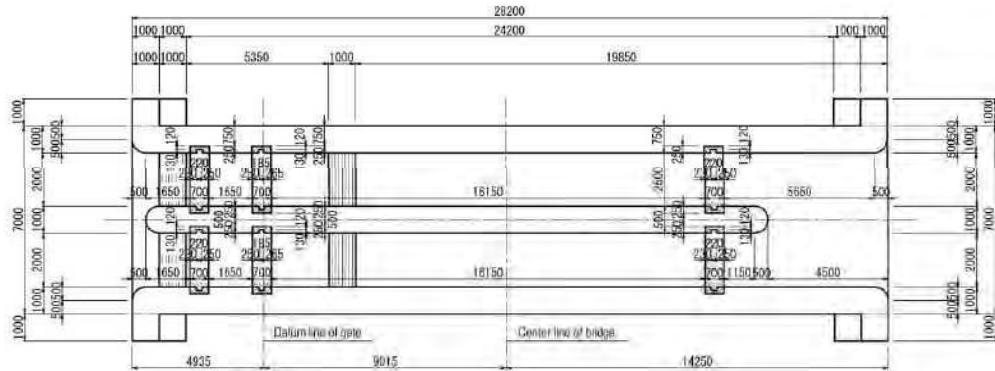
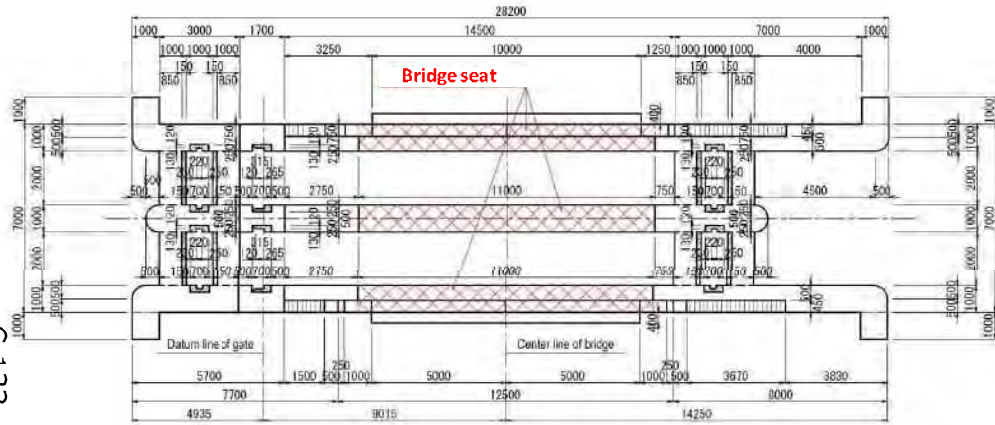
C-131

Schematic design of rebar  
(Approach cushion slab)

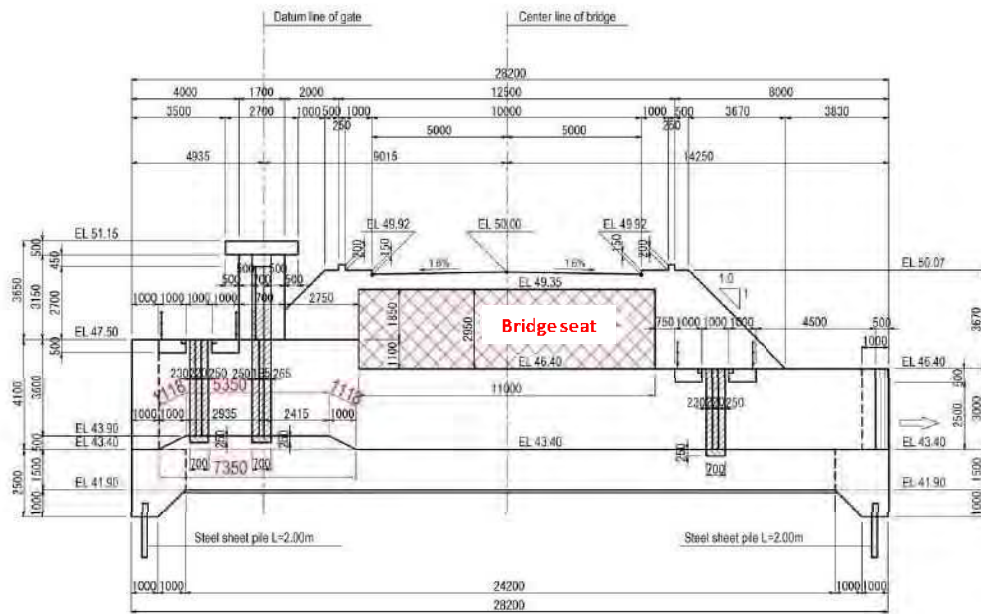


# Concrete / Formwork (1/2)

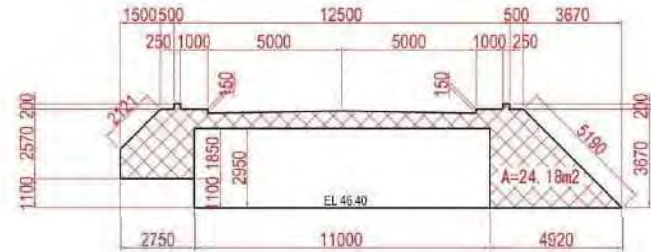
C-133



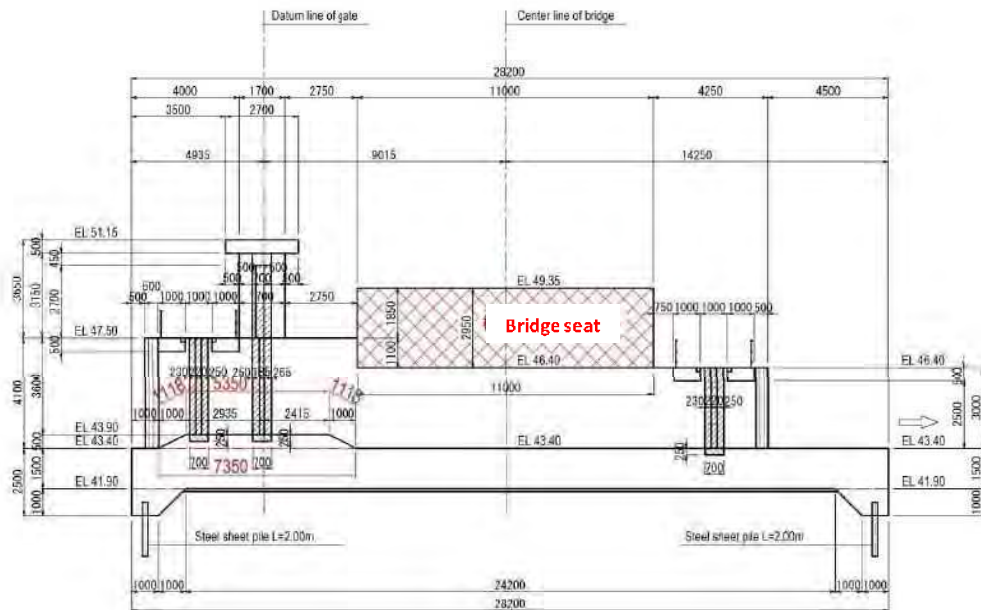
**Concrete / Formwork (2/2)**



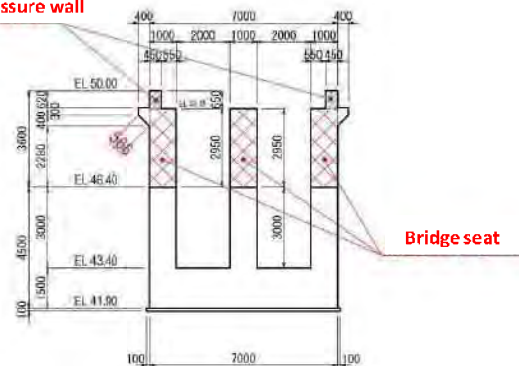
**Earth pressure wall (t=450)**



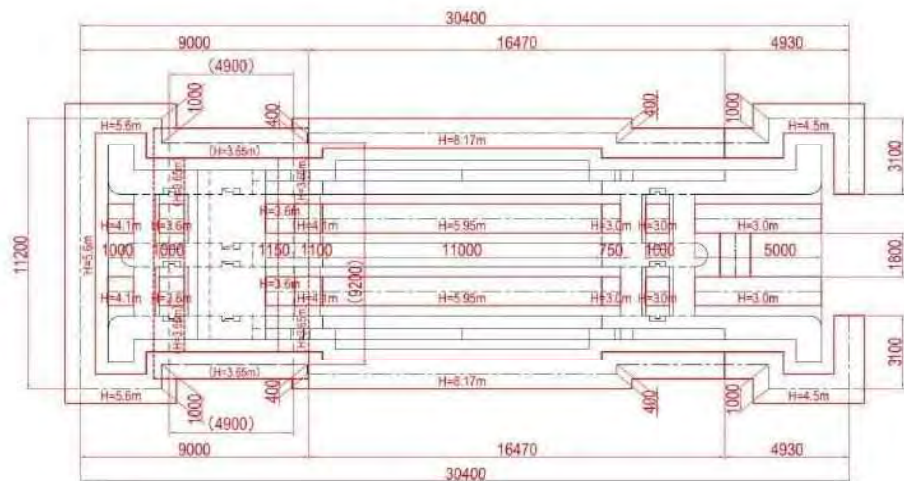
C-134



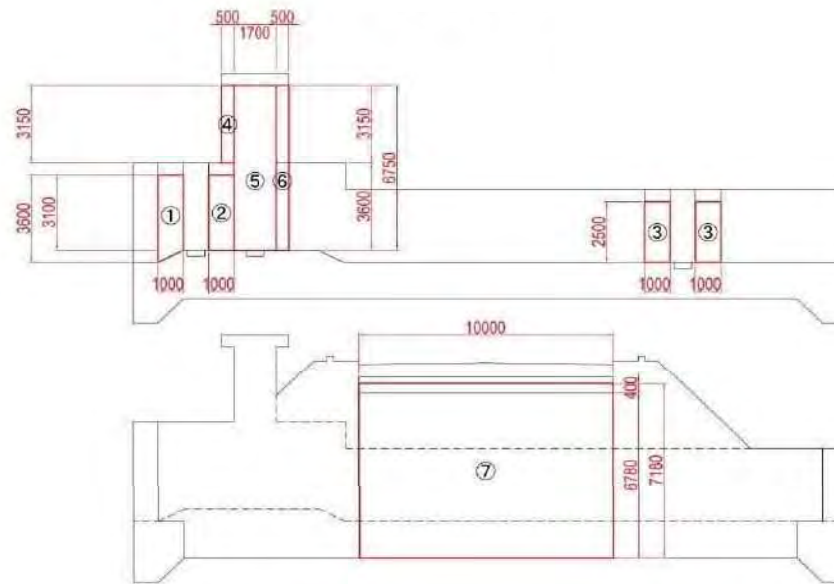
**Earth pressure wall**



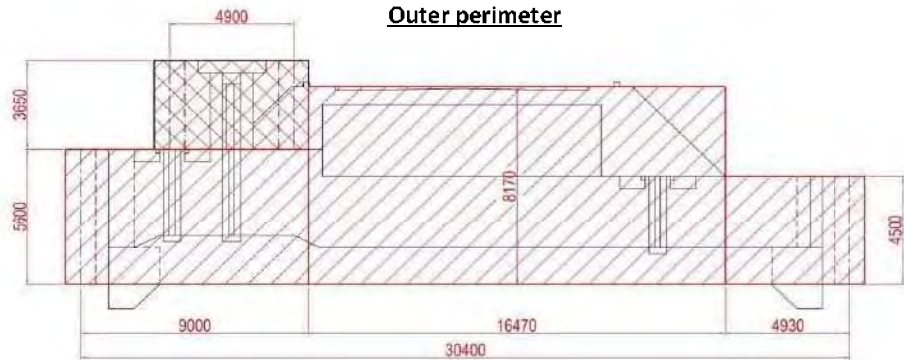
## Scaffolding



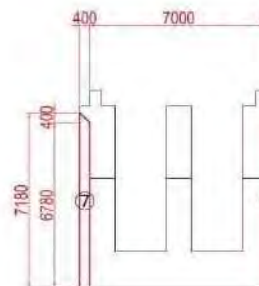
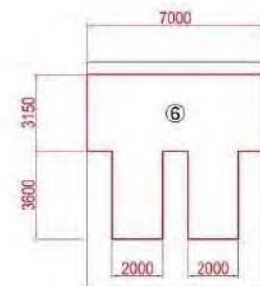
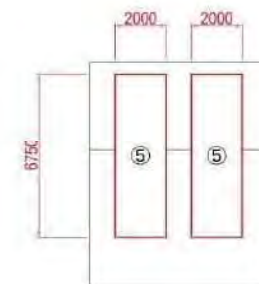
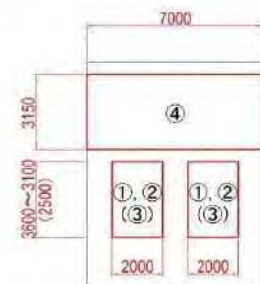
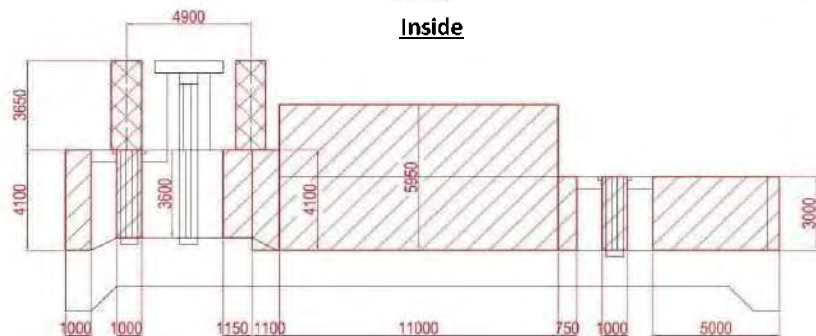
## Falsework



## Outer perimeter



## Inside



C-135

## (4) Diroutiah regulator

Diroutiah Weir body		Quantity Summary Sheet				
Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Remarks
Weir body	Main body	Leveling con	$\sigma_{ck}=20N/mm^2$	$m^3$	24.28	
		formwork	Leveling con.	$m^2$	4.76	
		Reinforced con.	$\sigma_{ck}=25N/mm^2$	$m^3$	987.61	
		Fomwork	RC structure	$m^2$	1,350.14	
		Circular formwork	RC structure $\phi 1000-1/2$ Circle	$m^2$	20.42	
		"	RC structure $\phi 1000-1/4$ Circle	$m^2$	10.21	
		Seco nd stage con.	$\sigma_{ck}=25N/mm2$	$m^3$	14.47	
		Fomwork	RC structure	$m^2$	70.87	
		Rebar	D12	t	2.686	
		"	D16	t	40.722	
		"	D22	t	2.424	
		Concrete curing	PC structure	$m^3$	24.28	
		"	RC structure	$m^3$	1,002.08	
		Scaffolding	Prefabricated scaffold $H \leq 30m$	$m^2$	933.33	
		Falsework	Pipe support falsework. $H < 4.0m$ Support beaing force $40KN/m^2$ and less	$m^3$	61.50	
		"	Wedge linked falsework. $H \leq 30m$ , Support beaing force $40KN/m^2$ and less	$m^3$	76.98	
	Other miscellaneous	Joint bar	D16 $\times$ 200	nos	464	
		Gration	2000 $\times$ 1000	set	6	
	Safty facillities work	Guard fense	H=1.10m	m	59.20	





Diroutiah Weir body						Quantity calculation					
Description	Classification	Calculation				Quantity	Unit	Remarks			
Main body											
Leveling con.	$\sigma_{ck}=20\text{N/mm}^2$										
		$10.20 \times (23.70 - 23.90) / 2 \times 0.10$	—	21.28	24.28	$\text{m}^3$					
Fomwork	Leveling con.	$(23.70 + 23.90) \times 2 \times 0.10 \times 2$	=	4.76	4.76	$\text{m}^2$					
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$										
	Cut-off	$1/2 \times (1.00 - 2.00) \times 1.00 \times 12.00 \times 2$	—	36.00							
	Invert	$1.00 \times 1.50 \times 2.00 \times 2 \times 2$	=	12.00							
	"	$10.00 \times 1.50 \times 27.70$	=	115.50							
	"	$1/2 \times (5.35 - 7.35) \times 0.50 \times 2.00 \times 3$	—	19.05							
	"	$-0.25 \times 0.70 \times 2.50 \times 3 \times 3$	=	-3.94			(-)Blockout				
	Wall A	$1.00 \times 3.80 \times 1.00$	=	3.80							
	"	$1.00 \times 3.80 \times 8.45$	=	32.11							
	"	$1.00 \times 2.70 \times 19.25$	—	51.98							
	"	$1.00 \times 2.70 \times 1.00$	—	2.70							
	"	$-(1 - \pi / 4) \times 0.50^2 \times (3.80 + 2.70)$	=	-0.35							
	"	$-0.25 \times 0.70 \times (3.30 \times 2 + 2.70)$	—	-1.63			(-)Blockout				
	Wall B	$1.00 \times 3.80 \times 7.95 \times 2$	=	60.42							
	"	$1.00 \times 2.70 \times 15.25 \times 2$	=	82.35							
	"	$-(1 - \pi / 4) \times 0.50^2 \times 2 \times (3.80 - 2.70) \times 2$	—	-1.39							
	"	$-0.25 \times 0.70 \times (3.30 \times 2 + 2.70) \times 2 \times 2$	—	-6.51			(-)Blockout				
	Wall C	$1.00 \times 3.80 \times 1.00$	—	3.80							
	"	$1.00 \times 3.80 \times 8.45$	=	32.11							
	"	$1.00 \times 2.70 \times 19.25$	—	51.98							
	"	$1.00 \times 2.70 \times 1.00$	—	2.70							
	"	$-(1 - \pi / 4) \times 0.50^2 \times (3.80 + 2.70)$	=	-0.35							
	"	$-0.25 \times 0.70 \times (3.30 \times 2 + 2.70)$	—	-1.63			(-)Blockout				

Description	Classification	Calculation	Quantity	Unit	Remarks
	Corridor	$1.00 \times 0.50 \times 2.00 \times 1 \times 3$	= 12.00		
	"	$-0.15 \times 0.15 \times 2.00 \times 4 \times 3$	= -0.51		(-)Secor:d stage con.
	Bridge seat	$1.00 \times 2.95 \times 11.00 \times 4$	= 129.80		
	Earth pressure wall	$0.45 \times 24.18 \times 2$	= 21.76		
	Pedestal	$1/2 \times (0.30 + 0.70) \times 0.40 \times 10.00 \times 2$	= 4.00		
	Gate pier	$1.00 \times 2.85 \times 1.70 \times 1$	= 19.38		
	"	$-0.25 \times 0.70 \times 2.85 \times 6$	= -2.99		(-)Blockout
	Top slab	$2.70 \times 0.50 \times 10.00$	= 13.50		
		Total =	987.61	987.61	m <sup>3</sup>
<b>Fomwork</b>	<b>RC structure</b>				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 2 \times 2$	= 6.00		Side
	"	$1.00 \times 12.00 \times 2$	= 24.00		End
	Invert	$(12.00 + 1.00 \times 2) \times 1.50 \times 2$	= 42.00		End
	"	$27.700 \times 1.50 \times 2$	= 83.10		Side
	"	$1.118 \times 2.00 \times 2 \times 3$	= 13.42		Raising part
	"	$(0.70 + 2.50) \times 2 \times 0.25 \times 3 \times 3$	= 11.40		Blockout part
	Wall A	$1.00 \times 3.80 \times 2$	= 7.60		
	"	$3.80 \times (0.50 + 8.45 + 7.95)$	= 64.22		
	"	$2.70 \times (0.50 + 19.25 + 18.75)$	= 103.95		
	"	$1.00 \times 2.70 \times 2$	= 5.40		
	"	$3.30 \times 0.25 \times 2 \times 2$	= 3.30		Blockout part
	"	$2.70 \times 0.25 \times 2$	= 1.35		"
	"	$-1/2 \times (5.35 + 7.35) \times 0.50$	= -3.18		(-)Raising part
	"	$-(1.00 \times 0.50 + 0.15 \times 0.15) \times 4$	= -1.91		(-)Corridor part
	Wall B	$3.80 \times 7.15 \times 2 \times 2$	= 113.21		
	"	$2.70 \times 14.75 \times 2 \times 2$	= 159.30		
	"	$3.30 \times 0.25 \times 2 \times 4 \times 2$	= 13.20		Blockout part
	"	$2.70 \times 0.25 \times 4 \times 2$	= 5.40		"
	"	$-1/2 \times (5.35 + 7.35) \times 0.50 \times 2 \times 2$	= -12.70		(-)Raising part

Description	Classification	Calculation	Quantity	Unit	Remarks
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4 \times 2 \times 2$	= -7.61		(-)Corridor part
	Wall C	$1.00 \times 3.80 \times 2$	= 7.60		
	"	$3.80 \times (0.50 + 8.45 + 7.95)$	= 64.22		
	"	$2.70 \times (0.50 + 19.25 + 18.75)$	= 103.95		
	"	$1.00 \times 2.70 \times 2$	= 5.40		
	"	$3.30 \times 0.25 \times 2 \times 2$	= 3.30		Blockout part
	"	$2.70 \times 0.25 \times 2$	= 1.35		"
	"	$-1.2 \times (5.35 + 7.35) \times 0.50$	= -3.18		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4$	= -1.91		(-)Corridor part
	Corridor	$(1.00 - 0.50 \times 2) \times 2.00 \times 1 \times 3$	= 48.00		
	Bridge seat	$(11.00 \times 2 + 0.55) \times 2.95 \times 2$	= 133.05		Abut pier
	"	$0.55 \times 1.85 \times 2$	= 2.01		"
	"	$-0.70 \times 10.00 \times 2$	= -14.00		(-)Bridge seat
	"	$(11.00 \times 2 + 1.00) \times 2.95 \times 2$	= 135.70		Middle pier
	"	$1.00 \times 1.85 \times 2$	= 3.70		"
	Earth pressure wall	$24.18 \times 2 \times 2$	= 96.72		
	"	$(2.121 + 0.20 \times 4 - 0.15 \times 2 + 5.19) \times 0.45 \times 2$	= 7.57		
	Pedestal	$(0.30 - 0.566) \times 10.00 \times 2$	= 17.32		
	"	$1/2 \times (0.30 + 0.70) \times 0.40 \times 2 \times 2$	= 0.80		
	Gate pier	$(1.70 + 1.00) \times 2 \times 2.85 \times 4$	= 61.56		
	"	$0.25 \times 2.85 \times 2 \times 6$	= 8.55		
	Top slab	$(2.70 + 10.00) \times 2 \times 0.50$	= 12.70		Side
	"	$2.70 \times 10.00$	= 27.00		Underside
	"	$-(1.00 \times 1.70 - 0.25 \times 0.70) \times 2$	= -3.05		(-)Gate pier part
	"	$-(1.00 \times 1.70 - 0.25 \times 0.70 \times 2) \times 2$	= -2.70		"
		Total	= 1350.11	1350.11	m <sup>3</sup>
Circular formwork	RC structure				
	φ1000-1/2Circle	$\pi \times 2 \times 1.00 \times (3.80 - 2.70) \times 2$	= 20.42	20.42	m <sup>2</sup>
	φ1000-1/4Circle	$\pi \times 4 \times 1.00 \times (3.80 - 2.70) \times 2$	= 10.21	10.21	m <sup>2</sup>

Description	Classification	Calculation	Quantity	Unit	Remarks
Second stage con.	$\sigma_{ck}=25\text{N/mm}^2$				
		$0.70 \times 2.50 \times 0.25 \times 3 \times 3 = 3.91$			Invert part
		$0.70 \times 0.25 \times (3.30 + 2.70) \times 6 = 6.30$			Stop log part
		$-0.22 \times 0.13 \times (3.30 + 2.70) \times 6 = -1.03$			"
		$(0.70 \times 0.25 - 0.185 \times 0.13) \times 3.30 \times 6 = 2.99$			Gate part
		$[1/2 \times (0.315 + 0.135) \times 0.12 + 0.265 \times 0.25] \times 2.40 \times 6 = 1.60$			Gate pier part
		$(0.15^2 - 0.05^2) \times 2.00 \times 4 \times 3 = 0.48$			Corridor part
		$0.05/6 \times [(2 \times 0.81 + 0.71) \times 1.72 + (2 \times 0.71 + 0.81) \times 1.62] \times 3 = 0.19$			Top slab part
		Total = 11.47	11.47	$\text{m}^3$	
Fomwork	RC structure				
		$(0.70 - 0.13 \times 2) \times 3.30 \times 2 \times 3 = 19.01$			Stop log part
		$(0.70 - 0.13 \times 2) \times 2.70 \times 2 \times 3 = 15.55$			"
		$(0.70 - 0.13 \times 2) \times 3.30 \times 2 \times 3 = 19.01$			Gate part
		$(0.265 + 0.13 + 0.315 + 0.12 \times \sqrt{(2)}) \times 2.40 \times 6 = 12.67$			Gate pier part
		$0.15 \times 2.00 \times 4 \times 3 = 3.60$			Corridor part
		$1/2 \times (0.71 - 0.81) \times 0.05 \times \sqrt{(2)} \times 2 \times 3 = 0.32$			Top slab part
		$1/2 \times (1.62 - 1.72) \times 0.05 \times \sqrt{(2)} \times 2 \times 3 = 0.71$			"
		Total = 70.87	70.87	$\text{m}^5$	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 = 2.686	2.686	t	
		D16 = 40.722	40.722	t	
		D22 = 2.424	2.424	t	
Concrete curing	PC structure	Leveling concrete = 24.28	24.28	$\text{m}^3$	
	RC structure	987.61 + 14.47 = 1002.08	1002.08	$\text{m}^3$	

Description	Classification	Calculation	Quantity	Unit	Remarks
Scaffolding	Prefabricated scaffold, H≤ 30m				
	Outer perimeter	$5.30 \times (11.20 + 9.00 + 1.00 + 0.10) \times 2$ = 185.50			
	"	$7.87 \times (16.47 + 0.40) \times 2$ = 265.53			
	"	$4.20 \times (4.43 + 1.00 + 3.10) \times 2$ = 71.65			
	Inside	$3.80 \times (1.00 + 1.10) \times 3$ = 23.94			
	"	$3.30 \times (1.00 + 1.15) \times 3$ = 21.29			
	"	$5.65 \times 11.00 \times 3$ = 186.45			
	"	$2.70 \times [(0.75 + 1.00 - 5.00) \times 3 + 1.80 \times 2]$ = 64.40			
	Gate pier part	$3.35 \times (4.90 + 12.20) \times 2$ = 114.57			
		Total = 933.33	933.33	m <sup>2</sup>	
Falsework	Pipe support falsework, H< 4.0m, Support bearing force 40KN/m <sup>2</sup> and less				
	①	$1.00 \times (3.30 + 2.80) / 2 \times 2.00 \times 3$ = 18.30			Corridor part
	②	$1.00 \times 2.80 \times 2.00 \times 3$ = 16.80			"
	③	$1.00 \times 2.20 \times 2.00 \times 3 \times 2$ = 26.40			"
		Total = 61.50	61.50	m <sup>3</sup>	
	Wedge linked falsework, H≤ 30m, Support bearing force 40KN/m <sup>2</sup> and less				
	④	$0.50 \times 2.85 \times 10.00$ = 14.25			Gate pier part
	⑤	$1.70 \times 6.15 \times 2.00 \times 3$ = 62.73			"
	⑥	$0.50 \times (10.00 \times 2.85 + 2.00 \times 3.30 \times 3)$ = 21.15			"
	⑦	$0.40 \times (6.88 + 6.48) / 2 \times 10.00 \times 2$ = 53.41			Pedestal part
		Total = 76.98	76.98	m <sup>3</sup>	
Other miscellaneous					
Joint bar	D16 × 200	$104 \times 2$ = 208.0			Gate part
	"	$(68 + 60) \times 2$ = 256.0			Stop log part
		Total = 464.0	464	nos	
Grating	2000 × 1000	= 6.0	6	set	

Description	Classification	Calculation	Quantity	Unit	Remarks
Safety facilities work					
Guard fence	H-1.10m	$9.70 \times 2 - 2.10 + 1.40 = 23.20$			Top slab part
		$9.00 \times 4 = 36.00$			Corridor part
		Total = 59.20	59.20	m	
Approach cushion slab					
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$				
		$10.00 \times 5.00 \times 0.40 \times 2 = 40.00$	40.00	$\text{m}^3$	
Fomwork	RC structure				
		$(10.00 + 5.00) \times 0.10 \times 2 \times 2 = 24.00$	24.00	$\text{m}^2$	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 $0.075 \times 2 = 0.150$	0.150	t	
		D16 $1.185 \times 2 = 2.370$	2.370	t	
		D18 $0.520 \times 2 = 1.040$	1.040	t	
Concrete curing	RC structure	Reinforced con.	40.00	$\text{m}^3$	
Anchor cap	SGP $\phi$ 40 $\times$ 180				
		$25 \times 2 = 50.0$	50	nos	
Anchor bar	D22 $\times$ 500 (including D10 spiral reinforcement)				
		$25 \times 2 = 50.0$	50	nos	
Rubber shoe	t=30mm				
		$10.00 \times 0.30 \times 2 = 6.00$	6.00	$\text{m}^2$	
Expansion joint	Elastic filler,t=20mm				
		$0.43 \times 10.00 \times 2 = 8.60$	8.60	$\text{m}^2$	





C-145

Diroutiah Reg. Rebar weight [1/8]																																	
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks																			
				a	b	c	d	e	φ																								
F1	D 16	A		9.460 <sup>x1</sup>	4.930 <sup>x1</sup>	4.930 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	20.86	43	896.98																				
F2	D 16	A		9.460 <sup>x1</sup>	6.780 <sup>x1</sup>	6.780 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	24.56	55	1350.80																				
F3	D 16	A		9.460 <sup>x1</sup>	3.830 <sup>x1</sup>	3.830 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	18.66	42	783.72																				
F4	D 16	C		9.800 <sup>x1</sup>							9.80	146	1430.80																				
F5	D 16	C		27.500 <sup>x1</sup>						1.000 <sup>x2</sup>	29.50	50	1475.00																				
F6	D 16	C		21.150 <sup>x1</sup>						1.000 <sup>x1</sup>	22.15	6	132.90																				
F7	D 16	C		5.450 <sup>x1</sup>							5.45	12	65.40																				
F8	D 16	C		1.900 <sup>x1</sup>							1.90	36	68.40																				
F9	D 16	G		1.866 <sup>x1</sup>	1.137 <sup>x1</sup>						3.01	30	90.30																				
F10	D 16	D		1.418 <sup>x1</sup>	1.134 <sup>x2</sup>						3.69	30	110.70																				
F11	D 16	H		1.134 <sup>x1</sup>	2.007 <sup>x1</sup>	1.859 <sup>x1</sup>					5.00	30	150.00																				
F12	D 16	E		13.884 <sup>x1</sup>	1.134 <sup>x1</sup>					1.000 <sup>x1</sup>	16.02	30	480.60																				
F13	D 16	E		5.434 <sup>x1</sup>	1.134 <sup>x1</sup>						6.57	30	197.10																				
F14	D 16	C		5.050 <sup>x1</sup>							5.05	18	90.90																				
F15	D 16	C		4.726 <sup>x1</sup>							4.73	30	141.90																				
F16	D 16	C		2.700 <sup>x1</sup>							2.70	48	129.60																				
F17	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	88	154.00																				
F18	D 16	D		9.800 <sup>x1</sup>	0.240 <sup>x2</sup>						10.28	12	123.36																				
Type	A			B			C			D			E			F			G			H			I			J			K		
	L			M																													
	Dia.			kg/m			R			L			J																				
	D12			0.888			130			201			1000																				
	D14			1.21			150			236			1000																				
	D16			1.58			170			267			1000																				
	D18			2.00			190			298			1000																				
	D20			2.47			210			330			1100																				
D22			2.98			240			377			1210																					
D25			3.85			270			424			1375																					
D28			4.83			300			471			1540																					
D32			6.31			340			534			1760																					

Diroutiah Reg. Rebar weight [2/8]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F19	D 16	C		27.500 <sup>x1</sup>						1.000 <sup>x2</sup>	29.50	12	354.00			
F20	D 16	C		1.900 <sup>x1</sup>							1.90	104	197.60			
F21	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	16	28.00			
F22	D 16	C		1.800 <sup>x1</sup>							1.80	24	43.20			
F23	D 16	C		1.000 <sup>x1</sup>							1.00	16	16.00			
F24	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	20	35.00			
F25	D 16	C		1.900 <sup>x1</sup>							1.90	120	228.00			
F26	D 16	C		2.273 <sup>x1</sup>							2.28	100	228.00			
F27	D 16	I		1.331 <sup>x1</sup>	1.000 <sup>x1</sup>						2.34	20	46.80			
F28	D 16	C		11.800 <sup>x1</sup>							11.80	20	236.00			
F29	D 12	M		0.428 <sup>x1</sup>	1.328 <sup>x1</sup>	1.328 <sup>x1</sup>	0.100 <sup>x2</sup>				3.29	8	26.32			
F30	D 12	M		0.428 <sup>x1</sup>	1.468 <sup>x1</sup> ~1.575	1.561 <sup>x1</sup> ~1.668	0.100 <sup>x2</sup>				3.77	30	113.10			
F31	D 12	M		0.428 <sup>x1</sup>	1.578 <sup>x1</sup>	1.578 <sup>x1</sup>	0.100 <sup>x2</sup>				3.79	15	56.85			
F32	D 12	M		0.428 <sup>x1</sup>	1.828 <sup>x1</sup>	1.828 <sup>x1</sup>	0.100 <sup>x2</sup>				4.29	45	193.05			
F33	D 12	M		0.428 <sup>x1</sup>	1.328 <sup>x1</sup>	1.328 <sup>x1</sup>	0.100 <sup>x2</sup>				3.29	341	1131.76			
F34	D 12	M		0.428 <sup>x1</sup>	1.078 <sup>x1</sup>	1.078 <sup>x1</sup>	0.100 <sup>x2</sup>				2.79	8	22.32			
W1	D 16	C		5.100 <sup>x1</sup>							5.10	108	550.80			
W2	D 16	C		6.950 <sup>x1</sup>							6.95	110	764.50			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

Diroutiah Reg. Rebar weight [3/8]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W3	D 16	C		4.000 <sup>x1</sup>							1.00	98	392.00			
W4	D 16	E		5.100 <sup>x1</sup>	0.240 <sup>x1</sup>						5.34	200	1068.00			
W5	D 16	E		6.950 <sup>x1</sup>	0.240 <sup>x1</sup>						7.19	220	1581.80			
W6	D 16	E		4.000 <sup>x1</sup>	0.240 <sup>x1</sup>						4.24	108	457.92			
W7	D 16	C		3.850 <sup>x1</sup>							3.85	24	92.40			
W8	D 16	C		5.100 <sup>x1</sup>							5.10	20	102.00			
W9	D 16	C		4.000 <sup>x1</sup>							4.00	20	80.00			
W10	D 16	D		0.800 <sup>x1</sup>	1.000 <sup>x2</sup>						2.80	228	638.40			
W11	D 16	D		0.550 <sup>x1</sup>	1.000 <sup>x2</sup>						2.55	20	51.00			
W12	D 16	D		0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	20	25.60			
W13	D 16	D		0.747 <sup>x1</sup>	1.000 <sup>x2</sup>						2.75	4	11.00			
W14	D 16	D		0.800 <sup>x1</sup>	1.000 <sup>x2</sup>						2.80	184	515.20			
W15	D 16	D		0.300 <sup>x1</sup>	1.000 <sup>x2</sup>						2.30	20	46.00			
W16	D 16	D		0.693 <sup>x1</sup>	1.000 <sup>x2</sup>						2.70	4	10.80			
W17	D 16	C		27.500 <sup>x1</sup>						1.000 <sup>x2</sup>	29.50	30	885.00			
W18	D 16	C		8.250 <sup>x1</sup>							8.25	12	99.00			
W19	D 16	J		1.400 <sup>x1</sup>	1.534 <sup>x1</sup>	0.768 <sup>x1</sup>	0.603 <sup>x1</sup>		φ 384		4.31	40	172.40			
W20	D 16	D		1.418 <sup>x1</sup>	0.768 <sup>x2</sup>						2.96	40	118.40			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

Diroutiah Reg. Rebar weight [4/8]																	
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks			
				a	b	c	d	e	φ								
W21	D 16	D		15.918 <sup>x1</sup>	0.768 <sup>x2</sup>					1.000 <sup>x1</sup>	18.46	28	516.88				
W22	D 16	D		3.018 <sup>x1</sup>	0.768 <sup>x2</sup>						4.56	12	54.72				
W23	D 16	J		1.400 <sup>x1</sup>	5.034 <sup>x1</sup>	0.768 <sup>x1</sup>	0.603 <sup>x1</sup>		φ 384		7.81	28	218.68				
W24	D 16	C		5.050 <sup>x1</sup>							5.05	38	191.90				
W25	D 16	C		2.700 <sup>x1</sup>							2.70	32	86.40				
W26	D 16	C		10.800 <sup>x1</sup>							10.80	60	648.00				
W27	D 16	D		0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	60	75.00				
W28	D 16	C		1.800 <sup>x1</sup>							1.80	76	136.80				
W29	D 16	C		1.746 <sup>x1</sup>							1.75	4	7.00				
W30	D 16	K		1.050 <sup>x2</sup>	1.206 <sup>x1</sup>				φ 381		3.31	40	132.40				
W31	D 16	C		1.450 <sup>x1</sup>							1.45	80	116.00				
W32	D 16	C		15.950 <sup>x1</sup>						1.000 <sup>x1</sup>	16.95	56	949.20				
W33	D 16	C		3.050 <sup>x1</sup>							3.05	24	73.20				
W34	D 16	K		1.050 <sup>x2</sup>	1.206 <sup>x1</sup>				φ 381		3.31	28	92.68				
W35	D 16	C		5.050 <sup>x1</sup>							5.05	76	383.80				
W36	D 16	C		2.700 <sup>x1</sup>							2.70	64	172.80				
W37	D 16	C		10.800 <sup>x1</sup>							10.80	60	648.00				
W38	D 16	D		0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	208	260.00				
Type	A	B	C	D	E	F	G	H	I	J	K						
													Dia.	kg/m	R	L	J
													D12	0.888	130	201	1000
													D14	1.21	150	236	1000
													D16	1.58	170	267	1000
													D18	2.00	190	298	1000
													D20	2.47	210	330	1100
													D22	2.98	240	377	1210
													D25	3.85	270	424	1375
													D28	4.83	300	471	1540
												D32	6.31	340	534	1760	

Diroutiah Reg. Rebar weight [5/8]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W39	D 16	C	—	1.896 <sup>x1</sup>							1.90	2	3.80			
W40	D 16	C	—	4.300 <sup>x1</sup>							4.30	4	17.20			
W41	D 16	C	—	1.450 <sup>x1</sup>							1.45	2	2.90			
W42	D 16	C	—	4.150 <sup>x1</sup>							4.15	6	24.90			
W13	D 16	C	—	10.800 <sup>x1</sup>							10.80	6	64.80			
W14	D 16	C	—	2.800 <sup>x1</sup>							2.80	2	5.60			
W45	D 16	C	—	5.896 <sup>x1</sup>							5.90	2	11.80			
W46	D 16	C	—	9.650 <sup>x1</sup>							9.65	4	38.60			
W47	D 16	C	—	1.396 <sup>x1</sup>							1.40	4	5.60			
W18	D 16	C	—	3.800 <sup>x1</sup>							3.80	2	7.60			
W19	D 16	C	—	1.450 <sup>x1</sup>							1.45	1	5.80			
W50	D 16	C	—	4.150 <sup>x1</sup>							4.15	6	24.90			
W51	D 16	C	—	10.800 <sup>x1</sup>							10.80	6	64.80			
W52	D 16	C	—	2.800 <sup>x1</sup>							2.80	1	11.20			
W53	D 16	C	—	5.150 <sup>x1</sup>							5.15	2	10.30			
W54	D 16	C	—	1.396 <sup>x1</sup>							1.40	1	5.60			
W55	D 12	D	└┘	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	596	613.88			
W56	D 12	D	└┘	0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	44	34.32			
Type	A	B	C	D	E	F	G	H	I	J	K	Dia.	kg/m	R	L	J
	L	M	D12	0.888	130	201	1000									
			D14	1.21	150	236	1000									
			D16	1.58	170	267	1000									
			D18	2.00	190	298	1000									
			D20	2.47	210	330	1100									
			D22	2.98	240	377	1210									
			D25	3.85	270	424	1375									
			D28	4.83	300	471	1540									
			D32	6.31	340	534	1760									

C-150

Diroutiah Reg. Rebar weight [6/8]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W57	D 12	D		0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	30	30.90			
W58	D 12	D		0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	540	556.20			
W59	D 12	D		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	44	23.32			
P1	D 22	C		1.470 <sup>x1</sup>							1.47	204	299.88			
P2	D 22	C		1.620 <sup>x1</sup>							1.62	16	25.92			
P3	D 22	C		2.029 <sup>x1</sup> ~3.429							2.73	32	87.36			
P4	D 22	C		3.470 <sup>x1</sup>							3.47	24	83.28			
P5	D 22	C		4.570 <sup>x1</sup>							4.57	24	109.68			
P6	D 22	C		4.529 <sup>x1</sup> ~1.229							2.88	72	207.36			
P7	D 12	D		0.250 <sup>x1</sup>	0.180 <sup>x2</sup>						0.61	186	113.16			
P8	D 16	C		13.717 <sup>x1</sup> ~14.457						1.000 <sup>x1</sup>	15.09	12	181.08			
P9	D 16	C		2.929 <sup>x1</sup> ~3.479							3.21	16	51.36			
P10	D 16	C		3.650 <sup>x1</sup>							3.65	20	73.00			
P11	D 16	C		2.929 <sup>x1</sup> ~5.679							1.31	60	258.60			
P12	D 16	H		1.050 <sup>x1</sup>	1.609 <sup>x1</sup>	2.038 <sup>x1</sup>					1.70	1	18.80			
P13	D 16	H		1.050 <sup>x1</sup>	1.609 <sup>x1</sup>	6.049 <sup>x1</sup>					8.71	1	34.84			
P14	D 16	D		0.218 <sup>x1</sup>	0.240 <sup>x2</sup>						0.70	10	7.00			
P15	D 12	D		0.278 <sup>x1</sup>	0.100 <sup>x2</sup>						0.48	126	60.48			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	

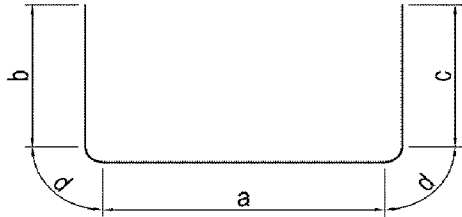
Diroutiah Reg. Rebar weight [7/8]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
S1	D 16	C		9.800 <sup>x1</sup>							9.80	41	431.20			
S2	D 16	C		0.800 <sup>x1</sup>							0.80	240	192.00			
S3	D 16	D		0.618 <sup>x1</sup>	0.268 <sup>x2</sup>						1.16	120	139.20			
C1	D 16	L		0.240 <sup>x1</sup>	0.650 <sup>x1</sup>	0.159 <sup>x1</sup>	0.919 <sup>x1</sup>				1.97	102	200.94			
C2	D 16	C		9.800 <sup>x1</sup>							9.80	8	78.40			
G1	D 16	A		9.460 <sup>x1</sup>	1.230 <sup>x1</sup>	1.630 <sup>x1</sup>	0.267 <sup>x2</sup>		φ 170	1.000 <sup>x1</sup>	13.86	8	110.88			
G2	D 16	B		1.024 <sup>x1</sup>	1.230 <sup>x1</sup> ~1.630	0.267 <sup>x1</sup>					2.73	2	5.46			
G3	D 16	B		1.053 <sup>x1</sup>	1.630 <sup>x1</sup> ~1.230	0.267 <sup>x1</sup>					2.75	2	5.50			
G4	D 16	C		0.789 <sup>x1</sup> ~0.845							0.82	6	4.92			
G5	D 16	C		1.589 <sup>x1</sup> ~1.645							1.62	1	6.48			
G6	D 16	C		1.250 <sup>x1</sup>							1.25	140	595.00			
G7	D 16	F		0.332 <sup>x2</sup>	0.832 <sup>x2</sup>	0.225 <sup>x2</sup>					2.78	120	333.60			
G8	D 16	F		0.582 <sup>x2</sup>	1.532 <sup>x2</sup>	0.225 <sup>x2</sup>					4.68	30	140.40			
G9	D 16	F		0.332 <sup>x2</sup>	1.532 <sup>x2</sup>	0.225 <sup>x2</sup>					1.18	30	125.40			
G10	D 12	D		0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	16	12.48			
G11	D 12	D		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	16	8.48			
T1	D 16	D		9.800 <sup>x1</sup>	0.300 <sup>x2</sup>						10.40	6	62.40			
T2	D 16	D		9.800 <sup>x1</sup>	0.240 <sup>x2</sup>						10.28	14	143.92			
Type	A	B	C	D	E	F	G	H	I	J	K	Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	

Diroutiah Reg. Rebar weight [8/8]																
No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
T3	D 16	E		$1.94^{x1}$ ~1.223	$0.240^{x1}$						1.45	1	5.80			
T4	D 16	C		$0.789^{x1}$ ~0.845							0.82	6	4.92			
T5	D 16	C		$1.589^{x1}$ ~1.645							1.62	4	6.48			
T6	D 16	D		$2.532^{x1}$	$0.332^{x2}$						3.20	45	144.00			
T7	D 16	D		$2.532^{x1}$	$0.240^{x2}$						3.02	45	135.90			
T8	D 16	E		$1.078^{x1}$ ~1.156	$0.332^{x1}$						1.45	12	17.40			
T9	D 16	E		$1.96^{x1}$ ~1.118	$0.332^{x1}$						1.49	12	17.88			
T10	D 16	E		$1.078^{x1}$ ~1.156	$0.240^{x1}$						1.36	12	16.32			
T11	D 16	E		$1.96^{x1}$ ~1.118	$0.240^{x1}$						1.40	12	16.80			
T12	D 12	D		$0.328^{x1}$	$0.100^{x2}$						0.53	53	28.09			
T13	D 16	C		$9.150^{x1}$							9.15	1	36.60			
T14	D 16	C		$2.150^{x1}$							2.15	72	154.80			
				D12	$3025.01 \times 0.888$	=	2686.21 kg	=	2.686 t							
				D16	$25773.12 \times 1.58$	=	40722.00 kg	=	40.722 t							
				D22	$813.48 \times 2.98$	=	2424.17 kg	=	2.424 t							
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

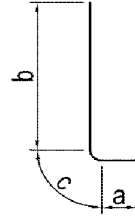


# Schematic design of rebar (1/2)

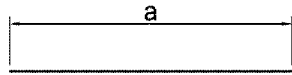
A



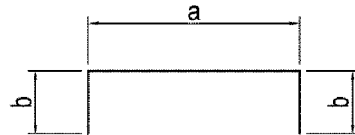
B



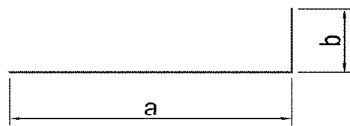
C



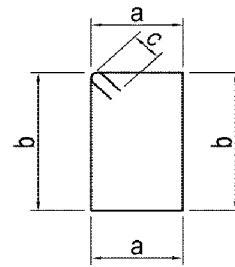
D



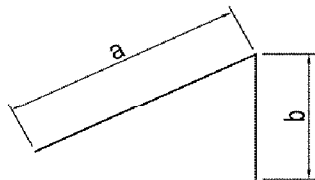
E



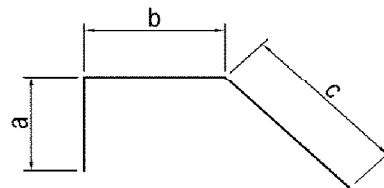
F



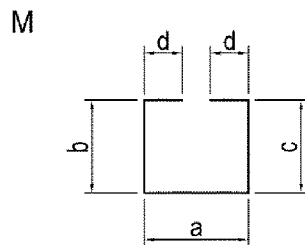
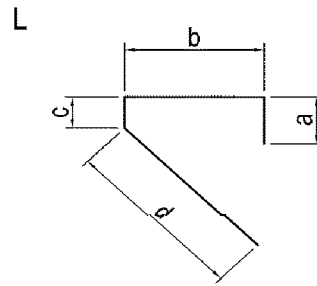
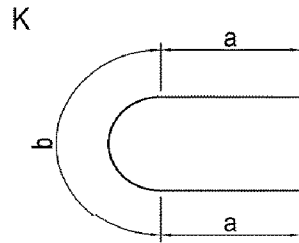
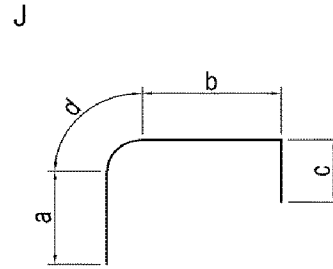
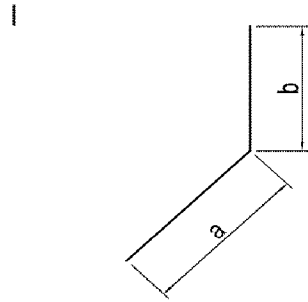
G



H



## Schematic design of rebar (2/2)



Diroutiah Reg. (Approach cushion slab)

Rebar weight [1/1]

per a place

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
F1	D 18	B	┌	4.800 <sup>x1</sup>	0.200 <sup>x2</sup>						5.20	50	260.00	
F2	D 16	B	┌	4.800 <sup>x1</sup>	0.200 <sup>x2</sup>						5.20	50	260.00	
F3	D 16	A	—	9.800 <sup>x1</sup>							9.80	25	245.00	
F4	D 16	A	—	9.800 <sup>x1</sup>							9.80	25	245.00	
F5	D 12	B	┐	0.229 <sup>x1</sup>	0.180 <sup>x2</sup>						0.59	144	84.96	
				D12	84.96 × 0.888	=	75.44 kg	=	0.075 t					
				D16	750.00 × 1.58	=	1185.00 kg	=	1.185 t					
				D18	260.00 × 2.00	=	520.00 kg	=	0.520 t					
Type	A          B		┌	Dia.	kg/m	R	L	J						
				D12	0.888	130	204	1000						
				D14	1.21	150	236	1000						
				D16	1.58	170	267	1000						
				D18	2.00	190	298	1000						
				D20	2.47	210	330	1100						
				D22	2.98	240	377	1210						
				D25	3.85	270	424	1375						
				D28	4.83	300	471	1540						
				D32	6.31	340	534	1760						

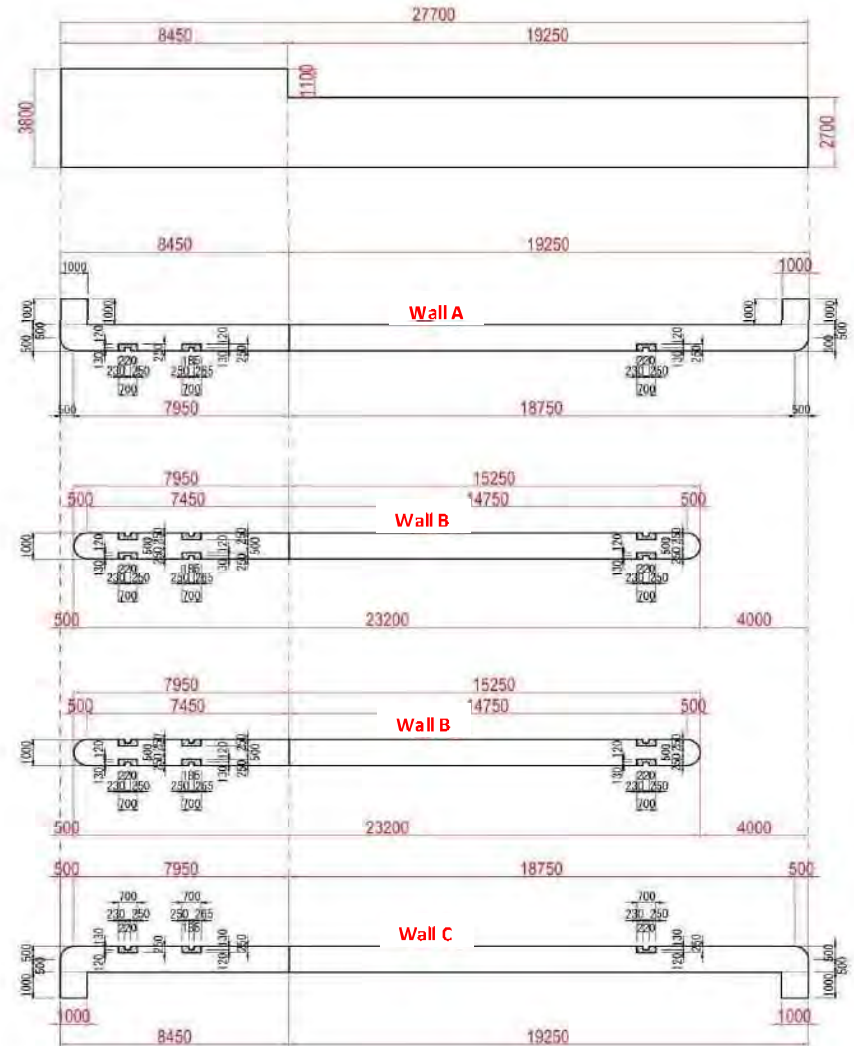
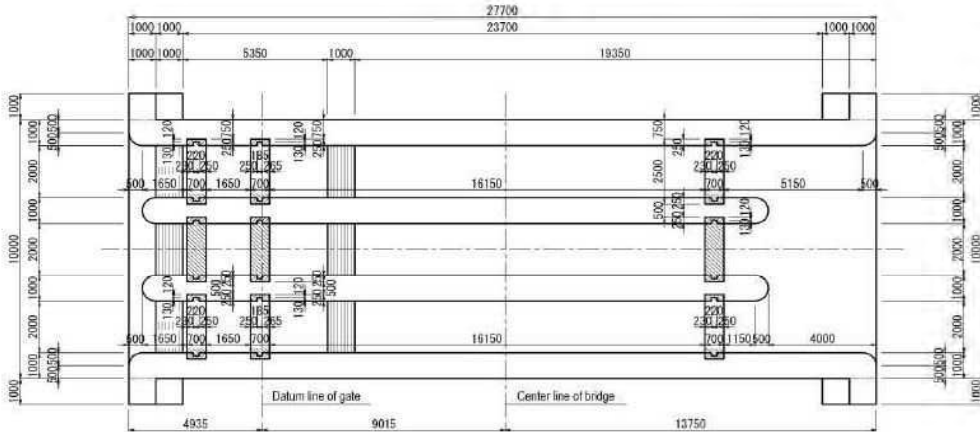
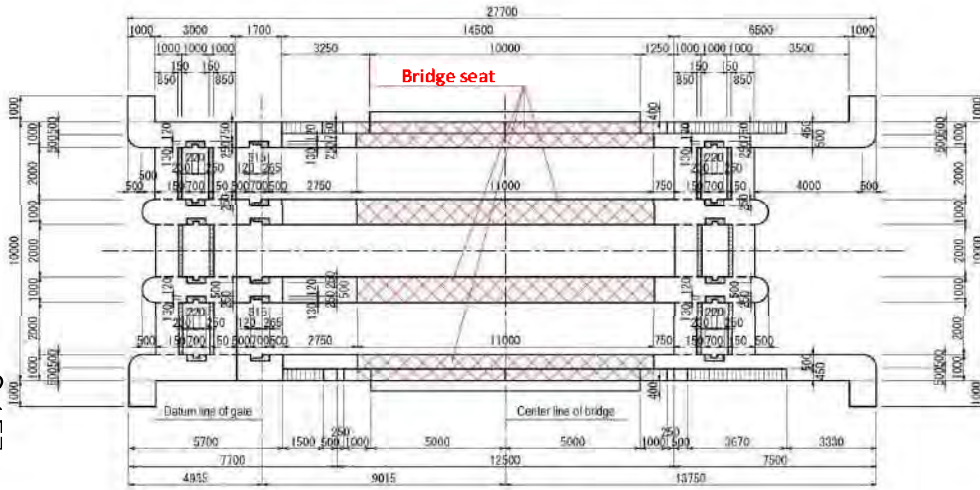
C-155

Schematic design of rebar  
(Approach cushion slab)

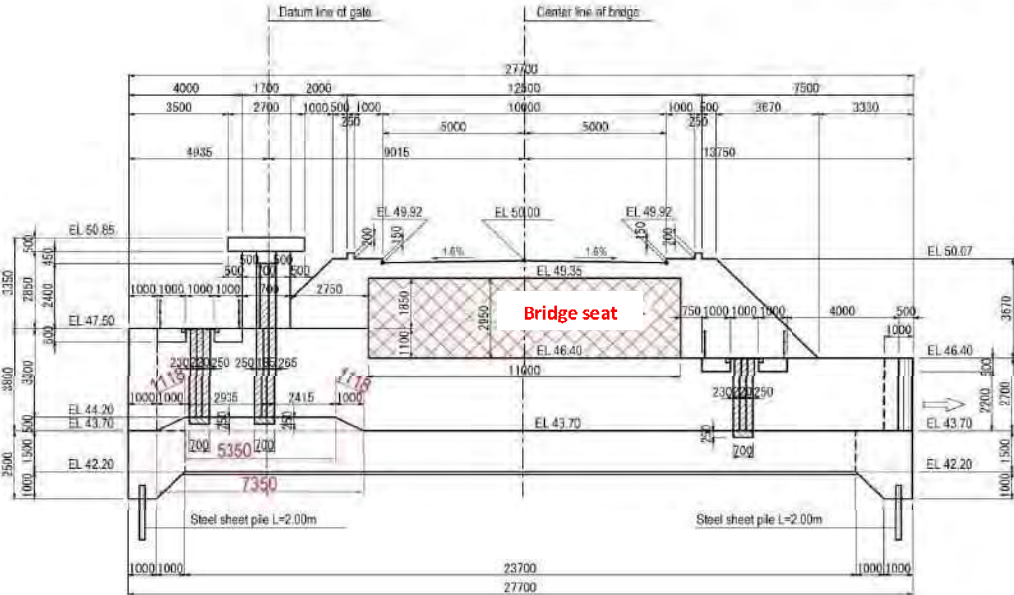


## Concrete / Formwork (1/2)

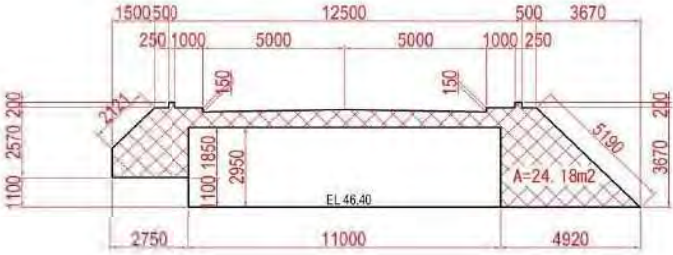
C-157



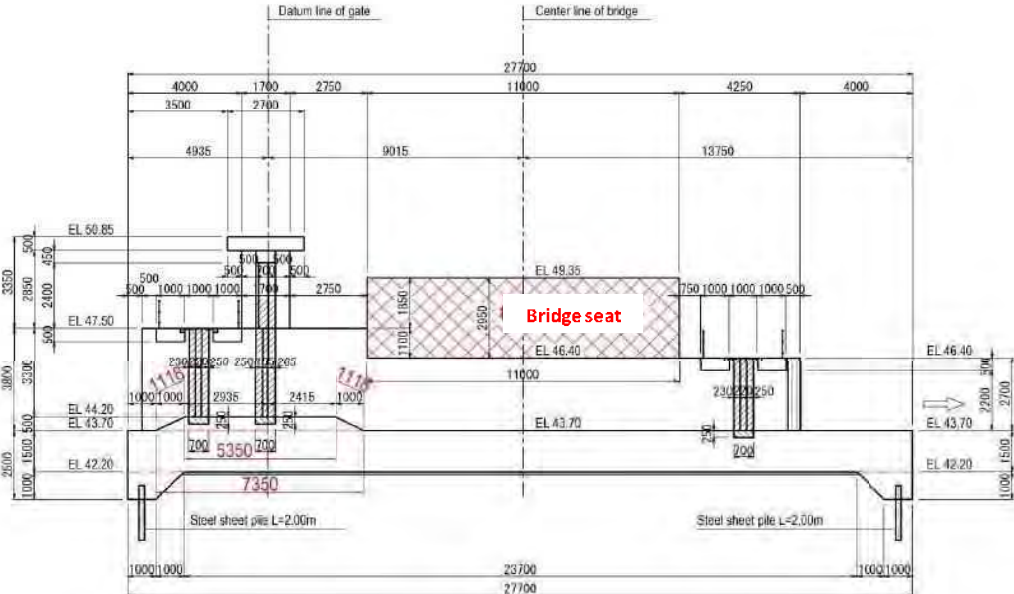
# Concrete / Formwork (2/2)



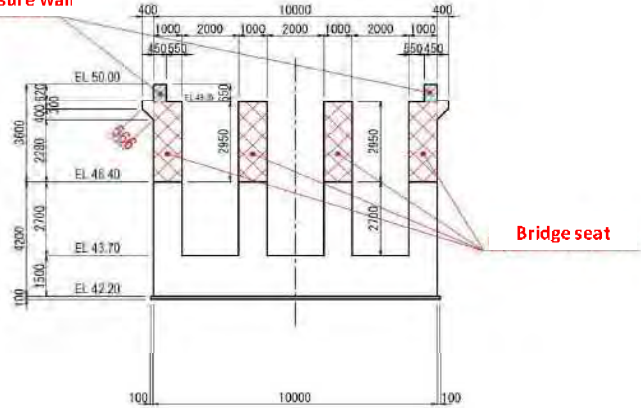
**Earth pressure wall (t=450)**



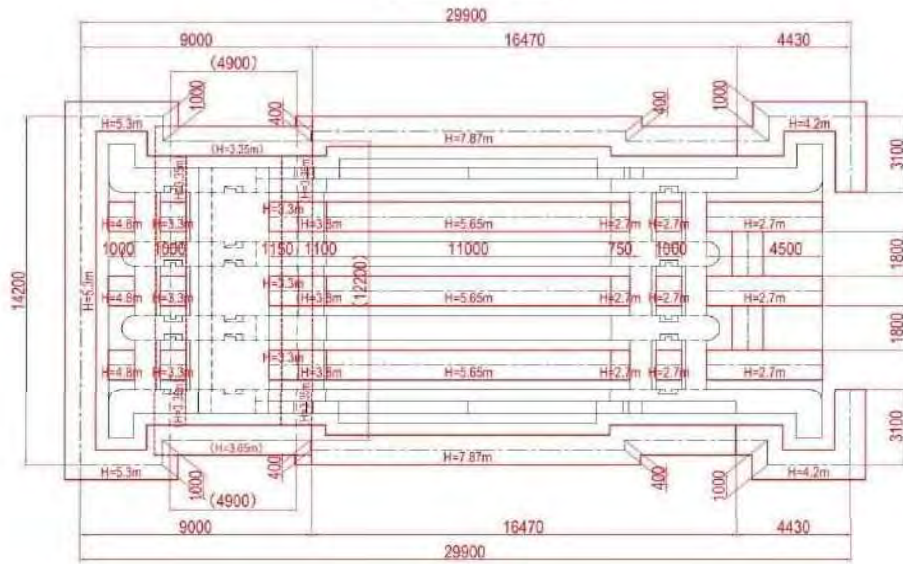
C-158



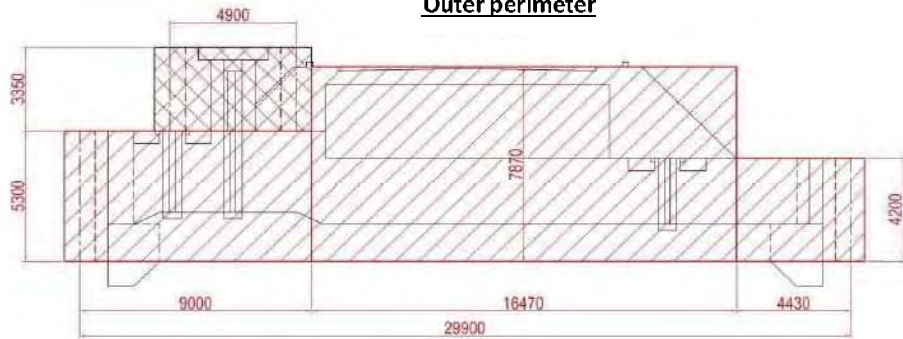
**Earth pressure wall**



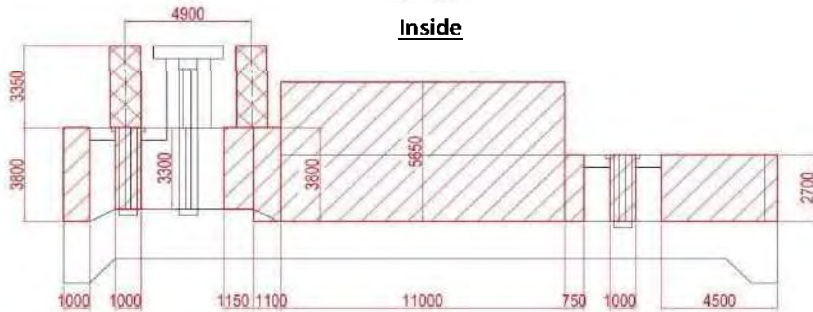
### Scaffolding



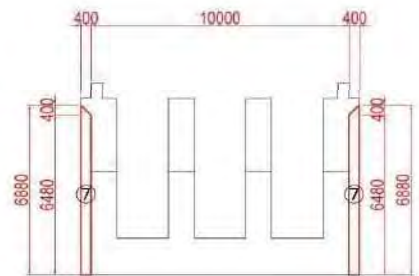
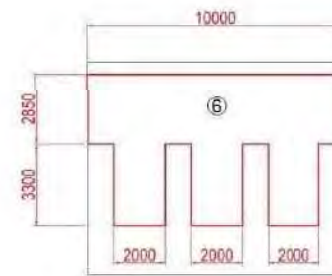
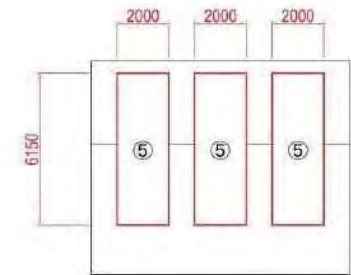
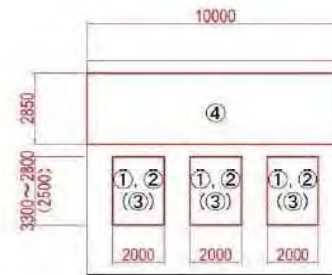
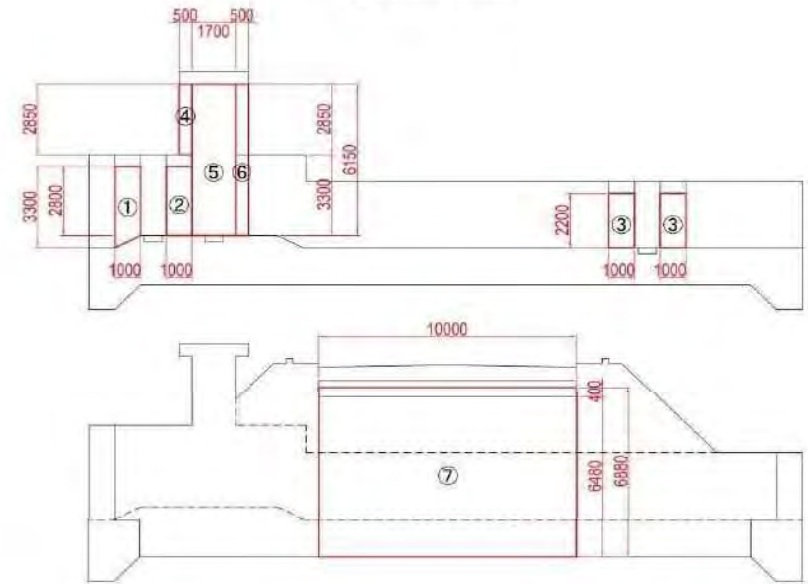
### Outer perimeter



### Inside



### Falsework



## (5) Abo Gabal regulator

Abo Gabal Weir body Quantity Summary Sheet							
Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Up stream	Down stream
Weir body	Demolition works	Concrete demolition	PC structure	m <sup>3</sup>	3.64	—	3.64
			RC structure	m <sup>3</sup>	123.61	—	123.61
	Main body	Leveling con.	$\sigma_{ck}=20N/mm^2$	m <sup>3</sup>	37.86	18.81	19.05
		Formwork	Leveling con.	m <sup>2</sup>	6.74	4.17	2.57
		Reinforced con.	$\sigma_{ck}=25N/mm^2$	m <sup>3</sup>	1,441.42	805.18	636.24
		Formwork	RC structure	m <sup>2</sup>	1,660.83	1,137.03	523.80
		Circular formwork	RC structure $\phi 1000-1/2$ Circle	m <sup>2</sup>	29.85	29.85	—
		"	RC structure $\phi 1000-1/4$ Circle	m <sup>2</sup>	5.26	5.26	—
		Second stage con.	$\sigma_{ck}=25N/mm^2$	m <sup>3</sup>	23.72	23.72	—
		Formwork	RC structure	m <sup>2</sup>	121.32	121.32	—
		Rcbar	D12	t	3.968	2.049	1.919
		"	D16	t	53.777	35.871	17.906
		Concrete curing	PC structure	m <sup>3</sup>	37.86	18.81	19.05
		"	RC structure	m <sup>3</sup>	1,465.14	828.90	636.24
		Scaffolding	Prefabricated Scaffold, $H \leq 30m$ tube support falsework, $H \leq 11m$	m <sup>2</sup>	975.38	593.54	381.84
		Falsework	wedge timber falsework, $H \leq 30m$ Support bearing force 40KN/m <sup>2</sup> and less	m <sup>3</sup>	60.58	60.58	—
		"	wedge timber falsework, $H \leq 30m$ Support bearing force 40KN/m <sup>2</sup> and less	m <sup>3</sup>	117.09	117.09	—
		Expansion joint	Elastic filler, t=20mm	m <sup>2</sup>	96.38	—	96.38
		Water stop	B=300mm	m	34.00	34.00	—
		Water-swelling water stop	Paste type, 9mm x 30mm	m	36.65	—	36.65
		Resin capsule anchor	D20 x 200, Drilling dia. $\phi 25$	nos	121	—	121
		Slip bar	D20 x 1.00m, VP $\phi 25 \times 0.50m$	nos	111	111	—
		"	D20 x 0.70m	nos	121	—	121



Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Up stream	Down stream
	Other miscellaneous	Joint bar	D16×200	nos	1,120	1,120	—
		Grating	2000×1000	set	1	1	—
	Safety facilities work	Guard fence	H=1.10m	m	29.80	29.80	—

Abo Gabal Weir body      Quantity calculation

【Upstream】

Discription	Classification	Calculation	Quantity	Unit	Remarks
Main body					
Leveling con.	$\sigma_{ck}=20\text{N/mm}^2$				
		$13.20 \times (11.20 + 11.30) / 2 \times 0.10$	— 18.81		
			Total = 18.81	18.81	$\text{m}^5$
Formwork	Leveling con.	$(14.20 - 14.30) / 2 \times 0.10 \times 2$	= 2.85		
		$13.20 \times 0.10$	— 1.32		
			Total = 4.17	4.17	$\text{m}^2$
Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$				
	Cut-off	$1.2 \times (1.00 + 2.00) \times 1.00 \times 15.00$	— 22.50		
	Invert	$1.00 \times 1.50 \times 2.00 \times 2$	= 6.00		
	"	$13.00 \times 1.50 \times 16.20$	— 315.90		
	"	$1.2 \times (3.75 + 3.96) \times 0.85 \times 2.00 \times 4$	= 26.21		Raising part
	"	$-0.70 \times 0.25 \times 2.50 \times 4 \times 3$	— -5.25		(-)Blockout
	Wall A	$1.00 \times 4.75 \times 1.00$	= 4.75		
	"	$1.00 \times 4.75 \times 16.20$	= 76.95		
	"	$-(1 - \pi/4) \times 0.50^2 \times 4.75$	— -0.25		
	"	$-0.70 \times 3.90 \times 0.25 \times 2$	= -1.37		(-)Blockout
	"	$-0.70 \times 4.75 \times 0.25$	= -0.83		"
	Wall B	$1.00 \times 4.75 \times 15.70$	— 74.58		
	"	$-(1 - \pi/4) \times 0.50^2 \times 4.75 \times 2$	= -0.51		
	"	$-0.70 \times 3.90 \times 0.25 \times 2 \times 2$	— -2.73		(-)Blockout
	"	$-0.70 \times 4.75 \times 0.25 \times 2$	= -1.66		"
	Wall C	$1.00 \times 4.75 \times 15.70$	— 74.58		
	"	$-(1 - \pi/4) \times 0.50^2 \times 4.75 \times 2$	— -0.51		
	"	$-0.70 \times 3.90 \times 0.25 \times 2 \times 2$	= -2.73		(-)Blockout
	"	$-0.70 \times 4.75 \times 0.25 \times 2$	— -1.66		"

Discription	Classification	Calculation	Quantity	Unit	Remarks
	Wall D	$1.00 \times 1.75 \times 15.20$	= 72.20		
	"	$-(1 - \pi / 1) \times 0.50^2 \times 4.75 \times 2 \times 2$	= -1.02		
	"	$-0.70 \times 3.90 \times 0.25 \times 2 \times 2$	= -2.73		(-)Blockout
	"	$-0.70 \times 4.75 \times 0.25 \times 2$	= -1.66		"
	Wall E	$1.00 \times 4.75 \times 1.00$	= 4.75		
	"	$1.00 \times 1.75 \times 16.20$	= 76.95		
	"	$-(1 - \pi / 4) \times 0.50^2 \times 4.75$	= -0.25		
	"	$-0.70 \times 3.90 \times 0.25 \times 2$	= -1.37		(-)Blockout
	"	$-0.70 \times 4.75 \times 0.25$	= -0.83		"
	Corrdor	$1.00 \times 0.50 \times 2.00 \times 4 \times 2$	= 8.00		
	"	$-0.15 \times 0.15 \times 2.00 \times 2 \times 4$	= -0.36		(-)Second stage cor.
	Bridge	$6.50 \times 0.50 \times 2.00 \times 4$	= 26.00		
	"	$0.25 \times 0.15 \times 13.00 \times 2$	= 0.98		
	Gate pier	$(0.75 \times 1.20 - 1.00 \times 0.50) \times 3.45 \times 2$	= 9.66		
	"	$(0.50 \times 1.20 - 1.00 \times 0.50) \times 3.45 \times 3$	= 11.39		
	Top slab	$3.00 \times 0.50 \times 13.00$	= 19.50		
		Total =	805.18	805.18	m <sup>3</sup>
Formwork	RC structure				
	Cut-off	$1/2 \times (1.00 + 2.00) \times 1.00 \times 2$	= 3.00		Side
	"	$1.00 \times 15.00$	= 15.00		Edge
	Invert	$(15.00 - 13.00) \times 1.50$	= 12.00		Edge
	"	$1.00 \times 1.50 \times 2$	= 3.00		Overhanging par.
	"	$1.50 \times 16.20 \times 2$	= 48.60		Side
	"	$(0.85 + 0.876) \times 2.00 \times 4$	= 13.81		Raising part
	"	$(0.70 + 2.50) \times 0.25 \times 3 \times 4$	= 9.60		Blockout part
	Wall A	$(1.50 + 1.00) \times 4.75$	= 11.88		
	"	$4.75 \times (16.20 + 15.70 + 1.00)$	= 156.28		
	"	$3.90 \times 0.25 \times 2 \times 2 + 4.75 \times 0.25 \times 2$	= 6.28		Blockout part
	"	$-1/2 \times (3.75 + 3.96) \times 0.85$	= -3.28		(-)Raising part

Discription	Classification	Calculation	Quantity	Unit	Remarks
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 2$	= -0.96		(-)Corridor
	"	$-6.50 \times 0.50$	= -3.25		(-)Bridge
	Wall B	$4.75 \times (15.20 \times 2 - 1.00)$	= 149.15		
	"	$3.90 \times 0.25 \times 4 \times 2 + 4.75 \times 0.25 \times 4$	= 12.55		Blockout part
	"	$-1/2 \times (3.75 + 3.96) \times 0.85 \times 2$	= -6.55		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 1$	= -1.91		(-)Corridor
	"	$-6.50 \times 0.50 \times 2$	= -6.50		(-)Bridge
	Wall C	$4.75 \times (15.20 \times 2 - 1.00)$	= 149.15		
	"	$3.90 \times 0.25 \times 4 \times 2 + 4.75 \times 0.25 \times 4$	= 12.55		Blockout part
	"	$-1/2 \times (3.75 + 3.96) \times 0.85 \times 2$	= -6.55		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 1$	= -1.91		(-)Corridor
	"	$-6.50 \times 0.50 \times 2$	= -6.50		(-)Bridge
	Wall D	$4.75 \times 14.20 \times 2$	= 134.90		
	"	$3.90 \times 0.25 \times 4 \times 2 + 4.75 \times 0.25 \times 4$	= 12.55		Blockout part
	"	$-1/2 \times (3.75 + 3.96) \times 0.85 \times 2$	= -6.55		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 1$	= -1.91		(-)Corridor
	"	$-6.50 \times 0.50 \times 2$	= -6.50		(-)Bridge
	Wall E	$(1.50 + 1.00) \times 4.75$	= 11.88		
	"	$4.75 \times (16.20 + 15.70 + 1.00)$	= 156.28		
	"	$3.90 \times 0.25 \times 2 \times 2 + 4.75 \times 0.25 \times 2$	= 6.28		Blockout part
	"	$-1/2 \times (3.75 + 3.96) \times 0.85$	= -3.28		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 2$	= -0.96		(-)Corridor
	"	$-6.50 \times 0.50$	= -3.25		(-)Bridge
	Corridor	$(1.00 + 0.50 \times 2) \times 2.00 \times 2 \times 4$	= 32.00		
	Bridge	$(6.50 + 0.50 \times 2) \times 2.00 \times 4$	= 60.00		
	"	$(0.25 + 13.00) \times 2 \times 0.15 \times 2$	= 7.95		
	"	$0.25 \times 0.15 \times 4$	= 0.15		
	Gate pier	$(1.00 + 1.70) \times 2 \times 3.45 \times 5$	= 93.15		
	Top slab	$(3.00 + 13.00) \times 2 \times 0.50$	= 16.00		
	"	$3.00 \times 13.00$	= 39.00		

Discription	Classification	Calculation	Quantity	Unit	Remarks
	"	$-(0.75 \times 1.20 + 1.00 \times 0.50) \times 2$	= -2.80		
	"	$-(0.50 \times 1.20 + 1.00 \times 0.50) \times 3$	= -3.30		
		Total =	1137.03	1137.03	m <sup>2</sup>
Circular formwork	RC structure				
	φ 1000-1/2Circle	$\pi/2 \times 1.00 \times 4.75 \times 1$	= 29.85	29.85	m <sup>2</sup>
	φ 1000-1/4Circle	$\pi/4 \times 1.00 \times 3.35 \times 2$	= 5.26	5.26	m <sup>2</sup>
Second stage con.	σ ck=25N/mm <sup>2</sup>				
		$0.70 \times 0.25 \times 2.50 \times 1 \times 3$	= 5.25		Invert part
		$(0.70 \times 0.25 - 0.185 \times 0.13) \times 3.90 \times 8$	= 4.71		Gate part
		$\{0.70 \times 0.25 - (0.315 \times 0.13 + 1/2 \times 0.12^2)\} \times 3.00 \times 8$	= 3.04		"
		$(0.70 \times 0.25 - 0.22 \times 0.13) \times (3.90 + 4.75) \times 8$	= 10.13		Stop log part
		$(0.15^2 - 0.05^2) \times 2.00 \times 2 \times 4$	= 0.32		Corridor part
		$0.05 \times 6 \times \{(2 \times 0.81 + 0.71) \times 1.82 + (2 \times 0.71 - 0.81) \times 1.72\} \times 1$	= 0.27		Top slab part
		Total =	23.72	23.72	m <sup>3</sup>
Formwork	RC structure				
		$(0.70 + 0.13 \times 2) \times 3.90 \times 8$	= 29.95		Gate part
		$\{0.265 - 0.315 + 0.13 + 0.12 \times \sqrt{(2)}\} \times 3.00 \times 8$	= 21.11		"
		$(0.70 + 0.13 \times 2) \times (3.90 + 4.75) \times 8$	= 66.43		Stop log part
		$0.15 \times 2.00 \times 2 \times 4$	= 2.40		Corridor part
		$1/2 \times (0.71 + 0.81) \times 0.05 \times \sqrt{(2)} \times 2 \times 1$	= 0.43		Top slab part
		$1/2 \times (1.72 + 1.82) \times 0.05 \times \sqrt{(2)} \times 2 \times 1$	= 1.00		"
		Total =	121.32	121.32	m <sup>2</sup>

Discription	Classification	Calculation	Quantity	Unit	Remarks
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 = 2.049	2.049	t	
		D16 = 35.871	35.871	t	
Concrete curing	PC structure	Leveling con. = 18.81	18.81	m <sup>3</sup>	
	RC structure	805.18+23.72 = 828.90	828.90	m <sup>3</sup>	
Sca.folding	Prefabricated Scaffold, H≤30m				
	Outer perimeter	$6.25 \times [17.20 + (17.13 + 1.00) \times 2] = 334.13$			
	Inside	$4.75 \times (1.00 - 1.35 + 2.00) \times 4 = 82.65$			
	"	$3.90 \times 1.00 \times 4 = 15.60$			
	Gate pier part	$3.95 \times (5.20 - 15.20) \times 2 = 161.16$			
		Total = 593.54	593.54	m <sup>2</sup>	
Falsework	Pipe support falsework, H<4.0m, Support bearing force 40KN/m <sup>2</sup> and less				
	②	$2.00 \times 3.40 \times 1.00 \times 4 = 27.20$			Corridor part
	④	$13.00 \times 3.95 \times 0.65 = 33.38$			Gate pier part
		Total = 60.58	60.58	m <sup>3</sup>	
	Wedge linked falsework, H≤30m, Support bearing force 40KN/m <sup>2</sup> and less				
	①	$2.00 \times 1.25 \times 1.00 \times 4 = 34.00$			Corridor part
	③	$2.00 \times 1.25 \times 6.50 \times 4 = 221.00$			Bridge part
	⑤	$(2.00 \times 3.90 - 2.50 \times 3.45) \times 1.20 \times 4 = 78.84$			Gate pier part
	⑥	$2.00 \times 7.35 \times 0.50 \times 4 = 29.40$			"
	⑦	$(2.00 \times 4.75 \times 4 + 13.00 \times 3.45) \times 0.65 = 53.85$			"
		Total = 417.09	417.09	m <sup>3</sup>	
Water stop	B=300mm	$12.00 + 5.50 \times 4 = 34.00$	34.00	m <sup>2</sup>	
Slip bar	D20×1.00m, VP φ 25 <0.50m	= 111.0	111	nos	



Abo Gabal Reg. (Upstream) Rebar weight [1/6]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks			
				a	b	c	d	e	φ								
F1	D 16	A		12.460 <sup>x1</sup>	5.880 <sup>x2</sup>	0.267 <sup>x2</sup>			φ 170	1.000 <sup>x3</sup>	27.76	82	2276.32				
F2	D 16	B		12.800 <sup>x1</sup>						1.000 <sup>x1</sup>	13.80	96	1324.80				
F3	D 16	B		16.000 <sup>x1</sup>						1.000 <sup>x1</sup>	17.00	65	1105.00				
F4	D 16	C		14.134 <sup>x1</sup>	1.134 <sup>x1</sup>					1.000 <sup>x1</sup>	16.27	8	130.16				
F5	D 16	B		2.750 <sup>x1</sup>							2.75	49	134.75				
F6	D 16	D		0.068 <sup>x1</sup>	1.734 <sup>x1</sup>	1.134 <sup>x1</sup>					2.91	40	117.60				
F7	D 16	E		1.418 <sup>x1</sup>	1.134 <sup>x2</sup>						3.69	40	147.60				
F8	D 16	F		0.193 <sup>x1</sup>	1.134 <sup>x1</sup>	1.756 <sup>x1</sup>					3.09	40	123.60				
F9	D 16	C		9.424 <sup>x1</sup>	1.134 <sup>x1</sup>						10.56	40	422.40				
F10	D 16	C		0.934 <sup>x1</sup>	1.134 <sup>x1</sup>						2.07	51	105.57				
F11	D 16	B		5.050 <sup>x1</sup>							5.05	16	80.80				
F12	D 16	B		3.637 <sup>x1</sup>							3.64	40	145.60				
F13	D 16	B		2.700 <sup>x1</sup>							2.70	56	151.20				
F14	D 16	E		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	102	178.50				
F15	D 16	E		12.768 <sup>x1</sup>	0.240 <sup>x2</sup>					1.000 <sup>x1</sup>	11.25	12	171.00				
F16	D 16	E		16.168 <sup>x1</sup>						1.000 <sup>x1</sup>	17.17	12	206.04				
F17	D 16	B		1.900 <sup>x1</sup>							1.90	75	142.50				
F18	D 16	B		2.273 <sup>x1</sup>							2.28	65	148.20				
Type	A	B	C	D	E	F	G	H	I	J							
											Dia.	kg/m	R	L	J		
	K	L	M														
	D																
													D12	0.888	130	201	1000
													D14	1.21	150	236	1000
													D16	1.58	170	267	1000
													D18	2.00	190	298	1000
													D20	2.47	210	330	1100
												D22	2.98	240	377	1210	
												D25	3.85	270	424	1375	
												D28	4.83	300	471	1540	
												D32	6.31	340	534	1760	

C-168



Abo Gabal Reg. (Upstream) Rebar weight [2/6]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F19	D 16	G		1.000 <sup>x1</sup>	1.331 <sup>x1</sup>						2.31	10	23.10			
F20	D 16	B		14.800 <sup>x1</sup>						1.000 <sup>x1</sup>	15.80	10	158.00			
F21	D 16	B		1.900 <sup>x1</sup>							1.90	26	49.40			
F22	D 16	B		1.800 <sup>x1</sup>							1.80	12	21.60			
F23	D 16	B		1.000 <sup>x1</sup>							1.00	8	8.00			
F24	D 16	E		1.300 <sup>x1</sup>	0.240 <sup>x2</sup>						1.78	8	14.24			
F25	D 16	E		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	10	17.50			
F26	D 12	II		0.428 <sup>x1</sup>	1.328 <sup>x2</sup>	0.100 <sup>x2</sup>					3.29	240	789.60			
F27	D 12	II		0.428 <sup>x1</sup>	1.928 <sup>x2</sup>	0.100 <sup>x2</sup>					4.49	40	179.60			
F28	D 12	H		0.428 <sup>x1</sup>	2.178 <sup>x2</sup>	0.100 <sup>x2</sup>					1.99	32	159.68			
F29	D 12	H		0.428 <sup>x1</sup>	1.078 <sup>x2</sup>	0.100 <sup>x2</sup>					2.79	20	55.80			
W 1	D 16	C		6.050 <sup>x1</sup>	0.240 <sup>x1</sup>						6.29	732	4604.28			
W 2	D 16	E		0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	72	92.16			
W 3	D 16	E		0.550 <sup>x1</sup>	0.240 <sup>x2</sup>						1.03	20	20.60			
W 4	D 16	E		0.300 <sup>x1</sup>	0.240 <sup>x2</sup>						0.78	30	23.40			
W 5	D 16	E		0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	10	12.80			
W 6	D 16	B		16.000 <sup>x1</sup>						1.000 <sup>x1</sup>	17.00	50	850.00			
W 7	D 16	I		1.400 <sup>x1</sup>	0.603 <sup>x1</sup>	1.534 <sup>x1</sup>	0.768 <sup>x1</sup>		φ 384		4.31	46	198.26			
Type	A	B	C	D	E	F	G	II	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M													
	D															
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
											D22	2.98	240	377	1210	
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

C-169

Abo Gabal Reg. (Upstream) Rebar weight [3/6]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W 8	D 16	E		1.418 <sup>x1</sup>	0.768 <sup>x2</sup>						2.96	46	136.16			
W 9	D 16	E		8.918 <sup>x1</sup>	0.768 <sup>x2</sup>						10.46	46	481.16			
W10	D 16	E		0.918 <sup>x1</sup>	0.768 <sup>x2</sup>						2.46	50	123.00			
W11	D 16	I		1.400 <sup>x1</sup>	0.603 <sup>x1</sup>	13.734 <sup>x1</sup>	0.768 <sup>x1</sup>		φ 384	1.000 <sup>x1</sup>	17.51	2	35.02			
W12	D 16	I		1.400 <sup>x1</sup>	0.603 <sup>x1</sup>	13.734 <sup>x1</sup>	0.768 <sup>x1</sup>		φ 384	1.000 <sup>x1</sup>	17.51	2	35.02			
W13	D 16	J		1.206 <sup>x1</sup>	1.050 <sup>x2</sup>				φ 384		3.31	69	228.39			
W14	D 16	B		1.450 <sup>x1</sup>							1.45	138	200.10			
W15	D 16	B		8.950 <sup>x1</sup>							8.95	138	1235.10			
W16	D 16	E		0.918 <sup>x1</sup>	0.768 <sup>x2</sup>						2.46	100	246.00			
W17	D 16	K		1.206 <sup>x1</sup>	0.034 <sup>x2</sup>	0.768 <sup>x1</sup>	0.240 <sup>x1</sup>		φ 384		2.29	25	57.25			
W18	D 16	J		1.206 <sup>x1</sup>	1.000 <sup>x2</sup>				φ 384		3.21	6	19.26			
W19	D 16	B		13.250 <sup>x1</sup>						1.000 <sup>x1</sup>	14.25	12	171.00			
W20	D 16	B		1.800 <sup>x1</sup>							1.80	50	90.00			
W21	D 16	E		0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	327	408.75			
W22	D 16	B		5.050 <sup>x1</sup>							5.05	176	888.80			
W23	D 16	B		2.700 <sup>x1</sup>							2.70	156	421.20			
W24	D 16	B		2.230 <sup>x1</sup>							2.23	26	57.98			
W25	D 16	B		4.300 <sup>x1</sup>							4.30	4	17.20			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M													
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

C-170

Abo Gabal Reg. (Upstream) Rebar weight [4/6]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks			
				a	b	c	d	e	φ								
W26	D 16	B	—	1.896 <sup>x1</sup>							1.90	2	3.80				
W27	D 16	B	—	10.800 <sup>x1</sup>							10.80	6	64.80				
W28	D 16	B	—	8.950 <sup>x1</sup>							8.95	8	71.60				
W29	D 16	B	—	10.300 <sup>x1</sup>							10.30	1	10.30				
W30	D 16	B	—	1.396 <sup>x1</sup>							1.40	6	8.40				
W31	D 16	B	—	1.450 <sup>x1</sup>							1.45	6	8.70				
W32	D 16	B	—	3.800 <sup>x1</sup>							3.80	3	11.40				
W33	D 16	B	—	0.950 <sup>x1</sup>							0.95	6	5.70				
W34	D 16	B	—	1.746 <sup>x1</sup> ~1.800							1.78	6	10.68				
W35	D 12	E	┌	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	863	888.89				
W36	D 12	E	┌	0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	61	49.92				
W37	D 12	E	┌	0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	96	50.88				
W38	D 12	E	┌	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	24	24.72				
S 1	D 16	B	—	12.800 <sup>x1</sup>						1.000 <sup>x1</sup>	13.80	22	303.60				
S 2	D 16	B	—	0.800 <sup>x1</sup>							0.80	160	128.00				
S 3	D 16	E	┌	0.618 <sup>x1</sup>	0.268 <sup>x2</sup>						1.16	80	92.80				
S 4	D 16	B	—	12.800 <sup>x1</sup>						1.000 <sup>x1</sup>	13.80	66	910.80				
S 5	D 16	E	┌	6.300 <sup>x1</sup>	0.268 <sup>x2</sup>						6.84	40	273.60				
Type	A	B	C	D	E	F	G	H	I	J							
											Dia.	kg/m	R	L	J		
	K	L	M														
	D																
													D12	0.888	130	201	1000
													D14	1.21	150	236	1000
													D16	1.58	170	267	1000
													D18	2.00	190	298	1000
													D20	2.47	210	330	1100
												D22	2.98	240	377	1210	
												D25	3.85	270	424	1375	
												D28	4.83	300	471	1540	
												D32	6.31	340	534	1760	

C-171

Abo Gabal Reg. (Upstream) Rebar weight [5/6]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
S 6	D 16	E		6.300 <sup>x1</sup>	0.240 <sup>x2</sup>						6.78	40	271.20			
G 1	D 16	A		12.460 <sup>x1</sup>	4.680 <sup>x2</sup>	0.267 <sup>x3</sup>			φ 170	1.000 <sup>x3</sup>	25.36	7	177.52			
G 2	D 16	L		0.978 <sup>x1</sup>	4.680 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	6.93	1	6.93			
G 3	D 16	L		0.978 <sup>x1</sup>	4.680 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	6.93	1	6.93			
G 4	D 16	L		0.943 <sup>x1</sup>	4.680 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	6.89	1	6.89			
G 5	D 16	L		0.943 <sup>x1</sup>	4.680 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	6.89	1	6.89			
G 6	D 16	B		0.825 <sup>x1</sup> ~0.896							0.87	8	6.96			
G 7	D 16	B		1.425 <sup>x1</sup> ~1.496							1.47	6	8.82			
G 8	D 16	B		4.850 <sup>x1</sup>							4.85	118	572.30			
G 9	D 16	M		0.832 <sup>x2</sup>	0.332 <sup>x2</sup>	0.225 <sup>x2</sup>					2.78	90	250.20			
G10	D 16	M		1.532 <sup>x2</sup>	0.582 <sup>x2</sup>	0.225 <sup>x2</sup>					1.68	36	168.18			
G11	D 16	M		1.532 <sup>x3</sup>	0.332 <sup>x2</sup>	0.225 <sup>x2</sup>					4.18	54	225.72			
G12	D 12	E		0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	28	21.84			
G13	D 12	E		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	42	22.26			
T 1	D 16	E		12.800 <sup>x1</sup>	0.300 <sup>x2</sup>					1.000 <sup>x1</sup>	11.40	8	115.20			
T 2	D 16	E		12.800 <sup>x1</sup>	0.240 <sup>x2</sup>					1.000 <sup>x1</sup>	11.28	15	214.20			
T 3	D 16	C		1.113 <sup>x1</sup> ~1.148	0.240 <sup>x1</sup>						1.38	4	5.52			
T 4	D 16	B		0.825 <sup>x1</sup> ~0.896							0.87	8	6.96			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L	M													
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
											D20	2.47	210	330	1100	
											D22	2.98	240	377	1210	
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

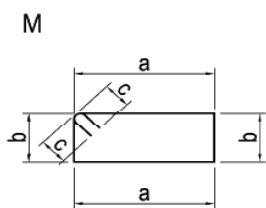
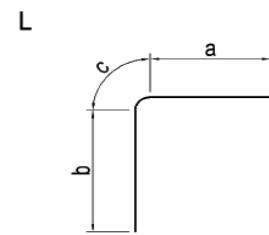
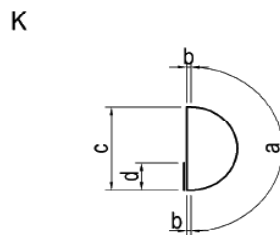
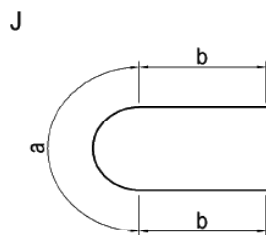
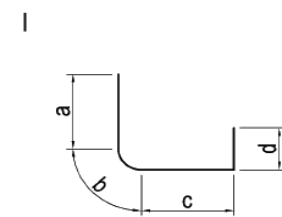
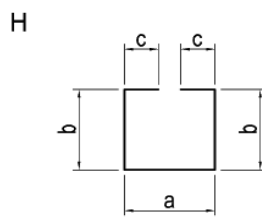
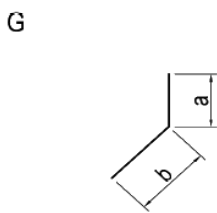
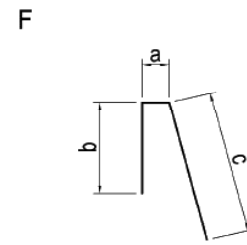
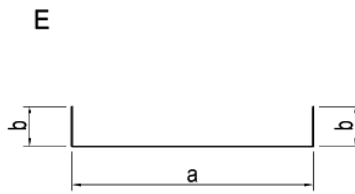
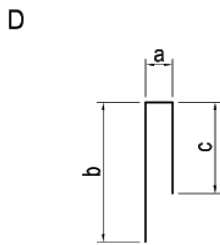
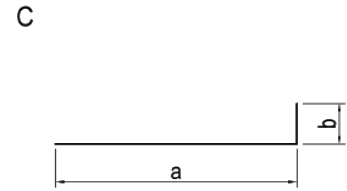
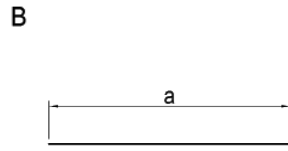
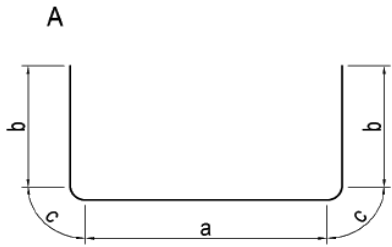
C-172

Abo Gabal Reg. (Upstream) Rebar weight [6/6]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
T 5	D 16	B	—	1.125 <sup>x1</sup> ~1,496							1.47	6	8.82			
T 6	D 16	E	┌	2.832 <sup>x1</sup>	0.332 <sup>x2</sup>						3.50	57	199.50			
T 7	D 16	E	┌	2.832 <sup>x1</sup>	0.240 <sup>x2</sup>						3.32	57	189.24			
T 8	D 16	C	┌	1.223 <sup>x1</sup>	0.332 <sup>x1</sup>						1.56	16	24.96			
T 9	D 16	C	┌	1.263 <sup>x1</sup>	0.332 <sup>x1</sup>						1.60	16	25.60			
T 10	D 16	C	┌	1.223 <sup>x1</sup>	0.240 <sup>x1</sup>						1.47	16	23.52			
T 11	D 16	C	┌	1.263 <sup>x1</sup>	0.240 <sup>x1</sup>						1.51	16	24.16			
T 12	D 16	B	—	12.800 <sup>x1</sup>						1.000 <sup>x1</sup>	13.80	4	55.20			
T 13	D 16	B	—	2.200 <sup>x1</sup>							2.20	64	140.80			
T 14	D 12	E	┌	0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	122	64.66			
D12                      2307.85 × 0.888                      =                      2049.37 kg                      =                      2.049 t																
D16                      22703.37 × 1.58                      =                      35871.32 kg                      =                      35.871 t																
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg/m	R	L	J	
											D12	0.888	130	201	1000	
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-173

## Schematic design of rebar (Upstream)





Abo Gabal Weir body      Quantity calculation

【Downstream】

Discription	Classification	Calculation	Quantity	Unit	Remarks
Demolition works					
Concrete demolition	PC structure	$2.00 \times 0.10 \times 18.20 = 3.64$	3.64	m <sup>3</sup>	
	RC structure	$2.00 \times 1.50 \times 18.00 = 54.00$			
		$(4.24 + 3.09) \times 4.75 \times 2 = 69.64$			
		Total = 123.64	123.64	m <sup>3</sup>	
Main body					
Leveling con.	$\sigma_{ck}=20\text{N}/\text{mm}^2$				
		$(13.20 - 16.30) \times 2 \times 9.60 \times 0.10 = 14.16$			
		$16.30 \times 3.00 \times 0.10 = 4.89$			
		Total = 19.05	19.05	m <sup>3</sup>	
Formwork	Leveling con.				
		$(10.088 + 3.00 + 12.60) \times 0.10 = 2.57$			
		Total = 2.57	2.57	m <sup>2</sup>	
Reinforced con.	$\sigma_{ck}=25\text{N}/\text{mm}^2$				
	Invert	$(13.00 - 16.10) \times 2 \times 1.50 \times 9.60 = 209.52$			
	"	$16.10 \times 1.50 \times 3.00 = 72.45$			
	Wall A	$1.00 \times 4.75 \times 12.60 = 59.85$			
	Wall B	$1/2 \times (1.00 + 2.10) \times 4.75 \times 9.60 = 70.68$			
	"	$2.10 \times 1.75 \times 3.00 = 29.93$			
	Wall C	$1/2 \times (1.00 + 2.10) \times 1.75 \times 9.60 = 70.68$			
	"	$2.10 \times 4.75 \times 3.00 = 29.93$			
	Wall E	$1/2 \times (1.00 + 1.90) \times 4.75 \times 9.60 = 66.12$			
	"	$1.90 \times 4.75 \times 3.00 = 27.08$			
		Total = 636.24	636.24	m <sup>3</sup>	



Discription	Classification	Calculation	Quantity	Unit	Remarks
Formwork	RC structure				
	Invert	$1.50 \times (10.088 + 3.00 + 12.60) = 38.53$			
	Wall A	$4.75 \times (10.088 + 3.00) \times 2 = 124.34$			
	Wall B	$4.75 \times (9.827 + 9.652 + 3.00 \times 2) = 121.03$			
	Wall C	$4.75 \times (12.60 + 9.663 + 3.00) = 120.00$			
	Wall E	$4.75 \times (9.642 + 3.00 - 12.60) = 119.90$			
		Total = 523.80	523.80	m <sup>2</sup>	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 = 1.919	1.919	t	
		D16 = 17.906	17.906	t	
Concrete string	PC structure	Leveling con. = 19.05	19.05	m <sup>3</sup>	
	RC structure	Reinforced con. = 636.24	636.24	m <sup>3</sup>	
Scaffolding	Prefabricated scaffold, H ≤ 30m				
	Outer Perimeter	$6.25 \times (10.09 + 3.17 - 12.60) = 161.63$			
	Inside	$4.75 \times (10.11 + 3.00 - 12.60 + 9.81 - 3.00 + 7.84) = 220.21$			
	"	$4.75 \times 14.60 = 220.21$			
		Total = 381.84	381.84	m <sup>2</sup>	
Expansion joint	Elastic filler, t=20mm				
		$13.00 \times 1.50 = 19.50$			Upstream
		$1.00 \times 4.75 \times 4 = 19.00$			"
		$16.10 \times 1.50 = 24.15$			Downstream
		$(1.00 + 2.10 \times 2 + 1.90) \times 4.75 = 33.73$			"
		Total = 96.38	96.38	m <sup>2</sup>	
Water-swelling water stop	Paste type, 9mm × 30mm				
		$14.65 + 5.50 \times 4 = 36.65$	36.65	m <sup>2</sup>	



Abo Gabal Reg. (Downstream) Rebar weight [1/5]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F1	D 16	A		12.187 <sup>×1</sup> ~15.458	5.880 <sup>×2</sup>	0.267 <sup>×2</sup>			φ 170	1.000 <sup>×4</sup>	30.27	21	726.18			
F2	D 16	A		15.560 <sup>×1</sup>	5.880 <sup>×2</sup>	0.267 <sup>×2</sup>			φ 170	1.000 <sup>×1</sup>	31.86	8	254.88			
F3	D 16	A		12.552 <sup>×1</sup> ~15.523	5.880 <sup>×2</sup>	0.267 <sup>×2</sup>			φ 170	1.000 <sup>×1</sup>	30.34	24	728.16			
F4	D 16	A		15.560 <sup>×1</sup>	5.880 <sup>×2</sup>	0.267 <sup>×2</sup>			φ 170	1.000 <sup>×1</sup>	31.86	7	223.02			
F5	D 16	B		12.827 <sup>×1</sup> ~15.863						1.000 <sup>×1</sup>	15.35	48	736.80			
F6	D 16	B		15.900 <sup>×1</sup>						1.000 <sup>×1</sup>	16.90	15	253.50			
F7	D 16	B		12.400 <sup>×1</sup>						1.000 <sup>×1</sup>	13.40	27	361.80			
F8	D 16	B		2.038 <sup>×1</sup> ~9.505							5.78	5	28.90			
F9	D 16	B		1.214 <sup>×1</sup> ~9.506							5.36	6	32.16			
F10	D 16	B		4.825 <sup>×1</sup> ~10.585							7.71	1	30.84			
F11	D 16	B		0.825 <sup>×1</sup> ~9.511							5.17	11	56.87			
F12	D 16	B		12.206 <sup>×1</sup>						1.000 <sup>×1</sup>	13.21	1	13.21			
F13	D 16	B		3.535 <sup>×1</sup> ~11.587							7.57	14	105.98			
F14	D 16	C		9.547 <sup>×1</sup>	2.895 <sup>×1</sup>					1.000 <sup>×1</sup>	13.45	1	13.45			
F15	D 16	C		9.569 <sup>×1</sup>	2.893 <sup>×1</sup>					1.000 <sup>×1</sup>	13.47	1	13.47			
F16	D 16	C		9.545 <sup>×1</sup>	2.906 <sup>×1</sup>					1.000 <sup>×1</sup>	13.46	1	13.46			
F17	D 16	C		9.737 <sup>×1</sup>	2.887 <sup>×1</sup>					1.000 <sup>×1</sup>	13.63	1	13.63			
F18	D 16	C		9.964 <sup>×1</sup>	2.918 <sup>×1</sup>					1.000 <sup>×1</sup>	13.89	1	13.89			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-179

Abo Gabal Reg. (Downstream) Rebar weight [2/5]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F19	D 16	C		10.002 <sup>x1</sup>	2.882 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	1	13.89			
F20	D 16	B		12.400 <sup>x1</sup>						1.000 <sup>x1</sup>	13.40	65	871.00			
F21	D 16	B		12.175 <sup>x1</sup>						1.000 <sup>x1</sup>	13.18	1	13.18			
F22	D 16	B		3.504 <sup>x1</sup> ~11.555							7.53	14	105.42			
F23	D 16	D		12.794 <sup>x1</sup>	0.240 <sup>x2</sup>					1.000 <sup>x1</sup>	11.28	6	85.68			
F24	D 16	D		15.868 <sup>x1</sup>	0.240 <sup>x2</sup>					1.000 <sup>x1</sup>	17.35	6	104.10			
F25	D 16	C		10.002 <sup>x1</sup>	2.884 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	3	41.67			
F26	D 16	C		10.002 <sup>x1</sup>	2.884 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	3	41.67			
F27	D 16	B		12.400 <sup>x1</sup>						1.000 <sup>x1</sup>	13.40	6	80.40			
F28	D 16	B		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	92	161.00			
F29	D 12	F		0.528 <sup>x1</sup>	1.328 <sup>x2</sup>	0.100 <sup>x2</sup>					3.39	333	1128.87			
W 1	D 16	E		6.050 <sup>x1</sup>	0.240 <sup>x1</sup>						6.29	421	2648.09			
W 2	D 16	C		10.002 <sup>x1</sup>	2.882 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	12	166.68			
W 3	D 16	C		10.002 <sup>x1</sup>	2.882 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	12	166.68			
W 4	D 16	C		9.964 <sup>x1</sup>	2.918 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	12	166.68			
W 5	D 16	C		9.964 <sup>x1</sup>	2.918 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	12	166.68			
W 6	D 16	C		9.737 <sup>x1</sup>	2.887 <sup>x1</sup>					1.000 <sup>x1</sup>	13.63	12	163.56			
W 7	D 16	C		9.737 <sup>x1</sup>	2.887 <sup>x1</sup>					1.000 <sup>x1</sup>	13.63	12	163.56			
Type	<div style="display: flex; justify-content: space-around; align-items: center;"> <span>A</span> <span>B</span> <span>C</span> <span>D</span> <span>E</span> <span>F</span> </div>											Dia.	kg/m	R	L	J
	D12	0.888	130	201	1000											
	D14	1.21	150	236	1000											
	D16	1.58	170	267	1000											
	D18	2.00	190	298	1000											
	D20	2.47	210	330	1100											
	D22	2.98	240	377	1210											
	D25	3.85	270	424	1375											
	D28	4.83	300	471	1540											
D32	6.31	340	534	1760												

C-180

Abo Gabal Reg. (Downstream) Rebar weight [3/5]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W 8	D 16	C		9.545 <sup>x1</sup>	2.906 <sup>x1</sup>					1.000 <sup>x1</sup>	13.46	12	161.52			
W 9	D 16	C		9.545 <sup>x1</sup>	2.906 <sup>x1</sup>					1.000 <sup>x1</sup>	13.46	12	161.52			
W10	D 16	B		12.400 <sup>x1</sup>						1.000 <sup>x1</sup>	13.40	12	160.80			
W11	D 16	C		9.569 <sup>x1</sup>	2.893 <sup>x1</sup>					1.000 <sup>x1</sup>	13.47	12	161.64			
W12	D 16	C		9.569 <sup>x1</sup>	2.893 <sup>x1</sup>					1.000 <sup>x1</sup>	13.47	12	161.64			
W13	D 16	C		9.547 <sup>x1</sup>	2.895 <sup>x1</sup>					1.000 <sup>x1</sup>	13.45	12	161.40			
W14	D 16	C		9.547 <sup>x1</sup>	2.895 <sup>x1</sup>					1.000 <sup>x1</sup>	13.45	12	161.40			
W15	D 16	D		0.800 <sup>x1</sup>	1.000 <sup>x2</sup>						2.80	14	39.20			
W16	D 16	D		0.790 <sup>x1</sup>	1.000 <sup>x2</sup>						2.79	47	131.13			
W17	D 16	D		1.900 <sup>x1</sup>	1.000 <sup>x2</sup>						3.90	11	51.60			
W18	D 16	D		0.831 <sup>x1</sup> ~1.886	1.000 <sup>x2</sup>						3.36	47	157.92			
W19	D 16	D		1.900 <sup>x1</sup>	1.000 <sup>x2</sup>						3.90	14	54.60			
W20	D 16	D		0.834 <sup>x1</sup> ~1.888	1.000 <sup>x2</sup>						3.37	47	158.39			
W21	D 16	D		1.700 <sup>x1</sup>	1.000 <sup>x2</sup>						3.70	11	51.80			
W22	D 16	D		0.828 <sup>x1</sup> ~1.690	1.000 <sup>x2</sup>						3.26	47	153.22			
W23	D 16	C		9.983 <sup>x1</sup>	2.900 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	2	27.78			
W24	D 16	C		9.983 <sup>x1</sup>	2.900 <sup>x1</sup>					1.000 <sup>x1</sup>	13.89	1	13.89			
W25	D 16	B		3.575 <sup>x1</sup> ~10.889							7.24	9	65.16			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	



C-181

Abo Gabal Reg. (Downstream) Rebar weight [4/5]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W26	D 16	B	—	7.575 <sup>×1</sup> ~8.670							8.13	2	16.26			
W27	D 16	B	—	1.815 <sup>×1</sup> ~5.655							3.74	2	7.48			
W28	D 16	B	—	12.400 <sup>×1</sup>						1.000 <sup>×1</sup>	13.40	4	53.60			
W29	D 16	B	—	4.203 <sup>×1</sup> ~11.185							7.70	5	38.50			
W30	D 16	B	—	4.495 <sup>×1</sup> ~10.895							7.70	4	30.80			
W31	D 16	B	—	12.400 <sup>×1</sup>						1.000 <sup>×1</sup>	13.40	4	53.60			
W32	D 16	D	[	0.756 <sup>×1</sup>	0.240 <sup>×2</sup>						1.24	24	29.76			
W33	D 16	D	[	0.776 <sup>×1</sup>	0.240 <sup>×2</sup>						1.26	24	30.24			
W34	D 16	D	[	0.779 <sup>×1</sup>	0.240 <sup>×2</sup>						1.26	24	30.24			
W35	D 16	D	[	0.777 <sup>×1</sup>	0.240 <sup>×2</sup>						1.26	24	30.24			
W36	D 16	D	]	0.768 <sup>×1</sup>	0.240 <sup>×2</sup>						1.25	24	30.00			
W37	D 16	D	]	1.868 <sup>×1</sup>	0.240 <sup>×2</sup>						2.35	48	112.80			
W38	D 16	D	]	1.668 <sup>×1</sup>	0.240 <sup>×2</sup>						2.15	24	51.60			
W39	D 12	D	]	0.818 <sup>×1</sup>	0.100 <sup>×2</sup>						1.02	132	134.64			
W40	D 12	D	]	0.828 <sup>×1</sup>	0.100 <sup>×2</sup>						1.03	39	40.17			
W41	D 12	D	]	0.859 <sup>×1</sup> ~1.914	0.100 <sup>×2</sup>						1.59	132	209.88			
W42	D 12	D	]	1.928 <sup>×1</sup>	0.100 <sup>×2</sup>						2.13	78	166.14			
W43	D 12	D	]	0.862 <sup>×1</sup> ~1.916	0.100 <sup>×2</sup>						1.59	132	209.88			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
											D32	6.31	340	534	1760	

C-182






Abo Gabal Reg. (Downstream) Rebar weight [5/5]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
W14	D 12	D		0.856 <sup>×1</sup> ~1.718	0.100 <sup>×2</sup>						1.49	132	196.68	
W45	D 12	D		1.728 <sup>×1</sup>	0.100 <sup>×2</sup>						1.93	39	75.27	

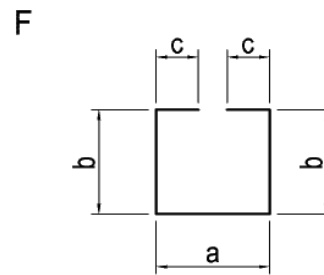
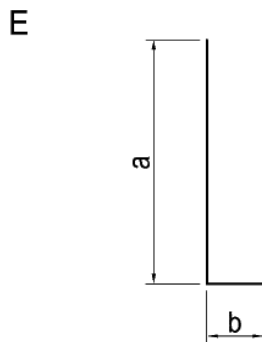
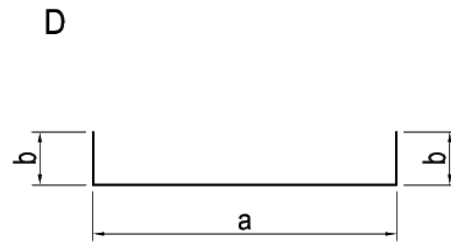
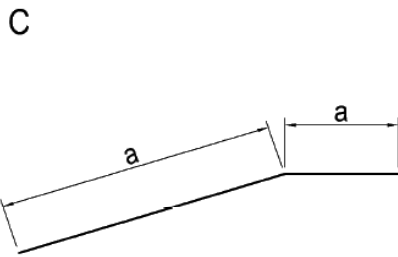
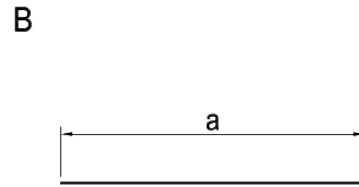
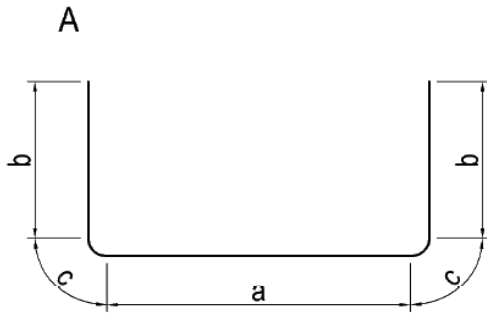
D12                      2161.53 > 0.888                      =                      1919.44 kg                      =                      1.919 t

D16                      11333.17 > 1.58                      =                      17906.41 kg                      =                      17.906 t

C-183

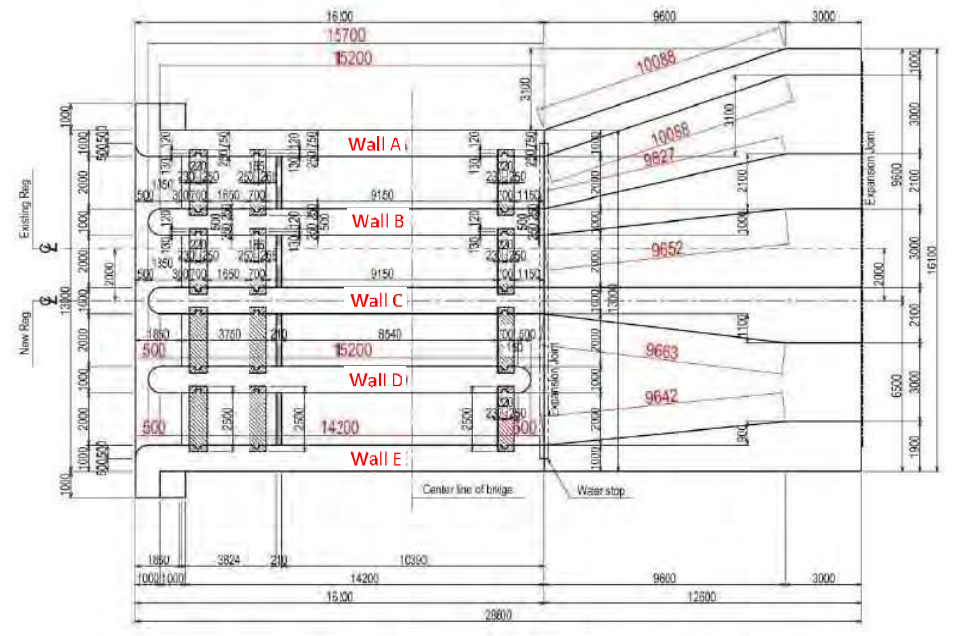
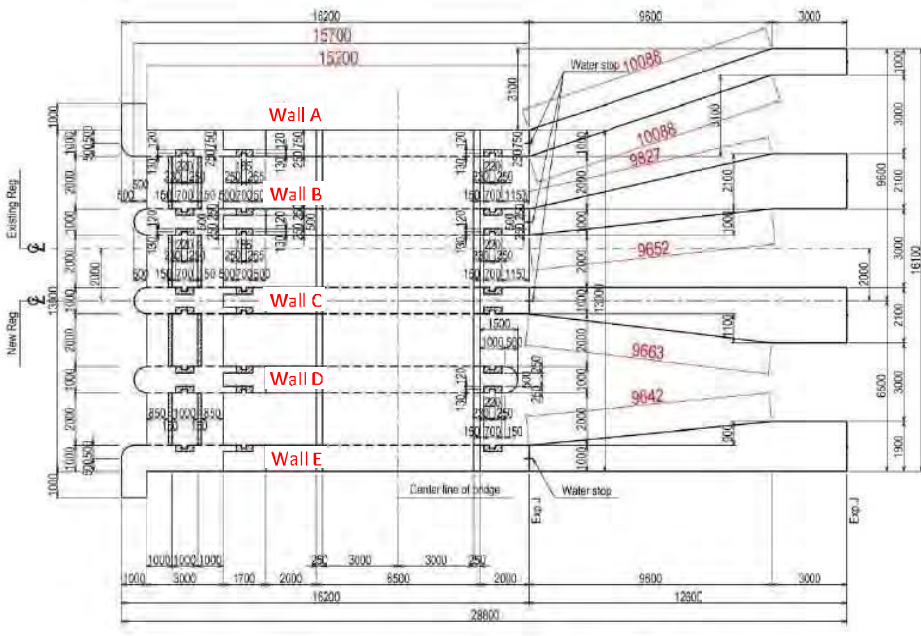
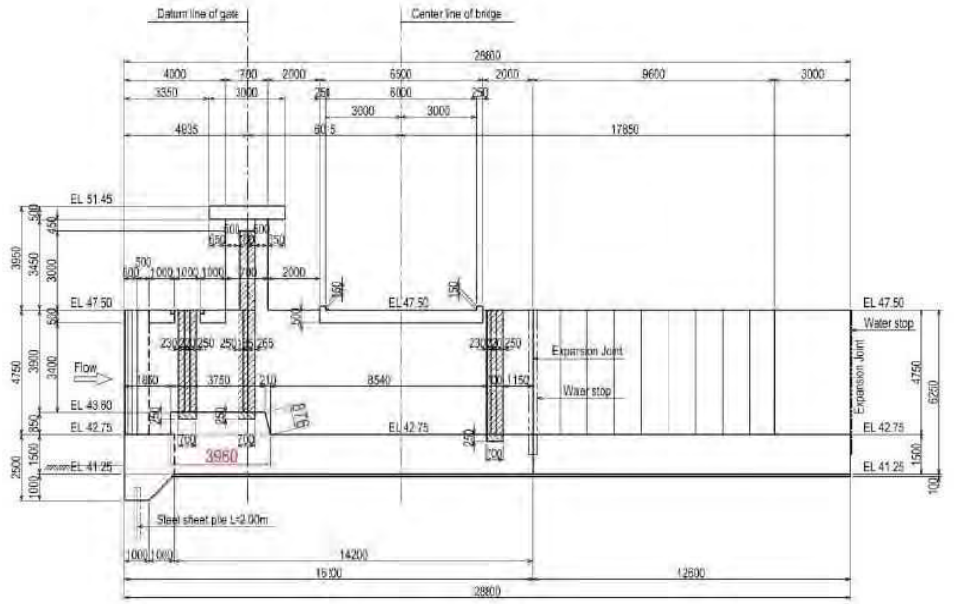
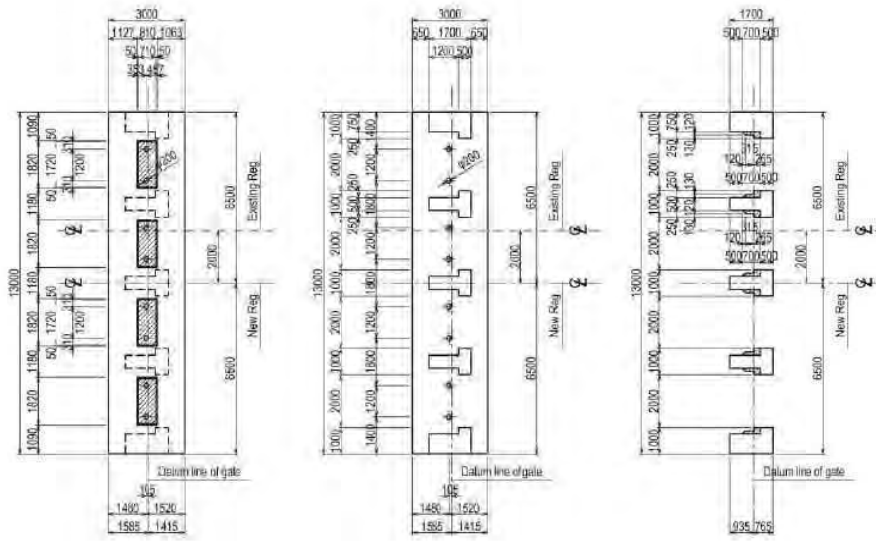
Type	A	B	C	D	E	F	Dia.	kg/m	R	L	J	
								D12	0.888	130	201	1000
								D14	1.21	150	236	1000
								D16	1.58	170	267	1000
								D18	2.00	190	298	1000
								D20	2.47	210	330	1100
								D22	2.98	240	377	1210
								D25	3.85	270	424	1375
								D28	4.83	300	471	1540
								D32	6.31	340	534	1760

Schematic design of rebar  
(Downstream)

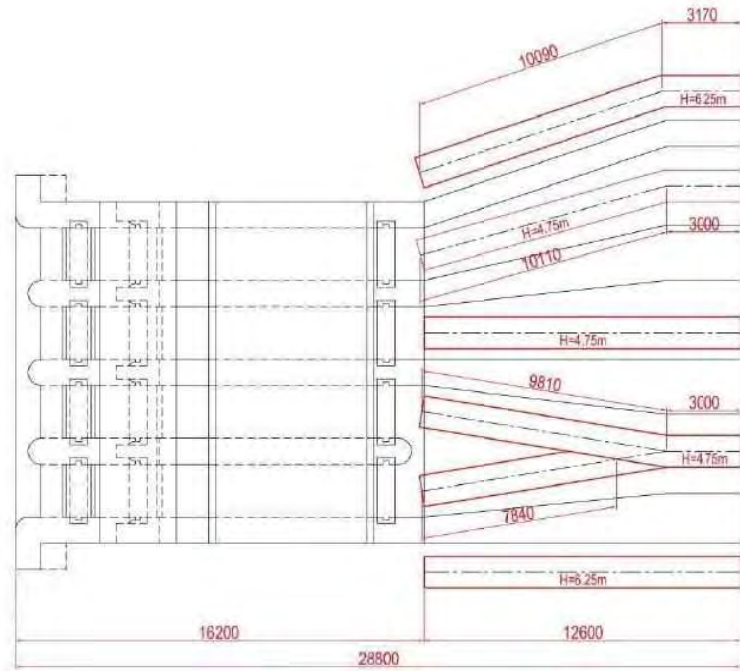
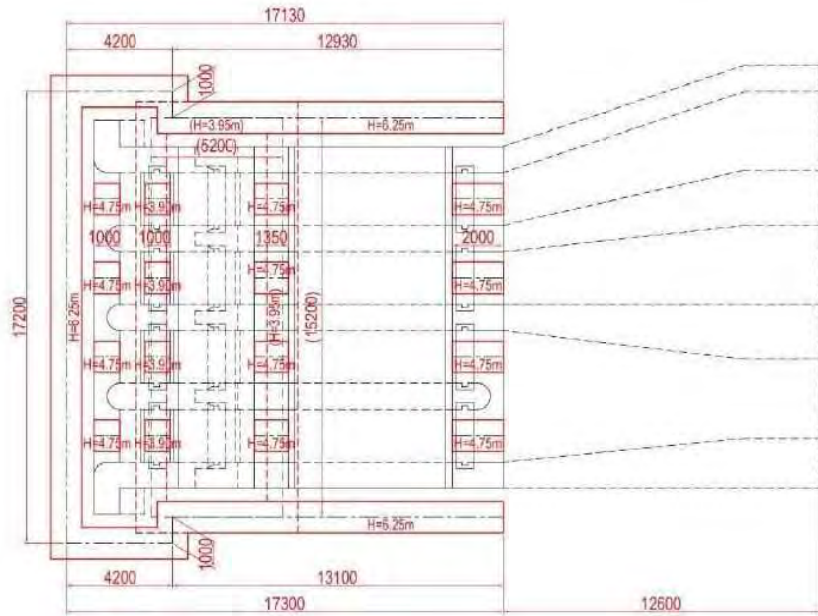
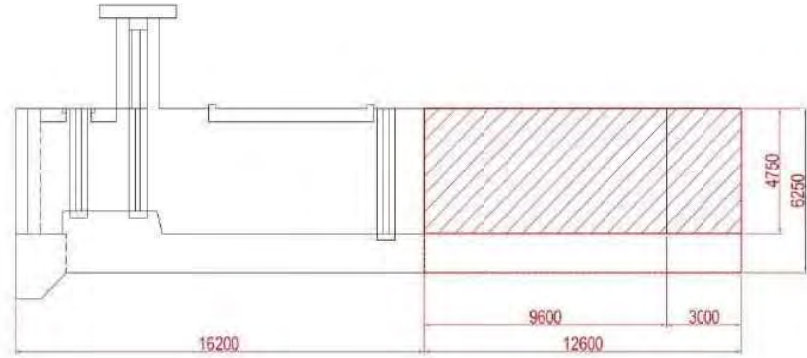
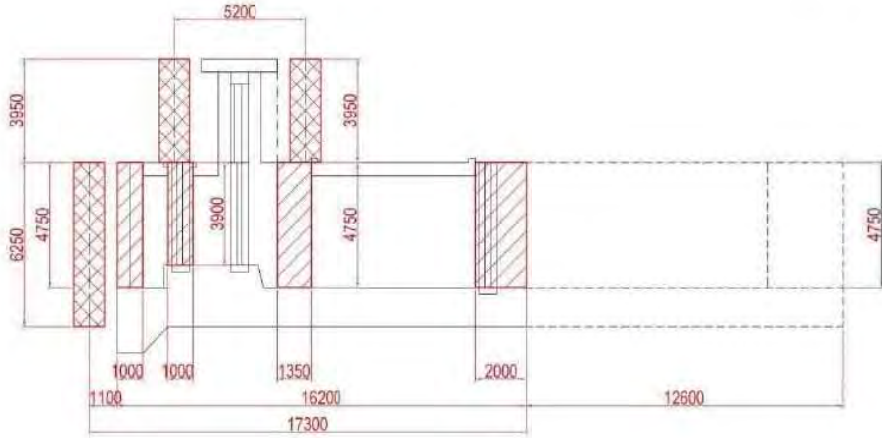




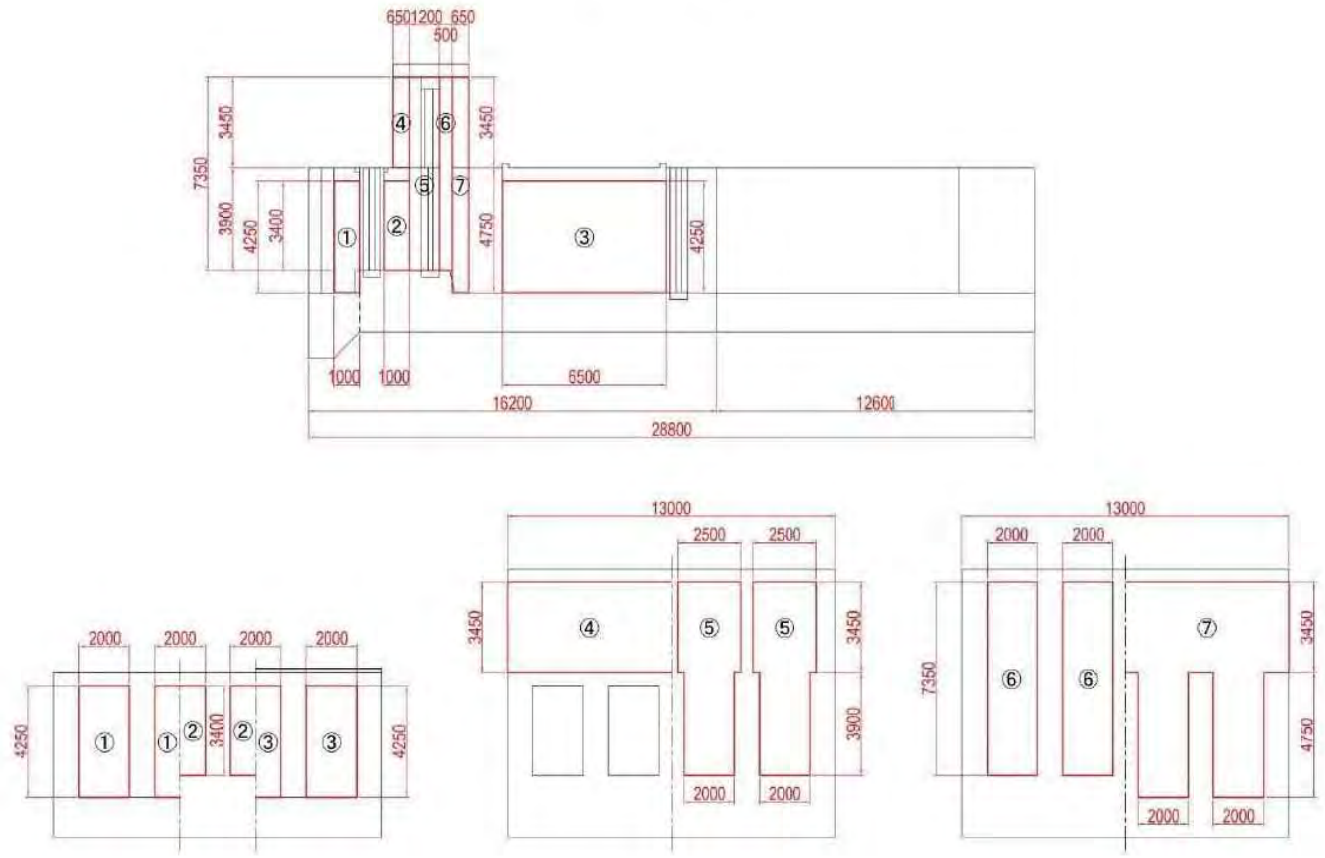
# Concrete / Formwork



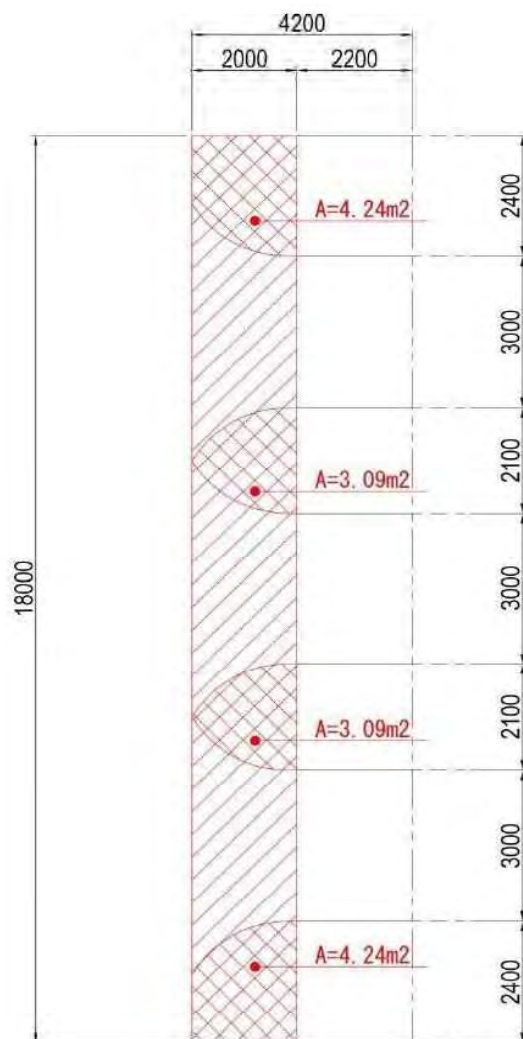
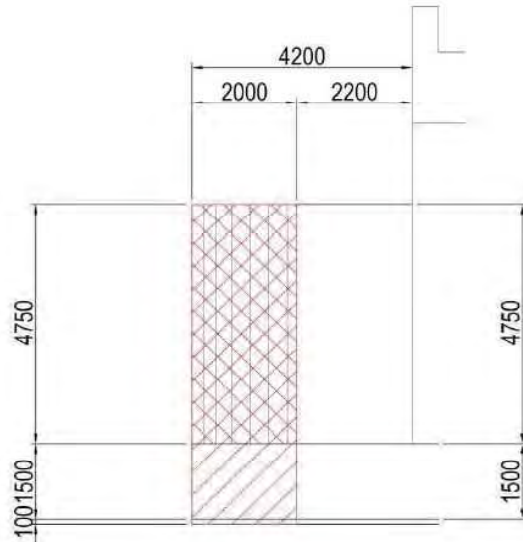
**Scaffolding**



# Falsework



## Concrete demolition



## (6) Sahelyia regulator

Sahelyia Weir body Quantity Summary Sheet							
Type of works	Class	Subdivision	Dimensions	Unit	Quantity	Up stream	Down stream
Weir body	Demolition works	Concrete demolition	PC structure	m <sup>3</sup>	2.14	—	2.14
			RC structure	m <sup>3</sup>	71.78	—	71.78
	Main body	Leveling con.	$\sigma_{ck}=20\text{N/mm}^2$	m <sup>3</sup>	15.61	9.91	5.70
		Formwork	Leveling con.	m <sup>2</sup>	4.76	3.54	1.22
		Reinforced con.	$\sigma_{ck}=25\text{N/mm}^2$	m <sup>3</sup>	752.37	526.13	226.24
		Formwork	RC structure	m <sup>2</sup>	1,060.31	830.05	230.26
		Circular formwork	RC structure $\phi 1000-1/2$ Circle	m <sup>2</sup>	9.35	9.35	—
		"	RC structure $\phi 1000-1/4$ Circle	m <sup>2</sup>	9.35	9.35	—
		Second stage con.	$\sigma_{ck}=25\text{N/mm}^2$	m <sup>3</sup>	13.38	13.38	—
		Formwork	RC structure	m <sup>2</sup>	72.00	72.00	—
		Rcbar	D12	t	1.831	1.201	0.630
		"	D16	t	31.097	23.901	7.193
		Concrete curing	PC structure	m <sup>3</sup>	15.61	9.91	5.70
		"	RC structure	m <sup>3</sup>	765.75	539.51	226.24
		Scaffolding	Prefabricated Scaffold, $11 \leq 30\text{m}$ average height, $1 \leq 30\text{m}$	m <sup>2</sup>	707.00	536.16	170.84
		Falsework	Support bearing force $40\text{KN/m}^2$ and less	m <sup>3</sup>	294.83	294.83	—
		Expansion joint	Elastic filler, $t=20\text{mm}$	m <sup>2</sup>	73.09	—	73.09
		Water stop	B-300mm	m	26.62	26.62	—
		Water-swelling water stop	Paste type, $9\text{mm} \times 30\text{mm}$	m	29.40	—	29.40
		Resin capsule anchor	D20 $\times$ 200, Drilling dia. $\phi 25$	nos	96	—	96
		Slip bar	D20 $\times$ 1.00m, VP $\phi 25 \times 0.50\text{m}$	nos	86	86	—
		"	D20 $\times$ 0.70m	nos	96	—	96
	Other miscellaneous	Joint bar	D16 $\times$ 200	nos	636	636	—
		Grating	2000 $\times$ 1000	set	2	2	—
	Safety facilities work	Guard fence	$11=1.10\text{m}$	m	29.80	29.80	—

Sahelyia Weir body Quantity calculation						
【Upstream】						
Discription	Classification	Calculation		Quantity	Unit	Remarks
Main body						
Leveling con.	σ ck=20N/mm <sup>2</sup>					
		$7.20 \times (13.20 + 13.30) / 2 \times 0.10$	—	9.51		
		$(7.20 - 7.746) / 2 \times 0.50 \times 0.10$	=	0.37		
			Total —	9.91	9.91	m <sup>3</sup>
Formwork	Leveling con.	$(13.20 + 13.30) / 2 \times 0.10 \times 2$	—	2.65		
		$(0.57 \times 2 + 7.746) \times 0.10$	—	0.89		
			Total =	3.51	3.51	m <sup>2</sup>
Reinforced con.	σ ck=25N/mm <sup>2</sup>					
	Cut-off	$1/2 \times (1.00 - 2.00) \times 1.00 \times 9.00$	=	13.50		
	Invert	$1.00 \times 1.50 \times 2.00 \times 2$	—	6.00		
	"	$7.00 \times 1.50 \times 15.20$	=	159.60		
	"	$(7.00 - 7.546) / 2 \times 1.50 \times 0.50$	—	5.45		
	"	$1/2 \times (3.75 - 4.11) \times 1.45 \times 2.00 \times 2$	=	22.79		Raising part
	"	$-0.70 \times 0.25 \times 2.50 \times 2 \times 3$	=	-2.63		(-)Blockout
	Wall A	$1.00 \times 5.95 \times 1.00$	—	5.95		
	"	$1.00 \times 5.95 \times 15.20$	=	90.41		
	"	$-(1 - \pi/4) \times 0.50^2 \times 5.95$	=	-0.32		
	"	$1/2 \times (1.00 - 1.031) \times 5.95 \times 0.50$	—	3.02		
	"	$-0.70 \times 4.50 \times 0.25 \times 2$	=	-1.58		(-)Blockout
	"	$-0.70 \times 5.95 \times 0.25$	—	-1.01		"
	Wall B	$1.00 \times 5.95 \times 11.70$	=	87.47		
	"	$-(1 - \pi/4) \times 0.50^2 \times 5.95 \times 2$	—	-0.64		
	"	$1/2 \times (1.00 - 1.172) \times 5.95 \times 0.50$	—	3.23		
	"	$-0.70 \times 4.50 \times 0.25 \times 2 \times 2$	=	-3.15		(-)Blockout
	"	$-0.70 \times 5.95 \times 0.25 \times 2$	—	-2.08		"

Discription	Classification	Calculation	Quantity	Unit	Remarks
	Wall C	$1.00 \times 5.95 \times 1.00$	= 5.95		
	"	$1.00 \times 5.95 \times 15.20$	= 90.11		
	"	$-(1 - \pi/4) \times 0.50^2 \times 5.95$	- 0.32		
	"	$1/2 \times (1.00 - 1.031) \times 5.95 \times 0.50$	- 3.02		
	"	$-0.70 \times 4.50 \times 0.25 \times 2$	= -1.58		(-)Blockout
	"	$-0.70 \times 5.95 \times 0.25$	- -1.01		"
	Corridor	$1.00 \times 0.50 \times 2.00 \times 2 \times 2$	= 4.00		
	"	$-0.15 \times 0.15 \times 2.00 \times 2 \times 2$	- -0.18		(-)Second stage con.
	Bridge	$6.50 \times 0.50 \times 2.00 \times 2$	= 13.00		
	"	$0.25 \times 0.15 \times 7.00 \times 2$	- 0.53		
	Gate pier	$(0.75 \times 1.20 + 1.00 \times 0.50) \times 1.05 \times 2$	- 11.31		
	"	$(0.50 \times 1.20 + 1.00 \times 0.50) \times 1.05$	= 4.46		
	Top slab	$3.00 \times 0.50 \times 7.00$	- 10.50		
		Total	- 526.13	526.13	m <sup>3</sup>
Formwork	RC structure				
	Cut-off	$1/2 \times (1.00 - 2.00) \times 1.00 \times 2$	= 3.00		Side
	"	$1.00 \times 9.00$	- 9.00		Edge
	Invert	$(9.00 - 7.546) \times 1.50$	= 24.82		Edge
	"	$1.00 \times 1.50 \times 2$	= 3.00		Overhanging part
	"	$1.50 \times (15.20 + 0.57) \times 2$	- 47.31		Side
	"	$(1.15 - 1.191) \times 2.00 \times 2$	= 11.78		Raising part
	"	$(0.70 - 2.50) \times 0.25 \times 3 \times 2$	= 4.80		Blockout part
	Wall A	$(1.50 - 1.00) \times 5.95$	- 14.88		
	"	$5.95 \times (15.20 + 0.57 - 1.00 + 14.70 - 0.556)$	- 190.55		
	"	$1.50 \times 0.25 \times 2 \times 2 + 5.95 \times 0.25 \times 2$	- 7.48		Blockout part
	"	$-1/2 \times (3.75 + 1.11) \times 1.45$	= -5.70		(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 2$	- -0.96		(-)Corridor
	"	$-6.50 \times 0.50$	- -3.25		(-)Bridge
	Wall B	$5.95 \times \{(14.20 + 0.507) \times 2 + 1.172\}$	= 181.99		

Discription	Classification	Calculation	Quantity	Unit	Remarks
	Wall B	$1.50 \times 0.25 \times 4 \times 2 + 5.95 \times 0.25 \times 1 = 14.95$			Blockout part
	"	$-1.2 \times (3.75 + 1.11) \times 1.45 \times 2 = -11.40$			(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 4 = -1.91$			(-)Corridor
	"	$-6.50 \times 0.50 \times 2 = -6.50$			(-)Bridge
	Wall C	$(1.50 - 1.00) \times 5.95 = 14.88$			
	"	$5.95 \times (15.20 + 0.57 - 1.00 + 14.70 - 0.556) = 190.55$			
	"	$4.50 \times 0.25 \times 2 \times 2 + 5.95 \times 0.25 \times 2 = 7.48$			Blockout part
	"	$-1.2 \times (3.75 + 4.11) \times 1.45 = -5.70$			(-)Raising part
	"	$-(1.00 \times 0.50 - 0.15 \times 0.15) \times 2 = -0.96$			(-)Corridor
	"	$-6.50 \times 0.50 = -3.25$			(-)Bridge
	Corridor	$(1.00 - 0.50 \times 2) \times 2.00 \times 2 \times 2 = 16.00$			
	Bridge	$(6.50 - 0.50 \times 2) \times 2.00 \times 2 = 30.00$			
	"	$(0.25 - 7.00) \times 2 \times 0.15 \times 2 = 4.35$			
	"	$0.25 \times 0.15 \times 4 = 0.15$			
	Gate pier	$(1.00 - 1.70) \times 2 \times 4.05 \times 3 = 65.61$			
	Top slab	$(3.00 - 7.00) \times 2 \times 0.50 = 10.00$			
	"	$3.00 \times 7.00 = 21.00$			
	"	$-(0.75 \times 1.20 + 1.00 \times 0.50) \times 2 = -2.80$			
	"	$-(0.50 \times 1.20 + 1.00 \times 0.50) = -1.10$			
		Total =	830.05	830.05	m <sup>2</sup>
Circular formwork	RC structure				
	ϕ 1000-1/2Circle	$\pi \times 2 \times 1.00 \times 5.95 = 9.35$	9.35	9.35	m <sup>2</sup>
	ϕ 1000-1/1Circle	$\pi / 1 \times 1.00 \times 5.95 \times 2 = 9.35$	9.35	9.35	m <sup>2</sup>



Discription	Classification	Calculation	Quantity	Unit	Remarks
Second stage con.	$\sigma_{ck}=25\text{N/mm}^2$				
		$0.70 \times 0.25 \times 2.50 \times 2 \times 3 = 2.63$			Invert part
		$(0.70 \times 0.25 - 0.235 \times 0.13) \times 4.50 \times 4 = 2.60$			Gate part
		$\{0.70 \times 0.25 - (0.365 \times 0.13 + 1/2 \times 0.12^2)\} \times 3.60 \times 4$			"
		$= 1.73$			
		$(0.70 \times 0.25 - 0.22 \times 0.13) \times (1.50 - 5.95) \times 1 = 6.12$			Stop log part
		$(0.15^2 - 0.05^2) \times 2.00 \times 2 \times 2 = 0.16$			Corridor part
		$0.05/6 \times \{(2 \times 0.83 + 0.73) \times 1.82 + (2 \times 0.73 + 0.83) \times 1.72\} \times 2$			Top slab part
		$= 0.14$			
		Total = 13.38	13.38	m <sup>3</sup>	
Formwork	RC structure				
		$(0.70 - 0.13 \times 2) \times 4.50 \times 4 = 17.28$			Gate part
		$\{0.365 + 0.215 - 0.13 + 0.12 \times \sqrt{(2)}\} \times 3.60 \times 4 = 12.67$			"
		$(0.70 - 0.13 \times 2) \times (4.50 + 5.95) \times 4 = 40.13$			Stop log part
		$0.15 \times 2.00 \times 2 \times 2 = 1.20$			Corridor part
		$1/2 \times (0.73 - 0.83) \times 0.05 \times \sqrt{(2)} \times 2 \times 2 = 0.22$			Top slab part
		$1/2 \times (1.72 - 1.82) \times 0.05 \times \sqrt{(2)} \times 2 \times 2 = 0.50$			"
		Total = 72.00	72.00	m <sup>2</sup>	
Rebar	Steel 360/520	refer to rebar weight calculation sheet			
		D12 = 1.201	1.201	t	
		D16 = 23.904	23.904	t	
Concrete curing	PC structure	Leveling con.	9.91	9.91	m <sup>3</sup>
	RC structure	526.13 + 13.38	539.51	539.51	m <sup>3</sup>

Discription	Classification	Calculation	Quantity	Unit	Remarks
Scaffolding	Prefabricated Scaffold, H ≤ 30m				
	Outer perimeter	$7.45 \times (11.20 + (16.02 + 1.00 - 0.89) \times 2)$ = 350.30			
	Inside	$5.95 \times (1.00 + 1.35 + 1.50) \times 2$ = 45.82			
	"	$4.50 \times 1.00 \times 2$ = 9.00			
	Gate pier part	$4.55 \times (5.20 + 9.20) \times 2$ = 131.04			
		Total = 536.16	536.16	m <sup>2</sup>	
Falsework	Wedge linked Falsework, H ≤ 30m, Support bearing force 40KN/m <sup>2</sup> and less				
	①	$2.00 \times 5.45 \times 1.00 \times 2$ = 21.80			Corridor part
	②	$2.00 \times 4.00 \times 1.00 \times 2$ = 16.00			"
	③	$2.00 \times 5.15 \times 6.50 \times 2$ = 141.70			Bridge part
	④	$7.00 \times 4.05 \times 0.65$ = 18.43			Gate pier part
	⑤	$(2.00 \times 4.50 + 2.50 \times 4.05) \times 1.20 \times 2$ = 45.90			"
	⑥	$2.00 \times 8.55 \times 0.50 \times 2$ = 17.10			"
	⑦	$(2.00 \times 5.95 \times 2 + 7.00 \times 4.05) \times 0.65$ = 33.90			"
		Total = 294.83	294.83	m <sup>3</sup>	
Water stop	B=300mm	$6.515 - 6.70 \times 3$ = 26.62	26.62	m <sup>2</sup>	
Slip bar	D20 × 1.00m, VP φ 25 × 0.50m	= 86.0	86	nos	
Other miscellaneous					
Joint bar	D16 × 200	$(126 + 84 + 108) \times 2$ = 636.0	636	nos	Gate, Stop log part
Grating	2000 × 1000	= 2.0	2	Set	
Safety facilities work					
Guard fence	H=1.10m	$6.70 \times 2 - 2.70 + 1.70$ = 17.80			Top slab
		$6.00 \times 2$ = 12.00			Corridor
		Total = 29.80	29.80	m	



Sahelyia Reg. (Upstream) Rebar weight [1/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F1	D 16	A		6.460 <sup>x1</sup>	7.080 <sup>x2</sup>	0.267 <sup>x2</sup>			φ 170	1.000 <sup>x3</sup>	21.16	77	1860.32			
F2	D 16	A		6.542 <sup>x1</sup> ~6.870	7.080 <sup>x2</sup>	0.267 <sup>x2</sup>			φ 170	1.000 <sup>x3</sup>	24.40	3	73.20			
F3	D 16	B		6.800 <sup>x1</sup>							6.80	100	680.00			
F4	D 16	B		6.881 <sup>x1</sup> ~7.210							7.05	3	21.15			
F5	D 16	C		15.500 <sup>x1</sup>						1.000 <sup>x1</sup>	16.50	35	577.50			
F6	D 16	B		1.400 <sup>x1</sup>							1.40	2	2.80			
F7	D 16	G		1.000 <sup>x1</sup>	0.422 <sup>x1</sup>						1.43	2	2.86			
F8	D 16	C		14.134 <sup>x1</sup>	1.134 <sup>x1</sup>					1.000 <sup>x1</sup>	16.27	4	65.08			
F9	D 16	C		0.500 <sup>x1</sup>	1.134 <sup>x1</sup>						1.64	2	3.28			
F10	D 16	C		0.457 <sup>x1</sup>	1.134 <sup>x1</sup>						1.60	2	3.20			
F11	D 16	B		2.750 <sup>x1</sup>							2.75	23	63.25			
F12	D 16	D		0.068 <sup>x1</sup>	2.334 <sup>x1</sup>	1.134 <sup>x1</sup>					3.54	20	70.80			
F13	D 16	E		1.418 <sup>x1</sup>	1.134 <sup>x2</sup>						3.69	20	73.80			
F14	D 16	F		0.193 <sup>x1</sup>	1.134 <sup>x1</sup>	2.374 <sup>x1</sup>					3.71	20	74.20			
F15	D 16	C		9.274 <sup>x1</sup>	1.134 <sup>x1</sup>						10.41	20	208.20			
F16	D 16	C		0.434 <sup>x1</sup>	1.134 <sup>x1</sup>						1.57	20	31.40			
F17	D 16	B		0.423 <sup>x1</sup>							0.43	2	0.86			
F18	D 16	B		5.050 <sup>x1</sup>							5.05	8	40.40			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L														
												D12	0.888	130	204	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
											D25	3.85	270	424	1375	
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

C-196

Sahelyia Reg. (Upstream) Rebar weight [2/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F19	D 16	B	—	3.634 <sup>x1</sup>							3.61	20	72.80			
F20	D 16	B	—	2.250 <sup>x1</sup>							2.25	28	63.00			
F21	D 16	E	┌	1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	47	82.25			
F22	D 16	E	└	6.768 <sup>x1</sup>	0.240 <sup>x2</sup>						7.25	6	43.50			
F23	D 16	E	┌	7.178 <sup>x1</sup>	0.240 <sup>x2</sup>						7.66	6	45.96			
F24	D 16	B	—	15.168 <sup>x1</sup>						1.000 <sup>x1</sup>	16.47	12	197.64			
F25	D 16	G	↘	1.000 <sup>x1</sup>	0.422 <sup>x1</sup>						1.43	12	17.16			
F26	D 16	B	—	1.900 <sup>x1</sup>							1.90	45	85.50			
F27	D 16	B	—	2.273 <sup>x1</sup>							2.28	35	79.80			
F28	D 16	G	↘	1.000 <sup>x1</sup>	1.331 <sup>x1</sup>						2.31	10	23.10			
F29	D 16	B	—	8.800 <sup>x1</sup>							8.80	10	88.00			
F30	D 16	B	—	1.900 <sup>x1</sup>							1.90	26	49.40			
F31	D 16	B	—	1.800 <sup>x1</sup>							1.80	12	21.60			
F32	D 16	B	—	1.000 <sup>x1</sup>							1.00	8	8.00			
F33	D 16	E	┌	1.300 <sup>x1</sup>	0.240 <sup>x2</sup>						1.78	8	14.24			
F34	D 16	E	└	1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	10	17.50			
F35	D 12	II	□	0.428 <sup>x1</sup>	1.328 <sup>x2</sup>	0.100 <sup>x2</sup>					3.29	110	361.90			
F36	D 12	II	□	0.428 <sup>x1</sup>	1.928 <sup>x2</sup>	0.100 <sup>x2</sup>					4.49	20	89.80			
Type	A	B	C	D	E	F	G	H	I	J						
											Dia.	kg/m	R	L	J	
	K	L										D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

C-197

Sahelyia Reg. (Upstream) Rebar weight [3/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
F37	D 12	H		0.428 <sup>x1</sup>	2.178 <sup>x2</sup>	0.100 <sup>x2</sup>					1.99	10	19.90	
F38	D 12	II		0.428 <sup>x1</sup>	1.078 <sup>x2</sup>	0.100 <sup>x2</sup>					2.79	10	27.90	
W 1	D 16	C		7.250 <sup>x1</sup>	0.240 <sup>x1</sup>						7.49	383	2868.67	
W 2	D 16	E		0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	42	53.76	
W 3	D 16	E		0.550 <sup>x1</sup>	0.240 <sup>x2</sup>						1.03	20	20.60	
W 4	D 16	E		0.781 <sup>x1</sup> ~0.791							0.79	4	3.16	
W 5	D 16	E		0.300 <sup>x1</sup>	0.240 <sup>x2</sup>						0.78	10	7.80	
W 6	D 16	E		0.831 <sup>x1</sup> ~0.883	0.240 <sup>x2</sup>						1.34	2	2.68	
W 7	D 16	E		0.800 <sup>x1</sup>	0.240 <sup>x2</sup>						1.28	10	12.80	
W 8	D 16	B		15.500 <sup>x1</sup>						1.000 <sup>x1</sup>	16.50	66	1089.00	
W 9	D 16	G		1.000 <sup>x1</sup>	0.422 <sup>x1</sup>						1.43	62	88.66	
W10	D 16	I		1.400 <sup>x1</sup>	0.603 <sup>x1</sup>	1.534 <sup>x1</sup>	0.768 <sup>x1</sup>		φ 384		4.31	52	224.12	
W11	D 16	E		1.418 <sup>x1</sup>	0.768 <sup>x2</sup>						2.96	52	153.92	
W12	D 16	E		8.918 <sup>x1</sup>	0.768 <sup>x2</sup>						10.46	52	543.92	
W13	D 16	E		0.464 <sup>x1</sup>	0.768 <sup>x1</sup>	0.763 <sup>x1</sup>					2.00	62	124.00	
W14	D 16	I		1.400 <sup>x1</sup>	0.603 <sup>x1</sup>	13.734 <sup>x1</sup>	0.768 <sup>x1</sup>		φ 384	1.000 <sup>x1</sup>	17.51	3	52.53	
W15	D 16	I		1.400 <sup>x1</sup>	0.603 <sup>x1</sup>	13.734 <sup>x1</sup>	0.768 <sup>x1</sup>		φ 384	1.000 <sup>x1</sup>	17.51	2	35.02	
W16	D 16	J		1.206 <sup>x1</sup>	1.050 <sup>x2</sup>				φ 384		3.31	26	86.06	

C-198

Type	A	B	C	D	E	F	G	II	I	J	Dia.	kg/m	R	L	J
											D12	0.888	130	201	1000
	K	L									D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

Sahelyia Reg. (Upstream) Rebar weight [4/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	$\phi$							
W17	D 16	B	—	1.450 <sup>x1</sup>							1.45	52	75.10			
W18	D 16	B	—	8.950 <sup>x1</sup>							8.95	52	465.40			
W19	D 16	E	┌	0.424 <sup>x1</sup>	0.753 <sup>x1</sup>	0.897 <sup>x1</sup>					2.08	62	128.96			
W20	D 16	J	└	1.206 <sup>x1</sup>	1.000 <sup>x2</sup>				$\phi$ 384		3.21	5	16.05			
W21	D 16	B	—	13.250 <sup>x1</sup>						1.000 <sup>x1</sup>	11.25	10	142.50			
W22	D 16	B	—	1.800 <sup>x1</sup>							1.80	62	111.60			
W23	D 16	E	┌	0.768 <sup>x1</sup>	0.240 <sup>x2</sup>						1.25	127	158.75			
W24	D 16	B	—	5.050 <sup>x1</sup>							5.05	100	505.00			
W25	D 16	B	—	2.250 <sup>x1</sup>							2.25	128	288.00			
W26	D 16	B	—	1.300 <sup>x1</sup>							1.30	1	17.20			
W27	D 16	B	—	1.896 <sup>x1</sup>							1.90	2	3.80			
W28	D 16	B	—	1.450 <sup>x1</sup>							1.45	4	5.80			
W29	D 16	B	—	10.300 <sup>x1</sup>							10.30	5	51.50			
W30	D 16	B	—	8.950 <sup>x1</sup>							8.95	1	35.80			
W31	D 16	B	—	0.440 <sup>x1</sup>							0.41	2	0.88			
W32	D 16	B	—	1.396 <sup>x1</sup>							1.40	2	2.80			
W33	D 16	B	—	3.800 <sup>x1</sup>							3.80	1	3.80			
W34	D 16	B	—	0.450 <sup>x1</sup>							0.45	2	0.90			
Type	A	B	C	D	E	F	G	H	I	J						
	K	L										Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
											D28	4.83	300	471	1540	
											D32	6.31	340	534	1760	

C-199

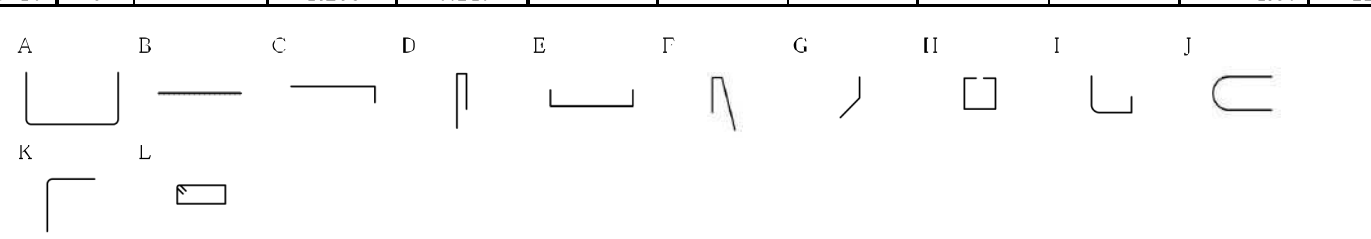
Sahelyia Reg. (Upstream) Rebar weight [5/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
W35	D 16	B	—	1.746 <sup>x1</sup> ~1,800							1.78	6	10.68		
W36	D 12	E	┌	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	603	621.09		
W37	D 12	E	┌	0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	78	60.84		
W38	D 12	E	┌	0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	39	20.67		
W39	D 12	E	┌	0.809 <sup>x1</sup>	0.100 <sup>x2</sup>						1.01	16	16.16		
W40	D 12	E	┌	0.859 <sup>x1</sup>	0.100 <sup>x2</sup>						1.06	8	8.48		
W41	D 12	E	┌	0.828 <sup>x1</sup>	0.100 <sup>x2</sup>						1.03	30	30.90		
S 1	D 16	B	—	6.800 <sup>x1</sup>							6.80	22	149.60		
S 2	D 16	B	—	0.800 <sup>x1</sup>							0.80	80	64.00		
S 3	D 16	E	┌	0.618 <sup>x1</sup>	0.268 <sup>x2</sup>						1.16	40	46.40		
S 4	D 16	B	—	12.800 <sup>x1</sup>						1.000 <sup>x1</sup>	13.80	66	910.80		
S 5	D 16	E	┌	6.300 <sup>x1</sup>	0.268 <sup>x2</sup>						6.84	20	136.80		
S 6	D 16	E	┌	6.300 <sup>x1</sup>	0.240 <sup>x2</sup>						6.78	20	135.60		
G 1	D 16	A	┌	6.460 <sup>x1</sup>	5.280 <sup>x2</sup>	0.267 <sup>x2</sup>			φ 170	1.000 <sup>x2</sup>	19.56	7	136.92		
G 2	D 16	K	┌	1.010 <sup>x1</sup>	5.280 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	7.56	1	7.56		
G 3	D 16	K	┌	1.010 <sup>x1</sup>	5.280 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	7.56	1	7.56		
G 4	D 16	K	┌	0.934 <sup>x1</sup>	5.280 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	7.49	1	7.49		
G 5	D 16	K	┌	0.934 <sup>x1</sup>	5.280 <sup>x1</sup>	0.267 <sup>x1</sup>			φ 170	1.000 <sup>x1</sup>	7.49	1	7.49		
Type	A	B	C	D	E	F	G	H	I	J					
											Dia.	kg/m	R	L	J
											D12	0.888	130	201	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
										D28	4.83	300	471	1540	
										D32	6.31	340	534	1760	

C-200



Sahelyia Reg. (Upstream) Rebar weight [6/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks			
				a	b	c	d	e	φ								
G 6	D 16	B	—	0.808 <sup>x1</sup> ~0.960							0.89	1	3.56				
G 7	D 16	B	—	1.408 <sup>x1</sup> ~1.560							1.49	2	2.98				
G 8	D 16	B	—	5.450 <sup>x1</sup>							5.45	62	337.90				
G 9	D 16	L	⌊	0.832 <sup>x2</sup>	0.332 <sup>x2</sup>	0.225 <sup>x2</sup>					2.78	63	175.14				
G10	D 16	L	⌊	1.532 <sup>x2</sup>	0.582 <sup>x2</sup>	0.225 <sup>x2</sup>					1.68	42	196.56				
G11	D 16	L	⌊	1.532 <sup>x2</sup>	0.332 <sup>x2</sup>	0.225 <sup>x2</sup>					1.18	21	87.78				
G12	D 12	E	⌊	0.578 <sup>x1</sup>	0.100 <sup>x2</sup>						0.78	30	23.40				
G13	D 12	E	⌊	0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	15	7.95				
T 1	D 16	E	⌊	6.800 <sup>x1</sup>	0.300 <sup>x2</sup>						7.40	8	59.20				
T 2	D 16	E	⌊	6.800 <sup>x1</sup>	0.240 <sup>x2</sup>						7.28	15	109.20				
T 3	D 16	C	⌊	1.114 <sup>x1</sup> ~1.180	0.240 <sup>x1</sup>						1.39	1	5.56				
T 4	D 16	B	—	0.809 <sup>x1</sup> ~0.960							0.89	4	3.56				
T 5	D 16	B	—	1.408 <sup>x1</sup> ~1.560							1.49	2	2.98				
T 6	D 16	E	⌊	2.832 <sup>x1</sup>	0.332 <sup>x2</sup>						3.50	31	108.50				
T 7	D 16	E	⌊	2.832 <sup>x1</sup>	0.240 <sup>x2</sup>						3.32	31	102.92				
T 8	D 16	C	⌊	1.253 <sup>x1</sup>	0.332 <sup>x1</sup>						1.59	12	19.08				
T 9	D 16	C	⌊	1.233 <sup>x1</sup>	0.332 <sup>x1</sup>						1.57	12	18.84				
T10	D 16	C	⌊	1.253 <sup>x1</sup>	0.240 <sup>x1</sup>						1.50	12	18.00				
Type	A	B	C	D	E	F	G	H	I	J							
	K	L											Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000	
												D14	1.21	150	236	1000	
												D16	1.58	170	267	1000	
												D18	2.00	190	298	1000	
												D20	2.47	210	330	1100	
												D22	2.98	240	377	1210	
												D25	3.85	270	424	1375	
												D28	4.83	300	471	1540	
											D32	6.31	340	534	1760		

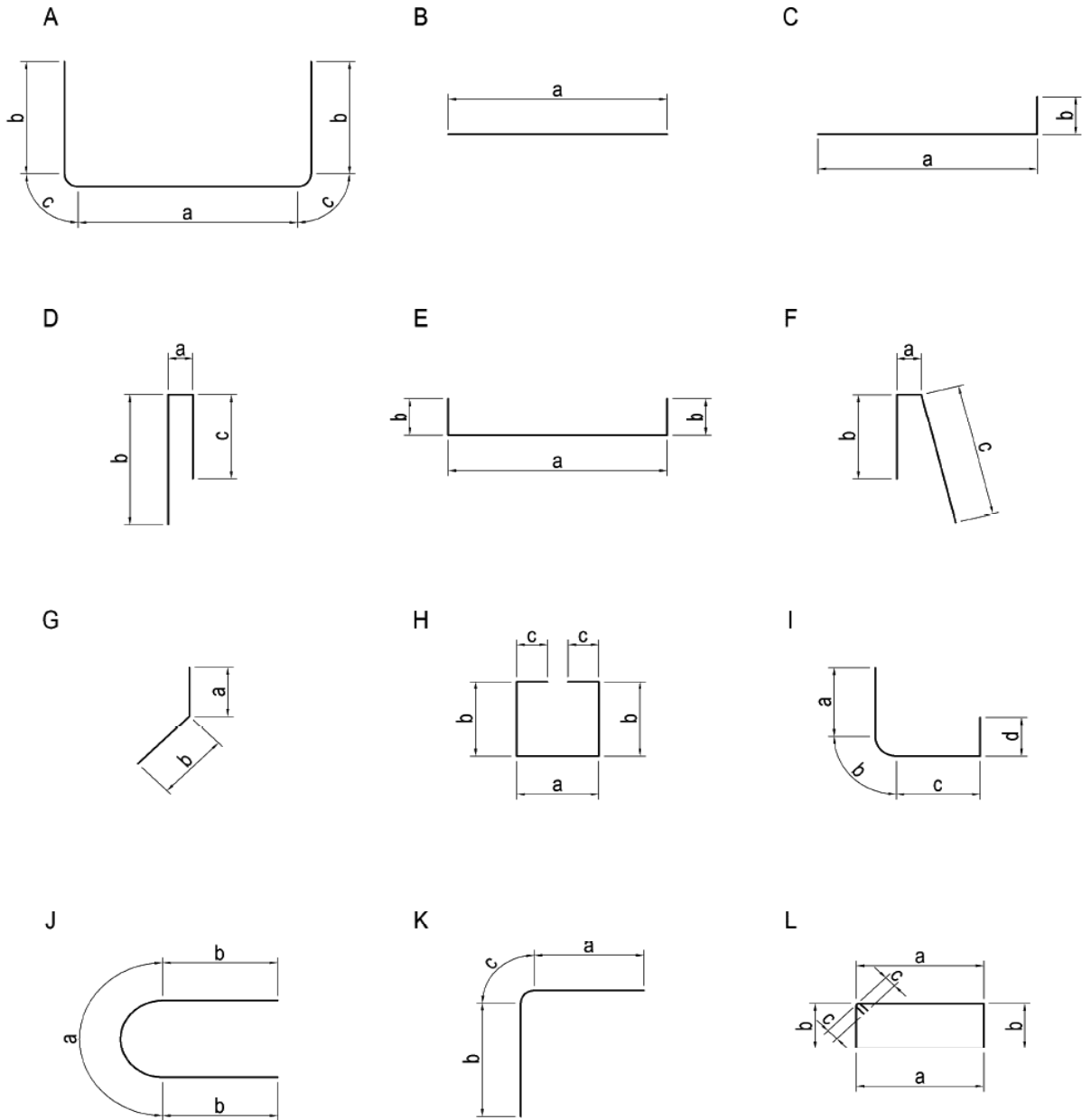
C-201

Sahelyia Reg. (Upstream) Rebar weight [7/7]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks	
				a	b	c	d	e	φ						
T11	D 16	C		1.233 <sup>x1</sup>	0.240 <sup>x1</sup>						1.48	12	17.76		
T12	D 16	B		6.400 <sup>x1</sup>							6.40	4	25.60		
T13	D 16	B		2.200 <sup>x1</sup>							2.20	32	70.40		
T14	D 12	E		0.328 <sup>x1</sup>	0.100 <sup>x2</sup>						0.53	64	33.92		
D12                      1352.91 × 0.888                      =                      1201.38 kg                      =                      1.201 t															
D16                      15129.31 × 1.58                      =                      23904.31 kg                      =                      23.904 t															
Type	A	B	C	D	E	F	G	H	I	J	Dia.	kg/m	R	L	J
											D12	0.888	130	201	1000
											D14	1.21	150	236	1000
											D16	1.58	170	267	1000
											D18	2.00	190	298	1000
											D20	2.47	210	330	1100
											D22	2.98	240	377	1210
											D25	3.85	270	424	1375
											D28	4.83	300	471	1540
											D32	6.31	340	534	1760

C-202

## Schematic design of rebar (Upstream)



Sahelyia Weir body      Quantity calculation						
【Downstream】						
Discription	Classification	Calculation		Quantity	Unit	Remarks
Demolition works						
Concrete demolition	PC structure	$2.00 \times 0.10 \times 10.70$	= 2.14	2.14	m <sup>3</sup>	
	RC structure	$2.00 \times 1.50 \times 10.50$	= 31.50			
		$(1.84 \times 2 + 3.09) \times 5.95$	= 40.28			
			Total = 71.78	71.78	m <sup>3</sup>	
Main body						
Leveling con.	$\sigma_{ck}=20N/mm^2$					
		$(7.746 + 10.70) \times 2 \times 2.70 \times 0.10$	= 2.49			
		$10.70 \times 3.00 \times 0.10$	= 3.21			
			Total = 5.70	5.70	m <sup>3</sup>	
Formwork	Leveling con.	$(3.077 + 3.00) \times 2 \times 0.10$	= 1.22			
			Total = 1.22	1.22	m <sup>3</sup>	
Reinforced con.	$\sigma_{ck}=25N/mm^2$					
	Invert	$(7.546 + 10.50) / 2 \times 1.50 \times 2.70$	= 36.51			
	"	$10.50 \times 1.50 \times 3.00$	= 47.25			
	Wall A	$1/2 \times (1.031 + 1.20) \times 5.95 \times 2.70$	= 17.92			
	"	$1.20 \times 5.95 \times 3.00$	= 21.42			
	Wall B	$1/2 \times (1.172 + 2.10) \times 5.95 \times 2.70$	= 26.28			
	"	$2.10 \times 5.95 \times 3.00$	= 37.49			
	Wall C	$1/2 \times (1.031 + 1.20) \times 5.95 \times 2.70$	= 17.92			
	"	$1.20 \times 5.95 \times 3.00$	= 21.42			
			Total = 226.24	226.24	m <sup>3</sup>	

Discription	Classification	Calculation	Quantity	Unit	Remarks
Formwork	RC structure				
	Invert	$1.50 \times (3.077 + 3.00) \times 2 = 18.23$			
	Wall A	$5.95 \times (3.077 + 3.00 + 3.00 \times 2) = 71.86$			
	Wall B	$5.95 \times (2.74 + 3.00) \times 2 = 68.31$			
	Wall C	$5.95 \times (3.077 + 3.00 + 3.00 \times 2) = 71.86$			
		Total — 230.26	230.26	m <sup>2</sup>	
Rebar	Steel 360/520	refer to rebar weight calculation sheet.			
		D12 = 0.630	0.630	t	
		D16 = 7.193	7.193	t	
Concrete curing	PC structure	Leveling con. = 5.70	5.70	m <sup>3</sup>	
	RC structure	Reinforced con. = 226.24	226.24	m <sup>3</sup>	
Scaffolding	Prefabricated scaffold, H ≤ 30m				
	Outer perimeter	$7.45 \times (3.36 + 3.17) \times 2 = 97.30$			
	Inside	$5.95 \times (3.18 + 3.00) \times 2 = 73.51$			
		Total — 170.84	170.84	m <sup>2</sup>	
Expansion joint	Elastic filler, t=20mm				
		$7.516 \times 1.50 = 11.32$			Upstream
		$(1.031 \times 2 + 1.172) \times 5.95 = 19.21$			"
		$10.50 \times 1.50 = 15.75$			Downstream
		$(1.20 \times 2 + 2.10) \times 5.95 = 26.78$			"
		Total — 73.09	73.09	m <sup>2</sup>	
Water-swelling water stop	Paste type, 9mm × 30mm				
		$9.30 + 6.70 \times 3 = 29.40$	29.40	m <sup>2</sup>	



Sahelyia Reg. (Downstream) Rebar weight [1/3]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
F1	D 16	A		7.088 <sup>x1</sup> ~9.879	7.080 <sup>x2</sup>	0.267 <sup>x2</sup>			φ 170	1.000 <sup>x3</sup>	26.18	11	366.52			
F2	D 16	A		9.960 <sup>x1</sup>	7.080 <sup>x2</sup>	0.267 <sup>x2</sup>			φ 170	1.000 <sup>x3</sup>	27.66	15	414.90			
F3	D 16	B		7.428 <sup>x1</sup> ~10.217							8.83	14	123.62			
F4	D 16	B		10.300 <sup>x1</sup>							10.30	15	154.50			
F5	D 16	B		5.500 <sup>x1</sup>							5.50	18	99.00			
F6	D 16	B		0.572 <sup>x1</sup> ~2.609							1.60	6	9.60			
F7	D 16	B		3.336 <sup>x1</sup> ~5.400							4.37	12	52.44			
F8	D 16	C		2.997 <sup>x1</sup>	2.870 <sup>x1</sup>						5.87	2	11.74			
F9	D 16	C		2.859 <sup>x1</sup>	2.927 <sup>x1</sup>						5.79	2	11.58			
F10	D 16	C		2.648 <sup>x1</sup>	2.890 <sup>x1</sup>						5.51	2	11.08			
F11	D 16	B		5.500 <sup>x1</sup>							5.50	37	203.50			
F12	D 16	B		3.240 <sup>x1</sup> ~5.434							4.34	14	60.76			
F13	D 16	D		7.392 <sup>x1</sup>	0.240 <sup>x2</sup>						7.88	6	47.28			
F14	D 16	D		10.268 <sup>x1</sup>	0.240 <sup>x2</sup>						10.75	6	64.50			
F15	D 16	C		2.997 <sup>x1</sup>	2.870 <sup>x1</sup>						5.87	12	70.44			
F16	D 16	D		1.268 <sup>x1</sup>	0.240 <sup>x2</sup>						1.75	56	98.00			
F17	D 12	F		0.528 <sup>x1</sup>	1.328 <sup>x2</sup>	0.100 <sup>x2</sup>					3.39	94	318.66			
W1	D 16	E		7.250 <sup>x1</sup>	0.240 <sup>x1</sup>						7.49	146	1093.54			
Type												Dia.	kg/m	R	L	J
												D12	0.888	130	201	1000
												D14	1.21	150	236	1000
												D16	1.58	170	267	1000
												D18	2.00	190	298	1000
												D20	2.47	210	330	1100
												D22	2.98	240	377	1210
												D25	3.85	270	424	1375
												D28	4.83	300	471	1540
												D32	6.31	340	534	1760

C-207






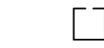
Sahelyia Reg. (Downstream) Rebar weight [2/3]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks		
				a	b	c	d	e	φ							
W 2	D 16	C		2.997 <sup>x1</sup>	2.870 <sup>x1</sup>						5.87	60	352.20			
W 3	D 16	C		2.859 <sup>x1</sup>	2.927 <sup>x1</sup>						5.79	60	347.40			
W 4	D 16	C		2.648 <sup>x1</sup>	2.890 <sup>x1</sup>						5.54	60	332.40			
W 5	D 16	D		0.822 <sup>x1</sup> ~0.972	1.000 <sup>x2</sup>						2.90	26	75.40			
W 6	D 16	D		1.000 <sup>x1</sup>	1.000 <sup>x2</sup>						3.00	28	84.00			
W 7	D 16	D		1.055 <sup>x1</sup> ~1.880	1.000 <sup>x2</sup>						3.47	13	45.11			
W 8	D 16	D		1.900 <sup>x1</sup>	1.000 <sup>x2</sup>						3.90	14	54.60			
W 9	D 16	C		2.925 <sup>x1</sup>	2.900 <sup>x1</sup>						5.83	8	46.64			
W10	D 16	B		5.500 <sup>x1</sup>							5.50	5	27.50			
W11	D 16	B		3.764 <sup>x1</sup> ~4.928							1.35	1	17.10			
W12	D 16	D		0.776 <sup>x1</sup>	0.240 <sup>x2</sup>						1.26	60	75.60			
W13	D 16	D		0.971 <sup>x1</sup>	0.240 <sup>x2</sup>						1.46	30	43.80			
W14	D 16	D		0.968 <sup>x1</sup>	0.240 <sup>x2</sup>						1.45	60	87.00			
W15	D 16	D		1.868 <sup>x1</sup>	0.240 <sup>x2</sup>						2.35	30	70.50			
W16	D 12	D		0.862 <sup>x1</sup> ~0.987	0.100 <sup>x2</sup>						1.13	81	91.92			
W17	D 12	D		1.028 <sup>x1</sup>	0.100 <sup>x2</sup>						1.23	98	120.54			
W18	D 12	D		1.152 <sup>x1</sup> ~1.839	0.100 <sup>x2</sup>						1.70	42	71.40			
W19	D 12	D		1.928 <sup>x1</sup>	0.100 <sup>x2</sup>						2.13	49	104.37			
Type												Dia.	kg/m	R	L	J
	D12	0.888	130	201	1000											
	D14	1.21	150	236	1000											
	D16	1.58	170	267	1000											
	D18	2.00	190	298	1000											
	D20	2.47	210	330	1100											
	D22	2.98	240	377	1210											
	D25	3.85	270	424	1375											
	D28	4.83	300	471	1540											
D32	6.31	340	534	1760												

C-208

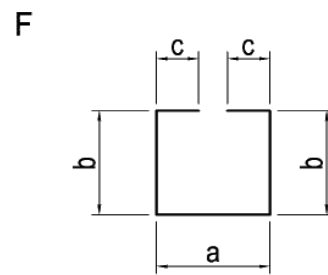
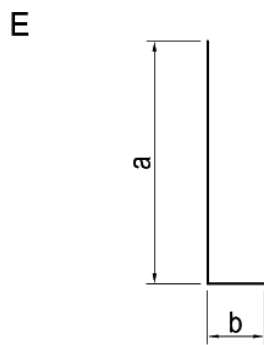
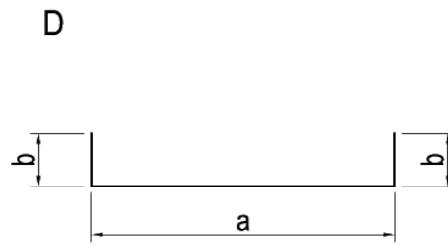
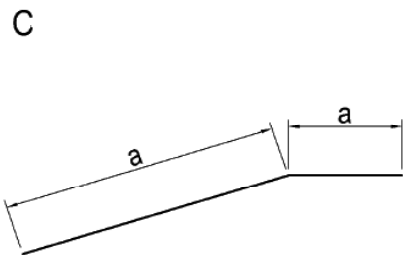
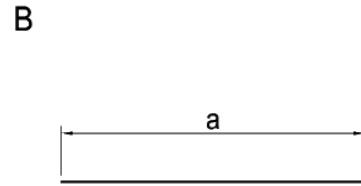
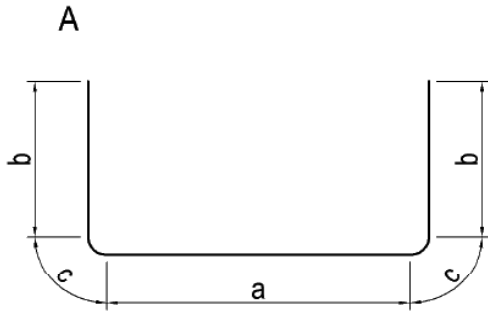


Sahelyia Reg. (Downstream) Rebar weight [3/3]

No.	Dia.	Type	Shape	L						Joint	length/nos	nos	total length	Remarks
				a	b	c	d	e	φ					
			D12	709.89 × 0.888		=	630.38 kg		=	0.630 t				
			D16	4552.55 × 1.58		=	7193.03 kg		=	7.193 t				
Type	A	B	C	D	E	F								
														
	Dia.	kg/m	R	L	J									
	D12	0.888	130	201	1000									
	D14	1.21	150	236	1000									
	D16	1.58	170	267	1000									
	D18	2.00	190	298	1000									
	D20	2.47	210	330	1100									
	D22	2.98	240	377	1210									
	D25	3.85	270	424	1375									
D28	4.83	300	471	1540										
D32	6.31	340	534	1760										

C-209

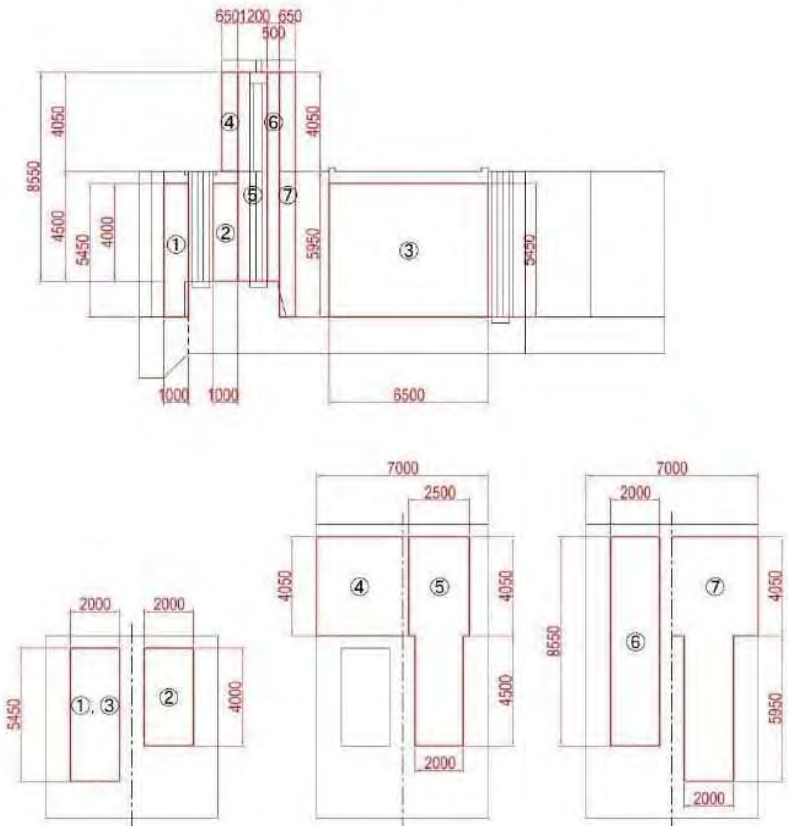
Schematic design of rebar  
(Downstream)



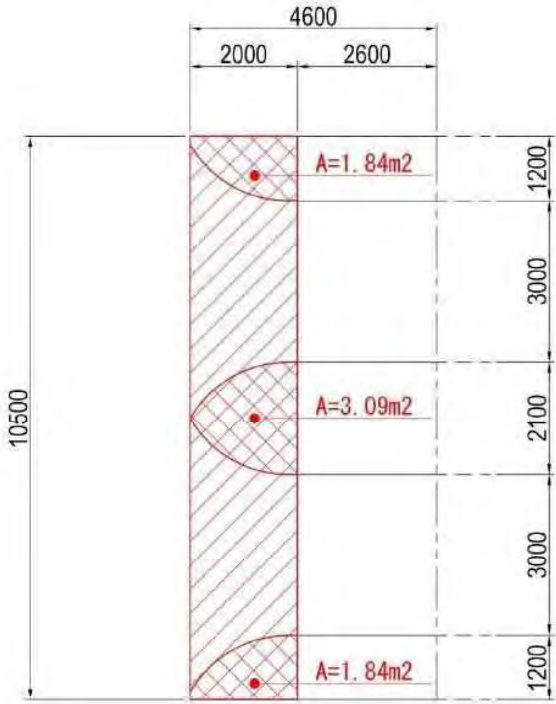
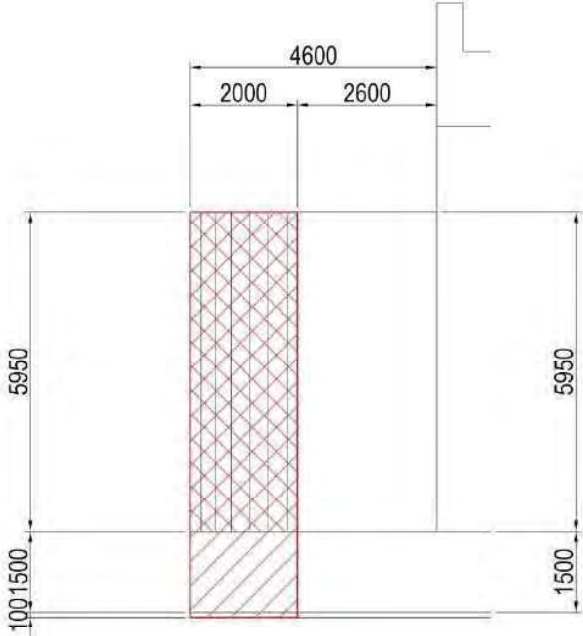




**Falsework**



**Concrete demolition**



## C-1-6. Quantity calculation of ancillary bridge works

### (1) Quantity summary table

Description	Classification	Unit	Bahr Yusef	Ibrahimia	Badraman		Quantity	Remarks
					Badraman	Dinoutiah		
1. Concrete works								
(1) Reinforced concrete	$\sigma_{ck}=25\text{N/mm}^2$ (cube)	m <sup>3</sup>	231.8	231.8	34.1	51.2	548.9	
(2) Form	RC stucture	m <sup>2</sup>	759.4	759.4	71.4	106.0	1696.3	
(3) Styrofoam	t=150	m <sup>2</sup>	19.2	19.2			38.4	
	t=100	m <sup>2</sup>	11.6	11.6			23.2	
	t=50	m <sup>2</sup>	36.8	36.8	11.4	11.4	96.3	
	t=20				22.0	33.0	55.0	
(4) Curing		m <sup>3</sup>	231.8	231.8	34.1	51.2	548.9	
(5) Reinforcing bar	Steel 360/520	kg	46,332	46,332	5,003	7,420	105,087	
(6) Falsework		m <sup>3</sup>	2,700	2,700	298	424	6,121	
2. Shoe								
(1) Rubber bearing	330×150×40	nos	10	10			20	
	330×300×40	nos	20	20			40	
	150×10,000×20	nos			1	1	2	
	300×10,000×20	nos			2	3	5	
(2) ST formula corrosion	F25D×710	set	4	4			8	
-pro of anchor	M25D×710	set	8	8			16	
	F25D×580	set			2	2	4	
	M25D×580	set			2	2	4	
(3) PD-Packings	150 <sup>b</sup> ×65×65	nos	4	4			8	
	150 <sup>b</sup> ×65×165	nos	8	8			16	
	20 <sup>t</sup> ×65×65	nos			2	2	4	
	20 <sup>t</sup> ×65×165	nos			2	2	4	
3. Expansion joint								
(1) Brof joint	Type NII - 20(Canige way)	m	20.2	20.2	20.2	20.2	80.8	
	Type NII - 20(Side walk)	m	4.0	4.0	4.0	4.0	16.0	
	Type NII - 35(Canige way)	m	10.1	10.1			20.2	
	Type NII - 35(Side walk)	m	2.0	2.0			4.0	
(2) Anchor bar		kg	205	205	102	102	614	
4. Asphalt pavement								
	Wearing course	m <sup>3</sup>	30.4	30.4	5.4	8.1	74.3	
5. Bulstrade								
(1) Bulstrade		m	67.6	67.6	12.0	18.0	165.2	
(2) Anchor bar		kg	625	625	108	159	1,517	

(2) Bahr Yusef and Ibrahimia reg.

### Ancillary bridge (Bahr Yusef , Ibrahimia) Quantity calculation

Description	Classification	Calculation	Quantity	Unit	Remarks
	Area	$a1 = 0.20 \times 0.25 \times 2$	= 0.10		
		$a2 = 1/2 \times (0.40 + 0.70) \times 1.25 \times 2$	= 1.38		
		$a3 = 0.30 \times 10.00$	= 3.00		
		$a4 = 1/2 \times 0.20 \times 0.60 \times 8$	= 0.48		
		$a5 = 0.75 \times 0.40 \times 5$	= 1.50		
		$A1 =$	6.46	m <sup>2</sup>	
		$a7 = 1/2 \times (0.80 + 2.00) \times 0.20 \times 4$	= 1.12		
		$a8 = 0.65 \times 2.00 \times 4$	= 5.20		
		$A2 =$	6.32	m <sup>2</sup>	
1. Concrete works					
(1) Reinforced concrete $\sigma_{ck}=25\text{N/mm}^2$ (cube)					
		$A1 \times 16.90 \times 2$	= 218.18		
		$A2 \times 0.40 \times 3 \times 2$	= 15.17		
		$-) (0.10 \times 10.10 + 0.08 \times 2.00) \times 0.33 \times 2 \times 2$	= -1.54		
		Total	231.80	m <sup>3</sup>	
(2) Form					
	RC structure	$(0.60 + (0.30^2 + 1.25^2)^{1/2}) \times 2 \times 16.90 \times 2$	= 127.46		
		$(0.75 \times 2 + 0.40) \times 5 \times 16.90 \times 2$	= 321.10		
		$((0.20^2 + 0.60^2)^{1/2} \times 2 + 0.80) \times 4 \times 16.90 \times 2$	= 279.18		



Description	Classification	Calculation	Quantity	Unit	Remarks
		-) $((0.65+(0.20^2+0.60^2)^{1/2}) \times 2 + 0.80) \times 0.40 \times 12 \times 2 =$	-32.30		
		$(0.20+0.20) \times 16.90 \times 2 \times 2 =$	27.04		
		-) $0.40 \times (1.20+1.00+1.20) \times 5 \times 2 =$	-13.60		
		$A2 \times 4 \times 2 =$	50.56		
		Total	759.43	m <sup>2</sup>	
(3) Styrofoam	t=150	$2.00 \times 0.40 \times 4 \times 3 \times 2$	19.20	m <sup>2</sup>	
	t=100	$A1+A2-0.10 \times 10.10-0.08 \times 2.00$	11.61	m <sup>2</sup>	
	t=50	$(A1+A2-0.10 \times 10.10-0.08 \times 2.00) \times 2 =$	23.21		
		$0.40 \times (1.20+1.00+1.20) \times 5 \times 2 =$	13.60		
		Total	36.81	m <sup>3</sup>	
(4) Curing			231.80	m <sup>3</sup>	
(5) Reinforcing bar (Steel 360/520)	D10		= 3,773		
	D12		= 5,763		
	D16		= 23,248		
	D25		= 13,548		
		Total	46,332	kg	
(6) Falsework		$(8.57 \times 12.50 + 1/2 \times (11.25+10.25) \times 0.50) \times 6.00 \times 4$	2,700	m <sup>3</sup>	
2. Shoe					
① Rubber bearing	330 × 150 × 40		10	nos	Fix.
	330 × 300 × 40		20	nos	Mov.
② ST formula corrosion-proof anchor					
	F25D × 710		4	set	Fix.



(3)Badraman (Badraman) reg.

Ancillary bridge (Badraman) Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
	Area	$a1 = 0.20 \times (0.25 + 1.25) \times 2 = 0.60$			
		$a2 = 0.30 \times 12.50 = 3.75$			
		$A1 = 4.35$	4.35	m <sup>2</sup>	
		$a3 = A1 = 4.35$			
		$a4 = 0.20 \times 12.50 = 2.50$			
		$A2 = 6.85$	6.85	m <sup>2</sup>	
1. Concrete works					
(1) Reinforced concrete $\sigma_{ck}=25\text{N/mm}^2$ (cube)					
		$A2 \times (0.50 + 1.00 + 0.50) = 13.70$			
		$1/2 \times (A1 + A2) \times 0.60 \times 4 = 13.44$			
		$A1 \times 0.80 \times 2 = 6.96$			
		<b>Total</b>	<b>34.10</b>	<b>m<sup>3</sup></b>	
(2) Form RC structure					
		$0.90 \times (0.50 + 1.00 + 0.50) \times 2 = 3.60$			
		$1/2 \times (0.90 + 0.70) \times 0.60 \times 4 \times 2 = 3.84$			
		$0.70 \times 0.80 \times 2 \times 2 = 2.24$			
		$0.20 \times 4 \times 6.00 = 4.80$			
		$(0.50 \times 2 + (0.20^2 + 0.60^2)^{1/2} \times 4 + 0.80 \times 2 + 1.00) \times 12.50 = 76.62$			

Description	Classification	Calculation	Quantity	Unit	Remarks
		-) $11.00 \times (0.50 + 1.00 + 0.50)$	= -22.00		
		$(0.10 \times 10.10 + 0.08 \times 2.00) \times 2$	= 2.34		
		Total	71.44	m <sup>2</sup>	
(3) Styrofoam	t=50	$(A2 - 0.10 \times 10.10 - 0.08 \times 2) \times 2$	11.36	m <sup>2</sup>	
	t=20	$11.00 \times (0.50 + 1.00 + 0.50)$	22.00	m <sup>2</sup>	
(4) Curing			34.10	m <sup>3</sup>	
(5) Reinforcing bar	D10	=			
(Steel 360/520)	D12	=	497		
	D16	=	4,506		
	D25	=			
		Total	5,003	kg	
(6) Falsework		$5.95 \times 12.50 \times 2.00 \times 2$	298	m <sup>3</sup>	
2. Shoe					
① Rubber bearing	150 × 10,000 × 20		1	nos	Fix.
	300 × 10,000 × 20		2	nos	Mov.
② ST formula corrosion-proof anchor					
	F25D × 580		2	set	Fix.
	M25D × 580		2	set	Mov.
③ PD-Packings					
	20 <sup>t</sup> × 65 × 65		2	nos	Fix.
	20 <sup>t</sup> × 65 × 165		2	nos	Mov.



(4)Badraman (Diroutiah) reg.

### Ancillary bridge (Diroutiah) Quantity calculation

Description	Classification	Calculation	Quantity	Unit	Remarks
	Area	$a1 = 0.20 \times (0.25 + 1.25) \times 2 = 0.60$			
		$a2 = 0.30 \times 12.50 = 3.75$			
		$A1 = 4.35$	4.35	m <sup>2</sup>	
		$a3 = A1 = 4.35$			
		$a4 = 0.20 \times 12.50 = 2.50$			
		$A2 = 6.85$	6.85	m <sup>2</sup>	
<b>1. Concrete works</b>					
(1) Reinforced concrete $\sigma_{ck}=25\text{N/mm}^2$ (cube)					
		$A2 \times (0.50 + 1.00 + 1.00 + 0.50) = 20.55$			
		$1/2 \times (A1 + A2) \times 0.60 \times 6 = 20.16$			
		$A1 \times 0.80 \times 3 = 10.44$			
		<b>Total</b>	<b>51.15</b>	<b>m<sup>3</sup></b>	
<b>(2) Form RC structure</b>					
		$0.90 \times (0.50 + 1.00 + 1.00 + 0.50) \times 2 = 5.40$			
		$1/2 \times (0.90 + 0.70) \times 0.60 \times 6 \times 2 = 5.76$			
		$0.70 \times 0.80 \times 3 \times 2 = 3.36$			
		$0.20 \times 4 \times 9.00 = 7.20$			
		$(0.50 \times 2 + (0.20^2 + 0.60^2)^{1/2} \times 6 + 0.80 \times 3 + 1.00 \times 2) \times 12.50 = 114.93$			

Description	Classification	Calculation	Quantity	Unit	Remarks
		-) $11.00 \times (0.50 + 1.00 + 1.00 + 0.50)$ = -33.00			
		$(0.10 \times 10.10 + 0.08 \times 2.00) \times 2$ = 2.34			
		Total	105.99	m <sup>2</sup>	
(3) Styrofoam	t=50	$(A2 - 0.10 \times 10.10 - 0.08 \times 2) \times 2$	11.36	m <sup>2</sup>	
	t=20	$11.00 \times (0.50 + 1.00 + 1.00 + 0.50)$	33.00	m <sup>2</sup>	
(4) Curing			51.15	m <sup>3</sup>	
(5) Reinforcing bar	D10	=			
(Steel 360/520)	D12	= 757			
	D16	= 6,663			
	D25	=			
		Total	7,420	kg	
(6) Falsework		$5.65 \times 12.50 \times 2.00 \times 3$	424	m <sup>3</sup>	
2. Shoe					
① Rubber bearing	150 × 10,000 × 20		1	nos	Fix.
	300 × 10,000 × 20		3	nos	Mov.
② ST formula corrosion-proof anchor					
	F25D × 580		2	set	Fix.
	M25D × 580		2	set	Mov.
③ PD-Packings					
	20 <sup>t</sup> × 65 × 65		2	nos	Fix.
	20 <sup>t</sup> × 65 × 165		2	nos	Mov.







(2) Steel sheet pile

Steel sheet pile revetment																							
Name of Reg.		Bahr-Yusef						Ibrahimia						Sahelyia	Diroutiah			Badraman		Abo Gabal	Total		
TYPE		A	BL-1	BL-2	BR-1	BR-2	C-1	C-2	D-1	D-2	E-1	E-2	F	G-1	G-2	H	I	J	K	L		M	
Construction length		66.60m	6.60m	6.60m	4.80m	9.00m	3.60m	9.00m	3.60m	5.40m	3.00m	1.80m	72.60m	6.60m	6.00m	19.20m	6.60m	4.20m	6.60m	3.60m	45.00m		
Front sheet pile	type		PU18	PU18	PU18	PU18	PU18	PU18	PU18	PU18	PU12	PU12	PU12	PU18	PU12	PU12	PU12	PU12	PU12	PU12	PU12		
	length	m	21.5	16.5	11.0	16.5	11.5	19.0	17.0	16.0	9.5	13.5	8.0	17.5	15.0	9.5	11.0	9.0	8.0	10.0	8.0	9.5	
	piece	piece	111	11	11	8	15	6	15	6	9	5	3	121	11	10	32	11	7	11	6	75	
	weight	t	183.52	13.96	9.31	10.15	13.27	8.77	19.61	7.38	5.65	5.29	1.59	162.84	10.91	6.28	23.27	6.54	3.70	8.46	3.17	47.10	
	Penetration depth	m	14.20	9.70	7.80	9.70	7.20	11.70	12.20	8.70	6.70	7.50	6.50	10.20	7.50	6.60	5.75	7.10	6.65	7.95	6.50	4.60	
painting	m <sup>2</sup>	1,105.56	102.96	145.20	74.88	95.40	59.76	104.40	59.76	102.60	42.00	28.80	1,205.16	92.40	114.00	240.00	118.80	67.20	132.00	57.60	468.00	4,416.48	
Bracing sheet pile	type		PU12	PU12		PU12	PU12	PU12	PU12	PU12		PU12		PU12	PU12		PU12				PU12		
	length	m	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0		6.0					6.0	
	piece	piece	94	11		8	15	6	15	6		5		104	11		32					72	
	weight	t	37.28	4.36		3.17	5.95	2.38	5.95	2.38		1.98		41.25	4.36		12.69						28.56
	Penetration depth	m	5.00	5.00		5.00	5.00	5.00	5.00	5.00		5.00		5.00	5.00		5.00						5.00
painting	m <sup>2</sup>	676.80	79.20		57.60	108.00	43.20	108.00	43.20		36.00		748.80	79.20		230.40						518.40	2,728.80
Tie-Rod	diameter	mm	46	42		42	25	46	32	46		42		46	42		42					32	
	length	m	12.0 ~14.0	12.0		12.0	7.5	11.0	7.5	10.0		9.0		11.5 ~13.5	10.0		8.5					7.5 ~9.0	
	number	nos.	28	3		2	4	2	4	2		2		31	3		9					20	
	total length	m	351.00	36.00		24.00	30.00	22.00	30.00	20.00		18.00		371.50	30.00		76.50						152.00
Waling	材料		UPN 220×80	UPN 200×75		UPN 200×75	UPN 180×70	UPN 220×80	UPN 180×70	UPN 220×80		UPN 200×75		UPN 220×80	UPN 200×75		UPN 180×70					UPN 160×65	
	延長	m	246.00	26.40		19.20	36.00	14.40	36.00	14.40		12.00		540.00	26.40		76.80					177.40	
	Weight	t	7.23	0.67		0.49	0.79	0.42	0.79	0.42		0.30		15.88	0.67		1.69					3.34	
Coping concrete																							
	Concrete	σ ck=25N/mm2	m3	128.43	9.90	4.90	7.31	13.18	7.81	18.97	7.81	7.15	6.65	2.47	140.81	13.92	8.06	40.11	2.66	1.80	2.66	1.58	91.71
Form	RC structure	m2	401.53	36.51	17.72	27.53	47.91	26.28	59.94	24.98	20.16	21.47	8.14	440.15	43.42	23.62	128.50	14.04	9.72	14.04	8.64	289.60	1,663.90
Re-bar	D12	t	3.387	0.310	0.154	0.158	0.289	0.207	0.506	0.207	0.172	0.176	0.059	3.716	0.367	0.138	1.065	0.084	0.057	0.084	0.049	2.432	13.62
Curing	RC structure	m3	128.43	9.90	4.90	7.31	13.18	7.81	18.97	7.81	7.15	6.65	2.47	140.81	13.92	8.06	40.11	2.66	1.80	2.66	1.58	91.71	517.89
Expantion joint	Elastic filler,t=20mm	m2	12.70	0.00	0.72	0.72	0.72	1.30	1.30	1.30	0.00	1.30	0.00	14.00	0.00	1.30	4.62	0.72	0.72	0.72	0.72	10.68	53.54

C-226

## Steel sheet pile revetment (TYPE A)      Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE A	$L = 14.30+19.125+33.80-0.25-0.375 = 66.60$	66.60	m	
Front sheet pile	PU18(S270GP), L=21.5m				
		$N = 66.60/0.60 = 111.0$	111	piece	
		$W = 111 \times 21.50 \times 0.0769 = 183.522$	183.52	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = (13.20+10.20+33.00)/0.6 = 94.0$	94	piece	
		$W = 94 \times 6.0 \times 0.0661 = 37.280$	37.28	t	
Penetration depth	PU18	$L = 22.20-8.00 = 14.20$	14.20	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (8.00-0.70+1.00) \times 0.60 \times 2 \times 111 = 1105.56$			PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 94 = 676.80$			PU12
		<b>Total = 1782.36</b>	1782.36	m <sup>2</sup>	
Tie-Rod	Φ46	$L = 12.00 \times 20 + 13.50 \times 2 + 14.00 \times 6 = 351.00$	351.00	m	
Waling	UPN 220×80	$L = (111+94) \times 0.6 \times 2 = 246.00$	246.00	m	
		$W = 246.00 \times 0.0294 = 7.232$	7.23	t	
Coping concrete	σ <sub>ck</sub> =25N/mm <sup>2</sup>	$1.30 \times 1.00 \times (14.30+19.125+33.80) = 87.39$			
		$0.80 \times 0.90 \times (13.40+10.40+33.20) = 41.04$			
		<b>Total = 128.43</b>	128.43	m <sup>3</sup>	
Form	RC structure	$(1.00 + 1.30 \times 2) \times (14.30+19.125+33.80) = 242.01$			
		$1.30 \times 1.00 \times 7 = 9.10$			
		$(0.90+0.80 \times 2) \times (13.40+10.40+33.20) = 142.50$			



## Steel sheet pile revetment (TYPE BL-1) Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE BL-1	$L = 6.60$	$= 6.60$	6.60	m
Front sheet pile	PU18(S270GP), L=16.5m				
		$N = 6.60/0.60$	$= 11.0$	11	piece
		$W = 11 \times 16.50 \times 0.0769$	$= 13.957$	13.96	t
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 6.60/0.6$	$= 11.0$	11	piece
		$W = 11 \times 6.0 \times 0.0661$	$= 4.363$	4.36	t
Penetration depth	PU18	$L = 16.70-7.00$	$= 9.70$	9.70	m/piece
	PU12	$L = 6.00-1.00$	$= 5.00$	5.00	m/piece
Painting on steel sheet pile	GL-1.00m both sides	$A = (7.00-0.20+1.00) \times 0.60 \times 2 \times 11$	$= 102.96$		PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 11$	$= 79.20$		PU12
		<b>Total =</b>	<b>182.16</b>	<b>182.16</b>	<b>m<sup>2</sup></b>
Tie-Rod	Φ42	$L = 12.00 \times 3$	$= 36.00$	36.00	m
Waling	UPN 200×75	$L = (11+11) \times 0.6 \times 2$	$= 26.40$	26.40	m
		$W = 26.40 \times 0.0253$	$= 0.668$	0.67	t
Coping concrete	σck=25N/mm <sup>2</sup>	$0.80 \times 0.90 \times 6.94$	$= 5.00$		
		$0.80 \times 0.90 \times 6.80$	$= 4.90$		
		<b>Total =</b>	<b>9.90</b>	<b>9.90</b>	<b>m<sup>3</sup></b>
Form	RC structure	$(0.90 + 0.80 \times 2) \times 6.94$	$= 17.35$		
		$0.80 \times 0.90 \times 1$	$= 0.72$		
		$(0.90 + 0.80 \times 2) \times 6.80$	$= 17.00$		



## Steel sheet pile revetment (TYPE BL-2) Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE BL-2	$L = 6.60$	= 6.60	6.60	m
Steel sheet pile	PU18(S270GP), L=11.0m				
		$N = 6.60/0.60$	= 11.0	11	piece
		$W = 11 \times 11.00 \times 0.0769$	= 9.305	9.31	t
Penetration depth	PU18	$L = 11.30-3.50$	= 7.80	7.80	m/piece
Painting on steel sheet pile	entire length both sides	$A = 11.00 \times 0.60 \times 2 \times 11$	= 145.20	145.20	m <sup>2</sup>
Coping concrete	$\sigma_{ck}=25\text{N/mm}^2$				
		$0.80 \times 0.90 \times 6.80$	= 4.90	4.90	m <sup>3</sup>
Form	RC structure				
		$(0.90 + 0.80 \times 2) \times 6.80$	= 17.00		
		$0.80 \times 0.90 \times 1$	= 0.72		
		<b>Total =</b>	<b>17.72</b>	<b>17.72</b>	<b>m<sup>3</sup></b>
Re-bar	D12				
		① $34 \times 2.15 \times 0.888 / 1000$	= 0.065		
		② $15 \times 6.70 \times 0.888 / 1000$	= 0.089		
		<b>Total =</b>	<b>0.154</b>	<b>0.154</b>	<b>t</b>
Curing	RC structure		= 4.90	4.90	m <sup>3</sup>
Expansion joint	elastic filler, t=20mm				
		$0.80 \times 0.90 \times 1$	= 0.72	0.72	m <sup>2</sup>

## Steel sheet pile revetment (TYPE BR-1) Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE BR-1	$L = 4.80$	= 4.80	4.80	m
Front sheet pile	PU18(S270GP), L=16.5m				
		$N = 4.80/0.60$	= 8.0	8	piece
		$W = 8 \times 16.50 \times 0.0769$	= 10.151	10.15	t
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 4.80/0.60$	= 8.0	8	piece
		$W = 8 \times 6.0 \times 0.0661$	= 3.173	3.17	t
Penetration depth	PU18	$L = 16.70-7.00$	= 9.70	9.70	m/piece
	PU12	$L = 6.00-1.00$	= 5.00	5.00	m/piece
Painting on steel sheet pile	GL-1.00m both sides	$A = (7.00-0.20+1.00) \times 0.60 \times 2 \times 8$	= 74.88		PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 8$	= 57.60		PU12
		<b>Total =</b>	<b>132.48</b>	<b>132.48</b>	<b>m<sup>2</sup></b>
Tie-Rod	Φ42	$L = 12.00 \times 2$	= 24.00	24.00	m
Waling	UPN 200 × 75	$L = 4.80 \times 2 \times 2$	= 19.20	19.20	m
		$W = 19.20 \times 0.0253$	= 0.486	0.49	t
Coping concrete	σck=25N/mm <sup>2</sup>				
		$0.80 \times 0.90 \times (5.148 + 5.00)$	= 7.31	7.31	m <sup>3</sup>
Form	RC structure				
		$(0.90 + 0.80 \times 2) \times (5.148 + 5.00)$	= 25.37		
		$0.80 \times 0.90 \times 3$	= 2.16		
		<b>Total =</b>	<b>27.53</b>	<b>27.53</b>	<b>m<sup>3</sup></b>





## Steel sheet pile revetment (TYPE BR-2) Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE BR-2	$L = 9.00$	= 9.00	9.00	m
Front sheet pile	PU18(S270GP), L=11.5m				
		$N = 9.00/0.60$	= 15.0	15	piece
		$W = 15 \times 11.50 \times 0.0769$	= 13.265	13.27	t
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 9.00/0.60$	= 15.0	15	piece
		$W = 15 \times 6.0 \times 0.0661$	= 5.949	5.95	t
Penetration depth	PU18	$L = 11.70-4.50$	= 7.20	7.20	m/piece
	PU12	$L = 6.00-1.00$	= 5.00	5.00	m/piece
Painting on steel sheet pile	GL-1.00m both sides	$A = (4.50-0.20+1.00) \times 0.60 \times 2 \times 15$	= 95.40		PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 15$	= 108.00		PU12
		<b>Total =</b>	<b>203.40</b>	<b>203.40</b>	<b>m<sup>2</sup></b>
Tie-Rod	Φ25	$L = 7.50 \times 4$	= 30.00	30.00	m
Waling	UPN 180 × 70	$L = 9.00 \times 2 \times 2$	= 36.00	36.00	m
		$W = 36.00 \times 0.022$	= 0.792	0.79	t
Coping concrete	σ <sub>ck</sub> =25N/mm <sup>2</sup>				
		$0.80 \times 0.90 \times (9.10+9.20)$	= 13.18	13.18	m <sup>3</sup>
Form	RC structure				
		$(0.90 + 0.80 \times 2) \times (9.10+9.20)$	= 45.75		
		$0.80 \times 0.90 \times 3$	= 2.16		
		<b>Total =</b>	<b>47.91</b>	<b>47.91</b>	<b>m<sup>3</sup></b>



## Steel sheet pile revetment (TYPE C-1) Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE C-1	$L = 3.60 = 3.60$	3.60	m	
Front sheet pile	PU18(S270GP), L=19.0m				
		$N = 3.60/0.60 = 6.0$	6	piece	
		$W = 6 \times 19.00 \times 0.0769 = 8.767$	8.77	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 3.60/0.60 = 6.0$	6	piece	
		$W = 6 \times 6.0 \times 0.0661 = 2.380$	2.38	t	
Penetration depth	PU18	$L = 19.70 - 8.00 = 11.70$	11.70	m/piece	
	PU12	$L = 6.00 - 1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (8.00 - 0.70 + 1.00) \times 0.60 \times 2 \times 6 = 59.76$			PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 6 = 43.20$			PU12
		<b>Total = 102.96</b>	102.96	m <sup>2</sup>	
Tie-Rod	Φ 46	$L = 11.00 \times 2 = 22.00$	22.00	m	
Waling	UPN 220 × 80	$L = 3.60 \times 2 \times 2 = 14.40$	14.40	m	
		$W = 14.40 \times 0.0294 = 0.423$	0.42	t	
Coping concrete	σ <sub>ck</sub> =25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 3.90 + 0.80 \times 0.90 \times 3.80 = 7.81$	7.81	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 3.90 + (0.90 + 0.80 \times 2) \times 3.80 = 23.54$			
		$1.30 \times 1.00 \times 1 + 0.80 \times 0.90 \times 2 = 2.74$			
		<b>Total = 26.28</b>	26.28	m <sup>3</sup>	



## Steel sheet pile revetment (TYPE C-2) Quantity calculation

Bahr Yusef Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE C-2	$L = 9.00 = 9.00$	9.00	m	
Front sheet pile	PU18(S270GP), L=17.0m				
		$N = 9.00/0.60 = 15.0$	15	piece	
		$W = 15 \times 17.00 \times 0.0769 = 19.610$	19.61	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 9.00/0.60 = 15.0$	15	piece	
		$W = 15 \times 6.0 \times 0.0661 = 5.949$	5.95	t	
Penetration depth	PU18	$L = 17.70 - 5.50 = 12.20$	12.20	m/piece	
	PU12	$L = 6.00 - 1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (5.50 - 0.70 + 1.00) \times 0.60 \times 2 \times 15 = 104.40$			PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 15 = 108.00$			PU12
		<b>Total = 212.40</b>	212.40	m <sup>2</sup>	
Tie-Rod	Φ 32	$L = 7.50 \times 4 = 30.00$	30.00	m	
Waling	UPN 180 × 70	$L = 9.00 \times 2 \times 2 = 36.00$	36.00	m	
		$W = 36.00 \times 0.022 = 0.792$	0.79	t	
Coping concrete	σ <sub>ck</sub> =25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 9.50 + 0.80 \times 0.90 \times 9.20 = 18.97$	18.97	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 9.50 + (0.90 + 0.80 \times 2) \times 9.20 = 57.20$			
		$1.30 \times 1.00 \times 1 + 0.80 \times 0.90 \times 2 = 2.74$			
		<b>Total = 59.94</b>	59.94	m <sup>3</sup>	



**Steel sheet pile revetment (TYPE D-1) Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE D-1	$L = 3.60 = 3.60$	3.60	m	
Front sheet pile	PU18(S270GP), L=16.0m				
		$N = 3.60/0.60 = 6.0$	6	piece	
		$W = 6 \times 16.00 \times 0.0769 = 7.382$	7.38	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 3.60/0.60 = 6.0$	6	piece	
		$W = 6 \times 6.0 \times 0.0661 = 2.380$	2.38	t	
Penetration depth	PU18	$L = 16.70-8.00 = 8.70$	8.70	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (8.00-0.70+1.00) \times 0.60 \times 2 \times 6 = 59.76$			PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 6 = 43.20$			PU12
		<b>Total = 102.96</b>	102.96	m <sup>2</sup>	
Tie-Rod	Φ 46	$L = 10.00 \times 2 = 20.00$	20.00	m	
Waling	UPN 220 × 80	$L = 3.60 \times 2 \times 2 = 14.40$	14.40	m	
		$W = 14.40 \times 0.0294 = 0.423$	0.42	t	
Coping concrete	σck=25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 3.90 + 0.80 \times 0.90 \times 3.80 = 7.81$	7.81	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 3.90 + (0.90 + 0.80 \times 2) \times 3.80 = 23.54$			
		$0.80 \times 0.90 \times 2 = 1.44$			
		<b>Total = 24.98</b>	24.98	m <sup>3</sup>	





**Steel sheet pile revetment (TYPE D-2) Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE D-2	$L = 5.40$	$= 5.40$	5.40	m
Steel sheet pile	PU12(S270GP), L=9.5m				
		$N = 5.40/0.60$	$= 9.0$	9	piece
		$W = 9 \times 9.50 \times 0.0661$	$= 5.652$	5.65	t
Penetration depth	PU12	$L = 10.20-3.50$	$= 6.70$	6.70	m/piece
Painting on steel sheet pile	entire length both sides	$A = 9.50 \times 0.60 \times 2 \times 9$	$= 102.60$	102.60	m <sup>2</sup>
Coping concrete	$\sigma_{ck}=25\text{N/mm}^2$				
		$1.30 \times 1.00 \times 5.50$	$= 7.15$	7.15	m <sup>3</sup>
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 5.50$	$= 19.80$		
		$0.60 \times 0.60 \times 1$	$= 0.36$		
		<b>Total =</b>	<b>20.16</b>	20.16	m <sup>3</sup>
Re-bar	D12				
		① $28 \times 3.25 \times 0.888 / 1000$	$= 0.081$		
		② $19 \times 5.40 \times 0.888 / 1000$	$= 0.091$		
		<b>Total =</b>	<b>0.172</b>	0.172	t
Curing	RC structure		$= 7.15$	7.15	m <sup>3</sup>
Expansion joint	elastic filler, t=20mm				
		$1.30 \times 1.00 \times 0$	$= 0.00$	0.00	m <sup>2</sup>

**Steel sheet pile revetment (TYPE E-1) Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE E-1	$L = 3.00 = 3.00$	3.00	m	
Front sheet pile	PU12(S270GP), L=13.5m				
		$N = 3.00/0.60 = 5.0$	5	piece	
		$W = 5 \times 16.00 \times 0.0661 = 5.288$	5.29	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 3.00/0.60 = 5.0$	5	piece	
		$W = 5 \times 6.0 \times 0.0661 = 1.983$	1.98	t	
Penetration depth	PU12	$L = 14.20-6.70 = 7.50$	7.50	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (6.70-0.70+1.00) \times 0.60 \times 2 \times 5 = 42.00$			PU12
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 5 = 36.00$			PU12
		<b>Total = 78.00</b>	78.00	m <sup>2</sup>	
Tie-Rod	Φ42	$L = 9.00 \times 2 = 18.00$	18.00	m	
Waling	UPN 200×75	$L = 3.00 \times 2 \times 2 = 12.00$	12.00	m	
		$W = 12.00 \times 0.0253 = 0.304$	0.30	t	
Coping concrete	σck=25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 3.343 + 0.80 \times 0.90 \times 3.20 = 6.65$	6.65	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 3.343 + (0.90 + 0.80 \times 2) \times 3.20 = 20.03$			
		$0.80 \times 0.90 \times 2 = 1.44$			
		<b>Total = 21.47</b>	21.47	m <sup>3</sup>	



**Steel sheet pile revetment (TYPE E-2) Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE E-2	$L = 1.80$	$= 1.80$	1.80	m
Steel sheet pile	PU12(S270GP), L=8.0m				
		$N = 1.80/0.60$	$= 3.0$	3	piece
		$W = 3 \times 8.00 \times 0.0661$	$= 1.586$	1.59	t
Penetration depth	PU12	$L = 8.70-2.20$	$= 6.50$	6.50	m/piece
Painting on steel sheet pile	entire length both sides	$A = 8.00 \times 0.60 \times 2 \times 3$	$= 28.80$	28.80	m <sup>2</sup>
Coping concrete	$\sigma_{ck}=25\text{N/mm}^2$				
		$1.30 \times 1.00 \times 1.90$	$= 2.47$	2.47	m <sup>3</sup>
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 1.90$	$= 6.84$		
		$1.30 \times 1.00 \times 1$	$= 1.30$		
		<b>Total</b>	<b>= 8.14</b>	8.14	m <sup>3</sup>
Re-bar	D12				
		① $10 \times 3.25 \times 0.888 / 1000$	$= 0.029$		
		② $19 \times 1.80 \times 0.888 / 1000$	$= 0.030$		
		<b>Total</b>	<b>= 0.059</b>	0.059	t
Curing	RC structure		$= 2.47$	2.47	m <sup>3</sup>
Expansion joint	elastic filler, t=20mm				
		$1.30 \times 1.00 \times 0$	$= 0.00$	0.00	m <sup>2</sup>

**Steel sheet pile revetment (TYPE F)      Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE F	$L = 14.30+19.125+40.00-0.25-0.575 = 72.60$	72.60	m	
Front sheet pile	PU18(S270GP), L=17.5m				
		$N = 72.60/0.60 = 121.0$	121	piece	
		$W = 121 \times 17.50 \times 0.0769 = 162.836$	162.84	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = (13.20+10.20+39.00)/0.6 = 104.0$	104	piece	
		$W = 104 \times 6.0 \times 0.0661 = 41.246$	41.25	t	
Penetration depth	PU18	$L = 18.20-8.00 = 10.20$	10.20	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (8.00-0.70+1.00) \times 0.60 \times 2 \times 121 = 1205.16$			PU18
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 104 = 748.80$			PU12
		<b>Total = 1953.96</b>	1953.96	m <sup>2</sup>	
Tie-Rod	Φ46	$L = 11.50 \times 23 + 13.00 \times 2 + 13.50 \times 6 = 371.50$	371.50	m	
Waling	UPN 220×80	$L = (121+104) \times 0.6 \times 2 = 540.00$	540.00	m	
		$W = 540.00 \times 0.0294 = 15.876$	15.88	t	
Coping concrete	σck=25N/mm <sup>2</sup>	$1.30 \times 1.00 \times (14.30+19.125+40.00) = 95.45$			
		$0.80 \times 0.90 \times (13.40+10.40+39.20) = 45.36$			
		<b>Total = 140.81</b>	140.81	m <sup>3</sup>	
Form	RC structure	$(1.00 + 1.30 \times 2) \times (14.30+19.125+40.00) = 264.33$			
		$1.30 \times 1.00 \times 8 = 10.40$			
		$(0.90+0.80 \times 2) \times (13.40+10.40+39.20) = 157.50$			



**Steel sheet pile revetment (TYPE G-1) Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE G-1	$L = 6.60 = 6.60$	6.60	m	
Front sheet pile	PU12(S270GP), L=15.0m				
		$N = 6.60/0.60 = 11.0$	11	piece	
		$W = 11 \times 15.00 \times 0.0661 = 10.907$	10.91	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 6.60/0.60 = 11.0$	11	piece	
		$W = 11 \times 6.0 \times 0.0661 = 4.363$	4.36	t	
Penetration depth	PU12	$L = 14.20-6.70 = 7.50$	7.50	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (6.70-0.70+1.00) \times 0.60 \times 2 \times 11 = 92.40$			PU12
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 11 = 79.20$			PU12
		<b>Total = 171.60</b>	171.60	m <sup>2</sup>	
Tie-Rod	Φ42	$L = 10.00 \times 3 = 30.00$	30.00	m	
Waling	UPN 200 × 75	$L = 6.60 \times 2 \times 2 = 26.40$	26.40	m	
		$W = 26.40 \times 0.0253 = 0.668$	0.67	t	
Coping concrete	σ <sub>ck</sub> =25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 6.94 + 0.80 \times 0.90 \times 6.80 = 13.92$	13.92	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 6.94 + (0.90 + 0.80 \times 2) \times 6.80 = 41.98$			
		$0.80 \times 0.90 \times 2 = 1.44$			
		<b>Total = 43.42</b>	43.42	m <sup>3</sup>	





**Steel sheet pile revetment (TYPE G-2) Quantity calculation**  
Ibrahimia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE G-2	$L = 6.00$	$= 6.00$	6.00	m
Steel sheet pile	PU12(S270GP), L=9.5m				
		$N = 6.00/0.60$	$= 10.0$	10	piece
		$W = 10 \times 9.50 \times 0.0661$	$= 6.280$	6.28	t
Penetration depth	PU12	$L = 9.80 - 3.20$	$= 6.60$	6.60	m/piece
Painting on steel sheet pile	entire length both sides	$A = 9.50 \times 0.60 \times 2 \times 10$	$= 114.00$	114.00	m <sup>2</sup>
Coping concrete	$\sigma_{ck} = 25 \text{N/mm}^2$				
		$1.30 \times 1.00 \times 6.20$	$= 8.06$	8.06	m <sup>3</sup>
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 6.20$	$= 22.32$		
		$1.30 \times 1.00 \times 1$	$= 1.30$		
		<b>Total =</b>	<b>23.62</b>	23.62	m <sup>3</sup>
Re-bar	D12				
		① $31 \times 3.25 \times 0.888 / 1000$	$= 0.089$		
		② $9 \times 6.10 \times 0.888 / 1000$	$= 0.049$		
		<b>Total =</b>	<b>0.138</b>	0.138	t
Curing	RC structure		$= 8.06$	8.06	m <sup>3</sup>
Expansion joint	elastic filler, t=20mm				
		$1.30 \times 1.00 \times 1$	$= 1.30$	1.30	m <sup>2</sup>

**Steel sheet pile revetment (TYPE H)      Quantity calculation**  
Shelyia Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE H	$L = 12.60+6.60 = 19.20$	19.20	m	
Front sheet pile	PU12(S270GP), L=11.0m				
		$N = 19.20/0.60 = 32.0$	32	piece	
		$W = 32 \times 11.00 \times 0.0661 = 23.267$	23.27	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = 19.20/0.60 = 32.0$	32	piece	
		$W = 32 \times 6.0 \times 0.0661 = 12.691$	12.69	t	
Penetration depth	PU18	$L = 11.70-5.95 = 5.75$	5.75	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (5.95-0.70+1.00) \times 0.60 \times 2 \times 32 = 240.00$			PU12
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 32 = 230.40$			PU12
		<b>Total = 470.40</b>	470.40	m <sup>2</sup>	
Tie-Rod	Φ42	$L = 8.50 \times 9 = 76.50$	76.50	m	
Waling	UPN 180×70	$L = 19.20 \times 2 \times 2 = 76.80$	76.80	m	
		$W = 76.80 \times 0.022 = 1.690$	1.69	t	
Coping concrete	σ <sub>ck</sub> =25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 20.00 + 0.80 \times 0.90 \times 19.60 = 40.11$	40.11	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 20.00 + (0.90 + 0.80 \times 2) \times 19.60 = 121.00$			
		$1.30 \times 1.00 \times 3 + 0.80 \times 0.90 \times 5 = 7.50$			
		<b>Total = 128.50</b>	128.50	m <sup>3</sup>	



**Steel sheet pile revetment (TYPE I)      Quantity calculation**  
 Badraman Reg. (Diroutiah)

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE I	$L = 3.00 + 3.60 = 6.60$	6.60	m	
Steel sheet pile	PU12(S270GP), L=9.0m				
		$N = 6.60/0.60 = 11.0$	11	piece	
		$W = 11 \times 9.00 \times 0.0661 = 6.544$	6.54	t	
Penetration depth	PU12	Average $L = (9.30 - 3.80) + (3.80 - 0.6) \times 0.5 = 7.10$	7.10	m/piece	
Painting on steel sheet pile	entire length both sides	$A = 9.00 \times 0.60 \times 2 \times 11 = 118.80$	118.80	m <sup>2</sup>	
Coping concrete	$\sigma_{ck} = 25 \text{ N/mm}^2$				
		$0.60 \times 0.60 \times (3.40 + 4.00) = 2.66$	2.66	m <sup>3</sup>	
Form	RC structure				
		$(0.60 + 0.60 \times 2) \times (3.40 + 4.00) = 13.32$			
		$0.60 \times 0.60 \times 2 = 0.72$			
		<b>Total = 14.04</b>	14.04	m <sup>3</sup>	
Re-bar	D12				
		① $(17+20) \times 1.20 \times 0.888/1000 = 0.039$			
		② $7 \times 3.30 \times 0.888/1000 = 0.021$			
		③ $7 \times 3.90 \times 0.888/1000 = 0.024$			
		<b>Total = 0.084</b>	0.084	t	
Curing	RC structure	$= 2.66$	2.66	m <sup>3</sup>	
Expansion joint	elastic filler, t=20mm				
		$0.60 \times 0.60 \times 2 = 0.72$	0.72	m <sup>2</sup>	

**Steel sheet pile revetment (TYPE J)      Quantity calculation**  
Badraman Reg. (Diroutiah)

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE J	$L = 1.80 + 2.40 = 4.20$	4.20	m	
Steel sheet pile	PU12(S270GP), L=8.0m				
		$N = 4.20 / 0.60 = 7.0$	7	piece	
		$W = 7 \times 8.00 \times 0.0661 = 3.702$	3.70	t	
Penetration depth	PU12	Average $L = (8.30 - 2.70) + (2.70 - 0.6) \times 0.5 = 6.65$	6.65	m/piece	
Painting on steel sheet pile	entire length both sides	$A = 8.00 \times 0.60 \times 2 \times 7 = 67.20$	67.20	m <sup>2</sup>	
Coping concrete	$\sigma_{ck} = 25 \text{ N/mm}^2$				
		$0.60 \times 0.60 \times (2.20 + 2.80) = 1.80$	1.80	m <sup>3</sup>	
Form	RC structure				
		$(0.60 + 0.60 \times 2) \times (2.20 + 2.80) = 9.00$			
		$0.60 \times 0.60 \times 2 = 0.72$			
		<b>Total = 9.72</b>	9.72	m <sup>3</sup>	
Re-bar	D12				
		① $(11 + 14) \times 1.20 \times 0.888 / 1000 = 0.027$			
		② $7 \times 2.10 \times 0.888 / 1000 = 0.013$			
		③ $7 \times 2.70 \times 0.888 / 1000 = 0.017$			
		<b>Total = 0.057</b>	0.057	t	
Curing	RC structure	$= 1.80$	1.80	m <sup>3</sup>	
Expansion joint	elastic filler, t=20mm				
		$0.60 \times 0.60 \times 2 = 0.72$	0.72	m <sup>2</sup>	

**Steel sheet pile revetment (TYPE K)      Quantity calculation**  
**Badraman Reg. (Badraman)**

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE K	$L = 3.60 + 3.00 = 6.60$	6.60	m	
Steel sheet pile	PU18(S270GP), L=10.0m				
		$N = 6.60 / 0.60 = 11.0$	11	piece	
		$W = 11 \times 10.00 \times 0.0769 = 8.459$	8.46	t	
Penetration depth	PU12	Average $L = (10.30 - 4.10) + (4.10 - 0.6) \times 0.5 = 7.95$	7.95	m/piece	
Painting on steel sheet pile	entire length both sides	$A = 10.00 \times 0.60 \times 2 \times 11 = 132.00$	132.00	m <sup>2</sup>	
Coping concrete	$\sigma_{ck} = 25 \text{ N/mm}^2$				
		$0.60 \times 0.60 \times (4.00 + 3.40) = 2.66$	2.66	m <sup>3</sup>	
Form	RC structure				
		$(0.60 + 0.60 \times 2) \times (4.00 + 3.40) = 13.32$			
		$0.60 \times 0.60 \times 2 = 0.72$			
		<b>Total = 14.04</b>	14.04	m <sup>3</sup>	
Re-bar	D12				
		① $(20 + 17) \times 1.20 \times 0.888 / 1000 = 0.039$			
		② $7 \times 3.90 \times 0.888 / 1000 = 0.024$			
		③ $7 \times 3.30 \times 0.888 / 1000 = 0.021$			
		<b>Total = 0.084</b>	0.084	t	
Curing	RC structure	$= 2.66$	2.66	m <sup>3</sup>	
Expansion joint	elastic filler, t=20mm				
		$0.60 \times 0.60 \times 2 = 0.72$	0.72	m <sup>2</sup>	

**Steel sheet pile revetment (TYPE L)      Quantity calculation**  
**Badraman Reg. (Badraman)**

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE L	$L = 1.80 + 1.80 = 3.60$	3.60	m	
Steel sheet pile	PU12(S270GP), L=8.0m				
		$N = 3.60 / 0.60 = 6.0$	6	piece	
		$W = 6 \times 8.00 \times 0.0661 = 3.173$	3.17	t	
Penetration depth	PU12	Average $L = (8.30 - 3.00) + (3.00 - 0.6) \times 0.5 = 6.50$	6.50	m/piece	
Painting on steel sheet pile	entire length both sides	$A = 8.00 \times 0.60 \times 2 \times 6 = 57.60$	57.60	m <sup>2</sup>	
Coping concrete	$\sigma_{ck} = 25 \text{ N/mm}^2$				
		$0.60 \times 0.60 \times (2.20 + 2.20) = 1.58$	1.58	m <sup>3</sup>	
Form	RC structure				
		$(0.60 + 0.60 \times 2) \times (2.20 + 2.20) = 7.92$			
		$0.60 \times 0.60 \times 2 = 0.72$			
		<b>Total = 8.64</b>	8.64	m <sup>3</sup>	
Re-bar	D12				
		① $(11 + 11) \times 1.20 \times 0.888 / 1000 = 0.023$			
		② $7 \times 2.10 \times 0.888 / 1000 = 0.013$			
		③ $7 \times 2.10 \times 0.888 / 1000 = 0.013$			
		<b>Total = 0.049</b>	0.049	t	
Curing	RC structure	$= 1.58$	1.58	m <sup>3</sup>	
Expansion joint	elastic filler, t=20mm				
		$0.60 \times 0.60 \times 2 = 0.72$	0.72	m <sup>2</sup>	



## Steel sheet pile revetment (TYPE M) Quantity calculation

Abo Gabal Reg.

Description	Classification	Calculation	Quantity	Unit	Remarks
Construction length	TYPE M	$L = 32.40+12.60 = 45.00$	45.00	m	
Front sheet pile	PU12(S270GP), L=9.5m				
		$N = 45.00/0.60 = 75.0$	75	piece	
		$W = 75 \times 9.50 \times 0.0661 = 47.096$	47.10	t	
Bracing sheet pile	PU12(S270GP), L=6.0m				
		$N = (30.60+12.60)/0.60 = 72.0$	72	piece	
		$W = 72 \times 6.0 \times 0.0661 = 28.555$	28.56	t	
Penetration depth	PU18	$L = 9.50-4.90 = 4.60$	4.60	m/piece	
	PU12	$L = 6.00-1.00 = 5.00$	5.00	m/piece	
Painting on steel sheet pile	GL-1.00m both sides	$A = (4.90-0.70+1.00) \times 0.60 \times 2 \times 75 = 468.00$			PU12
	entire length both sides	$A = 6.00 \times 0.60 \times 2 \times 72 = 518.40$			PU12
		<b>Total = 986.40</b>	986.40	m <sup>2</sup>	
Tie-Rod	Φ 32	$L = 7.50 \times 18 + 8.00 \times 1 + 9.00 \times 1 = 152.00$	152.00	m	
Waling	UPN 160 × 65	$L = (45.50+43.20) \times 2 = 177.40$	177.40	m	
		$W = 177.40 \times 0.0188 = 3.335$	3.34	t	
Coping concrete	σck=25N/mm <sup>2</sup>				
		$1.30 \times 1.00 \times 46.40 + 0.80 \times 0.90 \times 43.60 = 91.71$	91.71	m <sup>3</sup>	
Form	RC structure				
		$(1.00 + 1.30 \times 2) \times 46.40 + (0.90 + 0.80 \times 2) \times 43.60 = 276.04$			
		$1.30 \times 1.00 \times 6 + 0.80 \times 0.90 \times 8 = 13.56$			
		<b>Total = 289.60</b>	289.60	m <sup>3</sup>	



## (3) Wet stone pitching

Bank protection (wet stone pitching) Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
1. Wet stone pitching					
① Slope 1 : 2.0					
Bahr Yusef	(upstream)	A1 =	= 1041.97		
	(downstream)	A2 =	= 160.00		
		Total	1201.97	m <sup>2</sup>	
② Slope 1 : 1.5					
Ibrahimia	(upstream)		806.80	m <sup>2</sup>	
2. Back filling concrete		0.20 × 806.80	161.36	m <sup>3</sup>	
3. Crown works					
① Type A (Slope 1 : 2.0)					
Bahr Yusef	(upstream)	L1 =	= 38.50		
	(downstream)	L2 =	= 8.80		
		Total	47.30	m	
② Type B (Slope 1 : 1.5)					
Ibrahimia	(upstream)	L1 =	= 54.70		
		Total	54.70	m	
4. Foundation works					
① Type A (Slope 1 : 2.0)					
Bahr Yusef	(upstream)	L1 =	= 47.20		
	(downstream)	L2 =	= 8.70		
		Total	55.90	m	
② Type B (Slope 1 : 1.5)					
Ibrahimia	(upstream)	L1 =	= 50.60		
		Total	50.60	m	

Description	Classification	Calculation	Quantity	Unit	Remarks
(1) Crown concrete (per 10.0m)					
① Type A					
Plain concrete		$1/2 \times (0.40+0.25) \times 0.40 \times 10.00$	1.30	m <sup>3</sup>	
Leveling concrete		$0.05 \times 0.35 \times 10.00$	0.18	m <sup>3</sup>	
Form	Plain con.	$(0.40+(0.40^2+0.15^2)^{1/2}) \times 10.00+1/2 \times (0.40+0.25) \times 0.40$	8.40	m <sup>2</sup>	
Form	Leveling con.	$0.05 \times 10.00 \times 2$	1.00	m <sup>2</sup>	
Curing			1.30	m <sup>3</sup>	
Crushed stone		$0.10 \times 0.35 \times 10$	0.35	m <sup>3</sup>	
Expansion joint		$1/2 \times (0.40+0.25) \times 0.40$	0.13	m <sup>2</sup>	
② Type B					
Plain concrete		$(1/2 \times (0.40+0.25) \times 0.40+0.20 \times 0.25) \times 10.00$	1.80	m <sup>3</sup>	
Leveling concrete		$0.05 \times 0.35 \times 10.00$	0.18	m <sup>3</sup>	
Form	Plain con.	$(0.20+(0.40^2+0.15^2)^{1/2}+0.60) \times 10.00$	= 12.27		
		$1/2 \times (0.40+0.25) \times 0.40+0.20 \times 0.25$	= 0.18		
		Total	12.45	m <sup>2</sup>	
Form	Leveling con.	$0.05 \times 10.00 \times 2$	1.00	m <sup>2</sup>	
Curing			1.80	m <sup>3</sup>	
Crushed stone		$0.10 \times 0.35 \times 10$	0.35	m <sup>3</sup>	
Expansion joint		$1/2 \times (0.40+0.25) \times 0.40+0.20 \times 0.25$	0.18	m <sup>2</sup>	
(2) Foundation concrete (per 10.0m)					
① Type A					
Plain concrete		$(1/2 \times (0.20+0.40) \times 0.25+0.15 \times 0.40) \times 10.00$	1.35	m <sup>3</sup>	
Leveling concrete		$0.05 \times 0.50 \times 10.00$	0.25	m <sup>3</sup>	
Form	Plain con.	$(0.15+(0.20^2+0.25^2)^{1/2}+0.40) \times 10.00$	= 8.70		
		$1/2 \times (0.20+0.40) \times 0.25+0.15 \times 0.40$	= 0.14		
		Total	8.84	m <sup>2</sup>	
Form	Leveling con.	$0.05 \times 10.00 \times 2$	1.00	m <sup>2</sup>	
Curing			1.35	m <sup>3</sup>	



Description	Classification	Calculation	Quantity	Unit	Remarks
<p style="color: red; text-align: center;">Crown works (Type A)</p> <p style="color: red; text-align: center;">Foundation works (Type A)</p> <p style="text-align: center;">1:2</p> <p style="text-align: center;">WET STONE PITCHING t=350</p> <p style="text-align: right;">LEAN CONCRETE t=50 CRASHED STONE t=100</p>					
<p style="color: red; text-align: center;">Crown works (Type B)</p> <p style="color: red; text-align: center;">Foundation works (Type B)</p> <p style="text-align: center;">1:1.5</p> <p style="text-align: center;">WET STONE PITCHING t=350 BACK FILLING CONCRETE t=200</p> <p style="text-align: right;">LEAN CONCRETE t=50 CRASHED STONE t=100</p>					

## (4) Gabion

Gabion Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
1. Wire basket					
1.0m × 2.0m × 0.50m					
Bahr Yusef		$9 \times 30 + 6 \times 43$	528	pcs.	
		$18 + 38 + 14$	70	pcs.	
		Total	598	pcs.	
Ibrahimia		$8 \times 30 + 6 \times 43 + 5 \times 4 + 4 \times 4 + 3$	537	pcs.	
		$16 + 33$	49	pcs.	
		Total	586	pcs.	
1.0m × 2.0m × 1.0m					
Bahr Yusef		$(30 + 43) \times 2$	146	pcs.	
		$10 + 6$	16	pcs.	
		Total	162	pcs.	
Ibrahimia		$(30 + 43) \times 2$	146	pcs.	
		$6 + 12$	18	pcs.	
		$10 \times 32 + 24$	341	pcs.	left bank
		Total	508	pcs.	
2. Filled stone (Dia. 15 ~ 20cm.)					
Bahr Yusef		$A = 999.1 \times 1.118 = 1117.3 \text{m}^2$			
		$1117.3 \times 0.5 =$	558.7		
		$1.0 \times 2.0 \times 1.0 \times 162 =$	324.0		
		Total	882.7	m <sup>3</sup>	







## (2) Bed protection works

Bed protection works		Quantity calculation		
Item	Calculation	Quantity	Unit	Remarks
1. Bahr Yusef regulator				
Downstream	$30.00 \times 31.50$	= 945.00	m <sup>2</sup>	
Stone(Dia 40cm)	$945.00 \times 0.90$	= 850.50	m <sup>3</sup>	
Gravel	$945.0 \times 0.10$	= 94.50	m <sup>3</sup>	
Geotextile sheet		945.00	m <sup>2</sup>	
	Stone(Dia 20cm)			
Downstream	$5.00 \times 42.5 \times 0.5$	= 106.25	m <sup>3</sup>	
Upstream	$5.00 \times 132.7 \times 0.5$	= 331.75	m <sup>3</sup>	
	Total	438.00	m <sup>3</sup>	
2. Ibrahimia regulator				
Downstream	$30.00 \times 31.50$	= 945.00	m <sup>2</sup>	
Stone(Dia 40cm)	$945.00 \times 0.90$	= 850.50	m <sup>3</sup>	
Gravel	$945.0 \times 0.10$	= 94.50	m <sup>3</sup>	
Geotextile sheet		945.00	m <sup>2</sup>	
	Stone(Dia 20cm)			
Downstream	$5.00 \times 32.4 \times 0.5$	= 81.00	m <sup>3</sup>	
Upstream	$5.00 \times 143.8 \times 0.5$	= 359.50	m <sup>3</sup>	
	Total	440.50	m <sup>3</sup>	
3. Badrama regulator(Badraman canal)				
	$15.00 \times 5.00$	= 75.00	m <sup>2</sup>	
Stone(Dia 20cm)	$75.0 \times 0.40$	= 30.00	m <sup>3</sup>	
Gravel	$75.0 \times 0.10$	= 7.50	m <sup>3</sup>	
Geotextile sheet		75.00	m <sup>2</sup>	



### C-1-9. Quantity calculation of hydraulic pipe pit works

#### (1) Quantity summary table

Item	Calculation	Bahr Yusef	Ibrahimia	Unit	Remarks
1. Concrete					
(1) Reinforced concrete					
	Total	8.87	76.94	m <sup>3</sup>	
(2) Plain (Leveling) concrete					
	Total	2.96	7.56	m <sup>3</sup>	
2. Form work					
(1) Reinforced concrete	Total	53.34	321.39	m <sup>2</sup>	
(2) Plain (Leveling) concrete	=	3.03	6.08	m <sup>2</sup>	
3. Concrete curing	Reinforced concrete	8.87	76.94	m <sup>3</sup>	
	Plain concrete	2.96	7.56	m <sup>3</sup>	
4. Scaffolding	Total		160.14	m <sup>2</sup>	
5. Falsework	Total		91.97	m <sup>3</sup>	
6. Slip bar	D20 L=1.0m, Vinyl pipe $\Phi$ 25x0.5m				
	=		32	pcs	
7. Water stop					
B=300	=		16.80	m	
8. Elastic filter	Total		5.13	m <sup>2</sup>	
3. Contraction joint	Oil paint	=	0.62	m <sup>2</sup>	
9. Checker plate					
t=3.2mm	=		0.98	m <sup>2</sup>	
t=4.5mm	=	18.79		m <sup>2</sup>	
10. Anchor bar	L=0.20m				
D16	=		16	pcs	
11. Re-bar	D25		328	kg	
	D16		4,820	kg	
	D12	448	2,923	kg	

(2) Bahr Yosef reg.

Bahr Yusef hydraulic pipe pit		Quantity calculation		
Item	Calculation	Quantity	Unit	Remarks
1. Concrete				
(1) Reinforced concrete				
base slab	$0.20 \times 1.90 \times 14.00$	= 5.32		
side wall	$0.15 \times 0.80 \times 2 \times 13.85$	= 3.32		
	$0.15 \times 0.80 \times 1.90$	= 0.23		
	Total	8.87	m <sup>3</sup>	
(2) Plain (Leveling) concrete				
	$2.10 \times 14.10 \times 0.10$	2.96	m <sup>3</sup>	
2. Form work				
(1) Reinforced concrete				
side wall (Outside)	$1.00 \times (14.00 \times 2 + 1.90)$	= 29.90		
side wall (Inside)	$0.80 \times (13.85 \times 2 + 1.60)$	= 23.44		
	Total	53.34	m <sup>2</sup>	
(2) Plain (Leveling) concrete				
	$(14.10 \times 2 + 2.10) \times 0.10$	= 3.03	m <sup>2</sup>	
3. Contraction joint				
	Oil paint			
	$1.90 \times 0.20 + 0.15 \times 0.80 \times 2$	= 0.62	m <sup>2</sup>	
4. Concrete curing				
	Reinforced concrete	8.87	m <sup>3</sup>	
	Plain concrete	2.96	m <sup>3</sup>	
5. Checker plate				
t=4.5mm	$1.70 \times 11.05$	= 18.79	m <sup>2</sup>	
6. Rebar				
	D12	448	kg	



Item	Calculation	Quantity	Unit	Remarks
Haunch	$0.212 \times (26.35 + 3.825 + 1.65 + 24.7 + 2.175)$	= 12.44		
	$0.212 \times (25.825 + 2.425 + 23.375 + 0.775)$	= 11.11		
	$0.212 \times 2.40$	= 0.51		
parapet wall	$0.20 \times (0.15 + 2.40) \times 2$	= 1.02		
	Total	321.39	m <sup>2</sup>	
(2) Plain (Leveling) concrete				
	$(27.30 + 2.60 + 24.80 + 4.30 + 1.80) \times 0.10$	= 6.08	m <sup>2</sup>	
3. Concrete curing	Reinforced concrete	76.94	m <sup>3</sup>	
	Plain concrete	7.56	m <sup>3</sup>	
4. Scaffolding				
	$(29.30 + 5.30 + 4.60 + 24.70 + 0.70) \times 2.40$	= 155.04		
	$1.70 \times 2 \times (0.10 + 1.40)$	= 5.10		
	Total	160.14	m <sup>2</sup>	
5. Falsework				
	$(1.80 \times 1.80 - 1/2 \times 0.15 \times 0.15 \times 4) \times (26.50 + 0.70)$	= 86.90		
	$(3.20 \times 1.80 - 1/2 \times 0.15 \times 0.15 \times 2) \times 1.40$	= 8.03		
	$-(1.40 \times 0.80 + 0.60 \times 0.60) \times 2$	= -2.96		
	Total	91.97	m <sup>3</sup>	
6. Slip bar	D20 L=1.0m, Vinyl pipe $\Phi$ 25x0.5m			
	$16 \times 2$	= 32	pcs	
7. Water stop				
B=300	$2.10 \times 4 \times 2$	= 16.80	m	
8. Elastic filter				
	$(2.40 + 1.80) \times 0.30 \times 2 \times 2$	= 5.04		
	$1/2 \times 0.15 \times 0.15 \times 4 \times 2$	= 0.09		
	Total	5.13	m <sup>2</sup>	
9. Checker plate				
t=3.2mm	$0.70 \times 0.70 \times 2$	= 0.98	m <sup>2</sup>	
10. Anchor bar	L=0.20m			
D16	$8 \times 2$	= 16	pcs	
11. Rebar				
	D25	328	kg	
	D16	4,820	kg	
	D12	2,923	kg	

## C-1-10. Quantity calculation of ancillary road works

### (1) Quantity summary table

Description	Classification	Unit	Quantity	Remarks
(1) Earth work				
Embankment	sandy soil	m <sup>3</sup>	10,225.3	
(2) Asphalt pavement				
Roadwalk		m <sup>2</sup>	4,075.1	
	Weaving course (t=5cm)	m <sup>3</sup>	203.8	
	Binder course (t=6cm)	m <sup>3</sup>	244.5	
	Base course (t=20cm)	m <sup>3</sup>	815.0	
	Sub-base course (t=20cm)	m <sup>3</sup>	815.0	
Sidewalk		m <sup>2</sup>	623.7	
	Weaving course (t=5cm)	m <sup>3</sup>	31.2	
	Base course (t=15cm)	m <sup>3</sup>	93.6	
(3) Cub concrete				
Type 1		m	643.5	
	Plain concrete ( $\sigma_{ck}=20\text{N/mm}^2$ )	m <sup>3</sup>	38.6	
	Form (Plain concrete)	m <sup>2</sup>	390.1	
	Leveling concrete ( $\sigma_{ck}=20\text{N/mm}^2$ )	m <sup>3</sup>	9.7	
	Form (Leveling concrete)	m <sup>2</sup>	64.4	
	Crushed ston	m <sup>3</sup>	19.3	
Type 2		m	629.8	
	Plain concrete ( $\sigma_{ck}=20\text{N/mm}^2$ )	m <sup>3</sup>	70.9	
	Form (Plain concrete)	m <sup>2</sup>	574.0	
	Leveling concrete ( $\sigma_{ck}=20\text{N/mm}^2$ )	m <sup>3</sup>	11.0	
	Form (Leveling concrete)	m <sup>2</sup>	63.0	
	Crushed ston	m <sup>3</sup>	22.0	
(4) Bulstrade		m	629.8	
(5) Slope protection	Wet stone piching (1:15) t=35cm	m <sup>2</sup>	1987.0	
	Wet stone piching (1:10) t=35cm	m <sup>2</sup>	158.4	
	Backfilling concrete(t=10cm)	m <sup>3</sup>	15.8	$\sigma_{ck}=20\text{N/mm}^2$
	Crown works (Type A)	m	421.0	
	Crown works (Type B)	m	57.6	
	Foundation works	m	536.8	
(6) Step				
Type A	H=3.57m, 1:1.5	places	2	
Type B	H=2.57m, 1:1.5	places	4	
Type C	H=3.67m, 1:1.0	places	2	
Type D	H=2.57m, 1:1.0	places	2	
Type E	H=3.87m, 1:1.5	places	2	



## (2) Earth work

	Station	Distance (m)	Embankment (B0)		
			Area (m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Vol. (m <sup>3</sup> )
①Bahr Y usef					
	10.0	0.0	0.00		
	20.0	10.0	202.69	101.35	1,013.50
	40.2	20.2	62.49	132.59	2,678.32
	75.2	35.0	45.38	53.94	1,887.90
	sub-total				5,579.72
	110.7	0.0	45.38		
	137.3	26.6	45.38	45.38	1,207.11
	147.3	10.0	0.00	22.69	226.90
	sub-total				1,434.01
	Total			7,013.73	
②Badraman (Badraman)  (Diroutiah)					
	167.3	0.0	0.00		
	174.0	6.7	42.33	21.17	141.84
	sub-total				141.84
	181.0	0.0	42.33		
	189.5	8.5	11.67	27.00	229.50
	sub-total				229.50
		Total			371.34
	189.5	0.0	11.67		
	198.0	8.5	42.33	27.00	229.50
	sub-total				229.50
	208.0	0.0	42.33		
	214.0	6.0	4.90	23.62	141.72
	sub-total				141.72
	Total			371.22	
③Ibrahimia					
	259.5	0.0	0.00		
	266.1	6.6	48.89	24.45	161.37
	272.3	6.2	48.89	48.89	303.12
	sub-total				464.49
	307.5	0.0	48.89		
	343.5	36.0	48.89	48.89	1,760.04
	353.5	10.0	0.00	24.45	244.50
	sub-total				2,004.54
	Total			2,469.03	

## (3) Asphalt pavement

Asphalt pavement Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
1.Roadwalk	Area				
	~ Bahr Yusef	A1 = 1124.67+641.62 = 1766.29			
	Bahr Yusef~ Badraman	A2 = 630.10+37.05+37.23+64.62+84.38 = 853.38			
	Badraman~ Diroutiah	A3 = 156.00 = 156.00			
	Diroutiah~ Ibrahimia	A4 = 650.29+37.12+53.63+37.28+37.12+56.37			
		= 871.81			
	Ibrahimia~	A5 = 442.03+141.62 = 583.65			
	Abo Gabal	A6 = 148.50 = 148.50			
		Total	4,075.13	m <sup>2</sup>	
①Wearing course	t=5cm	4,075.13× 0.05	203.76	m <sup>3</sup>	
②Binder course	t=6cm	4,075.13× 0.06	244.51	"	
③Base course	t=20cm	4,075.13× 0.20	815.03	"	
④Sub-base course	t=20cm	4,075.13× 0.20	815.03	"	
2.Sidewalk	Area				
	~ Bahr Yusef	A1 = 117.83+110.35+13.43+64.83 = 306.44			
	Bahr Yusef~ Badraman	A2 = 38.47+4.21+6.21+41.96+6.02+6.16 = 103.03			
	Badraman~ Diroutiah	A3 = 16.00+16.00 = 32.00			
	Diroutiah~ Ibrahimia	A4 = 6.64+30.91+13.31+6.64+8.09+20.92+13.32			
		= 99.83			
	Ibrahimia~	A5 = 38.36+44.06 = 82.42			
		Total	623.72	m <sup>2</sup>	
①Wearing course	t=5cm	623.72× 0.05	31.19	m <sup>3</sup>	
②Base course	t=15cm	623.72× 0.15	93.56	"	

## (4) Curb concrete and Bulstrade

Curb concrete , Bulstrade Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
1.Curb concrete	Line				
①TYPE-1	Line				
	~ Bahr Yusef	$L1 = 113.89+118.45+15.02+65.41 = 312.77$			
	Bahr Yusef~ Badraman	$L2 = 38.69+4.07+6.16+41.71+5.98+6.16 = 102.77$			
	Badraman~ Diroutiah	$L3 = 16.00+16.00 = 32.00$			
	Diroutiah~ Ibrahimia	$L4 = 6.64 \times 2 + 44.81 + 8.09 + 20.75 + 13.32 \times 2 = 113.57$			
	Ibrahimia~	$L5 = 38.37+44.05 = 82.42$			
		Total	643.53	m	
Plain concrete	$\sigma_{ck}=20N/mm^2$	$0.30 \times 0.20 \times 643.53$	38.61	$m^3$	
Form	Plain con.	$0.30 \times 2 \times 643.53 + 0.30 \times 0.20 \times (643.53/10+1)$	390.08	$m^2$	
Leveling concrete	$\sigma_{ck}=20N/mm^2$	$0.05 \times 0.30 \times 643.53$	9.65	$m^3$	
Form	Leveling con.	$0.05 \times 2 \times 643.53$	64.35	$m^2$	
Crushed stone		$0.10 \times 0.30 \times 643.53$	19.31	$m^3$	
②TYPE-2	Line				
	~ Bahr Yusef	$L1 = 107.61+116.77+23.74+64.45 = 312.57$			
	Bahr Yusef~ Badraman	$L2 = 38.69+4.07+6.16+41.71+5.98+6.16 = 102.77$			
	Badraman~ Diroutiah	$L3 = 16.00+16.00 = 32.00$			
	Diroutiah~ Ibrahimia	$L4 = 6.64 \times 2 + 30.94 + 8.09 + 21.10 + 13.32 \times 2 = 100.05$			
	Ibrahimia~	$L5 = 38.37+44.05 = 82.42$			
		Total	629.81	m	
Plain concrete	$\sigma_{ck}=20N/mm^2$	$0.45 \times 0.25 \times 629.81$	70.85	$m^3$	
Form	Plain con.	$0.45 \times 2 \times 629.81 + 0.45 \times 0.25 \times (629.81/10+1)$	574.03	$m^2$	
Leveling concrete	$\sigma_{ck}=20N/mm^2$	$0.05 \times 0.35 \times 629.81$	11.02	$m^3$	
Form	Leveling con.	$0.05 \times 2 \times 629.81$	62.98	$m^2$	
Crushed stone		$0.10 \times 0.35 \times 629.81$	22.04	$m^3$	

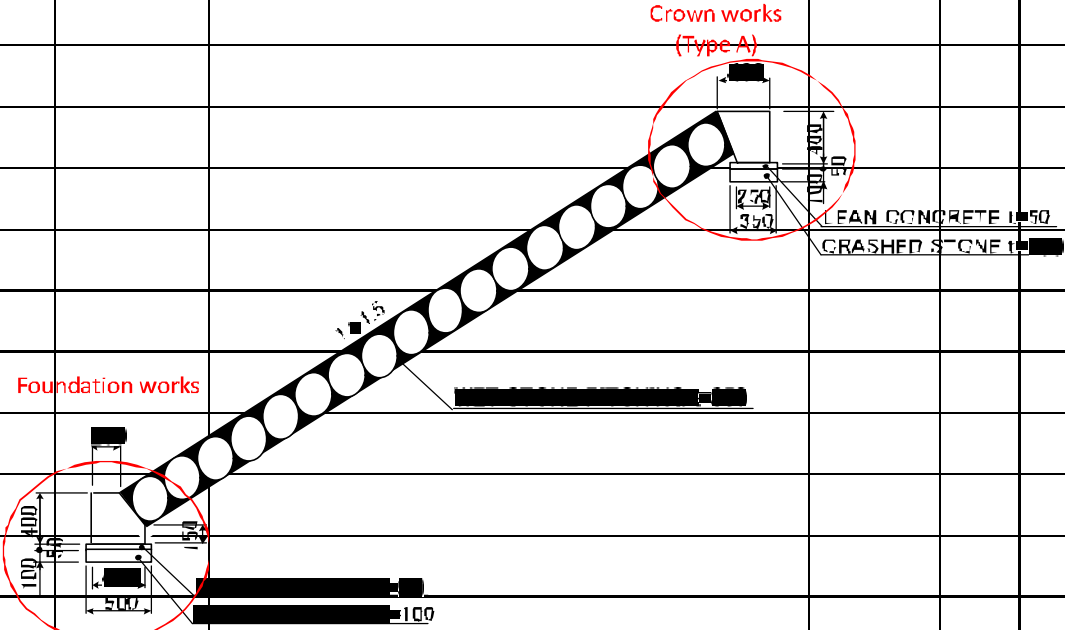
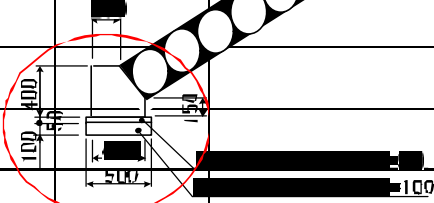
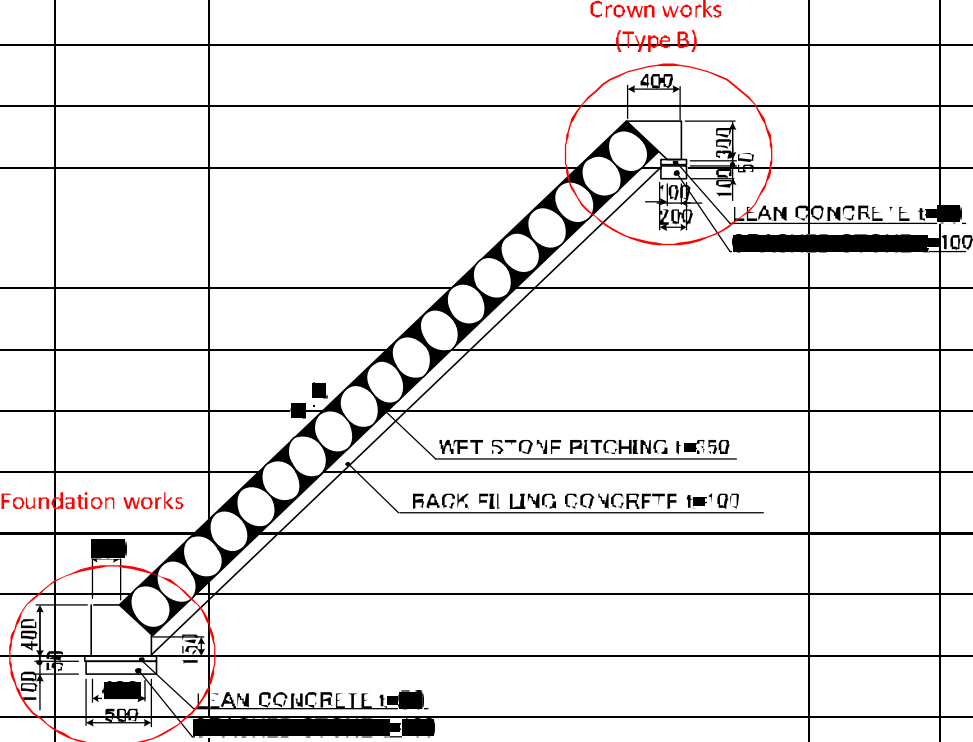
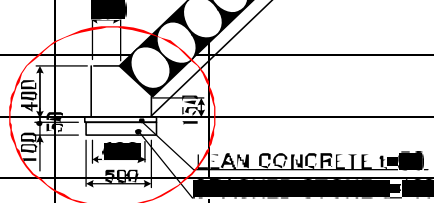


## (5) Wet stone pitching

Wet stone pitching Quantity calculation					
Description	Classification	Calculation	Quantity	Unit	Remarks
1. Wet stone pitching					
① Slope 1 : 1.5					
	~ Bahr Yusef	$A1 = 106.14 + 250.10 + 374.48 = 730.72$			
	Bahr Yusef ~ Badraman	$A2 = 111.13 + 21.84 + 157.00 + 40.48 = 330.45$			
	Diroutiah ~ Ibrahimia	$A3 = 3.06 + 39.46 + 4.50 + 1.62 + 59.67 = 108.31$			
	Ibrahimia ~	$A4 = 165.04 + 216.05 = 381.09$			
	Abo Gabal	$A5 = 102.68 = 102.68$			
		Total	1653.25	m <sup>2</sup>	
	Area	$1653.25 \times (1.0^2 + 1.5^2)^{1/2} / 1.5$	1986.96	m <sup>2</sup>	
② Slope 1 : 1.0					
	Bahr Yusef ~ Badraman	$A1 = 10.65 + 14.81 = 25.46$			
	Badraman ~ Diroutiah	$A2 = 24.89 + 30.55 = 55.44$			
	Diroutiah ~ Ibrahimia	$A3 = 12.45 + 18.65 = 31.10$			
		Total	112.00	m <sup>2</sup>	
	Area	$112.00 \times (1.0^2 + 1.0^2)^{1/2} / 1.0$	158.39	m <sup>2</sup>	
2. Back filling con.	$\sigma_{ck} = 20 \text{ N/mm}^2$	$0.10 \times 158.39$	15.84	m <sup>3</sup>	
3. Crown works					
① Type A (Slope 1 : 1.5)					
	~ Bahr Yusef	$L1 = 23.74 + 64.45 + 87.47 = 175.66$			
	Bahr Yusef ~ Badraman	$L2 = 38.39 + 4.02 + 42.01 + 6.05 = 90.47$			
	Diroutiah ~ Ibrahimia	$L3 = 30.94 + 8.09 + 33.42 = 72.45$			
	Ibrahimia ~	$L4 = 38.37 + 44.05 = 82.42$			
		Total	421.00	m	



Description	Classification	Calculation	Quantity	Unit	Remarks
(1) Crown concrete (per 10.0m)					
① Type A					
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.40+0.25) \times 0.40 \times 10.00$	1.30	$\text{m}^3$	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$0.05 \times 0.35 \times 10.00$	0.18	$\text{m}^3$	
Fom	Plain con.	$(0.40+(0.40^2+0.15^2)^{1/2}) \times 10.00+1/2 \times (0.40+0.25) \times 0.40$	8.40	$\text{m}^2$	
Fom	Leveling con.	$0.05 \times 10.00 \times 2$	1.00	$\text{m}^2$	
Curing			1.30	$\text{m}^3$	
Crushed store		$0.10 \times 0.35 \times 10$	0.35	$\text{m}^3$	
Expansion joint		$1/2 \times (0.40+0.25) \times 0.40$	0.13	$\text{m}^2$	
② Type B					
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.40+0.10) \times 0.30 \times 10.00$	0.75	$\text{m}^3$	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$0.05 \times 0.20 \times 10.00$	0.10	$\text{m}^3$	
Fom	Plain con.	$(0.30+(0.30^2+0.30^2)^{1/2}) \times 10.00+1/2 \times (0.40+0.10) \times 0.30$	7.32	$\text{m}^2$	
Fom	Leveling con.	$0.05 \times 10.00 \times 2$	1.00	$\text{m}^2$	
Curing			0.75	$\text{m}^3$	
Crushed store		$0.10 \times 0.20 \times 10$	0.20	$\text{m}^3$	
Expansion joint		$1/2 \times (0.40+0.10) \times 0.30$	0.08	$\text{m}^2$	
(2) Foundation concrete (per 10.0m)					
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$(1/2 \times (0.20+0.40) \times 0.25+0.15 \times 0.40) \times 10.00$	1.35	$\text{m}^3$	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$0.05 \times 0.50 \times 10.00$	0.25	$\text{m}^3$	
Fom	Plain con.	$(0.15+(0.20^2+0.25^2)^{1/2}+0.40) \times 10.00$	= 8.70		
		$1/2 \times (0.20+0.40) \times 0.25+0.15 \times 0.40$	= 0.14		
		Total	8.84	$\text{m}^2$	
Fom	Leveling con.	$0.05 \times 10.00 \times 2$	1.00	$\text{m}^2$	
Curing			1.35	$\text{m}^3$	
Crushed store		$0.10 \times 0.35 \times 10$	0.35	$\text{m}^3$	
Expansion joint		$1/2 \times (0.20+0.40) \times 0.25+0.15 \times 0.40$	0.14	$\text{m}^2$	

Description	Classification	Calculation	Quantity	Unit	Remarks
		 <p>Crown works (Type A)</p> <p>LEAN CONCRETE t=50</p> <p>CRASHED STONE t=100</p>			
	Foundation works				
		 <p>Crown works (Type B)</p> <p>LEAN CONCRETE t=100</p>			
	Foundation works	 <p>LEAN CONCRETE t=100</p> <p>WET STONE PITCHING t=350</p> <p>BACK FILLING CONCRETE t=100</p>			





Description	Classification	Calculation	Quantity	Unit	Remarks
①Type-A	H=3.57m,1:1.5				
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.27+0.586) \times 0.30 \times 1.20 = 0.15$			
		$1/2 \times (6.436+6.417) \times 0.425 \times 1.20 = 3.28$			
		$1/2 \times 0.311 \times 0.30 \times 1.20 = 0.06$			
		$-) 1/2 \times 0.12 \times 0.18 \times 0.80 = -0.01$			
		$-) 1/2 \times 0.15 \times 0.225 \times 23 \times 0.80 = -0.31$			
		Total	3.17	m <sup>3</sup>	
Fom	Plain con.	$1/2 \times (0.27+0.586) \times 0.30 \times 2 = 0.26$			
		$1/2 \times (6.436+6.417) \times 0.425 \times 2 = 5.46$			
		$1/2 \times 0.311 \times 0.30 \times 2 = 0.09$			
		$(3.57+0.30+0.311) \times 1.20 = 5.02$			
		$1/2 \times 0.12 \times 0.18 \times 2 = 0.02$			
		$1/2 \times 0.15 \times 0.225 \times 23 \times 2 = 0.78$			
		Total	11.63	m <sup>2</sup>	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$(0.586+6.417) \times 0.10 \times 1.20 = 0.84$	0.84	m <sup>3</sup>	
Fom	Leveling con.	$((0.586+6.417) \times 0.10 + 0.10 \times 1.20) \times 2 = 1.64$	1.64	m <sup>2</sup>	
Curing			11.63	m <sup>2</sup>	
Crushed stone		$(0.616+6.38) \times 0.15 \times 1.20 = 1.26$	1.26	m <sup>3</sup>	
Re-bar	D12	$(0.50+6.40) \times 7 \times 0.888\text{kg/m} = 42.89$	42.89	kg	

Description	Classification	Calculation	Quantity	Unit	Remarks
②Type-B	H=2.57m,1:1.5				
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.195+0.331) \times 0.30 \times 1.20 = 0.09$			
		$1/2 \times (4.633+4.765) \times 0.425 \times 1.20 = 2.40$			
		$1/2 \times 0.311 \times 0.30 \times 1.20 = 0.06$			
		$-) 1/2 \times 0.17 \times 0.225 \times 0.80 = -0.02$			
		$-) 1/2 \times 0.15 \times 0.225 \times 16 \times 0.80 = -0.22$			
		Total	2.32	m <sup>3</sup>	
Fom	Plain con.	$1/2 \times (0.195+0.331) \times 0.30 \times 2 = 0.16$			
		$1/2 \times (4.633+4.765) \times 0.425 \times 2 = 3.99$			
		$1/2 \times 0.311 \times 0.30 \times 2 = 0.09$			
		$(2.57+0.30+0.311) \times 1.20 = 3.82$			
		$1/2 \times 0.17 \times 0.225 \times 2 = 0.04$			
		$1/2 \times 0.15 \times 0.225 \times 16 \times 2 = 0.54$			
		Total	8.64	m <sup>2</sup>	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$(0.331+4.765) \times 0.10 \times 1.20 = 0.61$	0.61	m <sup>3</sup>	
Fom	Leveling con.	$((0.331+4.765) \times 0.10 + 0.10 \times 1.20) \times 2 = 1.26$	1.26	m <sup>2</sup>	
Curing			8.64	m <sup>2</sup>	
Crushed stone		$(0.361+4.728) \times 0.15 \times 1.20 = 0.92$	0.92	m <sup>3</sup>	
Re-bar	D12	$(0.20+4.70) \times 7 \times 0.888\text{kg/m} = 30.46$	30.46	kg	

Description	Classification	Calculation	Quantity	Unit	Remarks
③Type-C	H=3.67m,1:1.0				
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.38+0.654) \times 0.30 \times 1.20 = 0.19$			
		$1/2 \times (5.190+5.227) \times 0.406 \times 1.20 = 2.54$			
		$1/2 \times 0.274 \times 0.30 \times 1.20 = 0.05$			
		$-) 1/2 \times 0.07 \times 0.07 \times 0.80 = 0.00$			
		$-) 1/2 \times 0.20 \times 0.20 \times 18 \times 0.80 = -0.29$			
		Total	2.48	m <sup>3</sup>	
Fom	Plain con.	$1/2 \times (0.38+0.654) \times 0.30 \times 2 = 0.31$			
		$1/2 \times (5.190+5.227) \times 0.406 \times 2 = 4.23$			
		$1/2 \times 0.274 \times 0.30 \times 2 = 0.08$			
		$(3.67+0.30+0.274) \times 1.20 = 5.09$			
		$1/2 \times 0.07 \times 0.07 \times 2 = 0.00$			
		$1/2 \times 0.20 \times 0.20 \times 18 \times 2 = 0.72$			
		Total	10.44	m <sup>2</sup>	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$(0.654+5.227) \times 0.10 \times 1.20 = 0.71$	0.71	m <sup>3</sup>	
Fom	Leveling con.	$((0.654+5.227) \times 0.10 + 0.10 \times 1.20) \times 2 = 1.42$	1.42	m <sup>2</sup>	
Curing			10.44	m <sup>2</sup>	
Crushed stone		$(0.696+5.168) \times 0.15 \times 1.20 = 1.06$	1.06	m <sup>3</sup>	
Re-bar	D12	$(0.55+5.20) \times 7 \times 0.888\text{kg/m} = 35.74$	35.74	kg	

Description	Classification	Calculation	Quantity	Unit	Remarks
①Type-D	H=2.57m,1:1.0				
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.28+0.554) \times 0.30 \times 1.20 = 0.15$			
		$1/2 \times (3.635+3.671) \times 0.406 \times 1.20 = 1.78$			
		$1/2 \times 0.274 \times 0.30 \times 1.20 = 0.05$			
		$-) 1/2 \times 0.17 \times 0.17 \times 0.80 = -0.01$			
		$-) 1/2 \times 0.20 \times 0.20 \times 12 \times 0.80 = -0.19$			
		Total	1.78	m <sup>3</sup>	
Fom	Plain con.	$1/2 \times (0.28+0.554) \times 0.30 \times 2 = 0.25$			
		$1/2 \times (3.635+3.671) \times 0.425 \times 2 = 3.11$			
		$1/2 \times 0.274 \times 0.30 \times 2 = 0.08$			
		$(2.57+0.30+0.274) \times 1.20 = 3.77$			
		$1/2 \times 0.17 \times 0.17 \times 2 = 0.03$			
		$1/2 \times 0.20 \times 0.20 \times 12 \times 2 = 0.48$			
		Total	7.72	m <sup>2</sup>	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$(0.554+3.671) \times 0.10 \times 1.20$	0.51	m <sup>3</sup>	
Fom	Leveling con.	$((0.554+3.671) \times 0.10 + 0.10 \times 1.20) \times 2$	1.09	m <sup>2</sup>	
Curing			7.72	m <sup>2</sup>	
Crushed stone		$(0.596+3.612) \times 0.15 \times 1.20$	0.76	m <sup>3</sup>	
Re-bar	D12	$(0.45+3.65) \times 7 \times 0.888\text{kg/m}$	25.49	kg	

Description	Classification	Calculation	Quantity	Unit	Remarks
⑤Type-E	H=3.87m,1:1.5				
Plain concrete	$\sigma_{ck}=20\text{N/mm}^2$	$1/2 \times (0.27+0.586) \times 0.30 \times 1.20 = 0.15$			
		$1/2 \times (6.977+7.086) \times 0.425 \times 1.20 = 3.59$			
		$1/2 \times 0.311 \times 0.30 \times 1.20 = 0.06$			
		-) $1/2 \times 0.12 \times 0.18 \times 0.80 = -0.01$			
		-) $1/2 \times 0.15 \times 0.225 \times 25 \times 0.80 = -0.34$			
		Total	3.45	m <sup>3</sup>	
Fom	Plain con.	$1/2 \times (0.27+0.586) \times 0.30 \times 2 = 0.26$			
		$1/2 \times (6.977+7.086) \times 0.425 \times 2 = 5.98$			
		$1/2 \times 0.311 \times 0.30 \times 2 = 0.09$			
		$(3.57+0.30+0.311) \times 1.20 = 5.02$			
		$1/2 \times 0.12 \times 0.18 \times 2 = 0.02$			
		$1/2 \times 0.15 \times 0.225 \times 25 \times 2 = 0.84$			
		Total	12.21	m <sup>2</sup>	
Leveling concrete	$\sigma_{ck}=20\text{N/mm}^2$	$(0.586+7.086) \times 0.10 \times 1.20$	0.92	m <sup>3</sup>	
Fom	Leveling con.	$((0.586+7.086) \times 0.10 + 0.10 \times 1.20) \times 2$	1.77	m <sup>2</sup>	
Curing			12.21	m <sup>2</sup>	
Crushed stone		$(0.616+6.921) \times 0.15 \times 1.20$	1.36	m <sup>3</sup>	
Re-bar	D12	$(0.50+6.95) \times 7 \times 0.888\text{kg/m}$	46.31	kg	